CITY OF ALBUQUERQUE



March 14, 2018

J. Graeme Means, P.E. High Mesa Consulting Group 6010 B Midway Park Blvd NE Albuquerque, NM 87109

RE: Mark 3S Holly Development 9300 Holly NE Grading and Drainage Plan Engineer's Stamp Date 3/8/18 (File: C20D062)

Dear Mr. Means:

Based on the information provided in your submittal received on 3/8/18, this plan is approved for Grading Permit, Building Permit, and Work Order.

Prior to Certificate of Occupancy:

- 1. The Private Facility Drainage Covenant must be recorded with Bernalillo County and a copy included with the drainage certification.
 - 2. Either a recorded SIA with financial guarantee or City acceptance and close-out of the public Work Order is required.

NM 87103

PO Box 1293

Albuquerque

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

www.cabq.gov

Sincerely,

Dana Peterson, P.E. Senior Engineer, Planning Dept. Development Review Services



City of Albuquerque

Planning Department Development & Building Services Division DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

Project Title:	Building Permit #: City Drainage #:
DRB#: EPC#:	Work Order#:
Legal Description:	
City Address:	
Engineering Firm:	Contact:
Address:	
Phone#: Fax#:	E-mail:
Owner:	Contact:
Address:	
Phone#: Fax#:	E-mail:
Architect:	Contact:
Address:	
Phone#: Fax#:	E-mail:
Other Contact:	Contact:
Address:	
Phone#: Fax#:	E-mail:
TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTROL TYPE OF SUBMITTAL: ENGINEER/ ARCHITECT CERTIFICATION CONCEPTUAL G & D PLAN GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT	CERTIFICATE OF OCCUPANCY PRELIMINARY PLAT APPROVAL SITE PLAN FOR SUB'D APPROVAL FINAL PLAT APPROVAL SIA/ RELEASE OF FINANCIAL GUARANTEE FOUNDATION PERMIT APPROVAL GRADING PERMIT APPROVAL
CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT (TCL) TRAFFIC IMPACT STUDY (TIS-NIA) EROSION & SEDIMENT CONTROL PLAN (ESC)	SO-19 APPROVAL PAVING PERMIT APPROVAL GRADING/ PAD CERTIFICATION WORK ORDER APPROVAL CLOMR/LOMR
OTHER (SPECIFY) IS THIS A RESUBMITTAL?: YesNo	PRE-DESIGN MEETING OTHER (SPECIFY)
	Ву:

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ____

DRAINAGE PLAN

EXECUTIVE SUMMARY AND INTRODUCTION

THE PROPOSED MARK 3S HOLLY EXPANSION DEVELOPMENT IS LOCATED WITHIN THE LA CUEVA SECTOR PLAN AREA OF NORTH ALBUQUERQUE ACRES. THE EXPANSION INCORPORATES ADDITIONAL PROPERTY WEST OF THE EXISTING DEVELOPMENT TO EXPAND THE EXISTING SCHOOL CAMPUS. THE SITE DEVELOPMENT PLANS FOR THE PROPOSED EXPANSION WERE APPROVED THROUGH EPC AND DRB AS AN AMENDMENT TO A PREVIOUSLY APPROVED PLAN. A CONCEPTUAL GRADING PLAN WAS INCLUDED IN THE EPC SET AND THIS SUBMITTAL IS CONSISTENT WITH THAT PLAN. MOST OF THE REQUIRED PUBLIC INFRASTRUCTURE IN HOLLY AVENUE NE HAS BEEN CONSTRUCTED BY PREVIOUS PROJECTS DESIGNED AND INSPECTED BY THIS ENGINEER (LOS VIGILS, VINEYARD COURT ESTATES, MARK 3S HOLLY IMPROVEMENTS). THE REMAINING PORTIONS REQUIRED FOR THIS PROJECT ARE THE PERMANENT HALF-WIDTH PAVING AND THE CONSTRUCTION OF TWO STORM INLETS. THE SITE DISCHARGES FREELY TO PUBLIC DRAINAGE IMPROVEMENTS IN HOLLY THAT WERE DESIGNED AND SIZED FOR THIS DISCHARGE. SITE RUNOFF WILL BE DIRECTED TO DEPRESSED WATER QUALITY AREAS PRIOR TO DISCHARGING TO HOLLY. CONCURENT WITH THE DRB SITE PLAN APPROVALS. A PLATTING ACTION WAS ALSO DONE TO SUPPORT THE LOT LINE ELIMINATIONS, NEW LOTS LINES AND VACATION OF 2 FEET OF HOLLY RIGHT-OF-WAY BEING INCORPORATED INTO THE SITE. THE PURPOSE OF THIS PLAN IS TO OBTAIN BUILDING PERMIT APPROVAL.

PROJECT DESCRIPTION: 11.

AS SHOWN BY VICINITY MAP C-20 LOCATED HEREON. THE SITE IS LOCATED IN THE NORTH ALBUQUERQUE ACRES SECTION OF ALBUQUERQUE, ON HOLLY AVE NE BETWEEN VENTURA ST. N.E. AND HOLBROOK STREET N.E. THE EXISTING LEGAL DESCRIPTION IS TRACT A-1 MARK 3S HOLLY DEVELOPMENT. THE SITE IS ZONED SU-2/MIXED USE AND THE PROPOSED DEVELOPMENT IS CONSISTENT WITH THE ZONING AND THE APPROVED SITE DEVELOPMENT PLANS.

AS SHOWN BY PANEL 141 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS. BERNALILLO COUNTY. NEW MEXICO, AND INCORPORATED AREAS, DATED SEPTEMBER 26, 2008, THE SITE DOES NOT LIE WITHIN THE VICINITY OF ANY DESIGNATED FLOOD HAZARD ZONES.

