

CITY OF ALBUQUERQUE

Planning Department
David Campbell, Director



Mayor Timothy M. Keller

September 28, 2018

David Thompson, P.E.
Thompson Engineering Consultants, Inc.
PO Box 65760
Albuquerque, NM 87122

**RE: Signal Village Subdivision
Signal and Ventura NE
Grading Plan Stamp Date: 9/10/18
Drainage Report Stamp Date: 9/10/18
Hydrology File: C20D078**

Dear Mr. Thompson:

PO Box 1293
Albuquerque
NM 87103
www.cabq.gov

Based on the submittal received 9/11/18, the above referenced project cannot be approved for Preliminary Plat until the following are corrected:

Prior to Preliminary Plat:

1. Show all infrastructure improvements on the grading plan, especially curb and gutter and sidewalk on Ventura. The return is not in the correct location at Signal/Ventura intersection. Ventura improvements must include curb and gutter and sidewalk and ½ section collector paving section for the full length of frontage. These improvements provide adequate protection from scour so a scour wall is not required on the east side of the project. Dimension face-to-face of curb, and sidewalks on both sides of the centerline. Show connections and transitions to the existing curb and pavement on Signal.
2. Show and label the existing and proposed contours, including all proposed grading both onsite and offsite and label limits of grading. Extend limits of existing topo to include the ROW on Ventura, south of the Ventura/Signal intersection; include curb returns.
3. Provide additional details for the first flush pond; show the water surface elevations for both the first flush volume and the 100-yr volume. Include supporting calculation for the overflow channel as well (Both the overflow channel and the pond will need to be dedicated as private drainage easements on the Plat). Correct notes on the grading and drainage plan under *Developed Drainage Condition* to describe the central first flush pond and its overflow north to the arroyo, not south to Signal.
4. Typical sections are required at all retaining walls (not just the scour wall) at the point of maximum retainage showing existing ground, proposed grades, lot lines, and dimensions.

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Typical sections are needed through north, south, east, west boundaries, and of all roads, onsite and frontage.

5. Typical sections need to show the ROW/property line or easement line. Show easement and lot lines in legend and on the grading and drainage plan and be sure they match the plat. Floodwall footings must be contained in an easement In accordance with DPM Ch.22, section 5 part B, Show that grading and wall construction near the property line does not endanger adjacent property or constrain its use. If any such encroachment is made the following must be obtained:
 - a. Any private encroachment into the public ROW will require a revocable permit.
 - b. Any private encroachment into a public easement will require an encroachment agreement.
 - c. Any private encroachment into neighboring private property will require written and signed permission from both property owners.
6. The HEC-RAS model of future conditions must include either 7 or 8 10'x6' CBCs with the invert at about 5562'. The future culvert must be 60' long and shown on the arroyo profile.
7. The typical section and profile of the east boundary must accommodate and show the future grade of Ventura and the grade proposed with this project.
8. The HEC-RAS model of proposed conditions must include a section at the east boundary of the subdivision showing the area south of the floodwall ineffective and a 45degree contraction upstream of that section. If the lot elevations next to Ventura are not elevated above the 100-yr WSEL, plus freeboard, then the east property line must be designated as a levee and all design requirements of FEMA (44CFR, Ch.1, Section 65.10) must be demonstrated in this report.
9. Show typical sections of the channel improvements on the grading and drainage plan with proposed contours in plan view and provide written permission and public easements for offsite grading.
10. Use HEC-RAS results to compare bed load sediment transport rates for reaches east of Ventura to arroyo north of the subdivision. Equilibrium slope and grade control structures will probably be required. Discuss and show all calculations.
11. The scour wall is shown as being constructed from CMU block which is constructed in level runs, but the profile shows the scour wall as sloping along its length. The location of the steps should be called out on the plan for the footer and top of wall.
12. The footing of the scour wall looks too small. It has to support the wall after the soil is removed from the low side to the scour depth. Provide structural calculations including loading assumptions.

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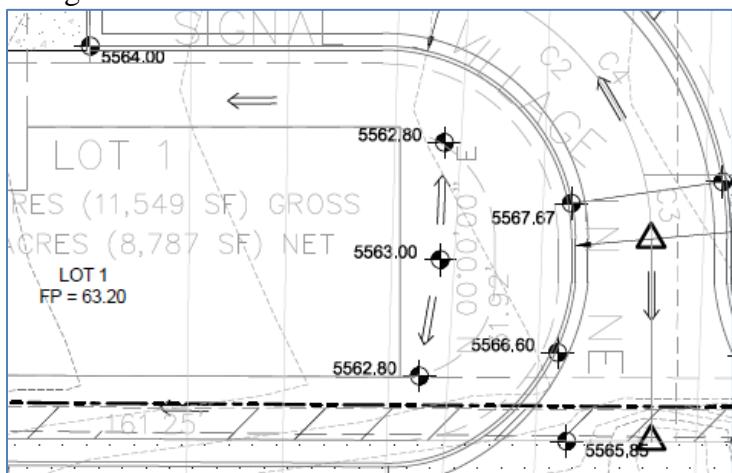
13. Provide the scour depth calculations (equations and the excel file). Be sure to update after correcting the HEC-RAS model for future conditions with the culverts at Ventura.
14. On the profile of the arroyo and proposed scour wall, provide proposed grades through the future culverts, provide elevation of the finished grade on both sides of the wall, top and bottom of the wall (these should match those shown in plan view), correct *Bottom of Wall* label, confirm that the water surfaces shown are for the critical or subcritical depths (not supercritical). Show as built elevations of existing downstream floodwall based on new survey, not engineer's certification from Signal Point because that datum is inaccurate.
15. Pad elevation and label for Lot 5 is missing.
16. Will there be a retaining wall between Signal Village Ln and Lot 1 due to the ~4' grade change?:

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17. In the report under "Methodology" explain that the encroachment on north over bank is not part of this project but is instead part of a future project being modeled now as a part of this project so we can be sure that this project is designed to accommodate that separate future project on the north bank. The future project is not being designed or reviewed in detail with this plan and will not be constructed with this project nor included in the LOMR for this project. Add a note to this effect on the G&D Plan and show the future contours associated with the encroachment. If this encroachment is to be included with this submittal, than include HEC-RAS models both with the north over bank encroachment and without. Explain that there will be the following HEC-RAS models:
 - a. Duplicate effective
 - b. Corrected effective - "Existing Conditions"
 - c. Proposed Conditions - including only what this project will actually build and include in LOMR
 - d. Future Conditions w/ north over bank encroachment,
 - e. Future Conditions w/ north over bank encroachment and bridge at Ventura

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18. In the report under “Methodology”:
 - a. Delete the reference to the bridge under Alameda.
 - b. Delete the reference to Mixed /supercritical being used in this analysis
 - c. The reason for sub-critical is irregular channel shape and has little to do with sediment concentrations in the runoff, especially at these low concentrations
19. Manning’s ‘n’ value for the channel changed without good justification, change back to match duplicate effective or justify. If grading the channel then existing may be different than proposed.
20. Compare elevations of duplicate effective to corrected effective. Provide table comparing elevations from all models and include both the elevation and the change for each model. Add footnotes explaining what the change is relative to. The existing should be relative to the duplicate effective and all others should be relative to the existing.
21. In the report under “existing conditions” explain datum shift from current effective.
22. In the report under “Introduction” state that the flows match what FEMA used in previous LOMR.
23. Section 12+34 is not perpendicular to flow in left overbank, nor are sections upstream.
24. Show the stationing base line on the G&D Plan with bearings, distances, and curve data. Use the same survey base line for both the HEC-RAS model and the construction plan profile. The base line should be straighter to represent the center of the conveyance rather than trying to follow a particular branch of the thalweg.
25. Written concurrence with the grading and drainage plan is required from the USACE indicating compliance with section 404 of the clean water act prior to approval of the preliminary plat. The limits of any Waters of the US must be shown on the Grading and Drainage Plan and conditions of any Section 404 permits must be stated on the plan.
26. *Information.* An Infrastructure Improvements Agreement, IIA, must be recorded prior to issuance of a Floodplain Development Permit, prior to recording a Plat, prior to issuance of a Building permit, and prior to issuance of a Work Order.

Prior to Grading Permit:

27. An approved ESC Plan is required for this project, and an ESC Permit is required prior to any land disturbance on this site due to the close proximity to the floodplain.
28. A separate floodplain development permit must be obtained from Rudy Rael at rrael@cabq.gov prior to any work in the floodplain.

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Prior to Release of Financial Guarantee (For Information):

29. Engineer's Certification, per the DPM Chapter 22.7: *Engineer's Certification Checklist For Subdivision* is required.
30. Work Order Closeout Package for the floodplain infrastructure per DPM must be submitted.
31. A Letter of Map Revision (LOMR) must be obtained from FEMA after construction is complete. When a CLOMR has been issued by FEMA, a portion or all of the SIA and financial guarantees for the improvements may be released prior to the LOMR being issued by FEMA, but the financial guarantee for the LOMR will not be released prior to the effective date of the LOMR. Submittal of a copy of the LOMR from FEMA is required for release of the balance of the financial guarantees and SIA's when issuance is a condition of release.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

PO Box 1293
Albuquerque
NM 87103

Sincerely,

A handwritten signature in black ink, appearing to read "Dana Peterson".

Dana Peterson, P.E.
Senior Engineer, Planning Dept.
Development Review Services

www.cabq.gov

LA CUEVA FLOODPLAIN ANALYSIS

Prepared for:

Llave Enterprises, Inc



September 2018

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Introduction

The purpose of this report is to demonstrate that the proposed arroyo improvements associated with the Signal Village Subdivision are adequate to remove the project area from the FEMA floodplain, protect the project from flooding, and that it is adequately protected from scour. The area analyzed is located west of Ventura Road, south of Alameda Boulevard, east of Barstow Street, and north of Signal Avenue. As shown in Appendix A, The La Cueva Arroyo is within a FEMA Floodplain classified as Zone AE. This report uses the hydrologic analysis provided in the North Albuquerque Acres Master Drainage Plan (NAAMDP) by Resource Technology Inc. (RTI) dated October 1998. The NAAMDP should be referenced for general drainage background. The flow rate for the La Cueva Arroyo is 3,048 cfs at Ventura and 3,094 cfs at Barstow according to the NAAMDP, see Appendix B for Hydrology Background..

Existing Conditions

The area within the project limits is mostly undeveloped. There is a small subdivision located north of Signal and west of Ventura and also a single family home located south of Alameda and west of Ventura. There is a concrete channel that begins shortly after the La Cueva Arroyo crosses Alameda and flows northwest. An earthen berm was constructed south of the concrete channel to direct the flows in the arroyo into the channel. The surrounding area generally slopes at 3% from east to west. The arroyo was previously studied by Weston Solutions in 2012, which was used to create the existing floodplain. This analysis uses more current topographic data to model the floodplain.

Methodology

Flood Plain Modeling

The La Cueva arroyo was modeled using HEC-RAS 5.0.1 for both existing and proposed conditions. The manning's coefficient for the natural arroyo was assumed to be 0.025 for the stream bed and 0.035 for the banks. Concrete box culverts for the road crossings at Ventura and Alameda were modeled to have capacity for the 100-yr storm. Due to soil type and sediments load this type of the arroyo would typically run supercritical until it reaches a point where velocity is high enough for it to transport enough sediment to cause it to run critical. In this case the bed material is a course material and the arroyo in this area based on analysis and observation of flows in the channel would appear able to maintain subcritical flow. Weston's model from 2012 the arroyo ran in a mixed flow regime. This model was set to a mixed flow regime. By allowing supercritical flow in the upstream portion of the arroyo, a worst case velocity scenario is created to be able to design the scour wall depth. See model results in appendix C and full run included electronically.

Sediment Analysis

A sediment analysis was completed following the methods described in the AMAFCA Sediment and Erosion Design Guide. Basins were identified from the North Albuquerque Acres Master Drainage Plan. The project site is included in Basin 111.1. Basins 108, 109, and 110 upstream of the project site were also included in the analysis. The MUSLE equation was used to calculate the wash load from each basin. The bed load was calculated follow the MPM-Woo method described in the AMAFCA Guide. Basins 108 and 109, which drain to Basin 110, contributes a total of 4.02 acre-feet of sediment to Basin 110. Basin 110, which drains to Basin 111.1 contributes 1.30 acre-feet of sediment to Basin 111.1. And a total of 0.67 acre-feet of sediment is produced in Basin 111.1. Please refer to the Basin Map and Sediment Analysis spreadsheet in Appendix D.

Scour Analysis

The proposed scour wall along the north side of the proposed Signal Village Subdivision was analyzed per AMAFCA's Sediment and Erosion Design Guide from 1994. Using HECRAS results, the scour analysis was completed to determine the scour wall depth and height for the worst case assuming a box culvert under Ventura.

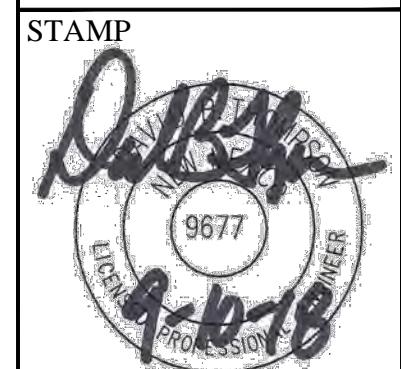
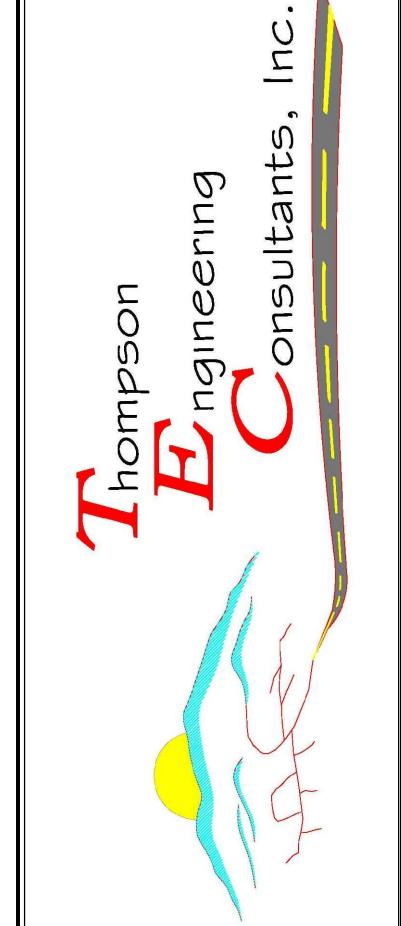
Proposed Design

A Scour Wall is proposed beginning at Ventura and will connect to the existing scour wall for the adjacent subdivision to the west. The purpose of the Scour Wall is to contain the incoming arroyo flows from the east, thus removing the floodplain and protecting the future development for this specific location. The proposed length of the scour wall is 420 feet starting near the northwest corner of the Ventura and Signal intersection. From there, it continues north and west following along the left bank of the arroyo until connecting to the existing wall. As shown in the scour analysis included in Appendix B, the total wall height varies from 14.99 to 19.44 ft and the estimated scour varies from 6.71 to 10.36 ft. The model run indicates that the scour wall reduces the floodplain on the property as shown in the attached Exhibit A. The proposed floodplain is mostly within the current floodplain boundary. See grading design in Appendix E.



WATER SURFACE ELEVATION DIFFERENCES BETWEEN EXISTING AND PROPOSED CONDITIONS

Model Scenario			
	Existing Water Surface (RESPEC Model)	Existing with Proposed Wall	Existing with Proposed Wall and Culvert @ Ventura
Cross Section	W.S Elevation	Δ W.S Elevation	Δ W.S Elevation
1937.32	5580.75	0	0
1849.64	5578.18	0	0
1714.73	5574.42	0	0
1625.11	5571.82	0	0
1565.71	5570.65	0	0.71
1565 (Culvert)			
1511.28	5567.91	0.09	0.11
1438.58	5565.53	0.46	0.46
1410.93	5564.72	0.61	0.61
1380.76	5563.87	0.79	0.79
1354.24	5563.39	0.49	0.49
1332.33	5562.92	0.03	0.03
1311.52	5562.38	-0.11	-0.11
1295.47	5561.87	-0.07	-0.07
1264.68	5560.95	-0.23	-0.23
1233.92	5561.03	-0.56	-0.43
1187.49	5559.39	-0.03	0
1146.73	5557.03	0	0
1104.38	5555.56	0	0
1056.97	5554.19	0	0
1011.94	5553.05	0	0
975.03	5552.37	0	0
916.13	5550.02	0	0
848.23	5547.96	0	0
804.54	5547.29	0	0
742.87	5545.55	0	0
711.63	5544.34	0	0
674.15	5543.75	0	0
618.69	5541.31	0	0
491.87	5538.41	0	0
450.17	5536.7	0	0
432.04	5535.95	0	0
356.79	5533.66	0	0
306.95	5532.38	0	0
247.1	5530.83	0	0
208.39	5529.99	0	0
172.73	5528.92	0	0
98.74	5527.5	0	0
1.24	5525.15	0	0



DATE: 09.10.2018

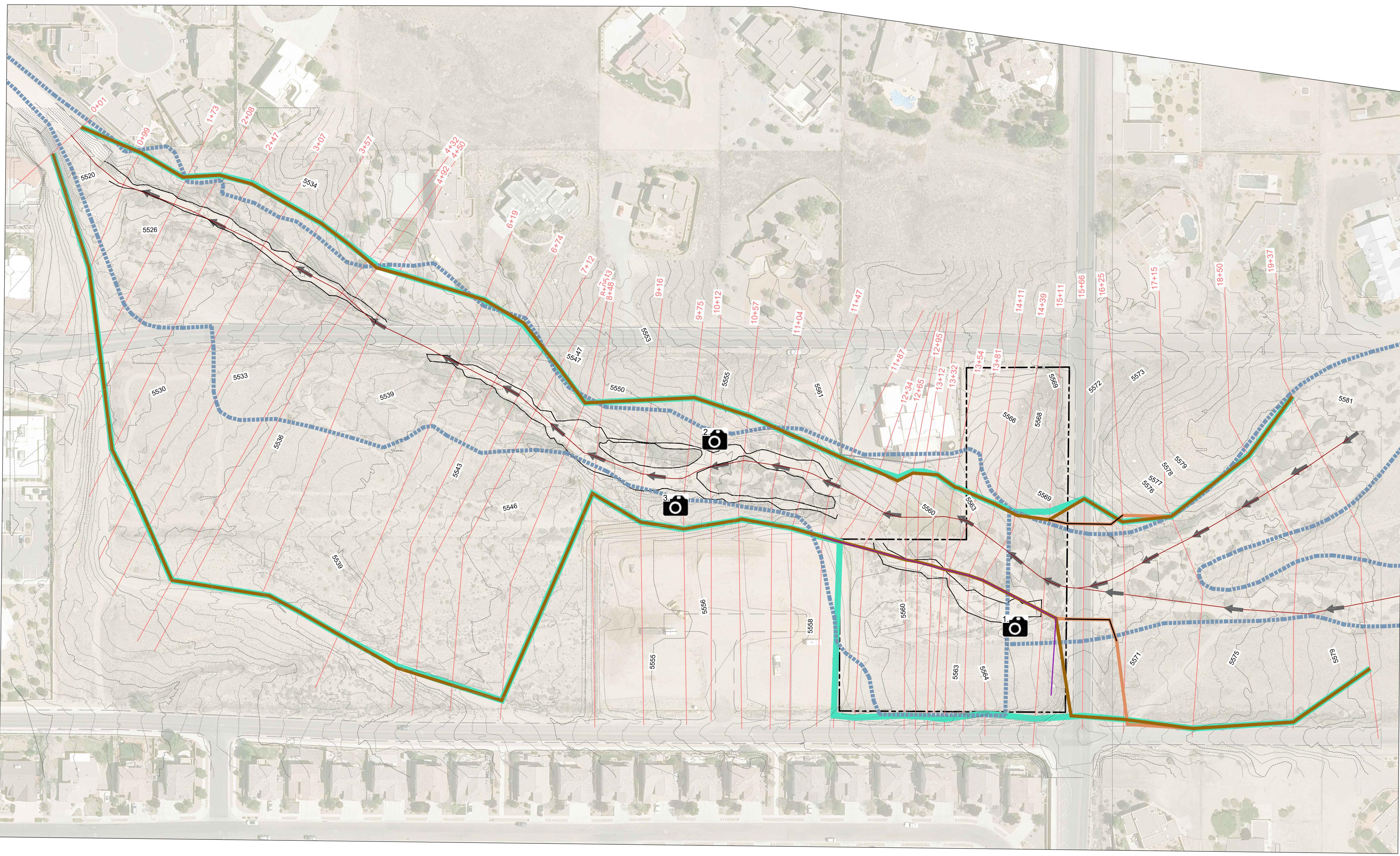
LA CUEVA ARROYO FLOODPLAIN ANALYSIS

EXHIBIT A

LEGEND
PROPERTY LINE
ARROYO CENTER LINE
FEMA CURRENT FLOODPLAIN ZONE AH/AE
PROPOSED HECRAS CROSS SECTIONS
EXIST WATER SURFACE MODEL - SUBCRITICAL FLOW REGIME
PROPOSED WATER SURFACE - SUBCRITICAL FLOW REGIME WITH PROPOSED WALL, NO CULVERT
PROPOSED WATER SURFACE - SUBCRITICAL FLOW REGIME WITH PROPOSED WALL, WITH CULVERT
PROPOSED SCOUR WALL
USACE OHWM
PROPOSED STRUCTURE



1 OF 1



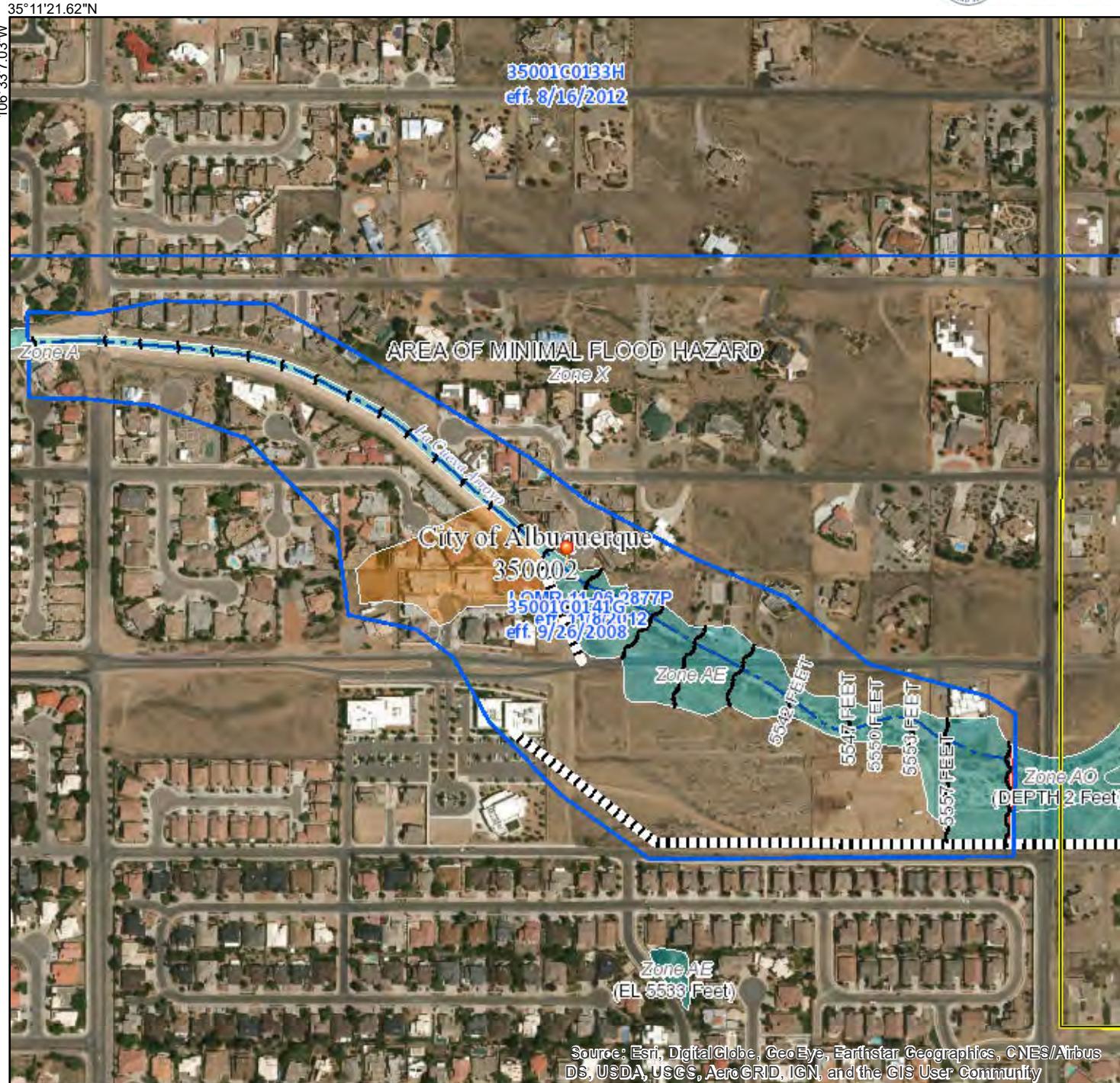
SURVEY SHOWN IS 1FT INTERVAL CONTOURS TAKEN FROM ALDRICH SURVEY PERFORMED ON APRIL 2018.