III. BACKGROUND DOCUMENTS

THE FOLLOWING IS A LIST OF DOCUMENTS RELATED TO THE SITE AND SURROUNDING AREA. THIS LIST MAY NOT BE INCLUSIVE. HOWEVER. REPRESENTS A SUMMARY OF RELEVANT PLANS AND DOCUMENTS WHICH ARE KNOWN TO THE ENGINEER AT THE TIME OF PLAN PRFPARATION

A. THE "NORTH AND SOUTH DOMINGO BACA ARROYO AND PASEO DEL NORTE CORRIDOR DRAINAGE MANAGEMENT PLAN" PREPARED FOR AMAFCA BY RESOURCE TECHNOLOGY, INC. (RTI) DATED DECEMBER, 1991. THIS PLAN HAS BEEN ADOPTED BY AMAFCA AS A GUIDELINE FOR DRAINAGE MANAGEMENT WITHIN THIS AREA WHICH INCLUDES THE NORTH DOMINGO BACA ARROYO (NDBA). AMAFCA RESOLUTION 1992-3 DATED JANUARY 03, 1992 FORMALLY ADOPTED THIS PLAN WHICH IDENTIFIES THE EXTENSION OF PERMANENT DRAINAGE IMPROVEMENTS WITHIN THE NDBA CORRIDOR, AND ESTABLISHED DEVELOPED DRAINAGE BASIN BOUNDARIES WITHIN THE PLAN AREAS. AS SHOWN BY THIS PLAN, THIS SITE IS IDENTIFIED TO DRAIN TO PUBLIC STORM DRAIN IMPROVEMENTS CONSTRUCTED WITHIN THE HOLLY RIGHT-OF-WAY

B. REQUEST FOR LETTER OF MAP REVISION (LOMR) FOR THE NORTH DOMINGO BACA ARROYO CARMEL AVENUE STORM DRAIN EXTENSION PREPARED BY JMA DATED 12/08/2003 AND APPROVED BY FEMA 03/23/2004 (FEMA CASE NUMBER 04-06-671P). THIS LOMR SUPPORTS THE COMPLETED NORTH DOMINGO BACA ARROYO CARMEL AVENUE STORM DRAIN EXTENSION COST SHARE PROJECT BY AMAFCA UPON FEMA APPROVAL, AND IT REMOVED THE ASSOCIATED FLOODPLAIN DESIGNATION FROM THE NDBA WEST OF A POINT MIDBLOCK BETWEEN HOLBROOK STREET AND EUBANK.

C. DRAINAGE REPORT FOR "LOS VIGILS SUBDIVISION" BY HIGH MESA CONSULTING GROUP FORMERLY KNOWN AS JEFF MORTENSEN & ASSOCIATES, INC, DATED 12/31/2002, HYDROLOGY FILE C20/D41. THIS PLAN THE CONSTRUCTION OF A 45 LOT RESIDENTIAL SUBDIVISION LOCATED TO THE NORTH OF THIS PROJECT ON THE NORTH SIDE OF HOLLY. THE LOS VIGILS PROJECT EXTENDED THE HOLLY STORM DRAIN ACROSS THE FRONTAGE OF THIS SITE AND DESIGNED THE REQUIRED INLETS ON THE SOUTH SIDE WHICH MUST NOW BE CONSTRUCTED. A BASIN MAP, STREET HYDRAULICS AND STORM DRAIN HYDRAULICS ANALYSIS WERE INCLUDED IN THIS SUBMITTAL TO ADDRESS THE EXTENSION OF HOLLY TO HOLBROOK ALL DEVELOPMENT ON HOLLY, INCLUDING THIS SITE.

D. GRADING PLAN FOR "DESERT RIDGE OFFICE PARK" BY JMA, HYDROLOGY FILE C20/D51. THIS PLAN WAS APPROVED FOR THE UPSTREAM SITE IMMEDIATELY TO THE EAST (PROJECT # 1003277). E. DRAINAGE REPORT FOR "VINEYARD COURT ESTATES" BY HIGH MESA CONSULTING GROUP FORMERLY KNOWN AS JEFF MORTNESEN &

ASSOCIATES, INC, DATED 08/21/2003. THIS PLAN WAS FOR THE CONSTRUCTION OF A 45 LOT RESIDENTIAL SUBDIVISION LOCATED TO THE NORTHEAST OF THIS PROJECT ON THE NORTH SIDE OF HOLLY. THE VINEYARD COURT ESTATES PROJECT EXTENDED THE HOLLY STORM DRAIN AND STREET IMPROVEMENTS ACROSS ITS FRONTAGE. A BASIN MAP, STREET HYDRAULICS AND STORM DRAIN HYDRAULICS ANALYSIS WERE INCLUDED IN THIS SUBMITTAL TO ADDRESS THE EXTENSION OF HOLLY TO HOLBROOK AND ALL DEVELOPMENT ON HOLLY, INCLUDING THIS SITE.

F. GRADING AND DRAINAGE PLANS FOR MARK 3S HOLLY DEVELOPMENT BY HIGH MESA CONSULTING GROUP FORMERLY KNOWN AS JEFF MORTENSEN & ASSOCIATES, INC DATED 05/11/2015. THIS PLAN FOLLOWED THE DRAINAGE OF CONCEPT OF FREE DISCHARGE TO THE PERMANENT PUBLIC HOLLY PAVING AND STORM DRAINAGE IMPROVEMENTS WHICH IS CONSISTENT WITH PREVIOUSLY APPROVED PLANS FOR NDBA DEVELOPMENT WHICH INCLUDES THIS SITE. THE DRAINAGE CONCPETS PRESENTED THEREIN WILL BE CONSISTENT WITH THOSE NOW PROPOSED.

THE PROPOSED CONSTRUCTION DRAINING DIRECTLY AND FREELY TO PERMANENT HOLLY AVENUE NE DRAINAGE IMPROVEMENTS AS PROPOSED AND DESCRIBED HEREIN IS IN ACCORDANCE WITH THE POLICIES AND REQUIREMENTS OF THE ABOVE LISTED DOCUMENTS. AND IS CONSISTENT WITH THE CONCEPTS PREVIOUSLY ESTABLISHED BY THE CITY AND AMAFCA FOR NDBA DEVELOPMENT

IV. EXISTING CONDITIONS:

THE DEVELOPMENT TO BE EXPANDED IS CURRENTLY DEVELOPED AS A MONTESSORI SCHOOL WITH PAVED PARKING, UTILITY AND LANDSCAPING IMPROVEMENTS. THE ADJACENT LOT TO BE EXPANDED INTO WAS PREVIOUSLY DEVELOPED INTO A LANDSCAPING BUSINESS. EXISTING RUNOFF FOR BOTH SITES DRAIN TO HOLLY AVE NE TO EXISTING DOWNSTREAM PUBLIC STORM DRAIN FACILITIES THAT WERE CONSTRUCTED BY LOS VIGILS AND THE PREVIOUS MARK 3S HOLLY DEVELOPMENT (REF. C & H). HOLLY AVE NE TO THE NORTH IS A PUBLIC STREET WITH HALF-WIDTH (NORTH) PERMANENT PAVING IMPROVEMENTS. THE UPSTEAM SECTION OF HOLLY HAS PERMANENT FULL WIDTH IMPROVEMENTS. CONSTRUCTED BY VINEYARD COURT ESTATES, CPN 718781. PASEO DEL NORTE TO THE SOUTH IS A FULLY DEVELOPED PUBLIC STREET WITH A DRAINAGE DITCH, PUBLIC STORM DRAIN, AND PAVED ASPHALT TRAIL

OFFSITE FLOWS DO NOT ENTER THE SITE FROM THE DEVELOPED SITE TO THE EAST OR FROM THE PUBLIC STREETS TO THE NORTH AND SOUTH WHICH EXHIBIT PARALLEL TOPOGRAPHY. THE UNDEVELOPED SITE TO THE WEST IS TOPOGRAPHICALLY LOWER AND INCAPABLE OF CONTRIBUTING OFFSITE FLOWS.