Appendix A

National Flood Hazard Layer FIRMette



FEMA



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS

Without Base Flood Elevation (BFE)
Zone A, V, A99

With BFE or Depth

Regulatory Floodway Zone AE, AO, AH, VE, AR

0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X

Future Conditions 1% Annual Chance Flood Hazard Zone X

Area with Reduced Flood Risk due to Levee. See Notes. Zone X

Area with Flood Risk due to Levee Zone D

OTHER AREAS OF FLOOD HAZARD

NO SCREEN Area of Minimal Flood Hazard Zone X

Effective LOMRs

OTHER AREAS

Area of Undetermined Flood Hazard Zone D

GENERAL STRUCTURES

— Channel, Culvert, or Storm Sewer

||||| Levee, Dike, or Floodwall

OTHER FEATURES

20.2
17.5

Coastal Transect

Base Flood Elevation Line (BFE)

Limit of Study

Jurisdiction Boundary

Coastal Transect Baseline

Profile Baseline

Hydrographic Feature

MAP PANELS

Digital Data Available

No Digital Data Available

Unmapped



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The base map shown complies with FEMA's base map accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/30/2018 at 1:12:52 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: base map imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

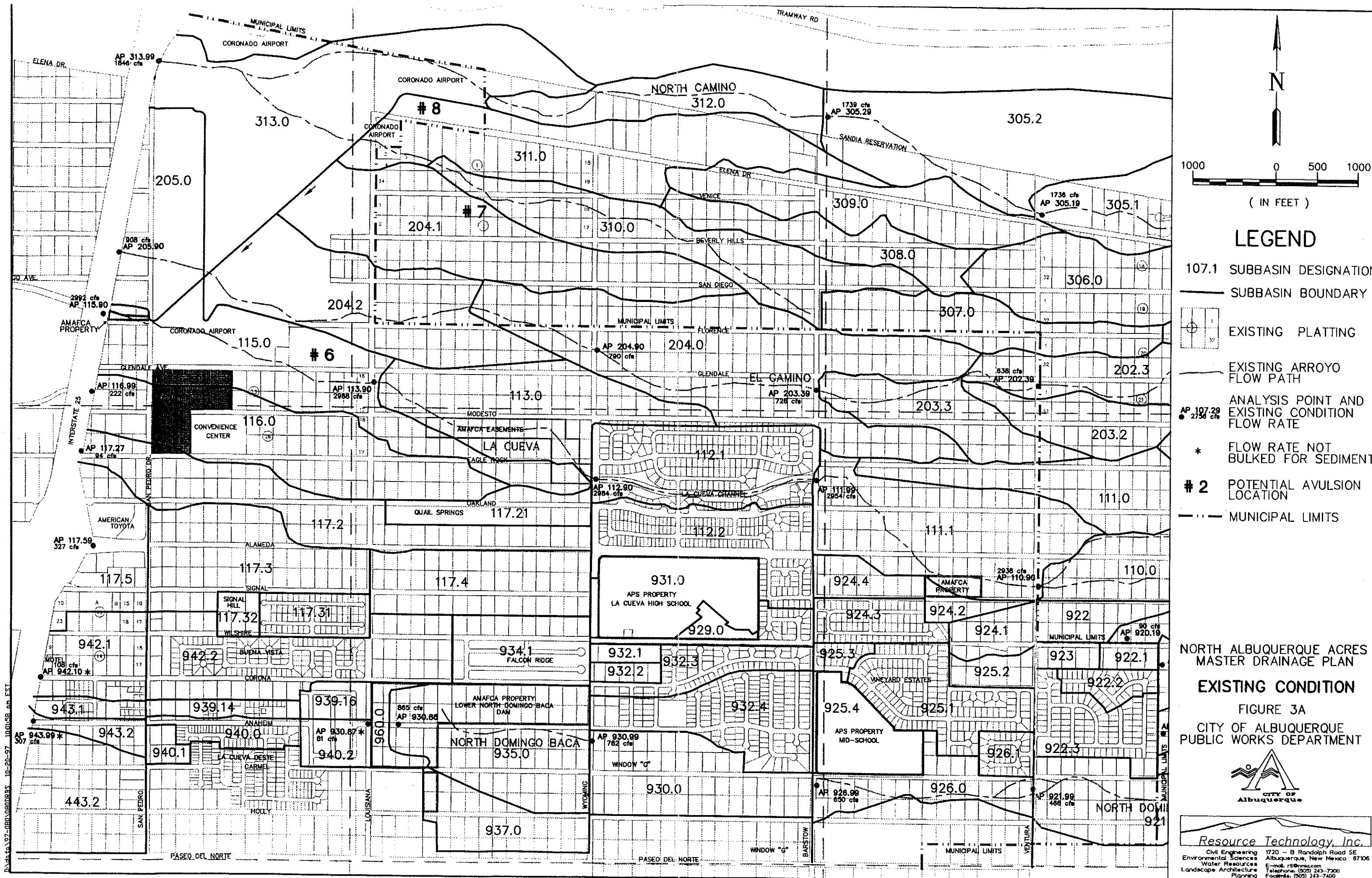
Appendix B

D. Proposed Condition Hydrology Results

Proposed condition flow rates for future fully developed conditions from the hydrology models incorporating the above facilities are reported in Table 6. All of the interim detention ponds have been eliminated. A comparison with the hydrology results from the **1996 AMAFCA Hydrology Report** for future fully developed conditions without storm drainage or channel improvements for selected points on the main arroyos is made in Tables 7a and 7b. Except where specifically labeled as “worst case” all flow rates reported assume avulsion control.

TABLE 6a						
FUTURE CONDITIONS HYDROLOGY SUMMARY (BULKED FLOW RATES)						
	AREA (Sq mi)	AP #	VOL- 10-YR (ac-ft)	Qp 10-YR (cfs)	VOL- 100-YR (ac-ft)	Qp 100-YR (cfs)
NORTH DOMINGO BACA ARROYO						
Holbrook	3.166	919.99	129.0	305	231.1	616
Ventura	3.446	921.99	142.4	609	255.7	1171
Barstow	3.562	926.99	149.2	758	267.5	1813
Wyoming	3.870	930.99	167.4	1096	298.8	1984
Inflow to LNDB Dam	4.259	930.86	189.5	1313	337.0	2442
Out flow from LNDB Dam	4.259	930.87	189.5	170	337.0	200
I-25	4.526	943.99	205.7	400	364.7	731
LA CUEVA ARROYO¹						
Ventura	3.766	110.90	130.0	1359	265.8	3048
Barstow	4.017	111.99	140.5	1374	284.5	3094
Wyoming	4.189	112.90	150.3	1383	301.0	3128
Louisiana	5.462	113.90	194.5	1632	390.1	3908
I-25	5.582	115.9	202.1	1640	402.6	3923
North Diversion Channel	6.871	128.90	270.8	2901	519.8	5551

¹La Cueva and El Camino Arroyo Flow Rates reflect diversion of El Camino to the La Cueva west of Wyoming.



LA Cueva 100-YR FUTURE

-(s16.67h8.5vOT-&18D
AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = a:lcwyol00.fut

COMMAND	HYDROGRAPH IDENTIFICATION	FROM	TO	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO	CFS PER ACRE	PAGE =
		ID NO.	ID NO.					(HOURS)	NOTATION	1
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 4.050
COMPUTE NM HYD	101.00	-	1	.60700	953.84	45.841	1.41600	1.650	2.455	PER IMP= .00
ROUTE MCUNGE	101.80	1	2	.60700	912.90	45.469	1.40451	1.850	2.350	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 4.200
COMPUTE NM HYD	100.00	-	1	1.21400	1201.70	87.881	1.35730	1.850	1.547	PER IMP= .00
ROUTE MCUNGE	100.80	1	3	1.21400	1177.21	87.700	1.35451	2.100	1.515	CCODE = .1
ADD HYD	101.88	3& 2	5	1.82100	1882.13	133.168	1.37117	1.950	1.615	RAIN24= 3.650
RAINFALL TYPE= 2										RAIN24= .00
COMPUTE NM HYD	102.00	-	1	.87500	1033.60	54.313	1.16386	1.700	1.846	PER IMP= .00
ADD HYD	102.90	1& 5	5	2.69600	2554.30	187.482	1.30389	1.900	1.480	
*S LA CUEVA TRIBUTARY ARROYO @ TRAMWAY BLVD. (102.9)										
ROUTE MCUNGE	102.80	5	2	2.69600	2515.07	187.100	1.30123	2.050	1.458	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.370
COMPUTE NM HYD	107.20	-	1	1.7200	325.87	13.591	1.48157	1.550	2.960	PER IMP= 17.00
ADD HYD	107.29	1& 2	4	2.86800	2575.01	200.690	1.31205	2.050	1.403	
*S NORTH LA CUEVA AT GLENDALE AND BROWNING (107.29)										
RAINFALL TYPE= 2										RAIN24= 3.500
COMPUTE NM HYD	102.10	-	1	.09300	149.17	4.457	.89867	1.500	2.506	PER IMP= .00
*S LA CUEVA TRIBUTARY @ TRAMWAY BLVD.										
ROUTE MCUNGE	102.18	1	2	.09300	136.20	4.395	.88600	1.700	2.288	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.380
COMPUTE NM HYD	107.10	-	1	.18080	399.38	14.303	1.48327	1.500	3.452	PER IMP= 17.00
ADD HYD	107.19	1& 2	5	.27380	399.38	18.697	1.28040	1.500	2.279	
*S NORTH LA CUEVA (TRIB) AT FLORENCE AND BROWNING (107.19)										
ADD HYD	107.90	5& 4	3	3.14180	2666.73	219.388	1.30929	2.000	1.326	
RAINFALL TYPE= 2										RAIN24= 3.400
COMPUTE NM HYD	106.00	-	1	.04360	95.85	3.448	1.48281	1.500	3.435	PER IMP= 17.00
ROUTE MCUNGE	106.80	1	2	.04360	86.06	3.396	1.46043	1.700	3.084	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.340
COMPUTE NM HYD	106.10	-	1	.11160	238.47	8.685	1.45925	1.500	3.339	PER IMP= 17.00
ADD HYD	106.19	1& 2	6	.15520	257.25	12.081	1.45958	1.550	2.590	
*S LA CUEVA TRIBUTARY ARROYO @ BROWNING (106.19)										
ADD HYD	107.99	3& 6	5	3.29700	2733.21	231.469	1.31636	2.000	1.295	
ROUTE MCUNGE	107.80	5	2	3.29700	2713.95	231.319	1.31551	2.100	1.286	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.230
COMPUTE NM HYD	109.00	-	1	.10060	216.30	7.552	1.40751	1.500	3.359	PER IMP= 17.00
ADD HYD	109.90	1& 2	5	3.39760	2736.13	238.871	1.31823	2.100	1.258	
RAINFALL TYPE= 2										RAIN24= 3.250
COMPUTE NM HYD	108.00	-	7	.20550	404.63	15.573	1.42091	1.550	3.077	PER IMP= 17.00
ADD HYD	109.99	5& 7	5	3.60310	2789.17	254.444	1.32409	2.100	1.210	
*S LA CUEVA ARROYO @ EUBANK (MAIN) (109.99)										
ROUTE MCUNGE	109.88	5	2	3.60310	2763.62	254.113	1.32237	2.200	1.198	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.130
COMPUTE NM HYD	110.00	-	1	.16340	275.61	11.738	1.34689	1.550	2.636	PER IMP= 17.00
ADD HYD	110.90	1& 2	5	3.76650	2796.05	265.851	1.32343	2.200	1.160	
*S LA CUEVA ARROYO @ VENTURA (MAIN) (110.90) - FINAL										
*S ROUTE TO BARSTOW										
ROUTE MCUNGE	110.88	5	2	3.76650	2792.08	263.889	1.31366	2.219	1.158	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.020
COMPUTE NM HYD	111.00	-	1	.05330	108.83	3.739	1.31526	1.500	3.190	PER IMP= 17.00
COMPUTE NM HYD	111.40	-	4	.01410	28.80	.989	1.31527	1.500	3.192	PER IMP= 17.00
*S COMBINE HYD.'S 111.0 AND 111.4 AS 111.49										
ADD HYD	111.49	1& 4	4	.06740	137.63	4.728	1.31525	1.500	3.191	
*S ROUTE TO LA CUEVA CHANNEL @ OAKLAND										
*S PIPE ROUTING										
ROUTE	111.48	4	5	.06740	133.79	4.728	1.31527	1.550	3.102	
RAINFALL TYPE= 2										RAIN24= 3.050
COMPUTE NM HYD	111.10	-	1	.09690	195.97	7.699	1.48968	1.500	3.160	PER IMP= 26.00
RAINFALL TYPE= 2										RAIN24= 3.050
COMPUTE NM HYD	111.30	-	6	.04200	107.90	4.348	1.94094	1.500	4.014	PER IMP= 50.00
ADD HYD	111.39	1& 6	1	.13890	303.87	12.046	1.62612	1.500	3.418	
*S COMBINE HYD.'S 111.48 AND 111.39 AS 111.88										
ADD HYD	111.88	1& 5	1	.20630	436.39	16.774	1.52456	1.500	3.305	
*S LA CUEVA CHANNEL AT BARSTOW (111.89)-NOT FINAL										
ADD HYD	111.89	1& 2	5	3.97280	2827.78	280.143	1.32216	2.194	1.112	
*S DIVERSION FROM NDB ABOVE CARRINGTON										
COMPUTE NM HYD	924.10	-	6	.02500	64.23	2.588	1.94094	1.500	4.015	PER IMP= 50.00
COMPUTE NM HYD	924.20	-	7	.01900	48.82	1.967	1.94094	1.500	4.015	PER IMP= 50.00
*S COMBINE HYD.'S 924.10 AND 924.20 AS 924.22										

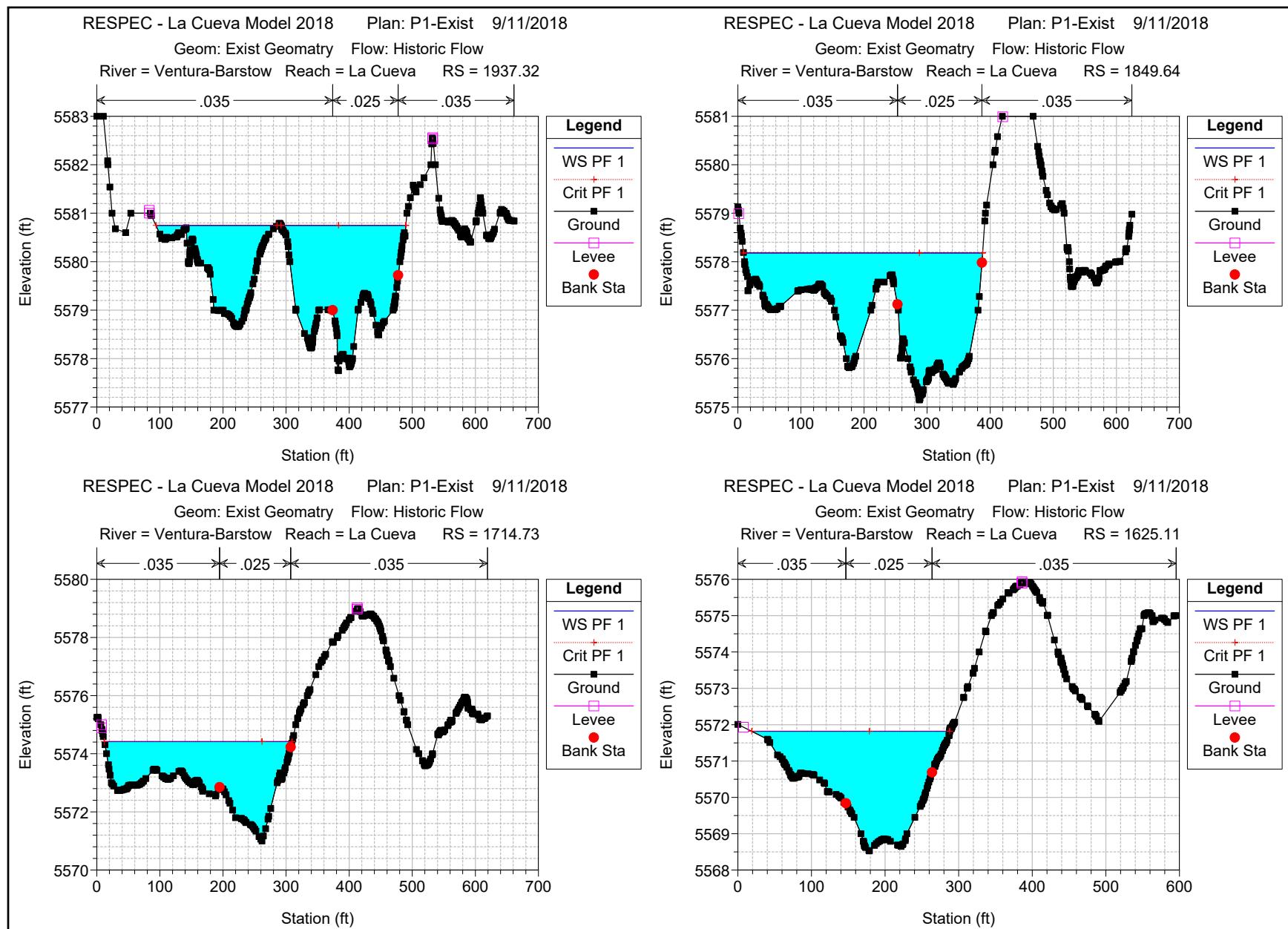
Appendix C

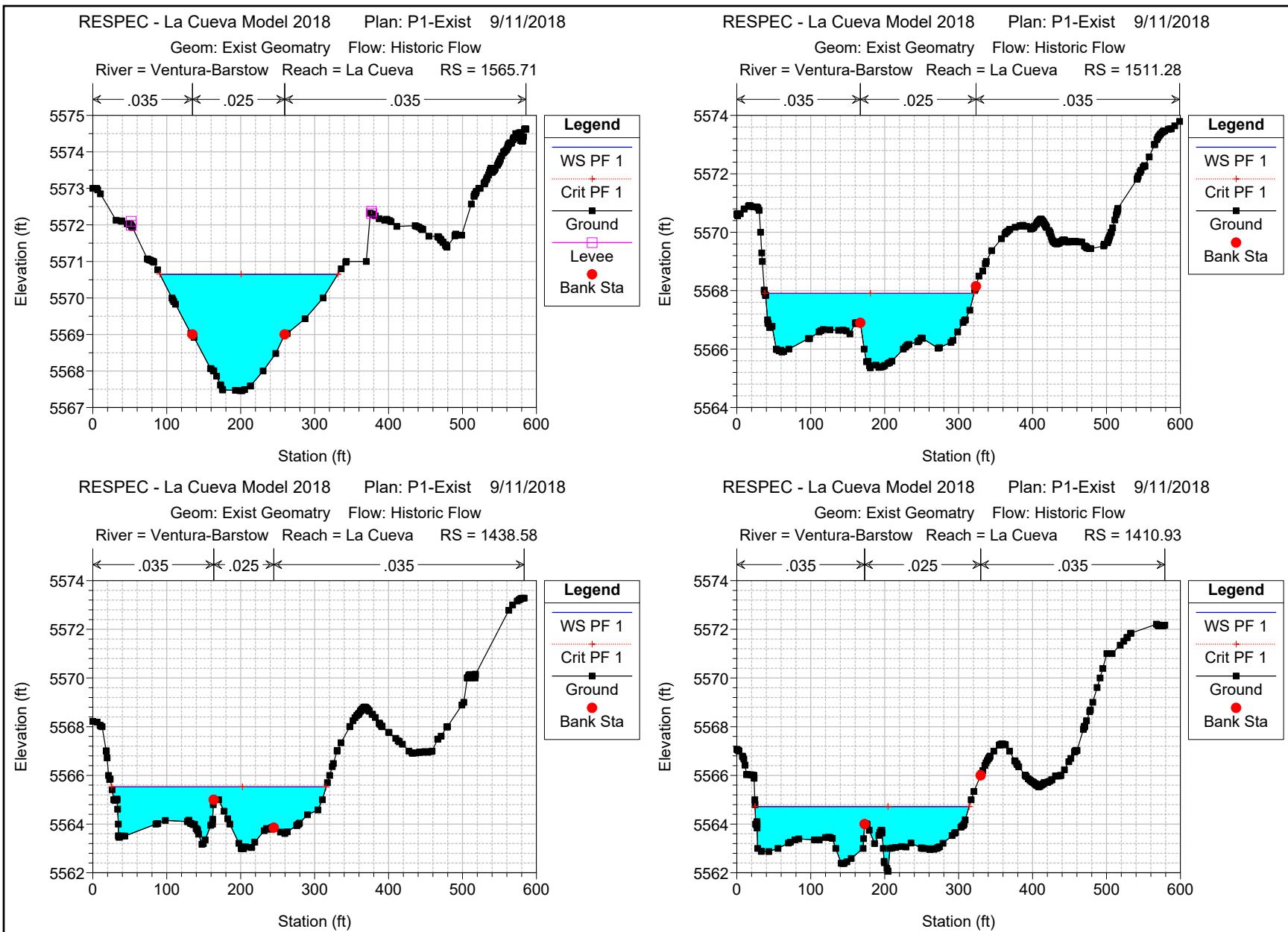
Existing Conditions

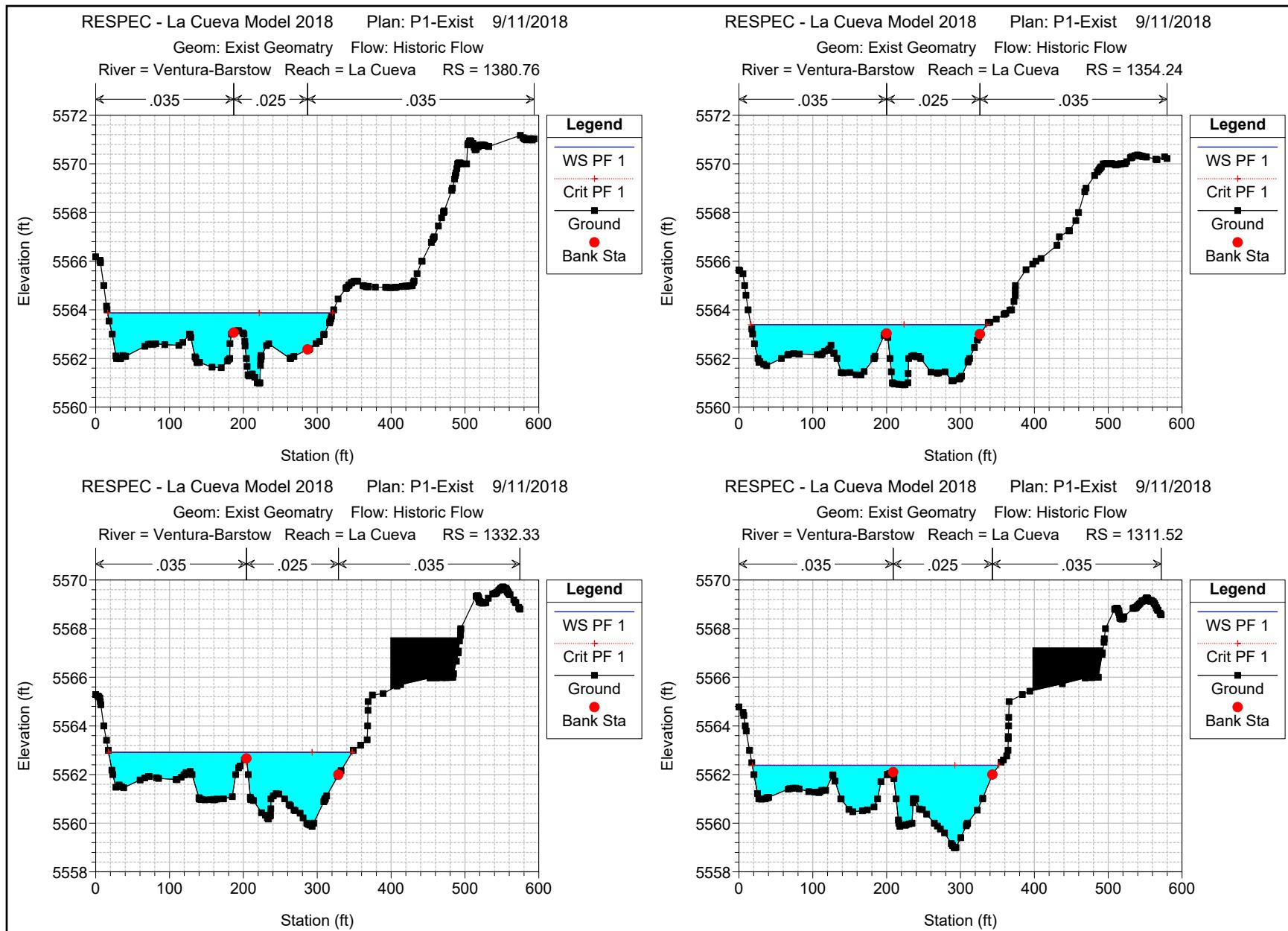
HEC-RAS Plan: P1-Exist River: Ventura-Barstow Reach: La Cueva Profile: PF 1