V. DEVELOPED CONDITIONS

THE PROPOSED IMPROVEMENTS CONSIST OF DEMOLITION AND REMOVAL OF EXISTING LANDSCAPING BUSINESS TO ALLOW FOR EXPANSION OF CURRENT MONTESSORI SCHOOL CAMPUS. THE EXPANSION WILL INCLUDE A NEW INFANT DAYCARE BUILDING WITH PAVED PARKING. UTILITY, AND LANDSCAPING IMPROVEMENTS. THE SITE WILL CONTINUE TO DISCHARGE FREELY INTO HOLLY AVE NE. PRIOR TO DISCHARGING OFF SITE, RUNOFF WILL BE DIRECTED TO LANDSCAPED AREAS DEPRESSED TO THE MAXIMUM EXTENT POSSIBLE TO RETAIN THE 80TH PERCENTILE FIRST FLUSH FOR WATER QUALITY AND ROOF DRAINAGE WILL BE PIPED TO A DIRECT STORM DRAIN CONNECTION BUT WILL FIRST BE ROUTED THROUGH A STORM WATER QUALITY MANHOLE TO TREAT FIRST FLUSH.

AS DEMONSTRATED BY THE STREET HYDRAULIC, STORM DRAIN AND INLET CALCULATIONS AND ANALYSIS CONTAINED WITHIN THE DRAINAGE REPORTS FOR LOS VIGILS AND VINEYARD COURT ESTATES. THE HOLLY STORM DRAIN AND STREET IS DESIGNED TO ACCEPT THE FREE DISCHARGE OF FULLY DEVELOPED RUNOFF FROM THE PROPERTIES FRONTING ON HOLLY, INCLUDING THIS SITE. ALL IMPROVEMENTS PROPOSED HEREIN ARE CONSISTENT WITH THE PREVIOUSLY APPROVED DEVELOPMENT PLANS FOR THIS SECTION OF HOLLY.

VI. GRADING PLAN

THE GRADING PLAN SHOWS: 1) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1 FT INTERVALS FROM THE HMCG TOPO SURVEY DATED 01/10/2017 & 05/16/2017, 2) PROPOSED GRADES INDICATED BY FINISHED FLOOR ELEVATIONS, SPOT ELEVATIONS, AND CONTOURS AT 1 FT INTERVALS, 3) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS, 4) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, AND 5) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES.

VII. CALCULATIONS

THE CALCULATIONS, REPRODUCED FORM THE APPROVED CONCEPTUAL GRADING PLAN AND WHICH APPEAR HEREON, ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40-ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY, 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS DEMONSTRATED BY THE LOMR AND APPROVED DRAINAGE REPORTS PREPARED BY THIS OFFICE TO SUPPORT THE CONSTRUCTED AMAFCA NDBA PROJECT AND FOR LOS VIGILS, VINEYARD COURT ESTATES, AND THE DESERT RIDGE OFFICE PARK PROJECT (SEE REFERENCES), THE PUBLIC STORM DRAIN IN HOLLY IS SIZED FOR FREE DISCHARGE OF FULLY DEVELOPED RUNOFF FROM THIS SITE.

IX. CONCLUSION

1) THE PROPOSED SITE IMPROVEMENTS AND DRAINAGE CONCEPT ARE CONSISTENT WITH THE DEVELOPMENT CRITERIA ESTABLISHED BY PREVIOUSLY APPROVED PLANS FOR NDBA DEVELOPMENT AND THIS SPECIFIC PROJECT. 2) DEVELOPED RUNOFF FROM THIS SITE WILL DRAIN FREELY TO PERMANENT PUBLIC HOLLY PAVING AND STORM DRAINAGE IMPROVEMENTS, WHICH WERE CONSTRUCTED FOR LOS VIGILS AND VINEYARD COURT ESTATES. 3) THERE ARE NO DPM DESIGN VARIANCES, DRAINAGE EASEMENTS OR DRAINAGE COVENANTS ANTICIPATED AT THIS TIME.



6010-B Midway Park Blvd. NE • Albuquerque, New Mexico 87109 Phone: 505.345.4250 • Fax: 505.345.4254 • www.highmesacg.com

MARK 3S HOLLY DEVELOPMENT

COVER SHEET

E. FIRST FLUSH CALCULATION	ONS -SEE SHEET	CG-101-B

 $Q_{CAP} = 6.58 \text{ CFS} > Q_{REQ} = 6.4 \text{ CFS}$

Q_{CAP}= 3.0*4.0*0.67^(3/2)

 $Q_{REQ} = 6.4CFS$ $Q_{CAP} = C^*L^*H^{(3/2)}$ C=3.0 ,L =4 FT, H = 0.67 FT (8" CURB)

D. BASIN A RUNDOWN OVERFLOW CALCULATIONS

$V_{100} = (E_W/12)A_T = (2.11/12)1.7$	3 = 0.3042	AC-FT =	13,250 CF
b. PEAK DISCHARGE			
$\mathbf{Q}_{P} = \mathbf{Q}_{PA}\mathbf{A}_{A} + \mathbf{Q}_{PB}\mathbf{A}_{B} + \mathbf{Q}_{PC}\mathbf{A}_{C} + \mathbf{Q}_{PD}\mathbf{A}_{D}$			
$Q_{P} = Q_{100} = ((0.00^{*}1.87) + (0.00^{*}2.6) + 0.00^{*}2.6))$	(0.40*3.45) + (1.34*5	.02)) =	8.1 CFS
2. BASIN B			
a. VOLUME			
$\mathbf{E}_{W} = (\mathbf{E}_{A}\mathbf{A}_{A} + \mathbf{E}_{B}\mathbf{A}_{B} + \mathbf{E}_{C}\mathbf{A}_{C} + \mathbf{E}_{D}\mathbf{A}_{D})/\mathbf{A}_{T}$			
$E_{VV} = ((0.00^*0.66) + (0.00^*0.92) +$	(0.12*1.29) + (0.65*	2.36))/0.77 =	2.20 IN
$V_{100} = (E_W/12)A_T = (2.20/12)0.7$	7 = 0.1412	AC-FT =	6,150 CF
b. PEAK DISCHARGE			
$Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B} + Q_{PC}A_{C} + Q_{PD}A_{D}$			
$Q_{\rm P} = Q_{100} = ((0.00^*1.87) + (0.00^*2.6) + (0.00^*2.6))$	(0 12*3 45) + (0 65*5	(02)) =	3.7 CFS
))	
C. COMPARISON			
1. BASIN A			
a. VOLUME			
ΔV ₁₀₀ = 13,250 - 7,420 =	5830.00 CF	78.6%	(INCREASE)
b. PEAK DISCHARGE			
$\Delta Q_{100} = 8.1 - 5.0 =$	3.10 CFS	62.0 %	(INCREASE)
2. BASIN B			
a. VOLUME			
ΔV ₁₀₀ = 6,150 - 9,040 =	-2890.00 CF	-32.0%	(DECREASE)
b. PEAK DISCHARGE			
$\Delta Q_{100} = 3.7 - 5.5 =$	-1.80 CFS	-32.7%	(DECREASE)
3. COMBINED OVERALL			
a. VOLUME			
ΔV ₁₀₀ = 19,400 - 16,460	2940.00 CF	17.9%	(INCREASE)
b. PEAK DISCHARGE			
$\Delta Q_{100} = = 11.8 - 10.5 =$	1.30 CFS	12.4%	(INCREASE)
c. DISCHARGE RATE			
11.8 CFS / 2.49 AC =	4.74 CFS/AC	(4.86 CFS/AC A	LLOWABLE)

$E_{VV} = ((0.00))$
V ₁₀₀ = (E _v

a. VOLUME $E_{W} = (E_{A}A_{A} + E_{B}A_{B} + E_{C}A_{C} + E_{D}A_{D})/A_{T}$ D*0.66) + (0.00*0.92) + (0.40*1.29) + (1.34*2.36))/1.73 =

B. DEVELOPED CONDITION 1. BASIN A

b. PEAK DISCHARGE

2. BASIN B
a. VOLUME
$\mathbf{E}_{W} = (\mathbf{E}_{A}\mathbf{A}_{A} + \mathbf{E}_{B}\mathbf{A}_{B} + \mathbf{E}_{C}\mathbf{A}_{C} + \mathbf{E}_{D}\mathbf{A}_{D})/\mathbf{A}_{T}$
$E_{W} = ((0.00^{*}0.66) + (0.12^{*}0.92) + (0.12^{*}1.29) + (0.95^{*}2.36))/1.18 =$

b. PEAK DISCHARGE

a. VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$

 $\mathbf{Q}_{\mathrm{P}} = \mathbf{Q}_{\mathrm{PA}}\mathbf{A}_{\mathrm{A}} + \mathbf{Q}_{\mathrm{PB}}\mathbf{A}_{\mathrm{B}} + \mathbf{Q}_{\mathrm{PC}}\mathbf{A}_{\mathrm{C}} + \mathbf{Q}_{\mathrm{PD}}\mathbf{A}_{\mathrm{D}}$

 $\mathbf{Q}_{\mathrm{P}} = \mathbf{Q}_{\mathrm{PA}}\mathbf{A}_{\mathrm{A}} + \mathbf{Q}_{\mathrm{PB}}\mathbf{A}_{\mathrm{B}} + \mathbf{Q}_{\mathrm{PC}}\mathbf{A}_{\mathrm{C}} + \mathbf{Q}_{\mathrm{PD}}\mathbf{A}_{\mathrm{D}}$

CALCULATIONS

B. P_{6,100} = P₃₆₀ =

I. SITE CHARACTERISTICS

D. LAND TREATMENTS

a. BASIN A

A. PRECIPITATION ZONE =

C. TOTAL PROJECT AREA $(A_T) = -$

1. EXISTING LAND TREATMENT

Treatment **A** Area

Treatment **B** Area

Treatment **C** Area

Treatment **D** Area

BASIN B	AREA (S	F/ AC)	
Total Area	33,410	/	0.77	%
Treatment A Area				
Treatment B Area				
Treatment C Area	5,025	/	0.12	15
Treatment D Area	28,385	1	0.65	85
	Treatment A Area Treatment B Area Treatment C Area	Treatment A AreaTreatment B AreaTreatment C Area5,025	Treatment A AreaTreatment B AreaTreatment C Area5,025 /	Treatment A AreaTreatment B AreaTreatment C Area5,025 / 0.12

 $V_{100} = (E_W/12)A_T = (1.56/12)1.31 =$

V₁₀₀ = (E_W/12)A_T = (2.11/12)1.18 =

 $Q_{P} = Q_{100} = ((0.00*1.87) + (0.00*2.6) + (0.98*3.45) + (0.33*5.02)) =$

 $Q_{\rm P} = Q_{100} = ((0.00*1.87) + (0.12*2.6) + (0.12*3.45) + (0.95*5.02)) =$

 $E_{W} = ((0.00*0.66) + (0.00*0.92) + (0.98*1.29) + (0.33*2.36))/1.31 =$

	Total Area	75,515	/	1.73	%
	Treatment A Area				
	Treatment B Area				
	Treatment C Area	17,300	/	0.40	23
	Treatment D Area	58,215	/	1.34	77
b.	BASIN B	AREA (S	F/ AC)	
	Total Area	33,410	/	0.77	%
	Treatment A Area				
	Treatment B Area				
	Treatment C Area	E 0.2E	1	0 1 2	15

<u> </u>					
a.	BASIN A	AREA (S	F/ AC)	
	Total Area	75,515	/	1.73	%
	Treatment A Area				
	Treatment B Area				
	Treatment C Area	17,300	/	0.40	23
	Treatment D Area	58,215	/	1.34	77
					_
b.	BASIN B	AREA (· · ·		
	Total Area	33,410	/	0.77	%
	Treatment A Area				

	Treatment D Area	41,200	/	0.95	80
2.	DEVELOPED LAND TREAT	MENT			
a.	BASIN A	AREA ((S	F/ AC)	
	Total Area	75,515	1	1.73	%
	Treatment A Area				
	Treatment B Area				
	Treatment C Area	17,300	/	0.40	23
	Treatment D Area	58,215	/	1.34	77
					_
b.	BASIN B	AREA ((S	F/ AC)	
	Total Area	22 /10	1	0.77	0/