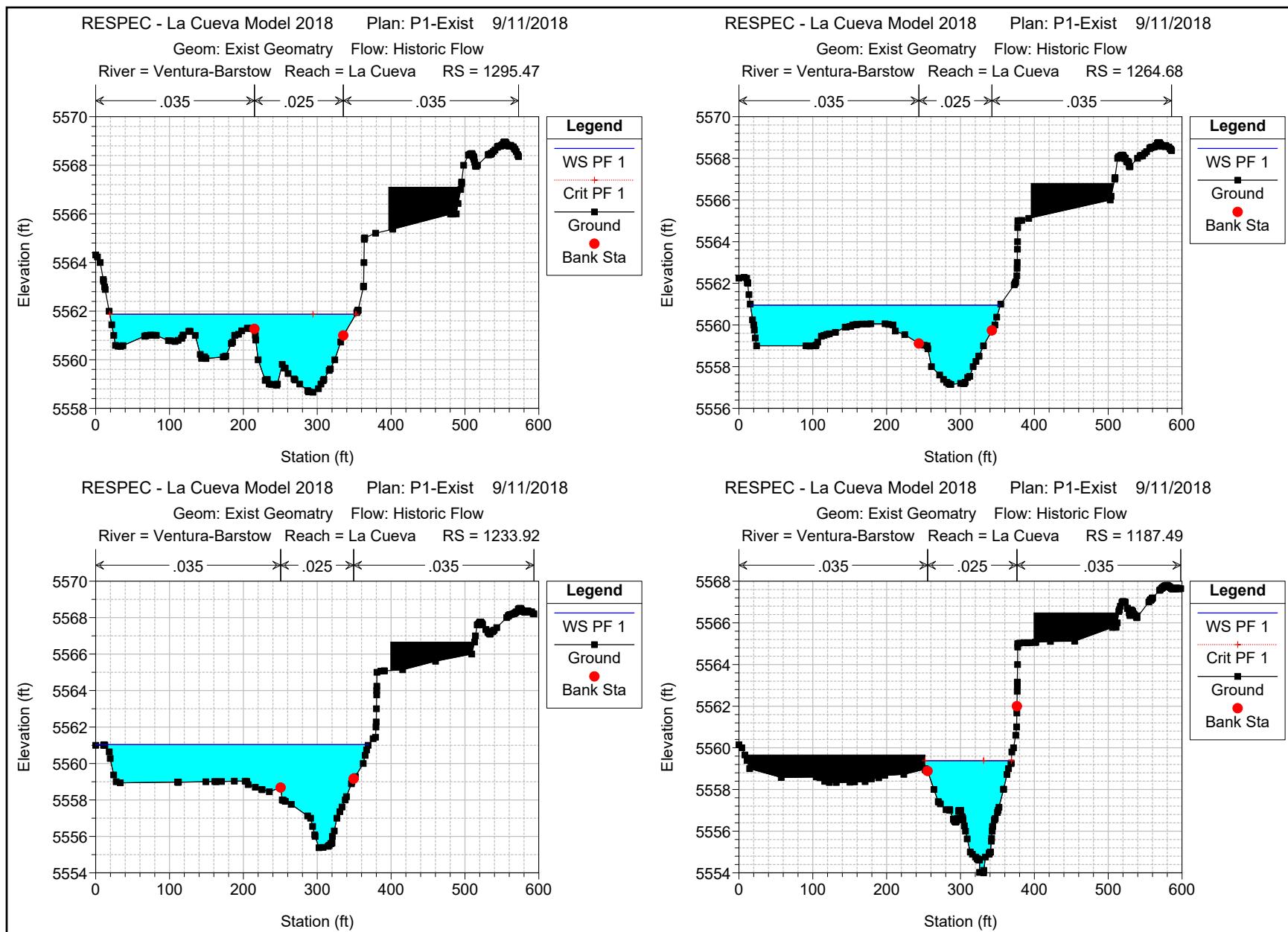
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
La Cueva	1937.32	PF 1	3090.00	5577.75	5580.75	5580.75	5581.52	0.007905	8.49	520.20	390.25	1.05
La Cueva	1849.64	PF 1	3090.00	5575.15	5578.18	5578.18	5578.86	0.005312	7.48	559.84	379.10	0.87
La Cueva	1714.73	PF 1	3090.00	5571.00	5574.42	5574.42	5575.20	0.006559	8.18	498.30	297.75	0.97
La Cueva	1625.11	PF 1	3090.00	5568.53	5571.82	5571.82	5572.79	0.005525	8.47	456.96	268.86	0.92
La Cueva	1565.71	PF 1	3090.00	5567.46	5570.65	5570.65	5571.65	0.005453	8.40	437.13	240.76	0.91
La Cueva	1511.28	PF 1	3090.00	5565.36	5567.91	5567.91	5568.71	0.008347	7.97	458.55	282.91	1.05
La Cueva	1438.58	PF 1	3090.00	5562.99	5565.53	5565.53	5566.34	0.010458	8.88	460.43	290.16	1.18
La Cueva	1410.93	PF 1	3090.00	5562.06	5564.72	5564.72	5565.51	0.010994	7.93	444.53	289.47	1.16
La Cueva	1380.76	PF 1	3090.00	5560.98	5563.87	5563.87	5564.65	0.010280	8.46	466.51	304.67	1.15
La Cueva	1354.24	PF 1	3090.00	5560.91	5563.39	5563.39	5564.15	0.008639	8.11	483.21	319.37	1.07
La Cueva	1332.33	PF 1	3090.00	5559.87	5562.92	5562.92	5563.67	0.006482	7.94	513.40	329.72	0.95
La Cueva	1311.52	PF 1	3090.00	5558.99	5562.38	5562.38	5563.13	0.006143	7.78	516.03	334.01	0.93
La Cueva	1295.47	PF 1	3090.00	5558.66	5561.87	5561.87	5562.65	0.005189	7.86	525.90	333.25	0.88
La Cueva	1264.68	PF 1	3090.00	5557.15	5560.95		5561.51	0.003404	7.05	625.76	338.54	0.73
La Cueva	1233.92	PF 1	3090.00	5555.38	5561.03		5561.31	0.001141	5.02	902.09	369.49	0.44
La Cueva	1187.49	PF 1	3090.00	5554.00	5559.39	5559.39	5560.85	0.006843	9.70	319.75	116.85	1.02
La Cueva	1146.73	PF 1	3090.00	5552.95	5557.03	5557.03	5558.39	0.007467	9.36	330.07	123.79	1.01
La Cueva	1104.38	PF 1	3090.00	5551.33	5555.56	5555.56	5556.88	0.006689	9.24	334.57	128.11	1.01
La Cueva	1056.97	PF 1	3090.00	5550.42	5554.19	5554.19	5555.61	0.007123	10.30	330.98	122.65	0.99
La Cueva	1011.94	PF 1	3090.00	5548.91	5553.05	5553.05	5554.54	0.005560	9.92	331.33	125.78	0.95
La Cueva	975.03	PF 1	3090.00	5547.84	5552.37	5552.37	5553.69	0.004434	9.42	373.61	179.46	0.86
La Cueva	916.13	PF 1	3090.00	5546.08	5550.02	5550.02	5551.26	0.007613	9.08	366.90	162.62	0.94
La Cueva	848.23	PF 1	3090.00	5543.83	5547.96	5547.96	5549.43	0.006180	9.75	322.31	118.12	0.99
La Cueva	804.54	PF 1	3090.00	5542.34	5547.29	5547.29	5548.23	0.003850	8.66	533.57	334.36	0.80
La Cueva	742.87	PF 1	3090.00	5540.82	5545.55	5545.55	5546.24	0.003047	7.50	647.08	461.93	0.71
La Cueva	711.63	PF 1	3090.00	5540.27	5544.34	5544.34	5545.11	0.005125	7.82	541.27	436.19	0.87
La Cueva	674.15	PF 1	3090.00	5538.78	5543.75	5543.75	5544.33	0.003327	8.03	737.10	506.26	0.74
La Cueva	618.69	PF 1	3090.00	5537.00	5541.31	5541.31	5542.19	0.004911	8.76	533.90	353.42	0.88
La Cueva	491.87	PF 1	3090.00	5533.46	5538.41	5538.41	5539.08	0.003589	7.68	653.45	473.47	0.76
La Cueva	450.17	PF 1	3090.00	5531.00	5536.70	5536.70	5537.33	0.004206	8.90	673.83	452.95	0.83
La Cueva	432.04	PF 1	3090.00	5530.99	5535.95	5535.95	5536.58	0.005290	8.03	618.73	440.66	0.89
La Cueva	356.79	PF 1	3090.00	5528.87	5533.66	5533.66	5534.28	0.004595	7.70	646.34	466.93	0.83
La Cueva	306.95	PF 1	3090.00	5528.01	5532.38	5532.38	5533.03	0.005150	8.12	622.76	444.36	0.88
La Cueva	247.1	PF 1	3090.00	5526.33	5530.83	5530.83	5531.53	0.004044	8.19	621.67	405.44	0.81
La Cueva	208.39	PF 1	3090.00	5525.00	5529.99	5529.99	5530.92	0.004653	9.61	545.83	321.98	0.88
La Cueva	172.73	PF 1	3090.00	5524.41	5528.92	5528.92	5529.95	0.005146	8.95	442.89	210.75	0.90
La Cueva	98.74	PF 1	3090.00	5521.70	5527.50		5527.99	0.001482	6.42	647.57	226.40	0.52
La Cueva	1.24	PF 1	3090.00	5518.52	5525.15	5525.15	5527.54	0.005708	12.42	248.98	53.21	0.99