2.	DEVELOPED LAND TREATIN	<u>MENT</u>			
a.	BASIN A	AREA (
	Total Area	75,515	1	1.73	%
	Treatment A Area				
	Treatment B Area				
	Treatment C Area	17,300	/	0.40	23
	Treatment D Area	58,215	/	1.34	77
b.	BASIN B	AREA (S	F/AC)	
	Total Area	33,410	/	0.77	%

		5,150	Ľ	0.12	10
	Treatment C Area	5,150	1	0.12	10
	Treatment D Area	41,200	/	0.95	80
<u>2.</u>	DEVELOPED LAND TREAT	<u>MENT</u>			
a.	BASIN A	AREA ((S	F/ AC)	ſ
	Total Area	75,515	1	1.73	%
	Treatment A Area				
	Treatment B Area				
	Treatment C Area	17,300	/	0.40	23
	Treatment D Area	58,215	/	1.34	77

b.	BASIN B	AREA ((S	F/AC)]
	Total Area	51,500	/	1.18	
	Treatment A Area				
	Treatment B Area	5,150	1	0.12	
	Treatment C Area	5,150	/	0.12	
	Treatment D Area	41,200	/	0.95	8
<u>2.</u>	DEVELOPED LAND TREAT	MENT			•
			~		Т

ASIN B	AREA ((S	F/ AC)	
Total Area	51,500	/	1.18	%
Treatment A Area				
Treatment B Area	5,150	/	0.12	10
Treatment C Area	5,150	/	0.12	10
Treatment D Area	41,200	/	0.95	80
EVELOPED LAND TREAT	MENT			
ASIN A	AREA ((S	F/ AC)	

Total Area	51,500	/	1.18	%
Treatment A Area				
Treatment B Area	5,150	/	0.12	10
Treatment C Area	5,150	/	0.12	10
Treatment D Area	41,200	/	0.95	80
DEVELOPED LAND TREAT	MENT			
BASIN A	AREA (S	F/ AC)	
Total Area	76 646	1	4 70	0/

Treatment A Area				
Treatment B Area	5,150	/	0.12	10
Treatment C Area	5,150	/	0.12	10
Treatment D Area	41,200	/	0.95	80
DEVELOPED LAND TREAT	<u>MENT</u>			
BASIN A	AREA ((S	F/ AC)	
Total Area	75,515	1	1.73	%

	fieatment A Alea				
	Treatment B Area	5,150	/	0.12	10
	Treatment C Area	5,150	/	0.12	10
	Treatment D Area	41,200	/	0.95	80
	DEVELOPED LAND TREAT	<u>MENT</u>			
۱.	BASIN A	AREA ((S	F/ AC)	
	Total Area	75,515	1	1.73	%

	Total Area	76 616	1	1 70	0/
a.	BASIN A	AREA (
2.	DEVELOPED LAND TREAT	<u>/IENT</u>			
	Treatment D Area	41,200	/	0.95	80
	Treatment C Area	5,150	/	0.12	10
	Treatment B Area	5,150	1	0.12	10
	Treatment A Area				

Treatment B Area	5,150	/	0.12	10
Treatment C Area	5,150	/	0.12	10
Treatment D Area	41,200	/	0.95	80
DEVELOPED LAND TREATIN	<u>/ENT</u>			
BASIN A	AREA ((S	F/ AC)	
Total Area	75,515	1	1.73	%
Treatment A Area				

Treatment B Area	5,150	/	0.12	10
Treatment C Area	5,150	/	0.12	10
Treatment D Area	41,200	1	0.95	80
EVELOPED LAND TREATIN	<u>/ENT</u>			
BASIN A	AREA ((S	F/ AC)	
Total Area	75,515	/	1.73	%

Treatment D Area	41,200	/	0.95	80
DEVELOPED LAND TREAT	<u>/ENT</u>			
BASIN A	AREA (
Total Area	75,515	/	1.73	%
Treatment A Area				
Treatment B Area				
Treatment C Area	17.300	1	0.40	23

Treatment D Area	41,200	/	0.95	80	
EVELOPED LAND TREAT					
ASIN A	AREA (AREA (SF/ AC)			
Total Area	75,515	/	1.73	%	
Treatment A Area					
Treatment B Area					
Treatment C Area	17,300	1	0.40	23	

Treatment D Area	41,200	/	0.95	80				
EVELOPED LAND TREATMENT								
ASIN A]							
Total Area	75,515	1	1.73	%				
Treatment A Area								
Treatment B Area								
Treatment C Area	17,300	/	0.40	23				

	0.,000	ľ		, [,] ,
Treatment A Area				
Treatment B Area	5,150	1	0.12	10
Treatment C Area	5,150	/	0.12	10
Treatment D Area	41,200	1	0.95	80
EVELOPED LAND TREAT	<u>MENT</u>			
ASIN A	AREA (SF/ AC)			
Total Area	75,515	/	1.73	%

2.60

Total Area 57,025 / 1.31

108.525 SF

2.49 AC

AREA (SF/AC)

42,769//0.98

14,256 / **0.33**

	01,000	'	1.10	/0
Treatment A Area				
Treatment B Area	5,150	1	0.12	10
Treatment C Area	5,150	/	0.12	10
Treatment D Area	41,200	1	0.95	80
EVELOPED LAND TREATIN	<u>MENT</u>			
ASIN A	AREA (S	F/ AC)	

	17,300			23
	58,215	/	1.34	77
	AREA (
rea	33,410	/	0.77	%
		-		

0.1703 AC-FT =

0.2075 AC-FT =

%

25

1.56 IN

7,420 CF

5.0 CFS

2.11 IN

9,040 CF

5.5 CFS

2.11 IN

MLP
OHC(1)
OHE(1)
OHG(2)
PG
PS
PVC
RCP
ROH
RR
RRT
SAS
SAS/PM
SB (

PIPE GATE SD/PM SDMH

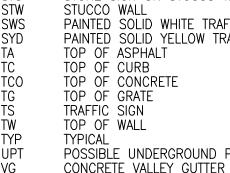
ANDSCAPING RAILROAD SANITARY SEWER SANITARY SEWER BY PAIL PAINTED TRAFFIC STOP E STORM DRAIN BY PAINT STORM DRAIN INLET STORM DRAIN MANHOLE SERVICE DROP POLE SILT FENCE (IN POOR CO

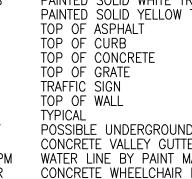
CMU SIGN SLIDING GATE

STEEL POLE

STONE SIGN ON STUCCO STUCCO WALL PAINTED SOLID WHITE TR PAINTED SOLID YELLOW TOP OF ASPHALT TOP OF CURB TOP OF CONCRETE TOP OF GRATE TRAFFIC SIGN TOP OF WALL