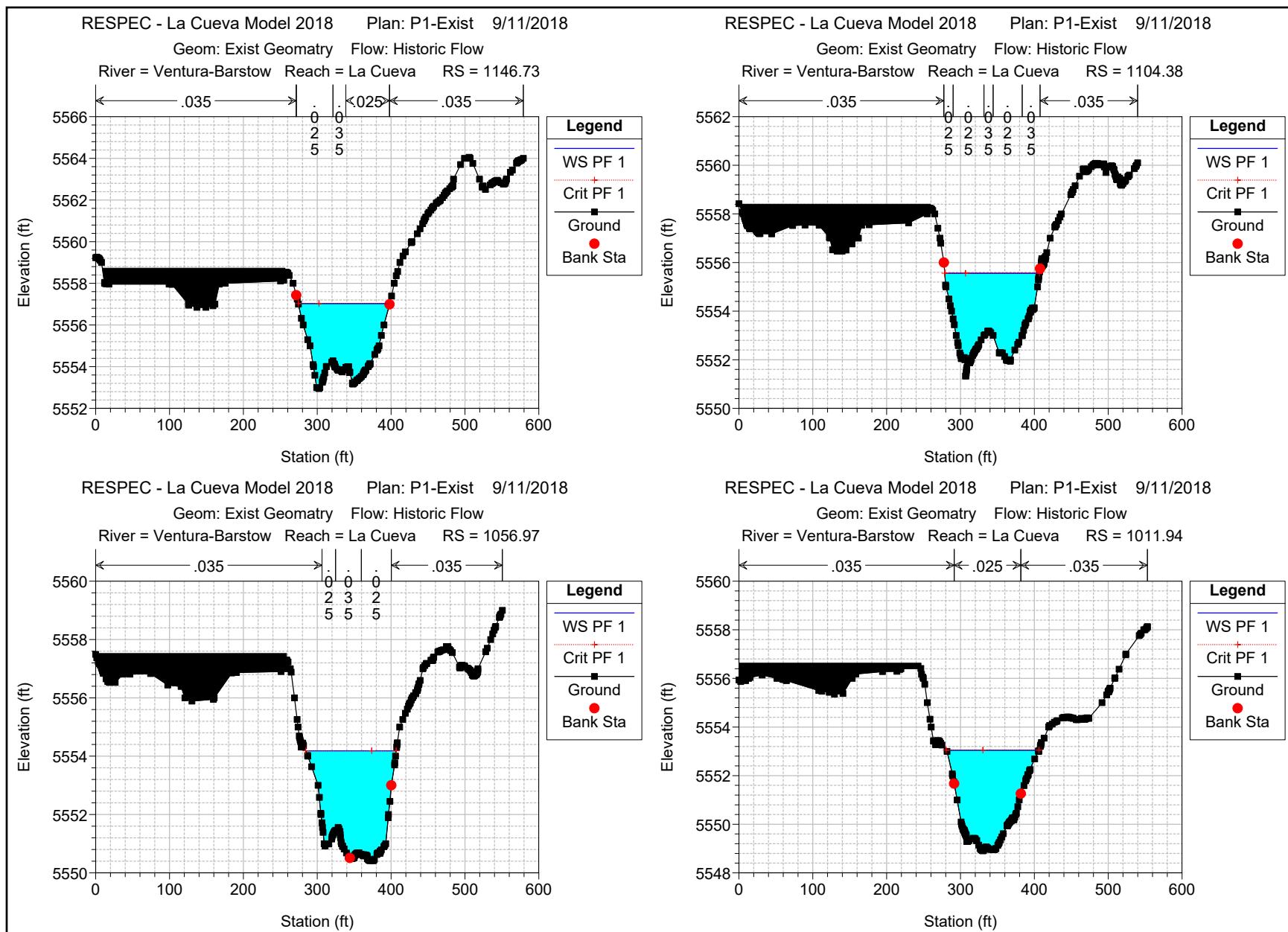
Existing Conditions

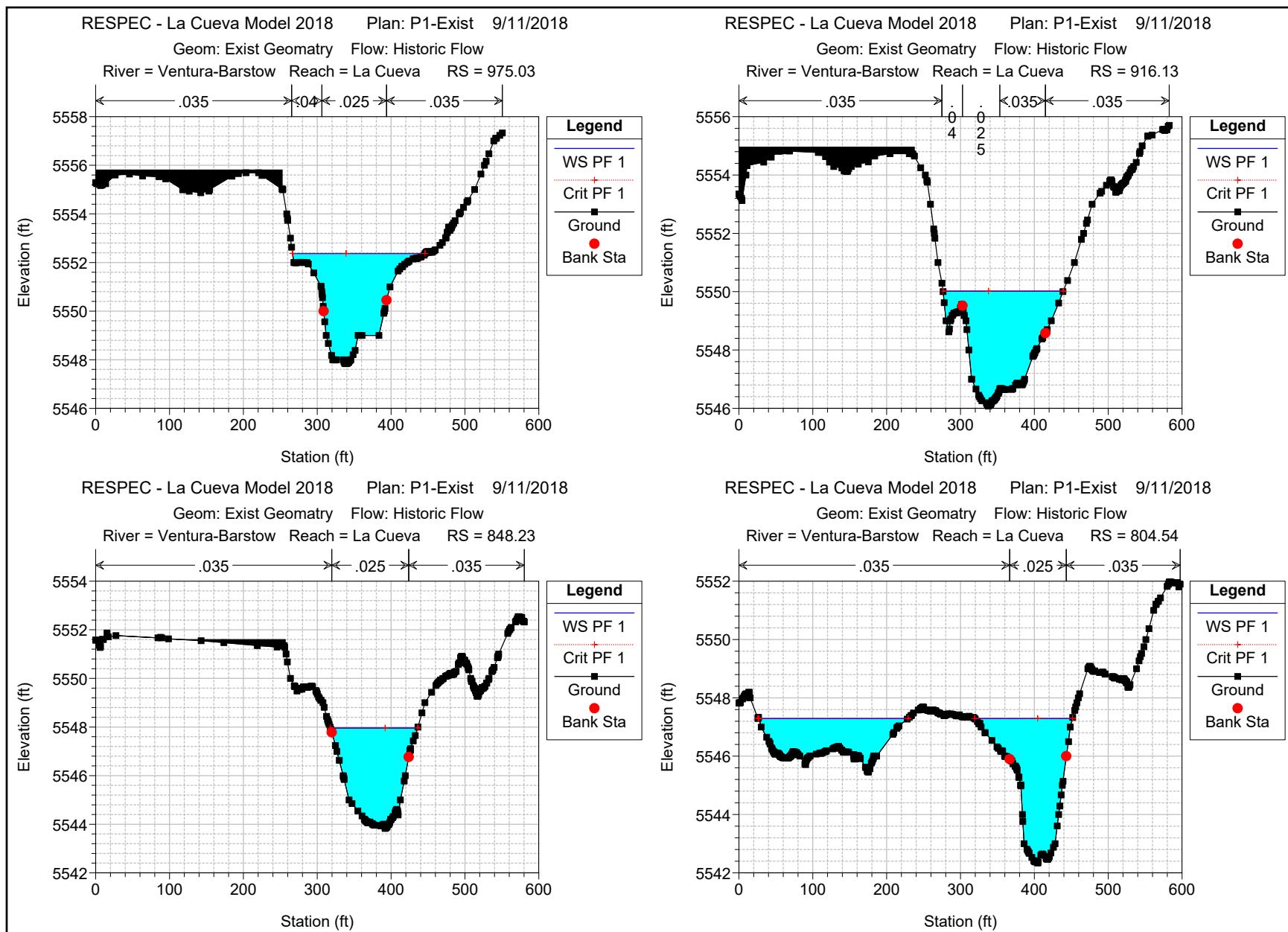


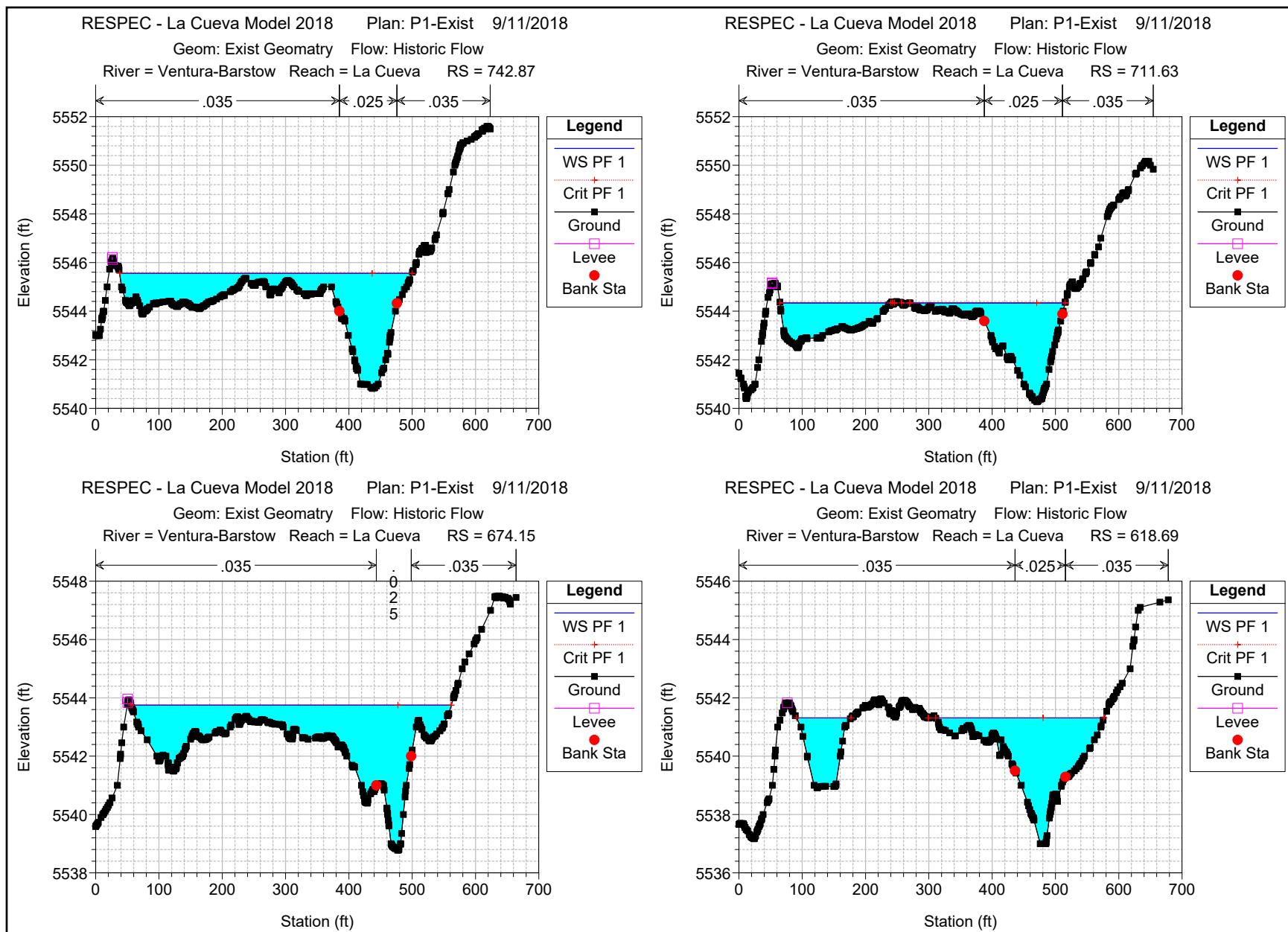


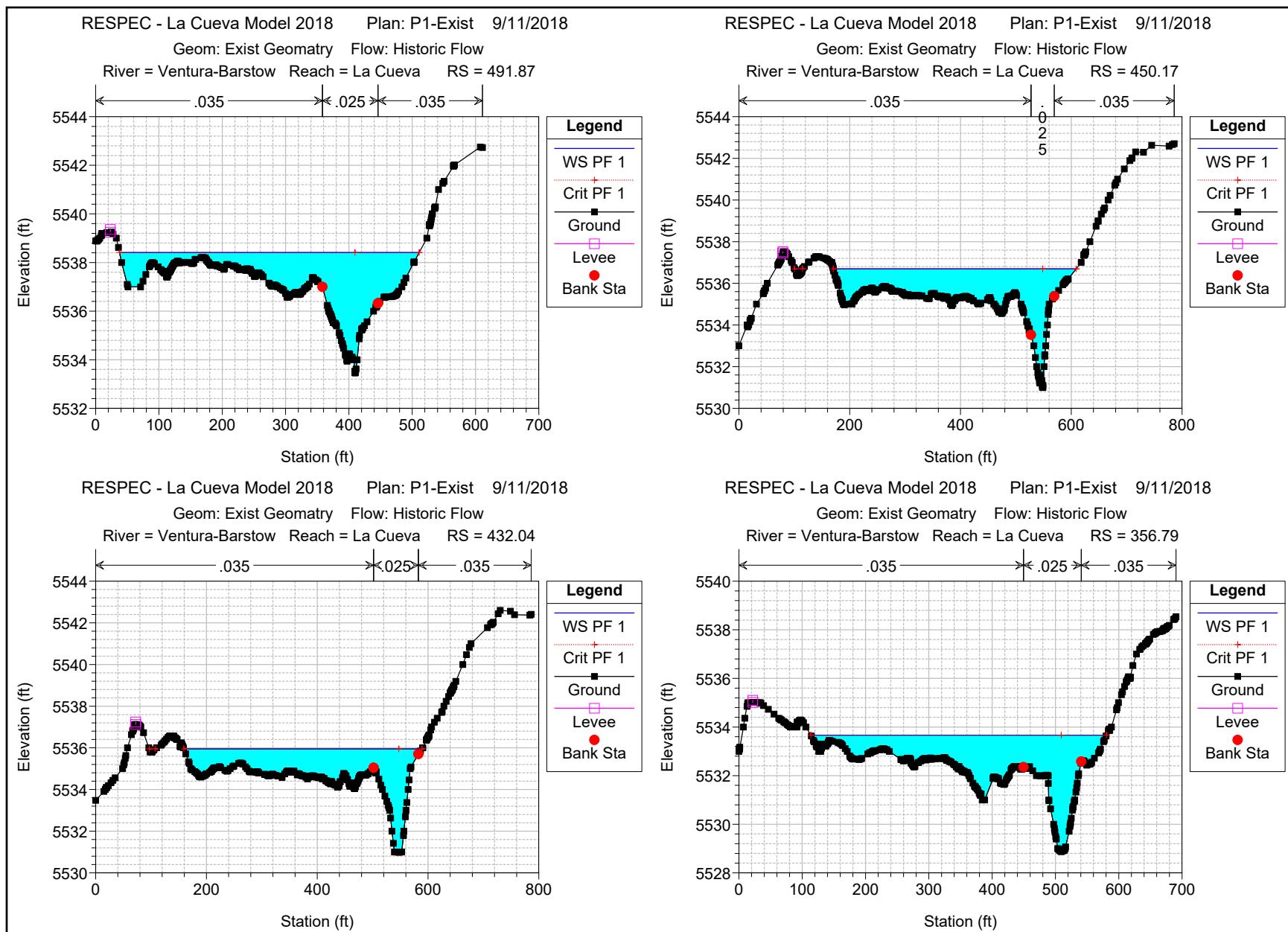


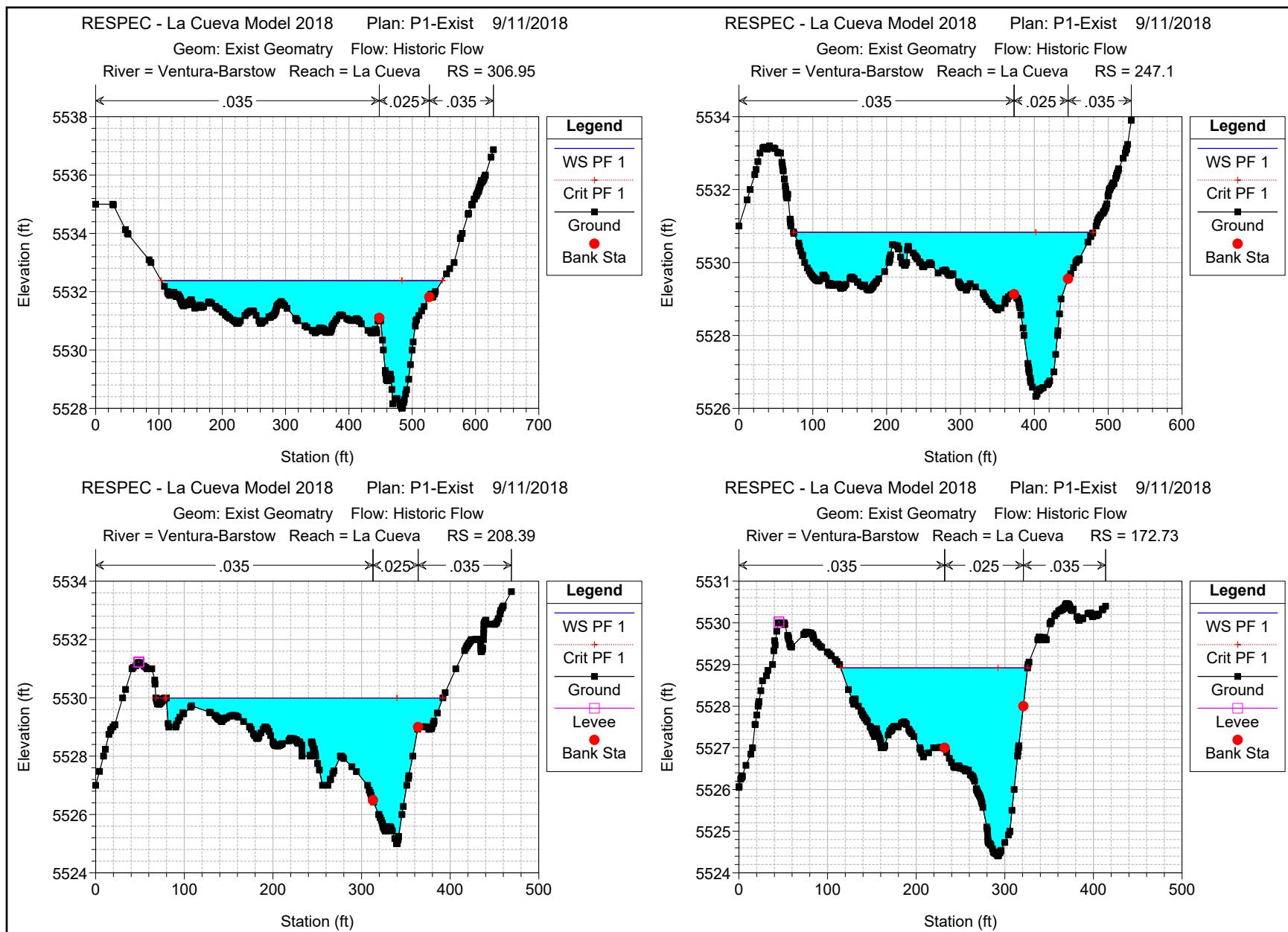


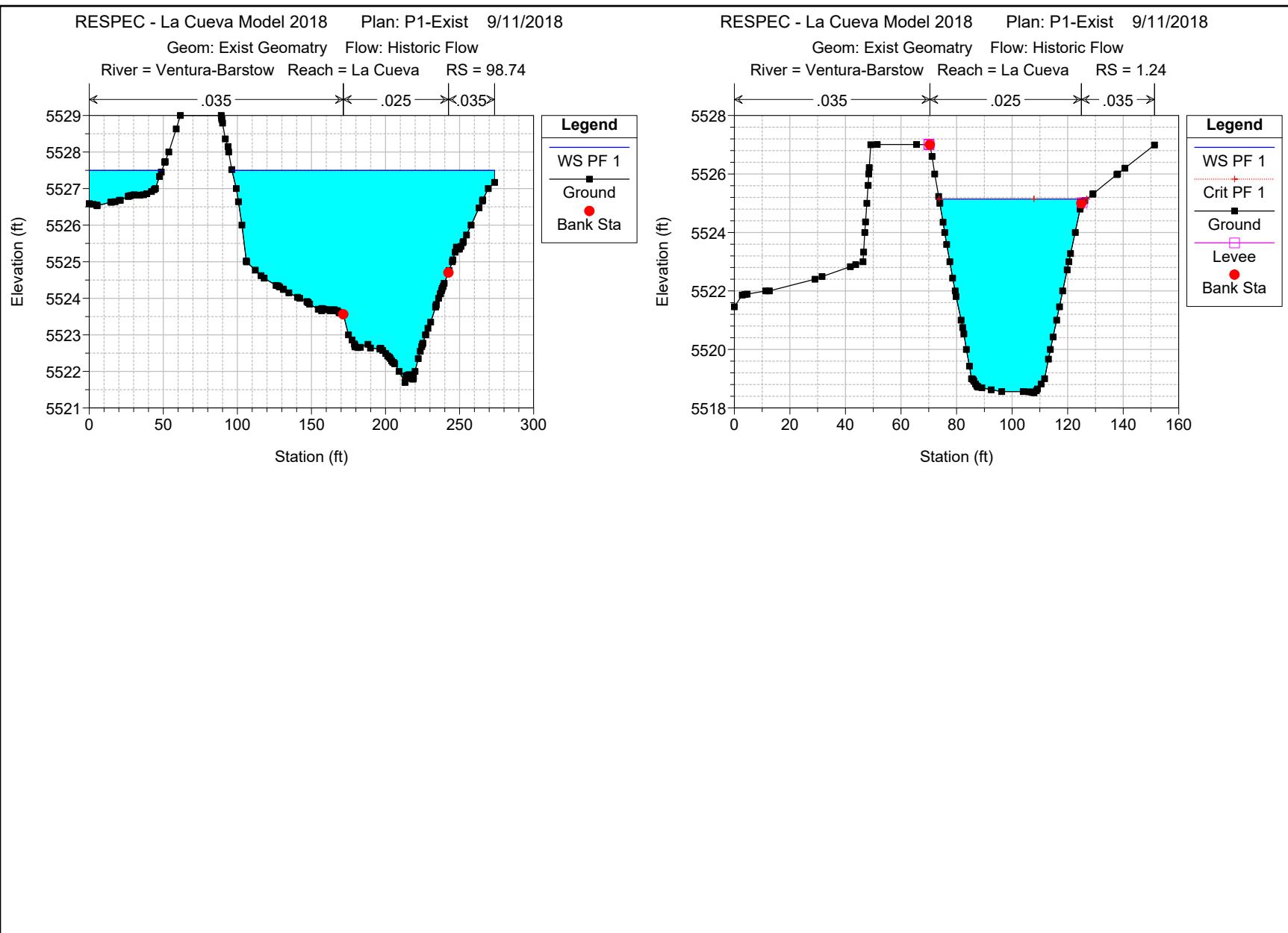












Proposed Scour Wall

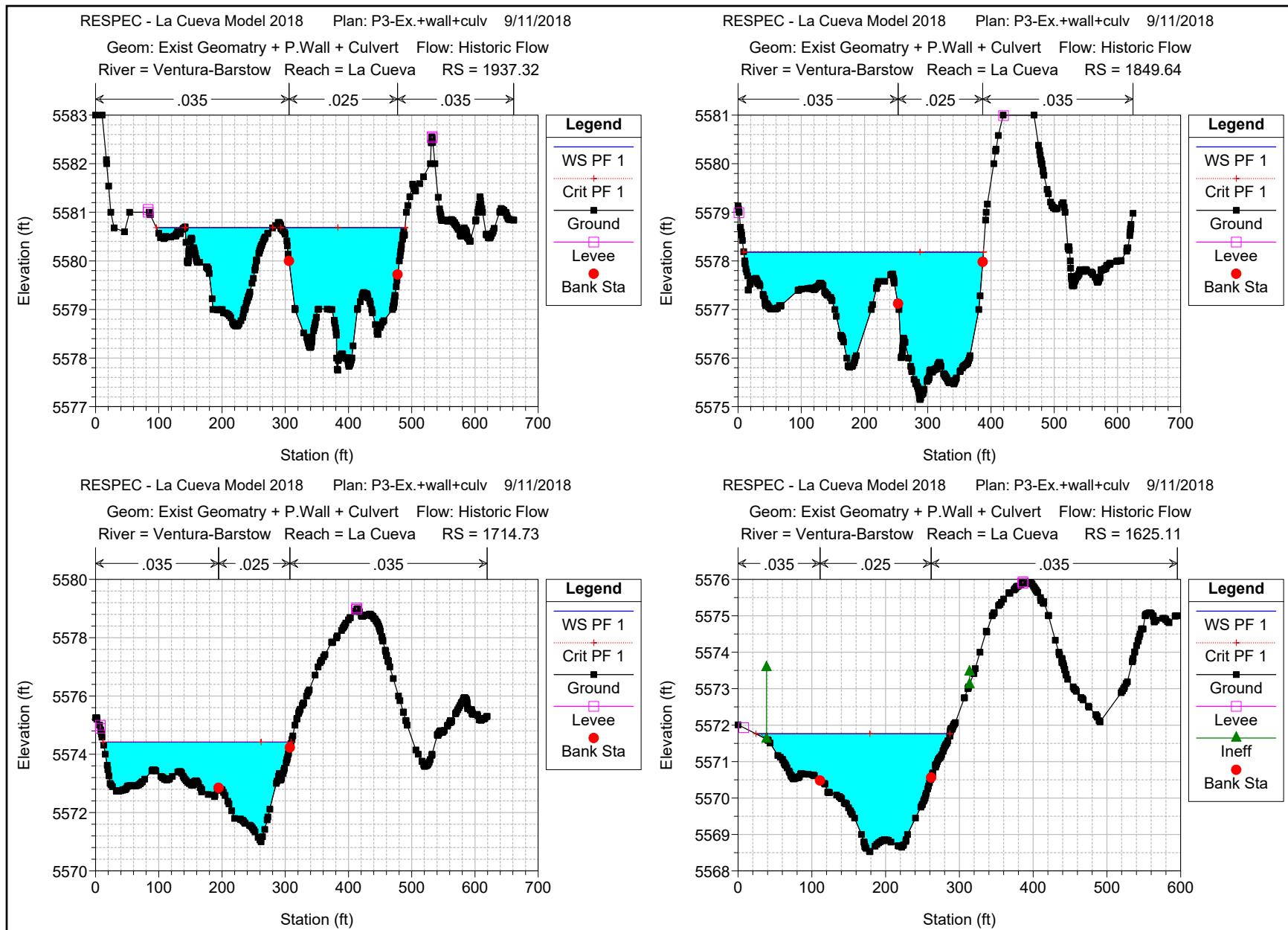
HEC-RAS Plan: P3-Ex.+wall+culv River: Ventura-Barstow Reach: La Cueva Profile: PF 1

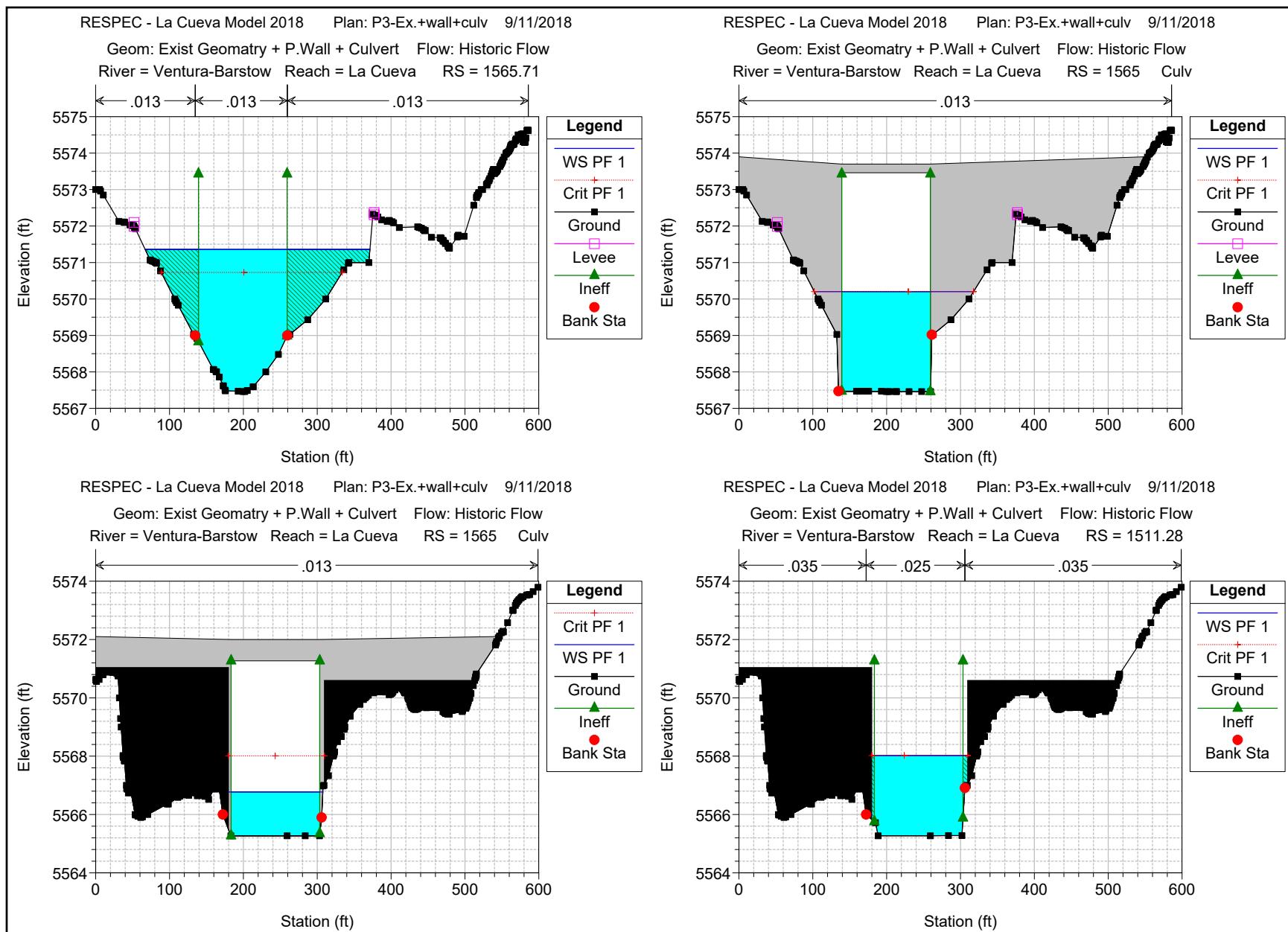
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
La Cueva	1937.32	PF 1	3090.00	5577.75	5580.68	5580.68	5581.45	0.006873	7.60	495.54	380.14	0.97
La Cueva	1849.64	PF 1	3090.00	5575.15	5578.18	5578.18	5578.86	0.005312	7.48	559.84	379.10	0.87
La Cueva	1714.73	PF 1	3090.00	5571.00	5574.42	5574.42	5575.20	0.006559	8.18	498.30	297.75	0.97
La Cueva	1625.11	PF 1	3090.00	5568.53	5571.76	5571.76	5572.68	0.005578	7.94	441.60	263.16	0.90
La Cueva	1565.71	PF 1	3090.00	5567.46	5571.36	5570.73	5572.26	0.000864	7.59	407.37	304.38	0.73
La Cueva	1565		Culvert									
La Cueva	1511.28	PF 1	3090.00	5565.27	5568.02	5568.02	5569.40	0.006598	9.43	327.80	129.80	1.01
La Cueva	1438.58	PF 1	3090.00	5562.96	5565.99	5565.99	5567.46	0.006790	9.75	316.88	108.64	1.01
La Cueva	1410.93	PF 1	3090.00	5562.08	5565.33	5565.33	5566.82	0.006504	9.80	315.16	104.39	0.99
La Cueva	1380.76	PF 1	3090.00	5561.02	5564.66	5564.66	5566.13	0.006422	9.75	318.83	108.31	0.98
La Cueva	1354.24	PF 1	3090.00	5560.01	5563.88	5563.88	5565.36	0.006773	9.78	318.91	111.53	1.00
La Cueva	1332.33	PF 1	3090.00	5559.18	5562.95	5562.95	5564.40	0.006421	9.66	327.31	124.97	0.98
La Cueva	1311.52	PF 1	3090.00	5558.38	5562.27	5562.27	5563.68	0.006749	9.51	325.61	121.14	1.00
La Cueva	1295.47	PF 1	3090.00	5557.77	5561.80	5561.80	5563.27	0.006546	9.73	322.45	120.27	0.99
La Cueva	1264.68	PF 1	3090.00	5556.60	5560.72	5560.72	5562.27	0.006168	10.02	314.72	106.97	0.99
La Cueva	1233.92	PF 1	3090.00	5555.00	5560.60		5561.05	0.001078	5.42	581.32	135.31	0.44
La Cueva	1187.49	PF 1	3090.00	5554.00	5559.39	5559.39	5560.85	0.006843	9.70	319.75	116.85	1.02
La Cueva	1146.73	PF 1	3090.00	5552.95	5557.03	5557.03	5558.39	0.007467	9.36	330.07	123.79	1.01
La Cueva	1104.38	PF 1	3090.00	5551.33	5555.56	5555.56	5556.88	0.006689	9.24	334.57	128.11	1.01
La Cueva	1056.97	PF 1	3090.00	5550.42	5554.19	5554.19	5555.61	0.007123	10.30	330.98	122.65	0.99
La Cueva	1011.94	PF 1	3090.00	5548.91	5553.05	5553.05	5554.54	0.005560	9.92	331.33	125.78	0.95
La Cueva	975.03	PF 1	3090.00	5547.84	5552.37	5552.37	5553.69	0.004434	9.42	373.61	179.46	0.86
La Cueva	916.13	PF 1	3090.00	5546.08	5550.02	5550.02	5551.26	0.007613	9.08	366.90	162.62	0.94
La Cueva	848.23	PF 1	3090.00	5543.83	5547.96	5547.96	5549.43	0.006180	9.75	322.31	118.12	0.99
La Cueva	804.54	PF 1	3090.00	5542.34	5547.29	5547.29	5548.23	0.003850	8.66	533.57	334.36	0.80
La Cueva	742.87	PF 1	3090.00	5540.82	5545.55	5545.55	5546.24	0.003047	7.50	647.08	461.93	0.71
La Cueva	711.63	PF 1	3090.00	5540.27	5544.34	5544.34	5545.11	0.005125	7.82	541.27	436.19	0.87
La Cueva	674.15	PF 1	3090.00	5538.78	5543.75	5543.75	5544.33	0.003327	8.03	737.10	506.26	0.74
La Cueva	618.69	PF 1	3090.00	5537.00	5541.31	5541.31	5542.19	0.004911	8.76	533.90	353.42	0.88
La Cueva	491.87	PF 1	3090.00	5533.46	5538.41	5538.41	5539.08	0.003589	7.68	653.45	473.47	0.76
La Cueva	450.17	PF 1	3090.00	5531.00	5536.70	5536.70	5537.33	0.004206	8.90	673.83	452.95	0.83
La Cueva	432.04	PF 1	3090.00	5530.99	5535.95	5535.95	5536.58	0.005290	8.03	618.73	440.66	0.89
La Cueva	356.79	PF 1	3090.00	5528.87	5533.66	5533.66	5534.28	0.004595	7.70	646.34	466.93	0.83
La Cueva	306.95	PF 1	3090.00	5528.01	5532.38	5532.38	5533.03	0.005150	8.12	622.76	444.36	0.88
La Cueva	247.1	PF 1	3090.00	5526.33	5530.83	5530.83	5531.53	0.004044	8.19	621.67	405.44	0.81
La Cueva	208.39	PF 1	3090.00	5525.00	5529.99	5529.99	5530.92	0.004653	9.61	545.83	321.98	0.88
La Cueva	172.73	PF 1	3090.00	5524.41	5528.92	5528.92	5529.95	0.005146	8.95	442.89	210.75	0.90
La Cueva	98.74	PF 1	3090.00	5521.70	5527.46	5526.20	5528.00	0.001587	6.61	606.35	185.85	0.54

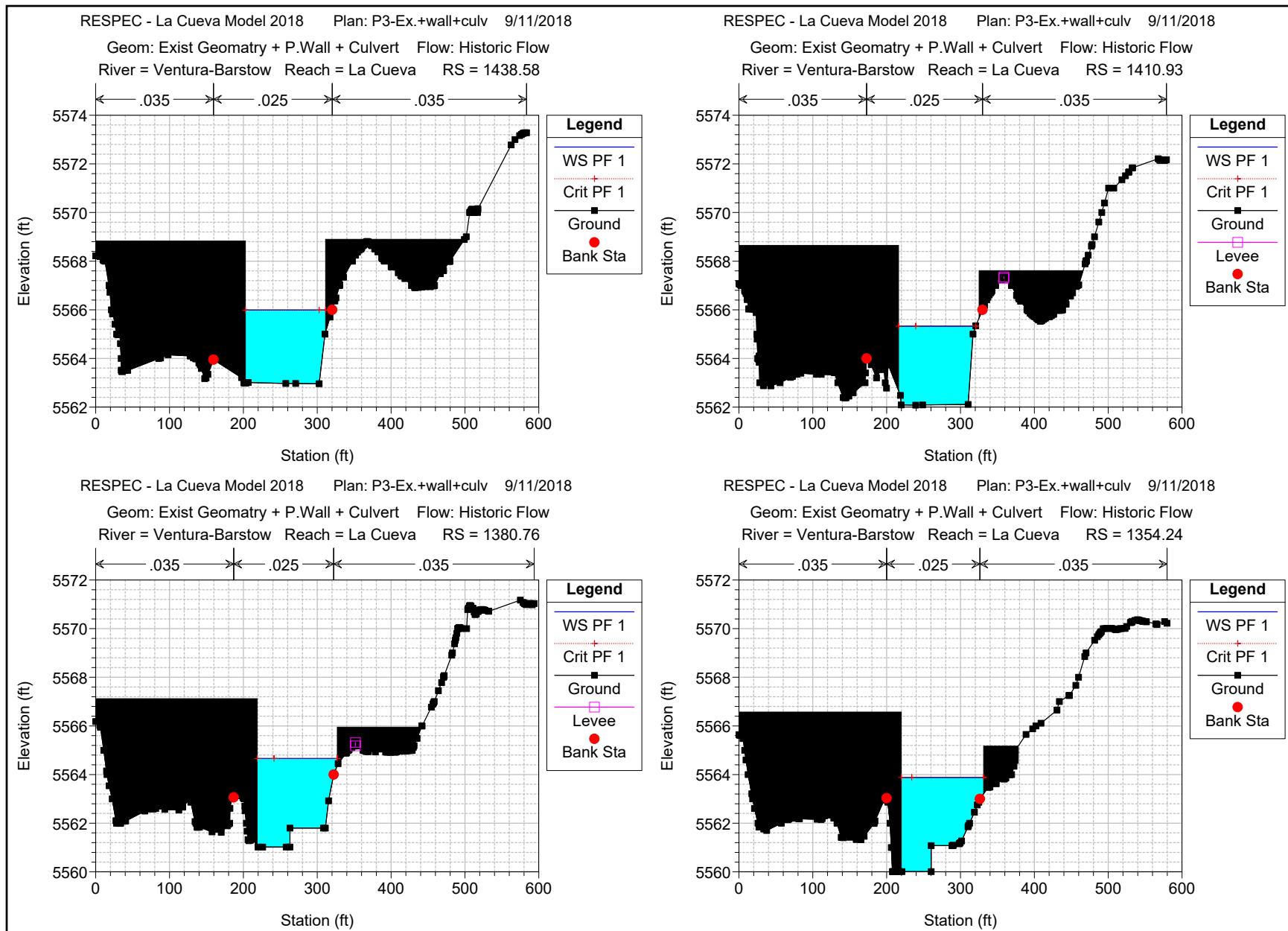
HEC-RAS Plan: P3-Ex.+wall+culv River: Ventura-Barstow Reach: La Cueva Profile: PF 1 (Continued)

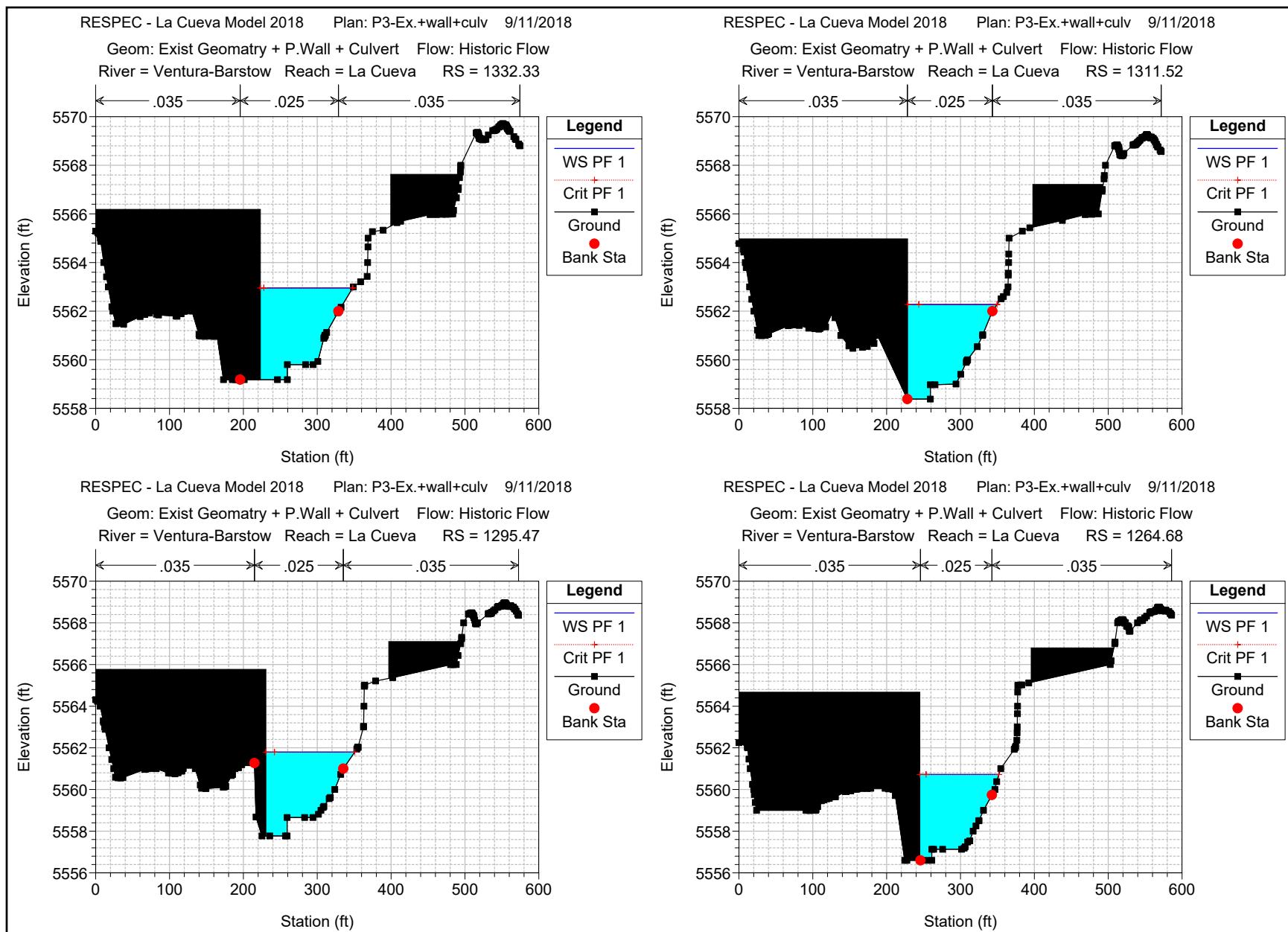
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
La Cueva	1.24	PF 1	3090.00	5518.52	5525.15	5525.15	5527.54	0.005708	12.42	248.98	53.21	0.99