TYPICAL POSSIBLE UNDERGROUND CONCRETE VALLEY GUTTE WATER LINE BY PAINT M







ASPHALT CURB

AREA LIGHT

LEGEND

AC

BBC BCSW

BTM

C&G

CMU

CND CONC CRW

CSW

E/PM

G/PM

GRV

GTS

GWAP

INV

IVB

KSW

LSD MED

MH

SDP

SGN SLGT SLP

STC

STS

WPP

WVB

WWC

C

Ö

 \bigcirc

XC

GW

DYS

СМ

LANDSCAPING CRUSHER POURED CONCRETE MOUN

CONCRETE MASONRY UNI

BRICK BUILDING COLUMN

BURIED CONCRETE SIDEW

CURB AND GUTTER

ELECTRIC CONDUIT CMU RETAINING WALL CONCRETE SIDEWALK

PAINTED DOUBLE YELLOW ELECTRIC LINE BY PAINT

LECTRIC METER

EDGE OF ASPHALT LECTRIC OUTLET

FIRE HYDRANT FLOWLINE GAS LINE BY PAINT MARK

LANDSCAPING GRAVEL GATE STOP POST GUY WIRE ANCHOR

GUY WIRE ANCHOR POLE

PIPE INVERT IRRIGATION VALVE BOX KEYSTONE WALL

LANDSCAPING IMPROVEM LANDSCAPING DIVIDER MFDIAN MANHOLF METAL LIGHT POLE OVERHEAD COMMUNICATIO OVERHEAD ELECTRIC (3 OVERHEAD GUY WIRE (#

PAINTED PARKING STALL POLYVINYL CHLORIDE PIF

REINFORCED CONCRETE ROOF OVERHANG LANDSCAPING RIVER ROC

SYNTHETIC LIGHT POLE SEPTIC TANK COVER

TYP UPT W/PM WCR OUTDOOR WATER FAUCET WLP WOOD LIGHT POLE

WMB

WATER METER BOX WOOD POWER POLE

WATER VALVE BOX

TREE TRUNK DIAMETER

EXTRUDED CONCRETE CU 1.0'ø

CONIFEROUS TREE

NML Ziz SMALL CONIFEROUS TREE

DECIDUOUS TREE

SHRUB

YUCCA

SMALL SHRUB

SMALL DECIDUOUS TREE

LANDSCAPING BOULDER

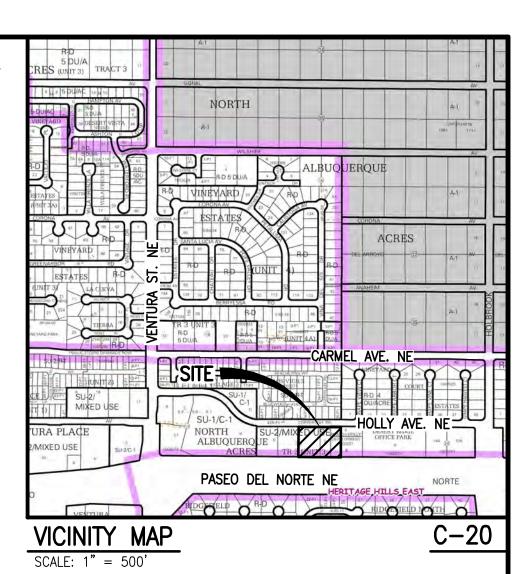
	FL	FL
	INV	IN
/ALK	MC	M(
FINES	TA	TC TC
ND T WALL	TC TG	TC TC
	+ 95.05	EX
	89.00	PF
TRAFFIC STRIPE	<u> </u>	EX
MARK		PF
	— — -5590- — —	EX
	<u> </u>	PF
K	\triangleleft	EX
	•	PF
		RI
		Ρl
		HI
INTS	*	RE
	¢	EX
	Ø	ΕX
ON (# OF LINES) OF LINES) OF LINES)	¥	PF
	\bigcirc	EX
STRIPE E		
PIPE	-	SA
K TIES	\bowtie	EX
		PF
NT MARK BAR		EX
MARK		PF
	Θ	EX
ONDITION)		PF
		EX
	Ð	PF
WALL	W	EX
AFFIC STRIPE TRAFFIC STRIPE	——	PF
	SAS	EX
	SAS	PF
	F	ЕX
	——F——	PF
PROPANE TANK R		EX
ARK RAMP		PF
		LC
		PF
IRB		PF

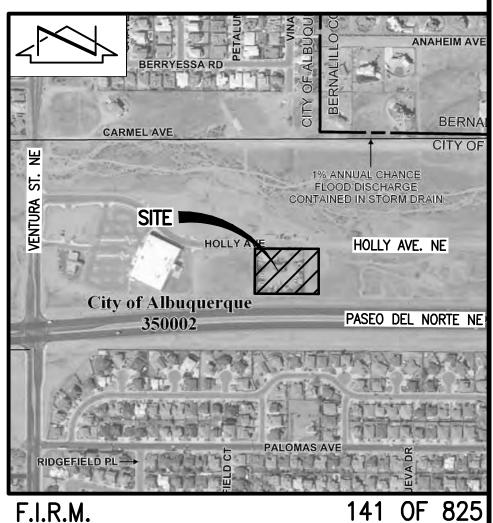
LOWLINE **VERT** MOTORCYCLE SPACES OP OF ASPHALT PAVEMENT OP OF CURB OP OF GRATE EXISTING SPOT ELEVATION PROPOSED SPOT ELEVATION EXISTING FLOWLINE PROPOSED FLOWLINE EXISTING CONTOUR PROPOSED CONTOUR EXISTING DIRECTION OF FLOW PROPOSED DIRECTION OF FLOW RIGHT OF WAY LINE UBLIC EASEMENT LINE HIGH POINT / DIVIDE ETAINING WALL EXISTING STORM DRAIN MANHOLE EXISTING FIRE HYDRANT PROPOSED FIRE HYDRANT EXISTING SANITARY SEWER MAN HOLE SANITARY SEWER MAN HOLE EXISTING VALVE BOX PROPOSED VALVE BOX EXISTING DOUBLE CLEANOUT PROPOSED DOUBLE CLEANOUT XISTING SINGLE CLEANOUT PROPOSED SINGLE CLEANOUT

EXISTING WATER SERVICE PROPOSED WATER SERVICE EXISTING WATER LINE PROPOSED WATER LINE EXISTING SANITARY SEWER LINE PROPOSED SANITARY SEWER LINE EXISTING FIRE LINE PROPOSED FIRE LINE XISTING POST INDICATOR VALVE PROPOSED POST INDICATOR VALVE OT LINE PROPOSED BASIN BOUNDARY

PROPOSED CONCRETE

PROPOSED ASPHALT PAVING





SCALE: 1'' = 500'

LEGAL DESCRIPTION

TRACT A-1, HOLLY MARK3S DEVELOPMENT FILED 12/18/2017 (2017C-0146)

TEMPORARY BENCHMARK #1 (T.B.M.)