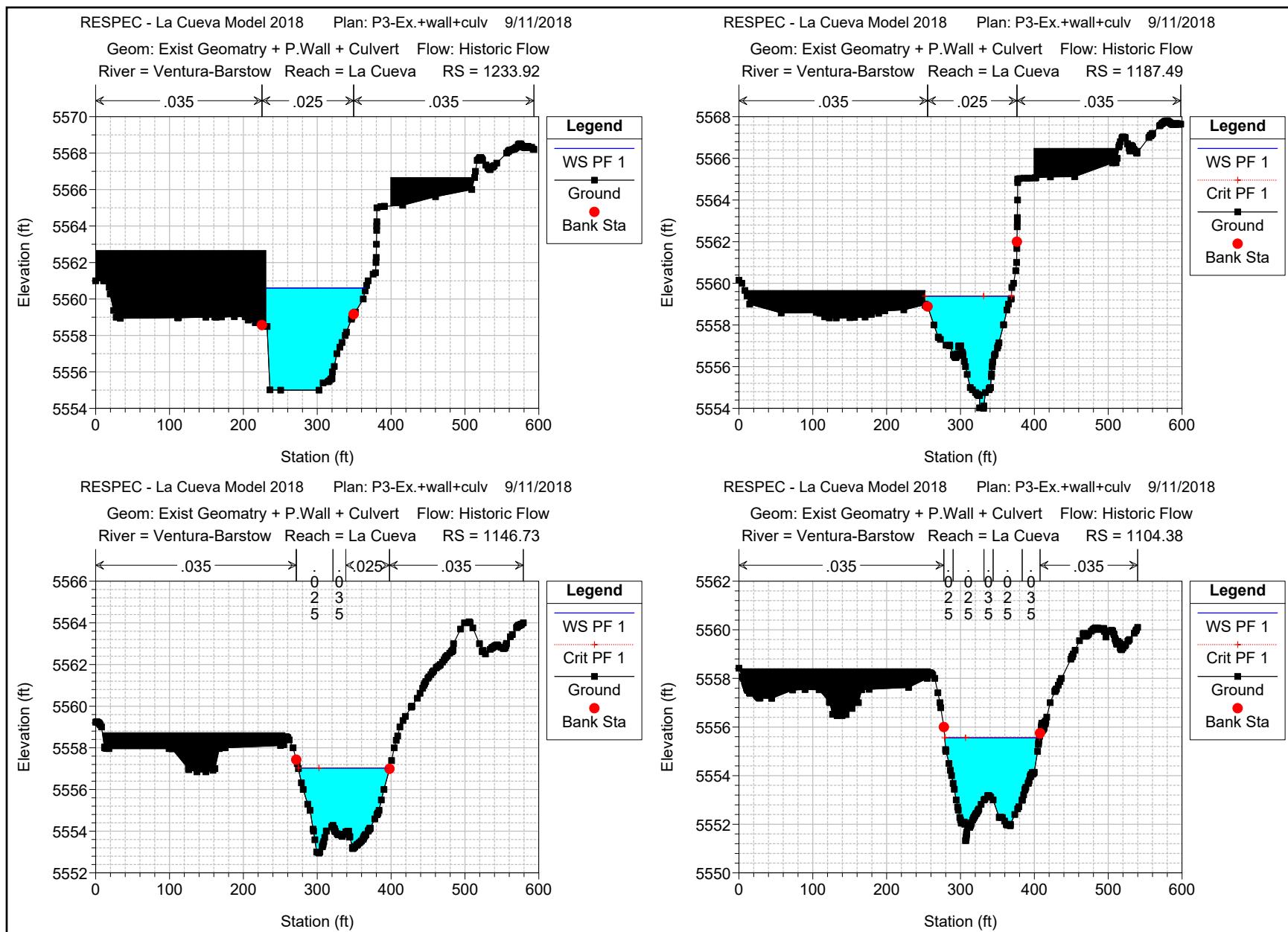
Proposed Scour Wall

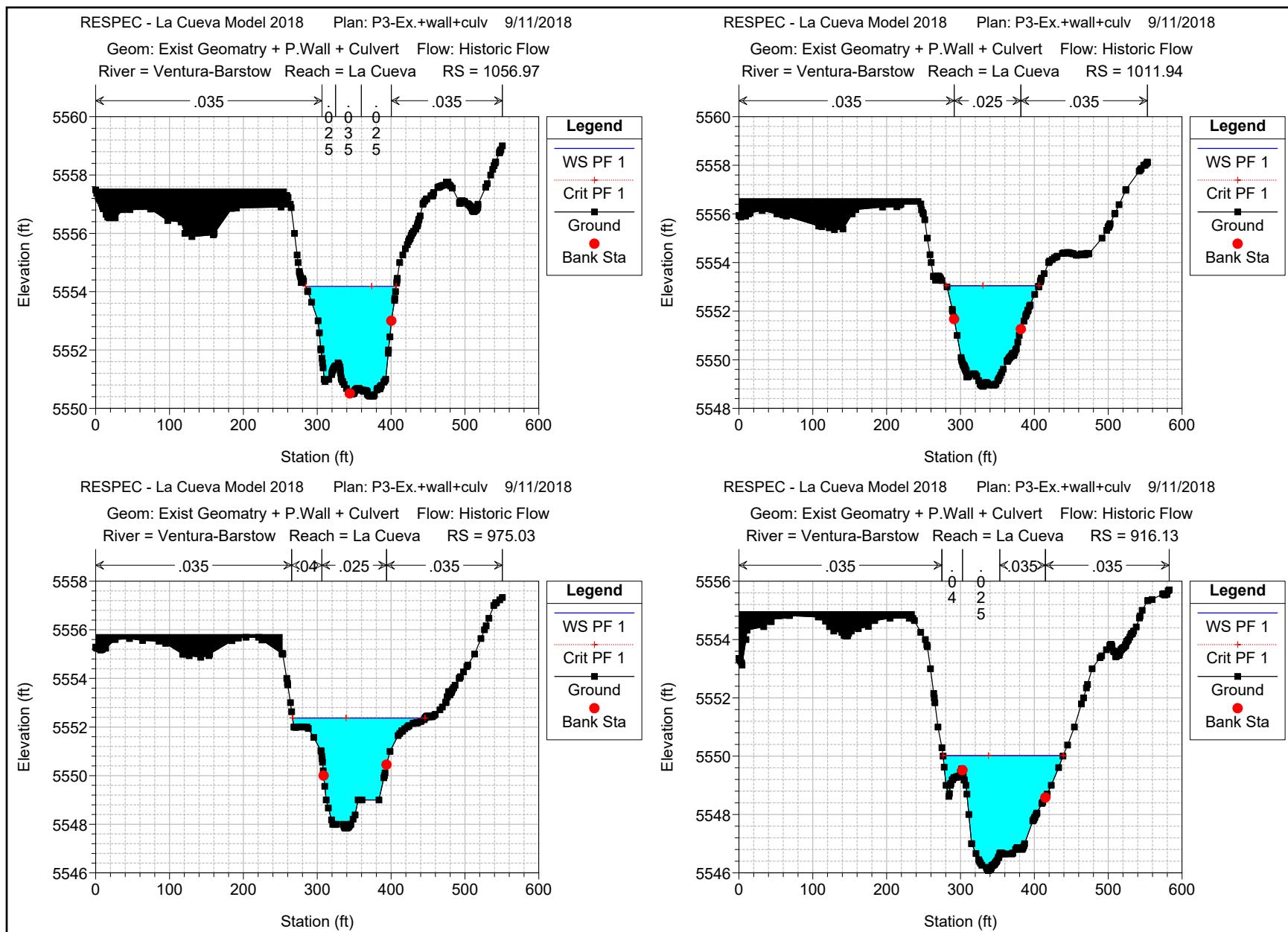


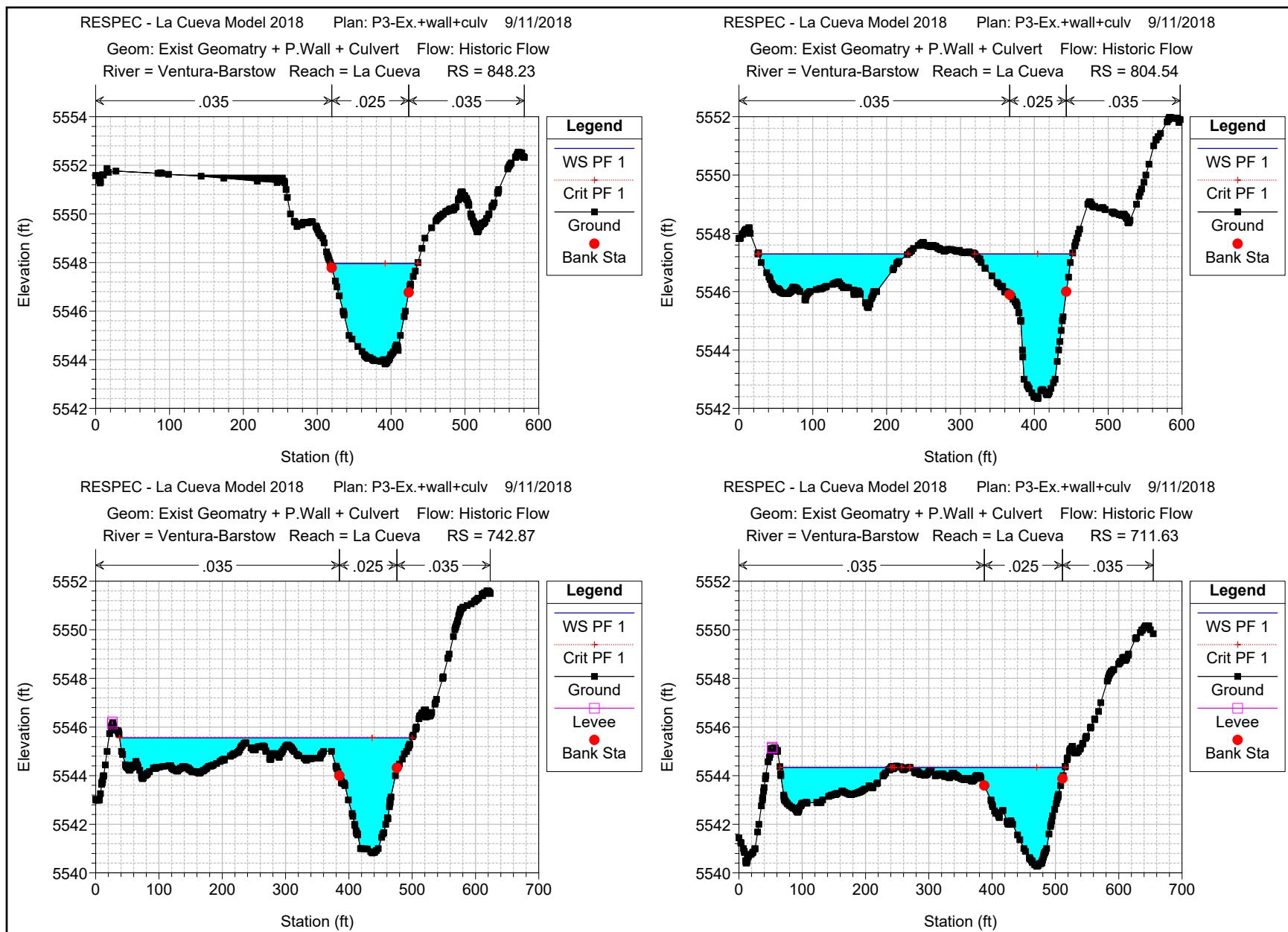


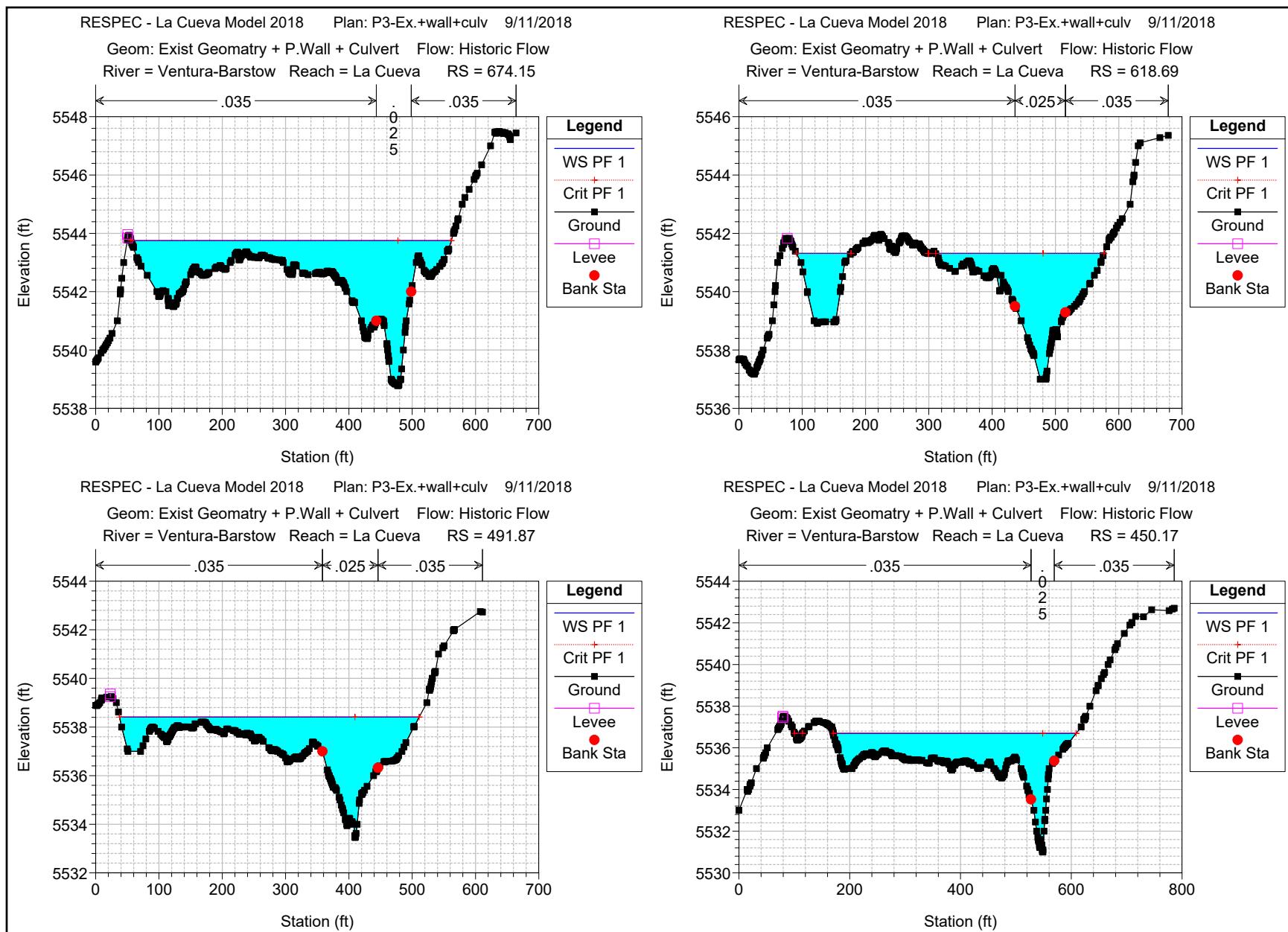


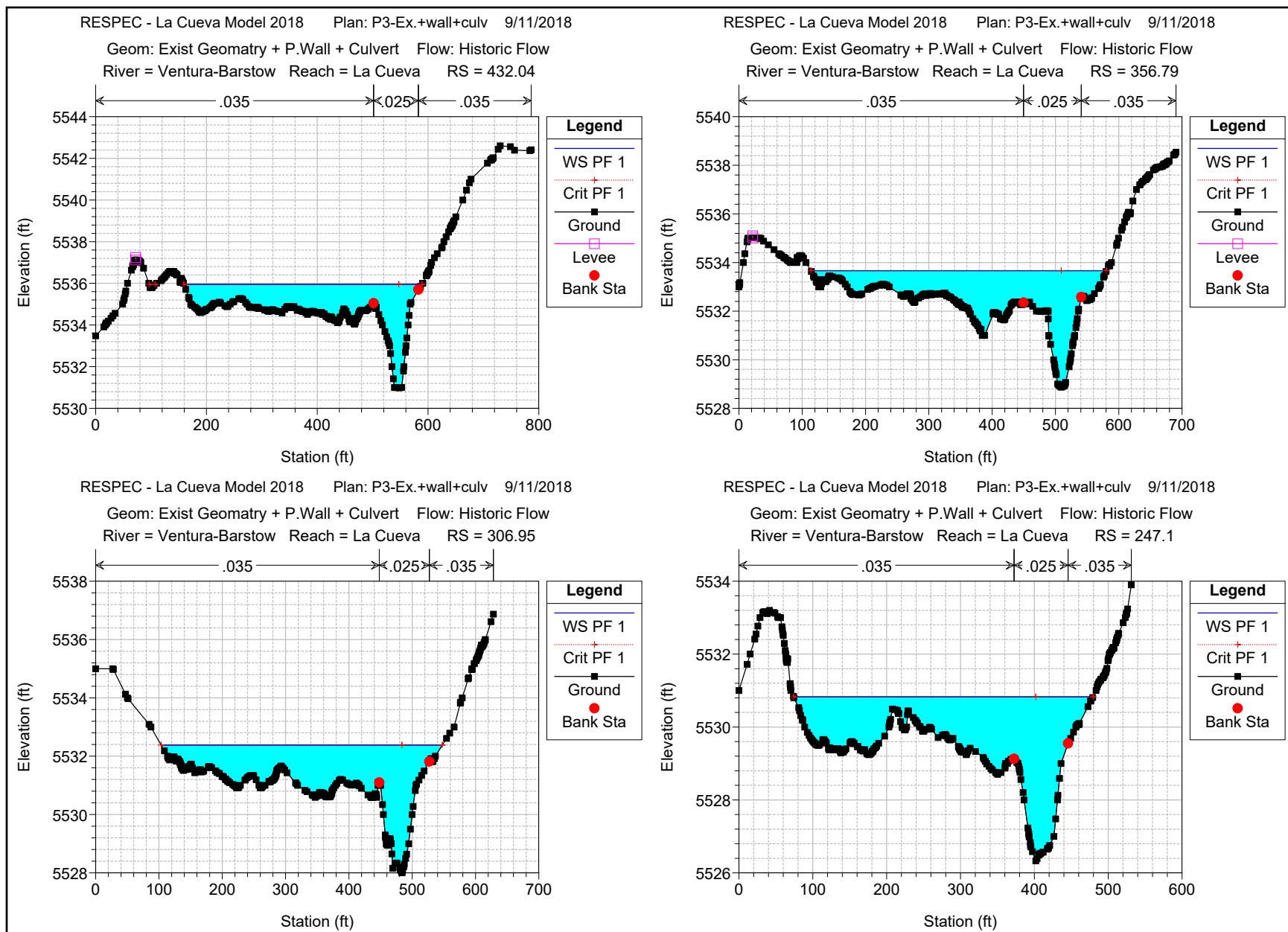


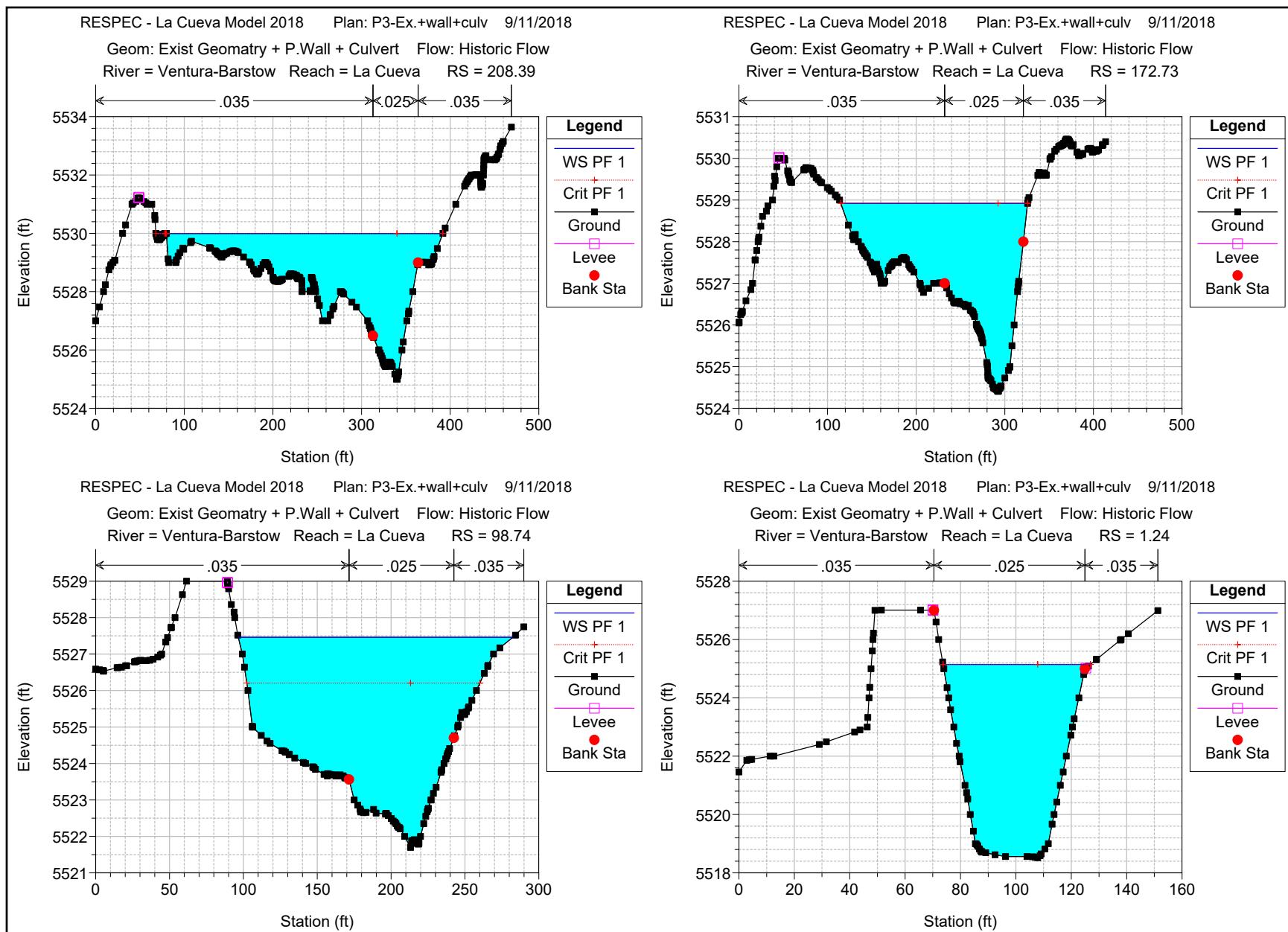










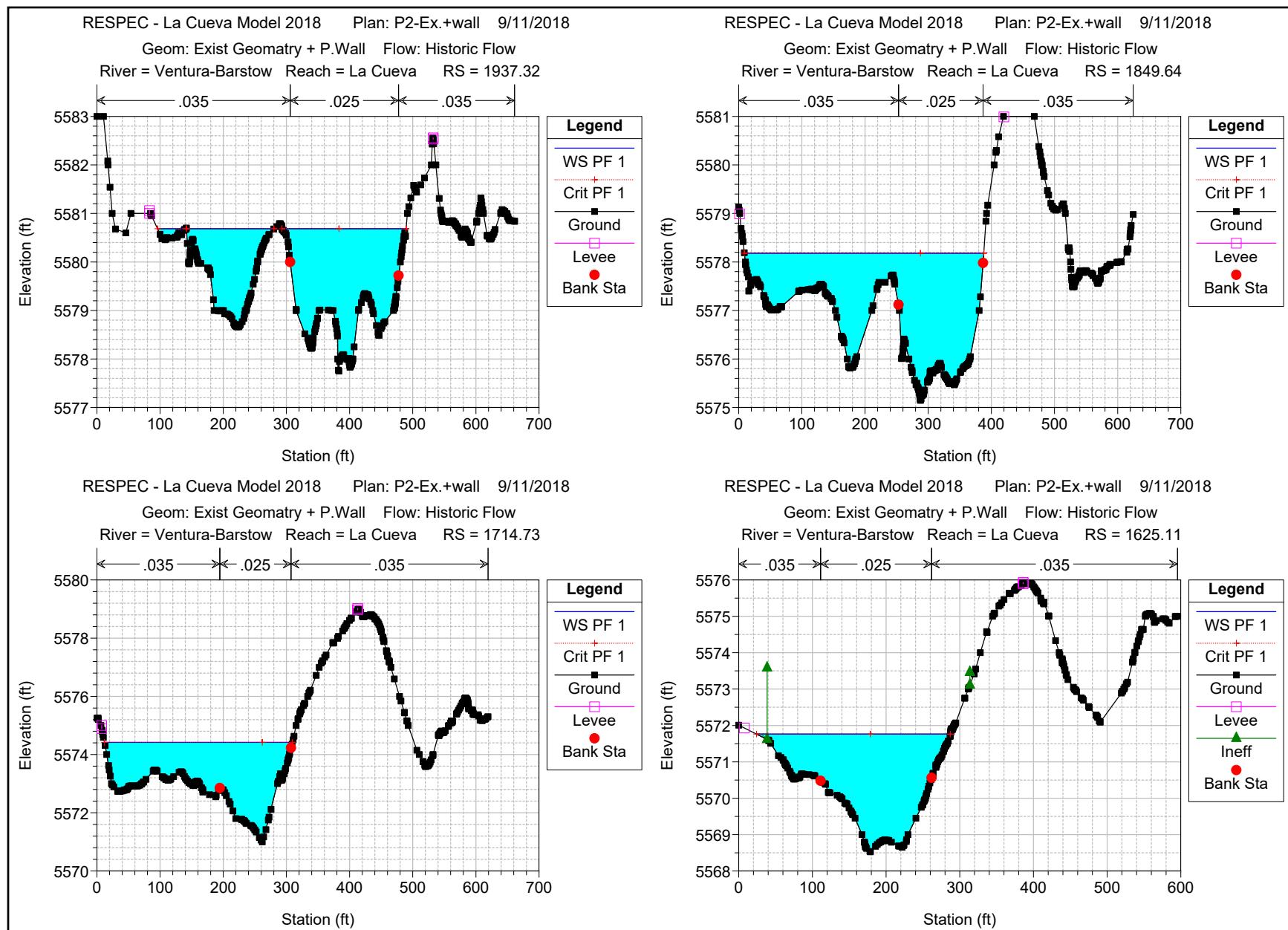


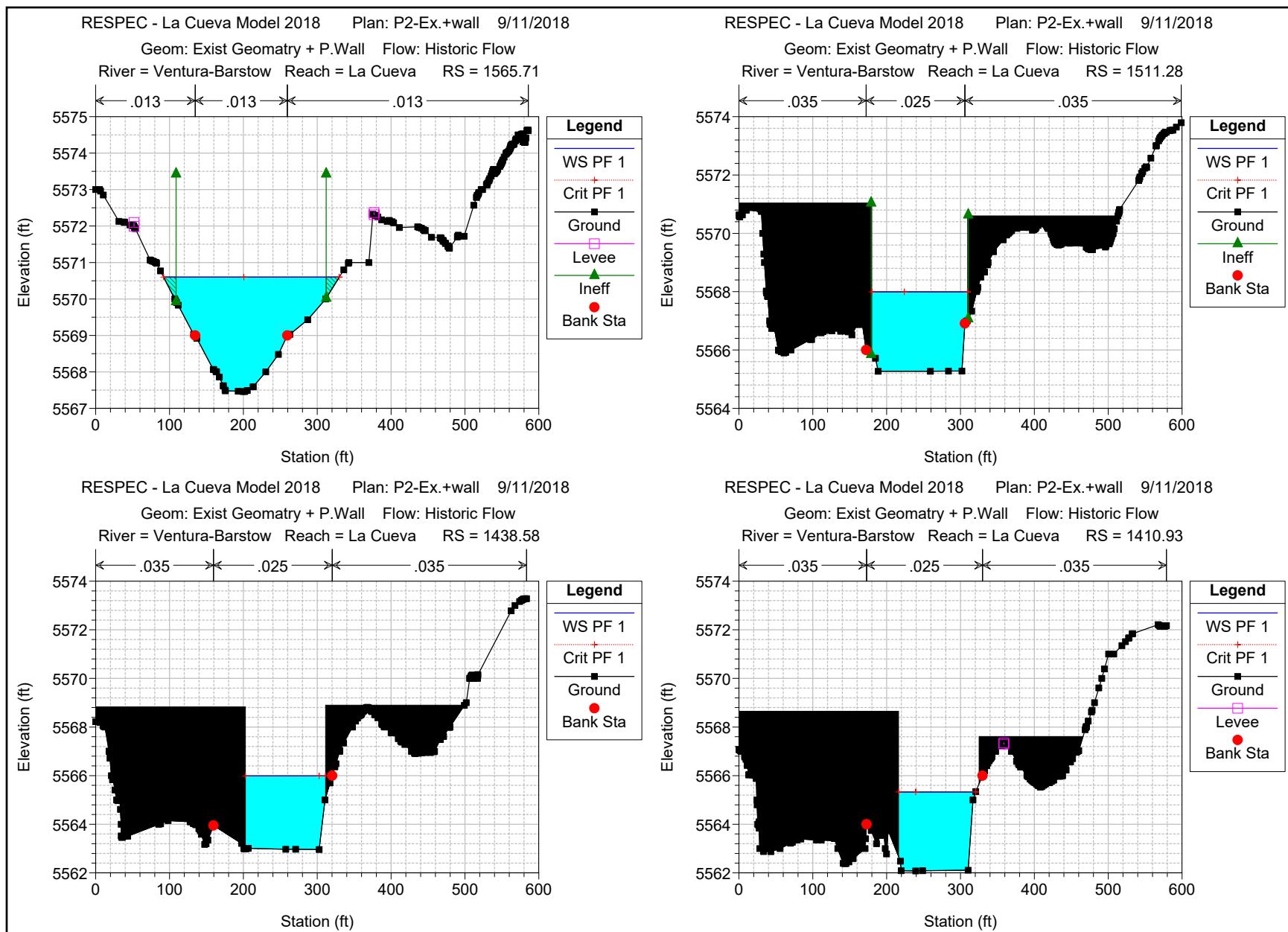
Proposed Scour wall and Culvert @ Ventura

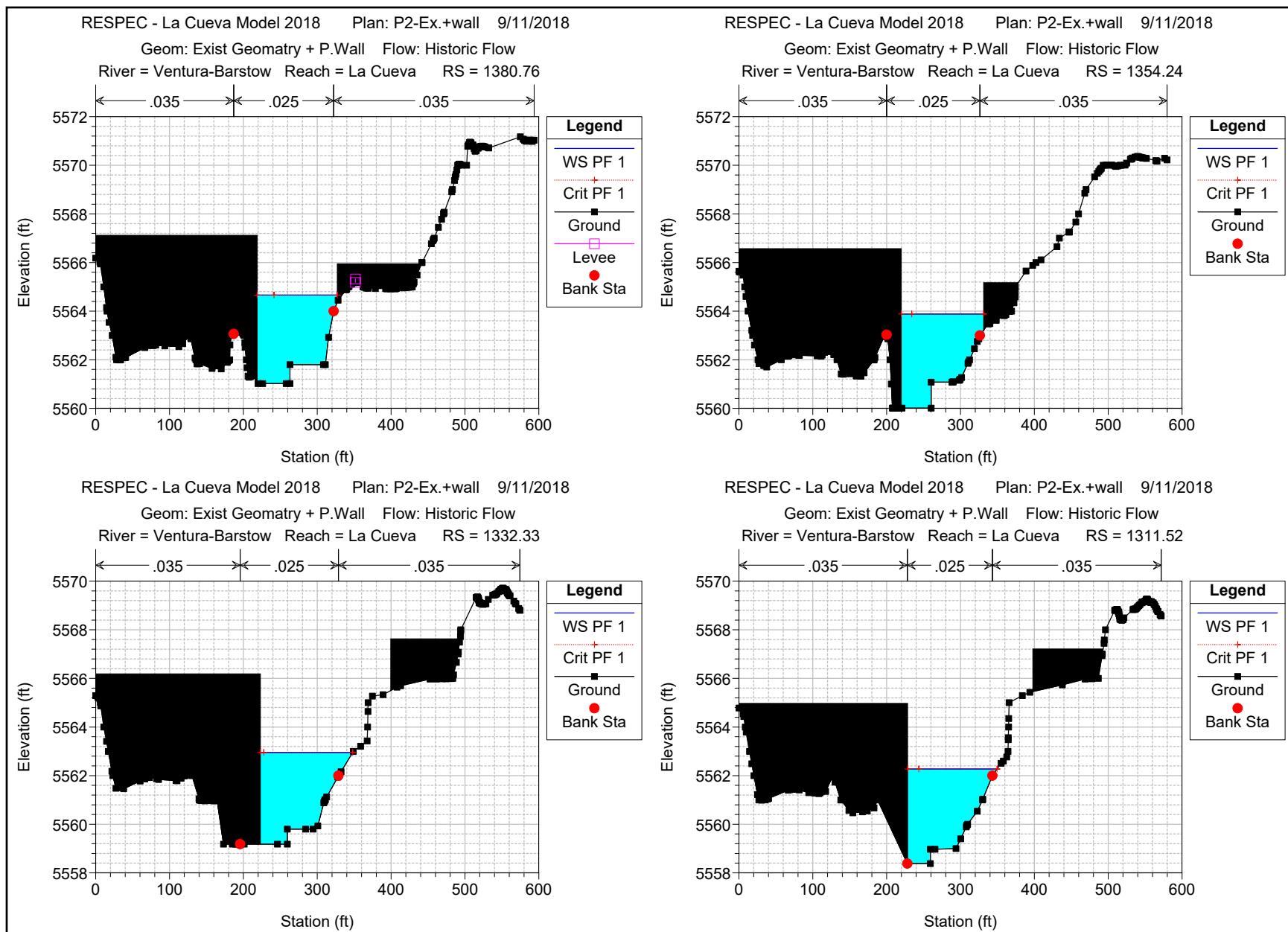
HEC-RAS Plan: P2-Ex.+wall River: Ventura-Barstow Reach: La Cueva Profile: PF 1

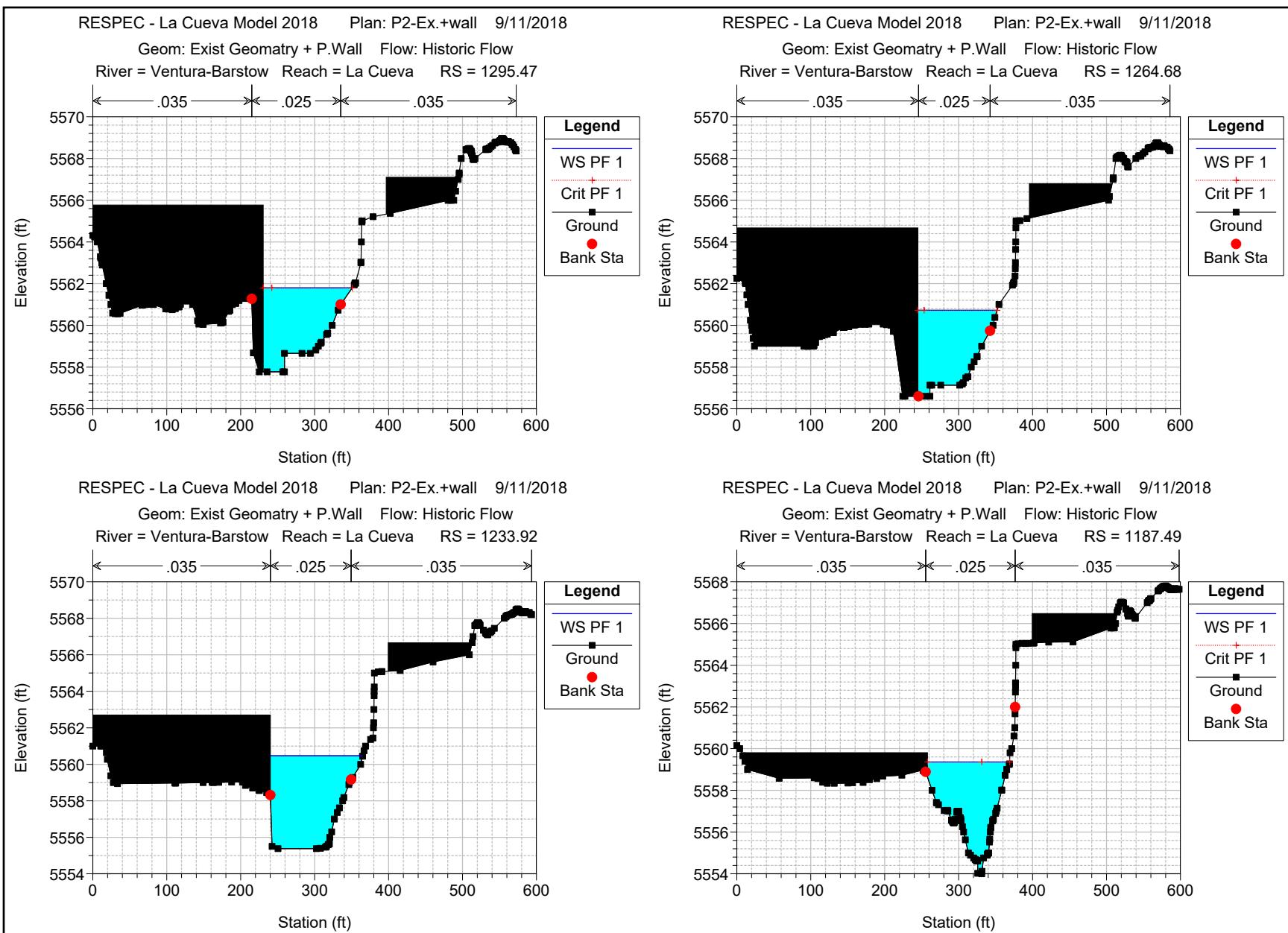
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
La Cueva	1937.32	PF 1	3090.00	5577.75	5580.68	5580.68	5581.45	0.006873	7.60	495.54	380.14	0.97
La Cueva	1849.64	PF 1	3090.00	5575.15	5578.18	5578.18	5578.86	0.005312	7.48	559.84	379.10	0.87
La Cueva	1714.73	PF 1	3090.00	5571.00	5574.42	5574.42	5575.20	0.006559	8.18	498.30	297.75	0.97
La Cueva	1625.11	PF 1	3090.00	5568.53	5571.76	5571.76	5572.68	0.005578	7.94	441.60	263.16	0.90
La Cueva	1565.71	PF 1	3090.00	5567.46	5570.60	5570.60	5571.55	0.001448	8.21	414.04	237.89	0.90
La Cueva	1511.28	PF 1	3090.00	5565.27	5568.00	5568.00	5569.29	0.006567	9.15	340.37	129.80	0.99
La Cueva	1438.58	PF 1	3090.00	5562.96	5565.99	5565.99	5567.46	0.006790	9.75	316.88	108.64	1.01
La Cueva	1410.93	PF 1	3090.00	5562.08	5565.33	5565.33	5566.82	0.006504	9.80	315.16	104.39	0.99
La Cueva	1380.76	PF 1	3090.00	5561.02	5564.66	5564.66	5566.13	0.006422	9.75	318.83	108.31	0.98
La Cueva	1354.24	PF 1	3090.00	5560.01	5563.88	5563.88	5565.36	0.006773	9.78	318.91	111.53	1.00
La Cueva	1332.33	PF 1	3090.00	5559.18	5562.95	5562.95	5564.40	0.006421	9.66	327.31	124.97	0.98
La Cueva	1311.52	PF 1	3090.00	5558.38	5562.27	5562.27	5563.68	0.006749	9.51	325.61	121.14	1.00
La Cueva	1295.47	PF 1	3090.00	5557.77	5561.80	5561.80	5563.27	0.006546	9.73	322.45	120.27	0.99
La Cueva	1264.68	PF 1	3090.00	5556.60	5560.72	5560.72	5562.27	0.006168	10.02	314.72	106.97	0.99
La Cueva	1233.92	PF 1	3090.00	5555.38	5560.47		5561.10	0.001652	6.36	495.10	124.81	0.53
La Cueva	1187.49	PF 1	3090.00	5554.00	5559.36	5559.36	5560.87	0.007049	9.85	313.62	110.91	1.03
La Cueva	1146.73	PF 1	3090.00	5552.95	5557.03	5557.03	5558.39	0.007467	9.36	330.07	123.79	1.01
La Cueva	1104.38	PF 1	3090.00	5551.33	5555.56	5555.56	5556.88	0.006689	9.24	334.57	128.11	1.01
La Cueva	1056.97	PF 1	3090.00	5550.42	5554.19	5554.19	5555.61	0.007123	10.30	330.98	122.65	0.99
La Cueva	1011.94	PF 1	3090.00	5548.91	5553.05	5553.05	5554.54	0.005560	9.92	331.33	125.78	0.95
La Cueva	975.03	PF 1	3090.00	5547.84	5552.37	5552.37	5553.69	0.004434	9.42	373.61	179.46	0.86
La Cueva	916.13	PF 1	3090.00	5546.08	5550.02	5550.02	5551.26	0.007613	9.08	366.90	162.62	0.94
La Cueva	848.23	PF 1	3090.00	5543.83	5547.96	5547.96	5549.43	0.006180	9.75	322.31	118.12	0.99
La Cueva	804.54	PF 1	3090.00	5542.34	5547.29	5547.29	5548.23	0.003850	8.66	533.57	334.36	0.80
La Cueva	742.87	PF 1	3090.00	5540.82	5545.55	5545.55	5546.24	0.003047	7.50	647.08	461.93	0.71
La Cueva	711.63	PF 1	3090.00	5540.27	5544.34	5544.34	5545.11	0.005125	7.82	541.27	436.19	0.87
La Cueva	674.15	PF 1	3090.00	5538.78	5543.75	5543.75	5544.33	0.003327	8.03	737.10	506.26	0.74
La Cueva	618.69	PF 1	3090.00	5537.00	5541.31	5541.31	5542.19	0.004911	8.76	533.90	353.42	0.88
La Cueva	491.87	PF 1	3090.00	5533.46	5538.41	5538.41	5539.08	0.003589	7.68	653.45	473.47	0.76
La Cueva	450.17	PF 1	3090.00	5531.00	5536.70	5536.70	5537.33	0.004206	8.90	673.83	452.95	0.83
La Cueva	432.04	PF 1	3090.00	5530.99	5535.95	5535.95	5536.58	0.005290	8.03	618.73	440.66	0.89
La Cueva	356.79	PF 1	3090.00	5528.87	5533.66	5533.66	5534.28	0.004595	7.70	646.34	466.93	0.83
La Cueva	306.95	PF 1	3090.00	5528.01	5532.38	5532.38	5533.03	0.005150	8.12	622.76	444.36	0.88
La Cueva	247.1	PF 1	3090.00	5526.33	5530.83	5530.83	5531.53	0.004044	8.19	621.67	405.44	0.81
La Cueva	208.39	PF 1	3090.00	5525.00	5529.99	5529.99	5530.92	0.004653	9.61	545.83	321.98	0.88
La Cueva	172.73	PF 1	3090.00	5524.41	5528.92	5528.92	5529.95	0.005146	8.95	442.89	210.75	0.90
La Cueva	98.74	PF 1	3090.00	5521.70	5527.46	5526.20	5528.00	0.001587	6.61	606.35	185.85	0.54
La Cueva	1.24	PF 1	3090.00	5518.52	5525.15	5525.15	5527.54	0.005708	12.42	248.98	53.21	0.99