A #5 REBAR W/CAP STAMPED "HMCG CONTROL NMPS 11184" SET IN DIRT, NEAR THE NORTHWEST CORNER OF LOT 7 IN THE NORTHWEST PORTION OF THE SITE. NOT SHOWN. ELEVATION = 5578.05 FEET (NAVD 88)

TEMPORARY BENCHMARK #2 (T.B.M.)

A MAG NAIL W/WASHER SET IN THE TOP OF A CONCRETE CURB, IN THE NORTHEASTERN PORTION OF THE SITE, AS SHOWN ON CG-101 AND CU-101. ELEVATION = 5592.04 FEET (NAVD 88)

TEMPORARY BENCHMARK #3 (T.B.M.)

A MAG NAIL W/WASHER SET IN AN ASPHALT PATH. IN THE SOUTHERN PORTION OF THE SITE, NOT SHOWN. ELEVATION = 5586.17 FEET (NAVD 88)

PAINTED UTILITY LINE MARK

SHEET

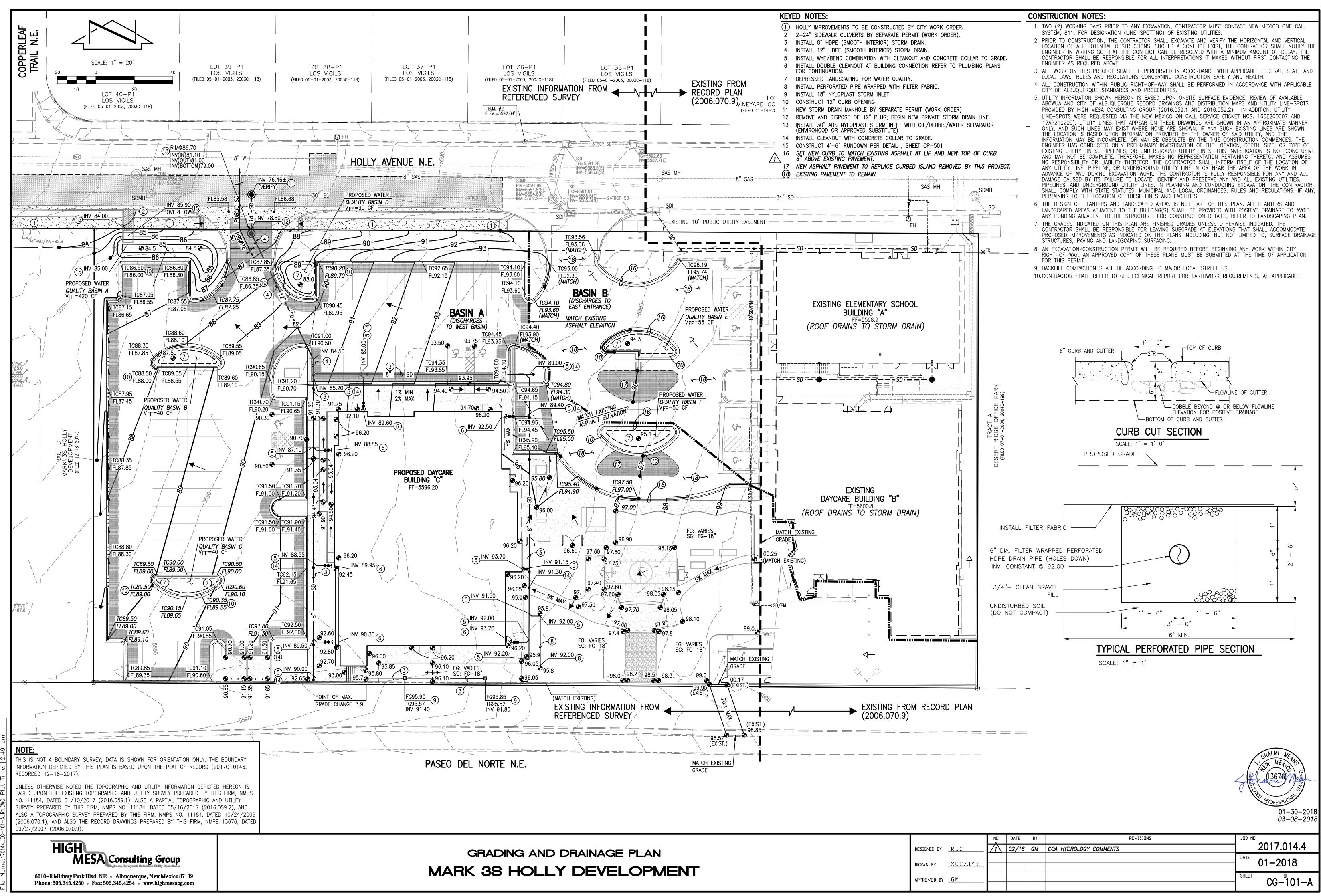
DES	CRIPT	ION	

INDEX OF DRAWINGS

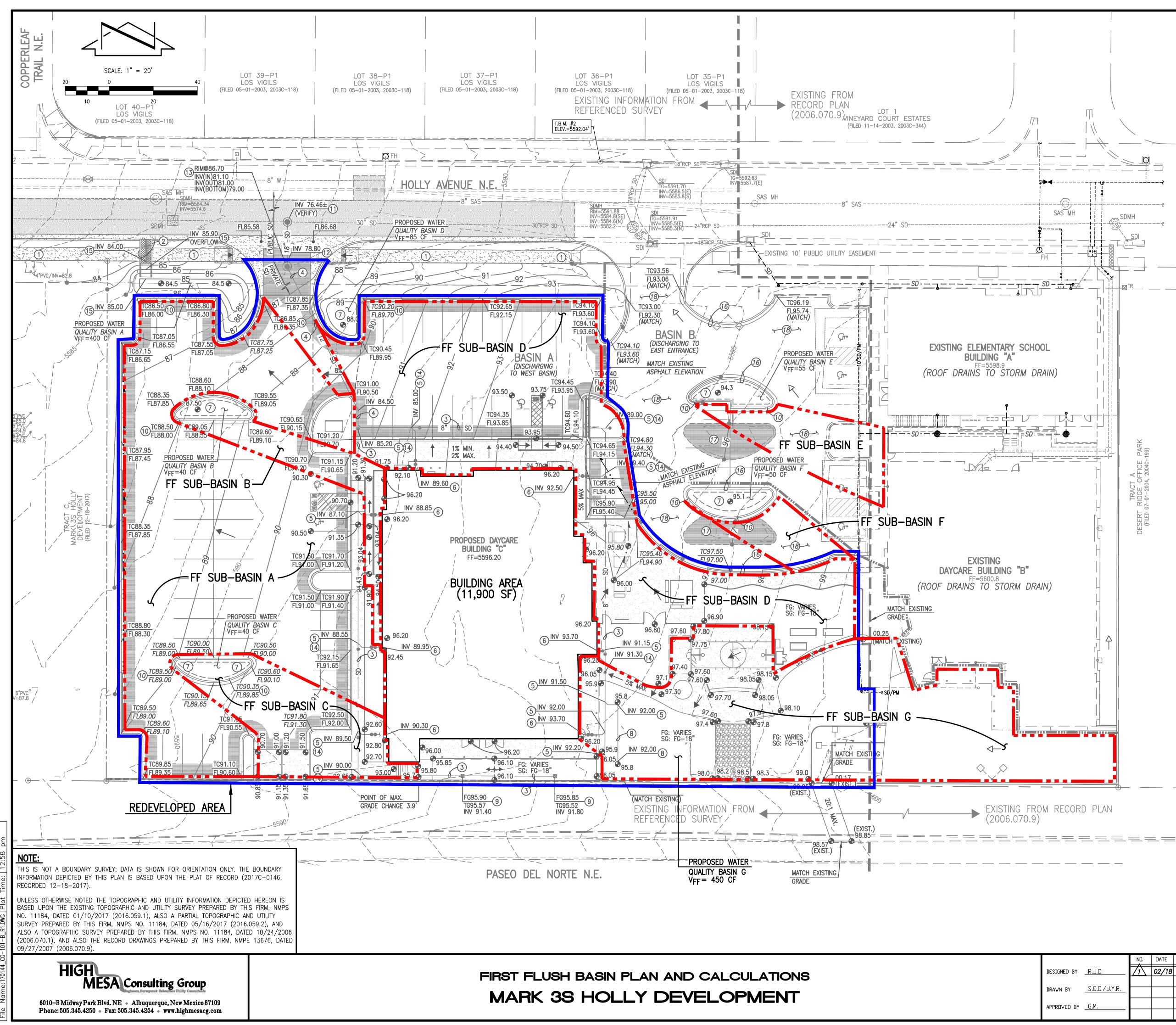
C-100 DRAINAGE PLAN, CALCULATIONS, VICINITY MAP, LEGEND AND INDEX OF DRAWINGS PREVIOUSLY CERTIFIED GRADING PLAN (FOR INFORMATION ONLY) C003 GRADING AND DRAINAGE PLAN CG-101-A /1 CG-101-B FIRST FLUSH BASIN PLAN AND CALCULATIONS CU-101 WATER AND SANITARY SEWER PLAN CP-501 PAVING SECTIONS AND DETAILS CU-501 WATER AND SANITARY SECTIONS AND DETAILS ESC-101 EROSION AND SEDIMENT CONTROL PLAN ESC-102 EROSION AND SEDIMENT CONTROL NOTES AND DETAILS



		ND.	DATE	BY	REVISIONS	JOB NO.
DESIGNED BY	R.J.C.	1	02/18	GM	COA HYDROLOGY COMMENTS	2017.014.4
						DATE 01-2018
DRAWN BY	<u>S.C.C./J.Y.R.</u>					01-2010
APPROVED BY	G.M.					SHEET C-100
						C-100



		ND.	DATE	BY	REVISIONS	JOB NO.
DESIGNED BY	R.J.C.	1	02/18	GM	COA HYDROLOGY COMMENTS	2017.014.4