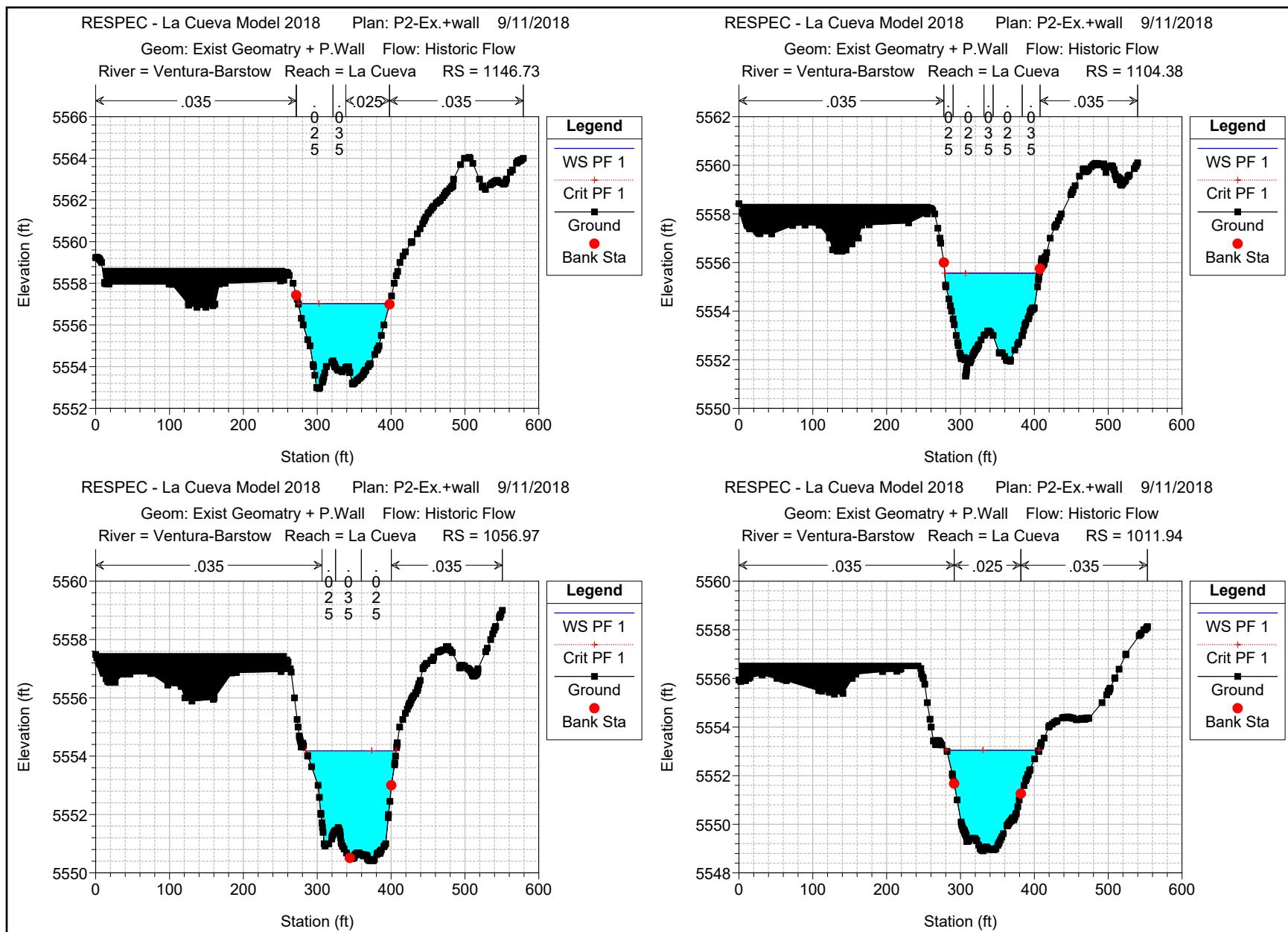
Proposed Scour wall and Culvert @ Ventura