						DATE 01-2018
DRAWN BY	<u>S.C.C./J.Y.R.</u>					01-2010
APPR⊡VED BY	G.M.					SHEET OF 101 A
						CG-101-A





FIRST FLUSH BASIN CALCULATIONS:

- 1. REDEVELOPED AREA
- a. REDEVELOPED AREA = 59,695 SF
- b. PERVIOUS AREA = 10,920 SF
- c. REDEVELOPED IMPERVIOUS AREA = 59,695 SF 10,920 SF = 48,775 SF
- 2. FIRST FLUSH VOLUME REDEVELOPED AREA $V_{FF REQ'D} = (0.26/12)(48,775 SF) = 1,060 CF$

3. PONDING / RETENTION AREA VOLUMES & IMPERVIOUS FF CONTRIBUTION

,	,			
AREA	$V_{POND CAP}$	A _{IMP} V _{II}	MP, FF = (0.26/12)(A _{IMP})	$V_{IMP, 100-YR}$
A =	410 CF	17,580 SF	380 CF	3,460 CF
B =	40 CF	1,490 SF	33 CF	290 CF
C =	40 CF	2,420 SF	53 CF	475 CF
D =	90 CF	12,460 SF	270 CF	2,450 CF
E =	55 CF	2,040 SF	45 CF	400 CF
F =	50 CF	1,150 SF	25 CF	230 CF
G =	810 CF*	4,550 SF	100 CF	900 CF

 $V_{TOTAL POND} = 1,480 \text{ CF} > V_{FF REQ'D} = 1,060 \text{ CF}$

AIMP = IMPERVIOUS AREA DISCHARGING TO POND

V_{IMP. FF} = FIRST FLUSH VOLUME GENERATED BY IMPERVIOUS AREA

V_{IMP.100-YR} = 100-YR VOLUME GENERATED BY IMPERVIOUS AREA TO EACH POND

*BASIN G IS A PROPOSED FIBER MULCH PLAY AREA WITH 18" DEPTH OF MULCH.

a. AREA= 3600 SF b. VOLUME = 0.3 (AIR VOIDS) x 3600 SF x 0.75 FT (AVG DEPTH)

VOLUME = 810 CF



		N□.	DATE	BY	REVISIONS	JOB NO	
DESIGNED BY	R.J.C.	1	02/18	GM	COA HYDROLOGY COMMENTS		2017.014.4
						DATE	01–2018
DRAWN BY	<u> </u>						01-2010
APPROVED BY	G.M.					SHEET	CG-101-B
							CG-101-D