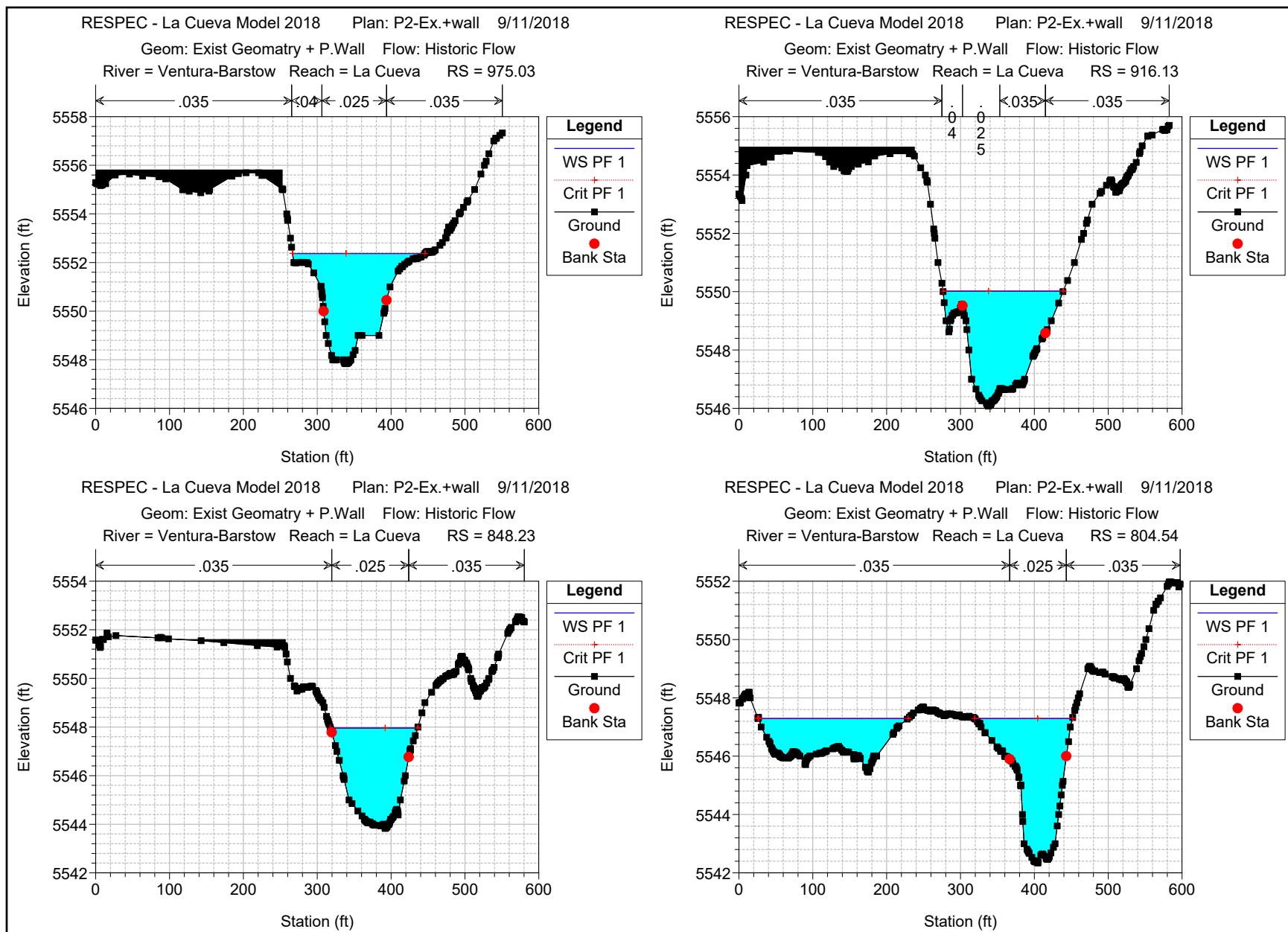


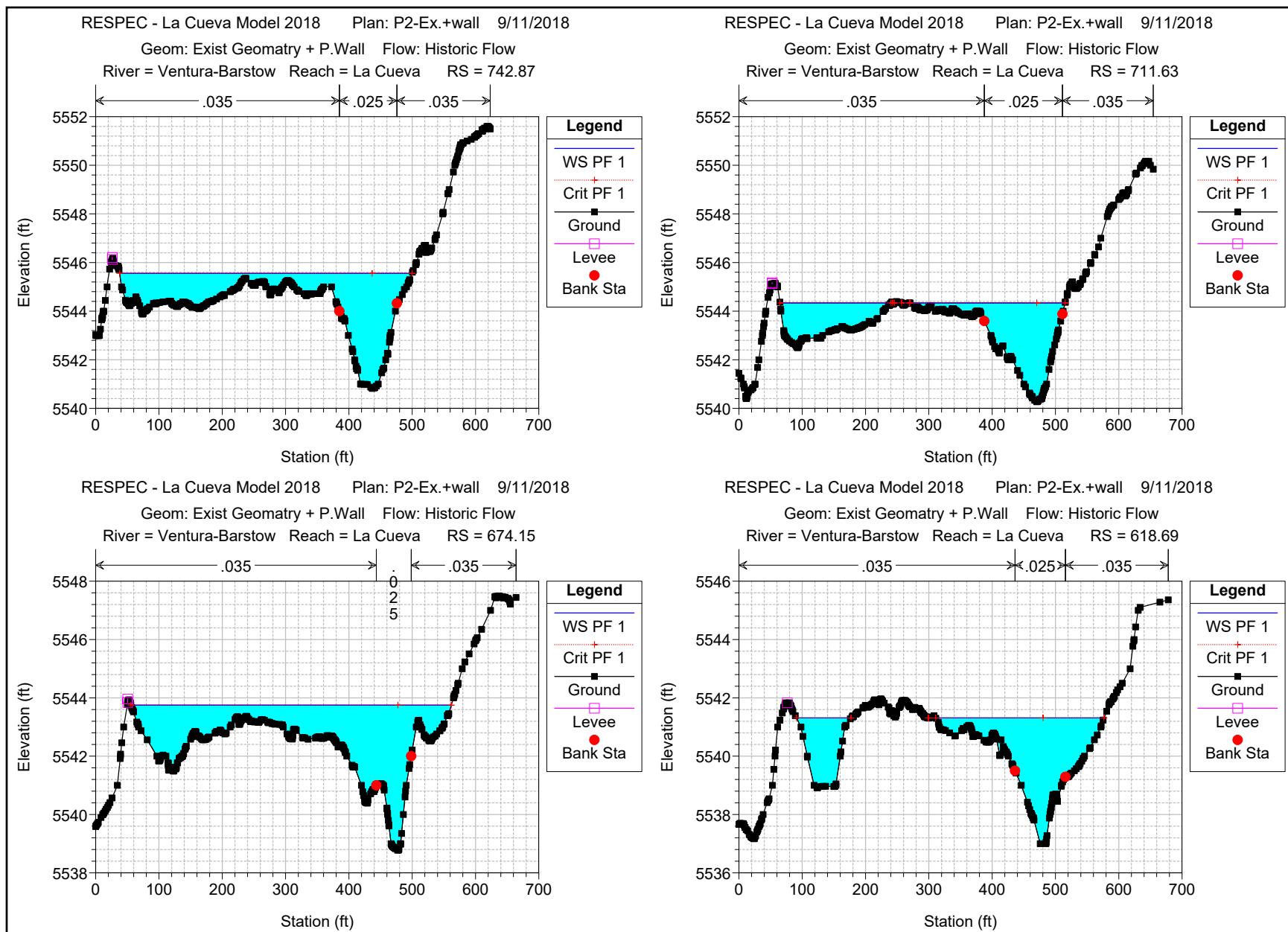


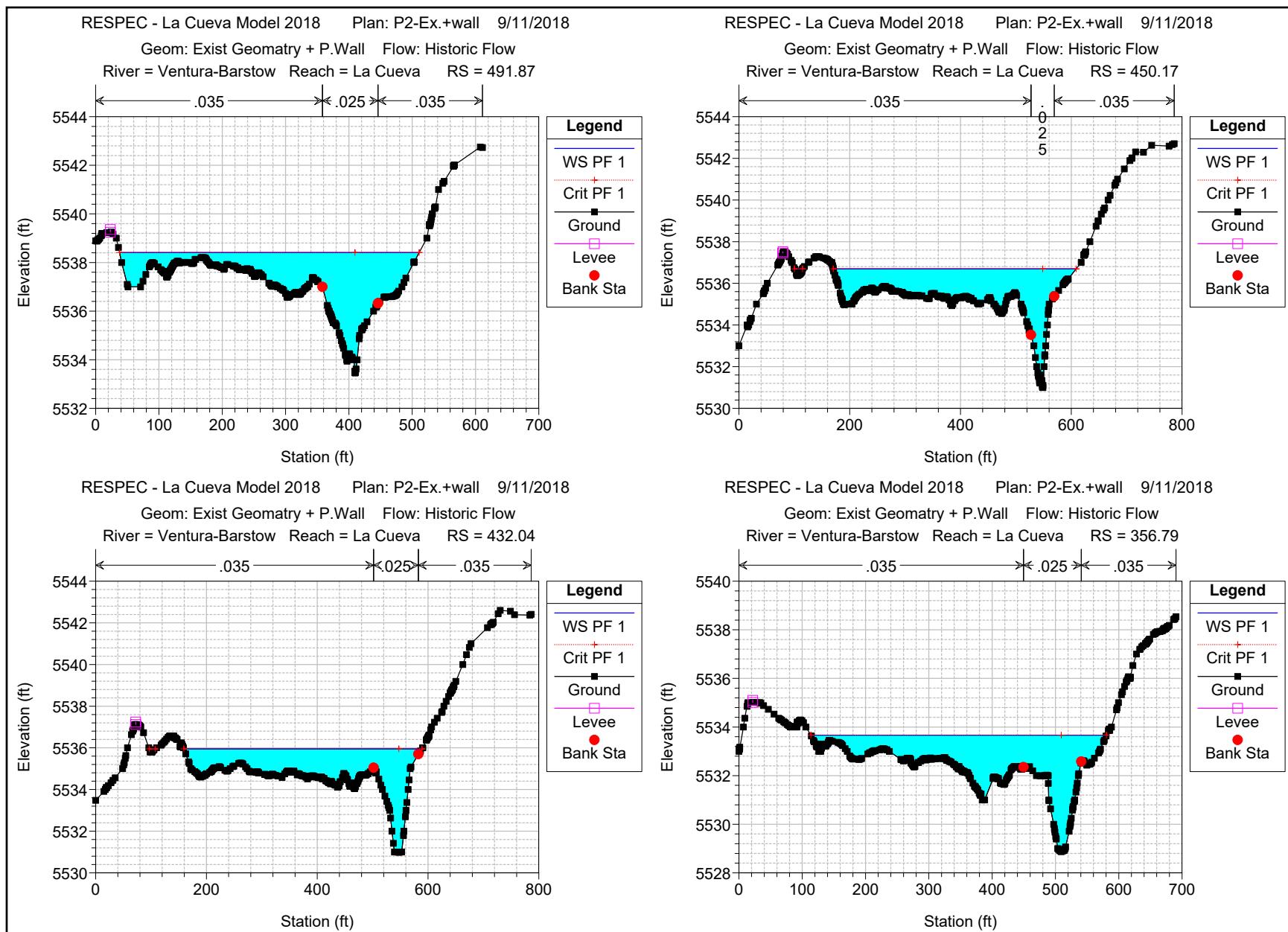


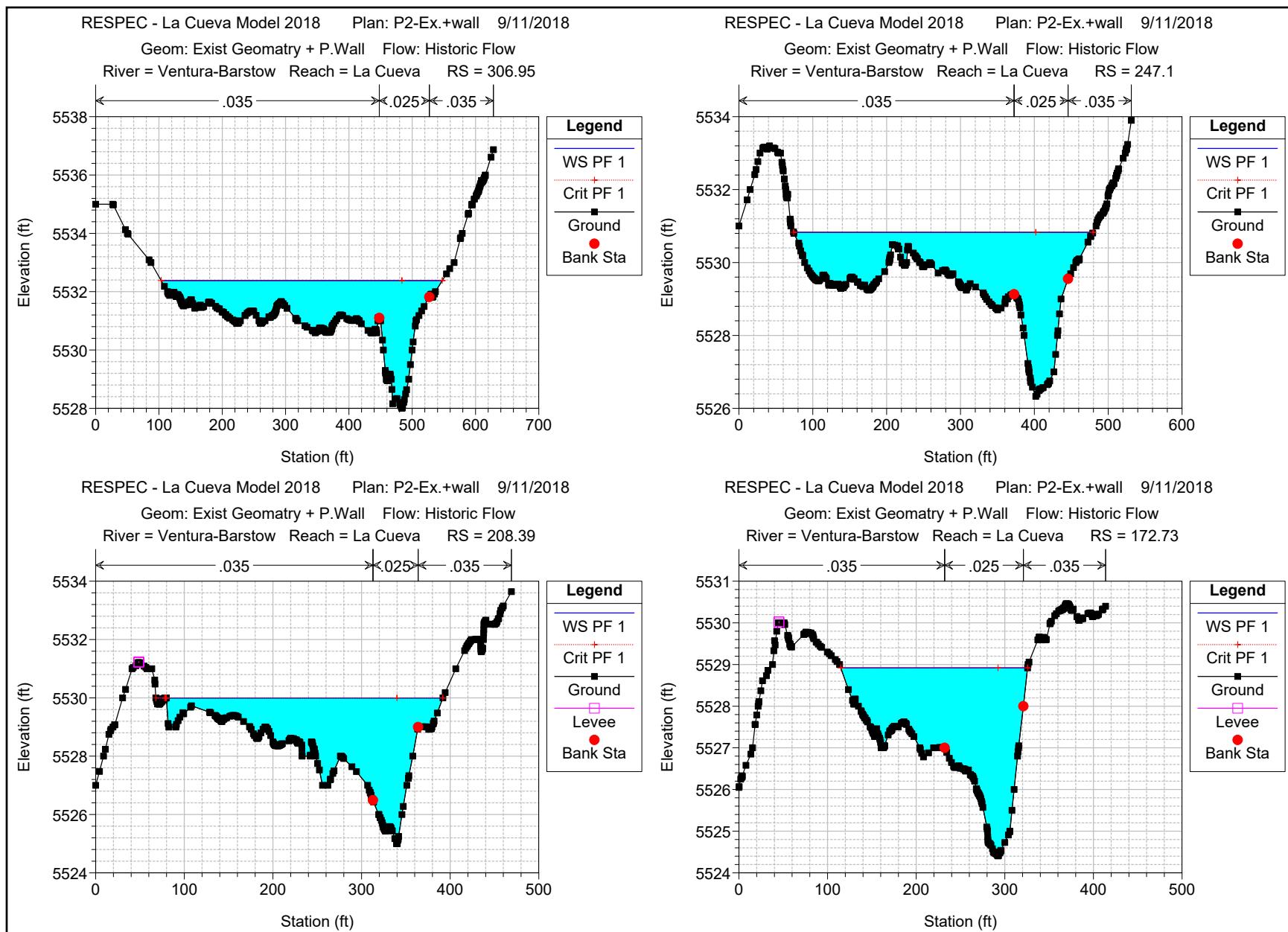


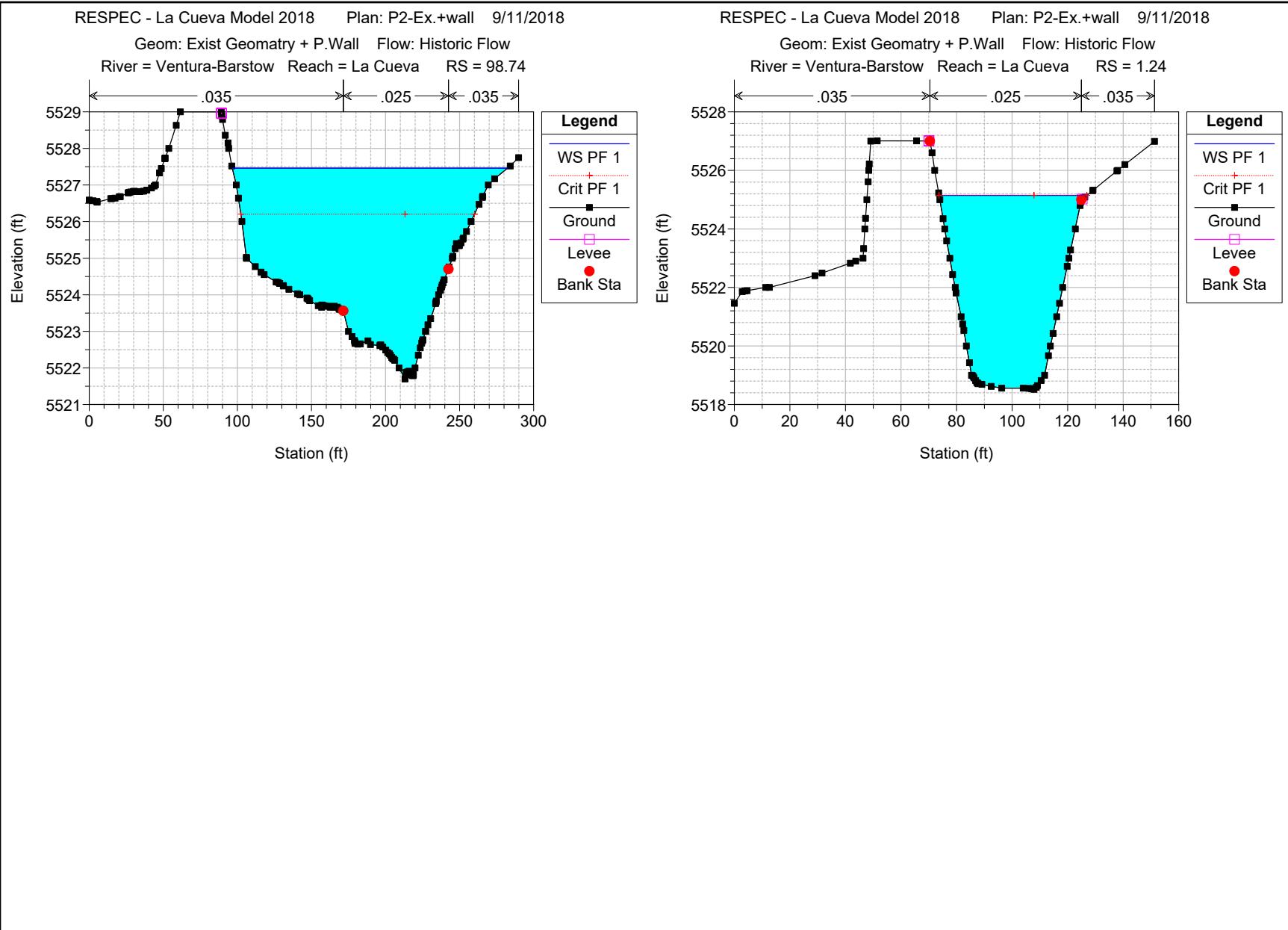












Appendix D

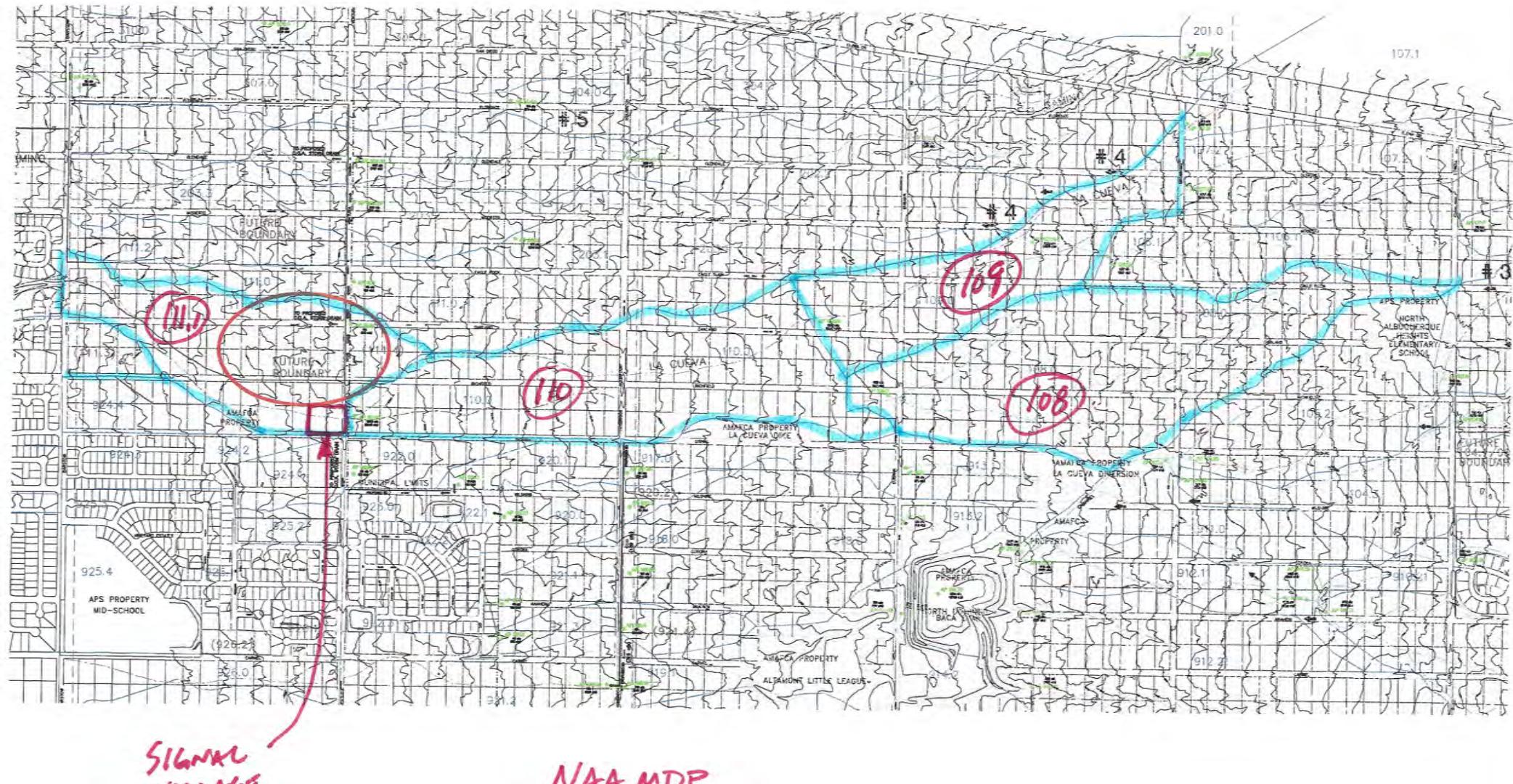
SIGNAL VILLAGE FINE SEDIMENT YIELD ANALYSIS USING MUSLE EQUATION

BASIN	AREA AC				PERCENT IMPERV	QP CFS	VOL AC-FT	WGHTD K	C	GAMMA	SLOPE %	N	LS	YS TONS	YS' TONS	CS PPM	QFS WASH LOAD
108	131.50				17%	404.63	15.57	0.150	0.20	400	4	0.4	0.69	795.91	660.60	30273	4.77
109	64.38				17%	216.30	7.55	0.150	0.20	400	4	0.4	0.69	373.69	310.16	29340	2.47
110	104.00				17%	275.61	11.74	0.150	0.20	400	4	0.4	0.69	548.03	454.87	27718	2.96
111.1	62.00				26%	195.97	7.70	0.150	0.20	400	4	0.4	0.69	357.51	264.56	24657	1.87

BED MATERIAL AND TOTAL SEDIMENT YIELD ANALYSIS USING MPM-WOO METHOD

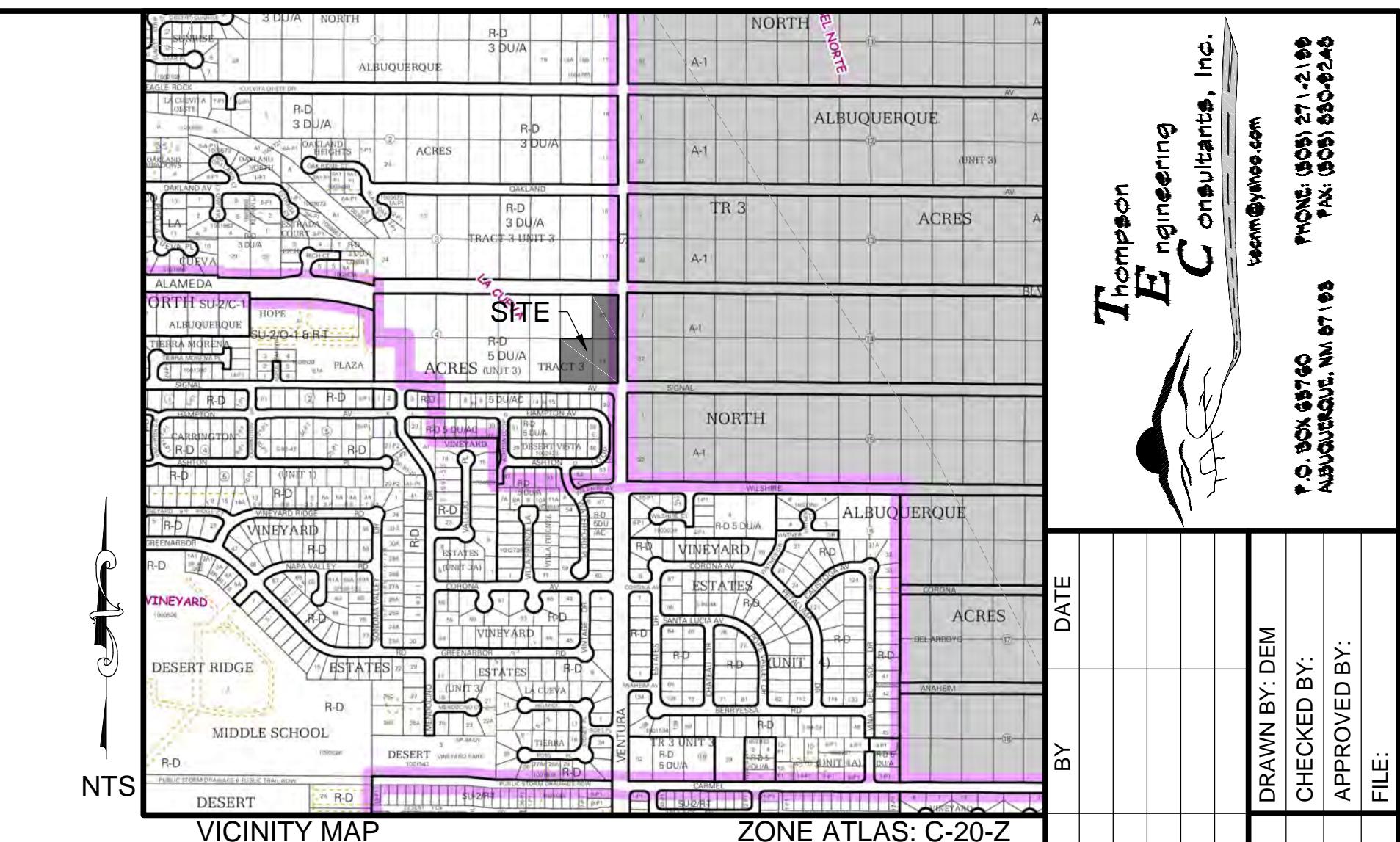
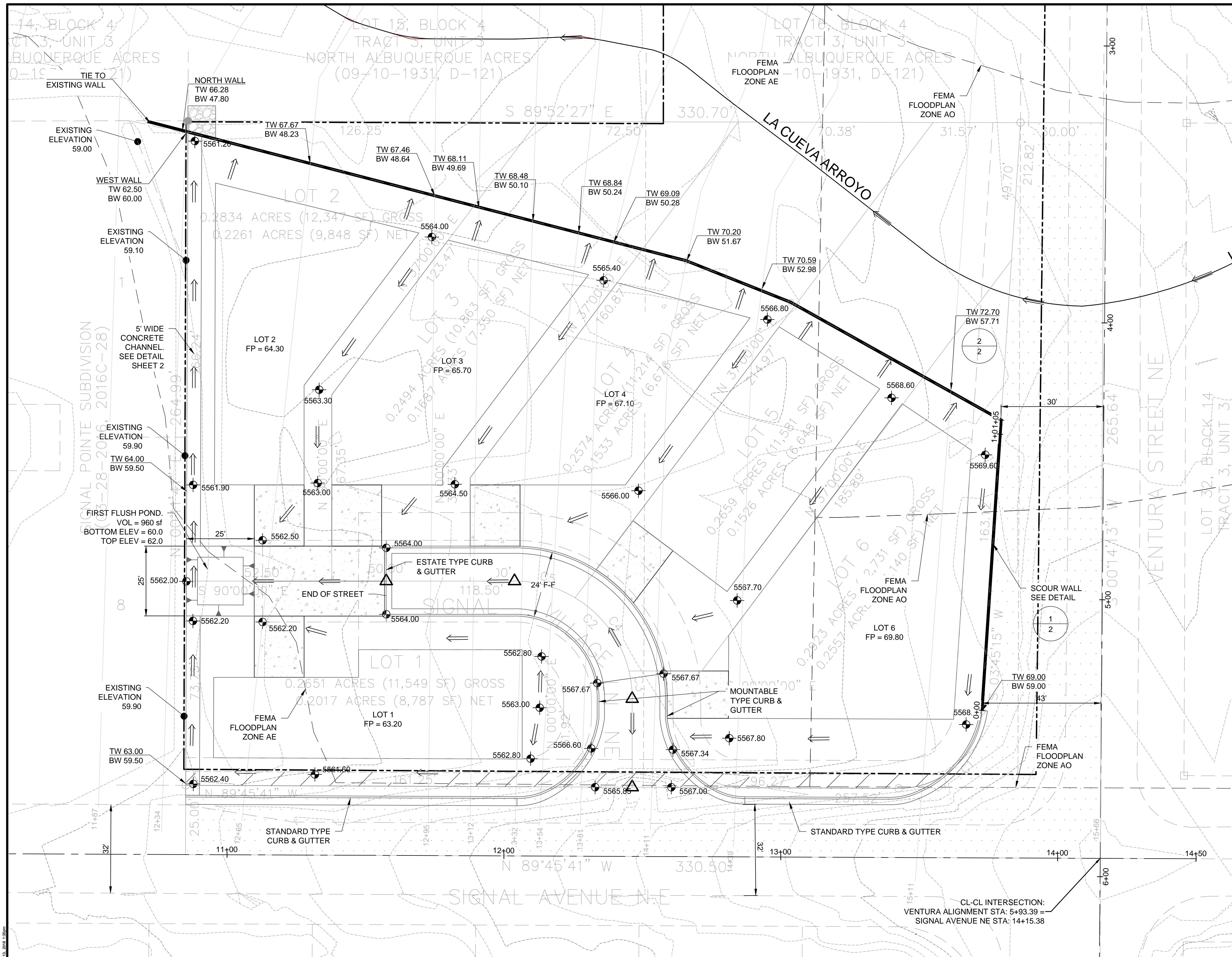
a = 5.00E-04
 b = 3.85
 c = 0.30
 d = -2.50

BASIN	AREA AC	QP CFS	VOL AC-FT	YS' TONS	SED. VOL. AC-FT	Cfs PPM	q unit width	Avg. slope	VEL	DEPTH	WD	qs (unit width bed load)	QS BED LOAD	QS TOTAL	BF	Q BULK	SED VOL AC-FT
108	131.50	404.63	15.57	660.60	0.3033	30273	6.74	0.050	7.64	0.88	60.0	1.31	78.46	83.22	1.21	487.85	3.20
109	64.38	216.30	7.55	310.16	0.1424	29340	3.61	0.040	5.56	0.65	60.0	0.35	21.02	23.48	1.11	239.78	0.82
110	104.00	275.61	11.74	454.87	0.2088	27718	4.59	0.035	5.89	0.78	60.0	0.46	27.54	30.51	1.11	306.12	1.30
111.1	62.00	195.97	7.70	264.56	0.1215	24657	3.27	0.035	5.14	0.64	60.0	0.25	15.20	17.07	1.09	213.04	0.67



NAA MDP
BASIN MAP

Appendix E



LEGAL DESCRIPTION: LOTS 17 & 18, BLOCK 4, TRACT 3, UNIT 3, NAA

SITE AREA: 1.6135 ACRES (TOTAL SITE), 1.2020 ACRES (DEVELOPED AREA)

FLOOD HAZARD STATEMENT: F.E.M.A. FLOODWAY BOUNDARY AND FLOODWAY MAP DATED AUGUST 16, 2012 (PANEL NO. 35001C0141H) INDICATES A FLOOD HAZARD ZONE AE WHICH IS AN AREA DETERMINED TO BE INSIDE THE 100-YEAR FLOODPLAIN WITH BASE FLOOD ELEVATIONS.

EXISTING DRAINAGE CONDITIONS:

THE DRAINAGE ANALYSIS FOR THIS SUBDIVISION IS IN ACCORDANCE WITH SECTION 22 OF THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL (DPM), ENTITLED "DRAINAGE, FLOOD CONTROL, AND EROSION CONTROL." THE DESIGN STORM USED FOR BOTH UNDEVELOPED AND DEVELOPED CONDITIONS IS THE 100-YEAR, 6-HOUR STORM EVENT FOR RUNOFF. THE SITE IS LOCATED IN ZONE 3 SO THE 100-YEAR, 6-HOUR STORM EVENT IS 2.60 INCHES.

LOTS 17 & 18 ARE LOCATED IN NORTH ALBUQUERQUE ACRES, AT THE NORTHWEST CORNER OF THE VENTURA STREET/SIGNAL AVENUE INTERSECTION. THE LOTS DRAIN FROM SOUTHEAST TO NORTHWEST TO THE LA CUEVA ARROYO. THE PEAK RUNOFF FROM THE LOTS UNDER EXISTING CONDITIONS IS 2.25 CFS DURING A 100-YEAR, 6-HOUR STORM. THE MAJORITY IS CURRENTLY ENCLOSED BY A FLOOD HAZARD ZONE AE.

DEVELOPED DRAINAGE CONDITIONS:

THIS PROJECT INVOLVES THE CONSTRUCTION OF A RESIDENTIAL SUBDIVISION WITH 6 LOTS. THE SUBDIVISION WILL DRAIN FROM EAST TO WEST IN THE PRIVATE STREET TO A 5-FOOT WIDE CONCRETE CHANNEL ALONG THE WEST PROPERTY BOUNDARY. THIS CONCRETE CHANNEL WILL DRAIN SOUTH TO A 24-INCH WIDE SIDEWALK CULVERT TO SIGNAL AVENUE. THE TOTAL DISCHARGE FROM THE SUBDIVISION DURING A 100-YEAR, 6-HOUR STORM IS 5.19 CFS. THE FIRST FLUSH VOLUME FROM THE SUBDIVISION FOR A 0.44-INCH RAIN IS 960 CUBIC FEET. ON EACH LOT THE AREA BEHIND THE BASK OF THE CURB WILL BE DEPRESSED TO ALLOW FOR THE RETENTION OF THE FIRST FLUSH VOLUME.

ALONG THE EASTERN AND NORTHERN BOUNDARY OF THE DEVELOPED AREA WILL BE A SCOUR WALL TO PROTECT THE SUBDIVISION FROM UPSTREAM FLOWS IN THE LA CUEVA ARROYO. THE SCOUR WALL WILL BE AS DEEP AS 8 FEET BELOW THE ARROYO BOTTOM TO PROTECT THE SUBDIVISION AGAINST SCOUR DURING STORM EVENTS. A HEC-RAS ANALYSIS IS CURRENTLY BEING PERFORMED TO DETERMINE THE IMPROVEMENTS WITHIN THE LA CUEVA ARROYO TO BE CONSTRUCTED TO BOTH PROTECT THE SUBDIVISION AND TO REMOVE THE FLOOD HAZARD ZONE THROUGH A LOMR TO FEMA. THIS ANALYSIS WILL BE SUBMITTED AS A SUPPLEMENT TO THIS GRADING AND DRAINAGE PLAN.

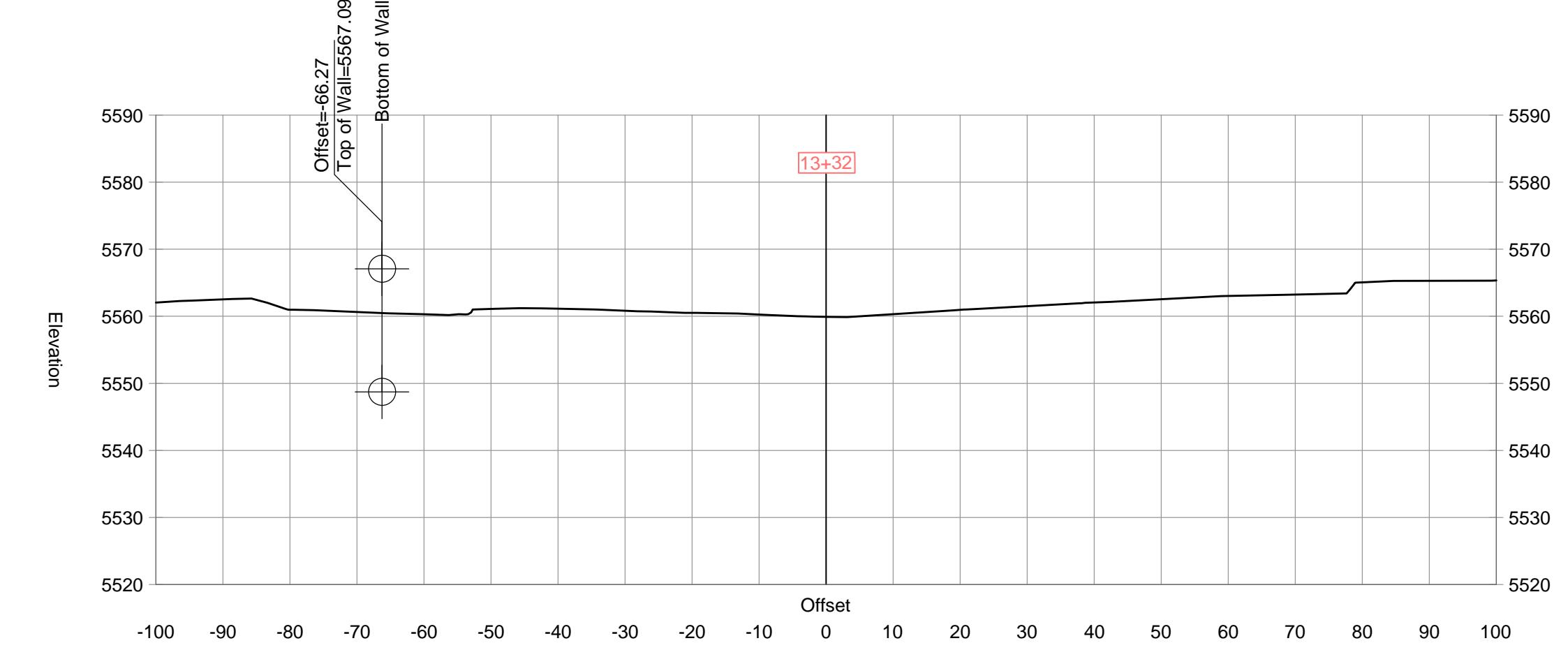
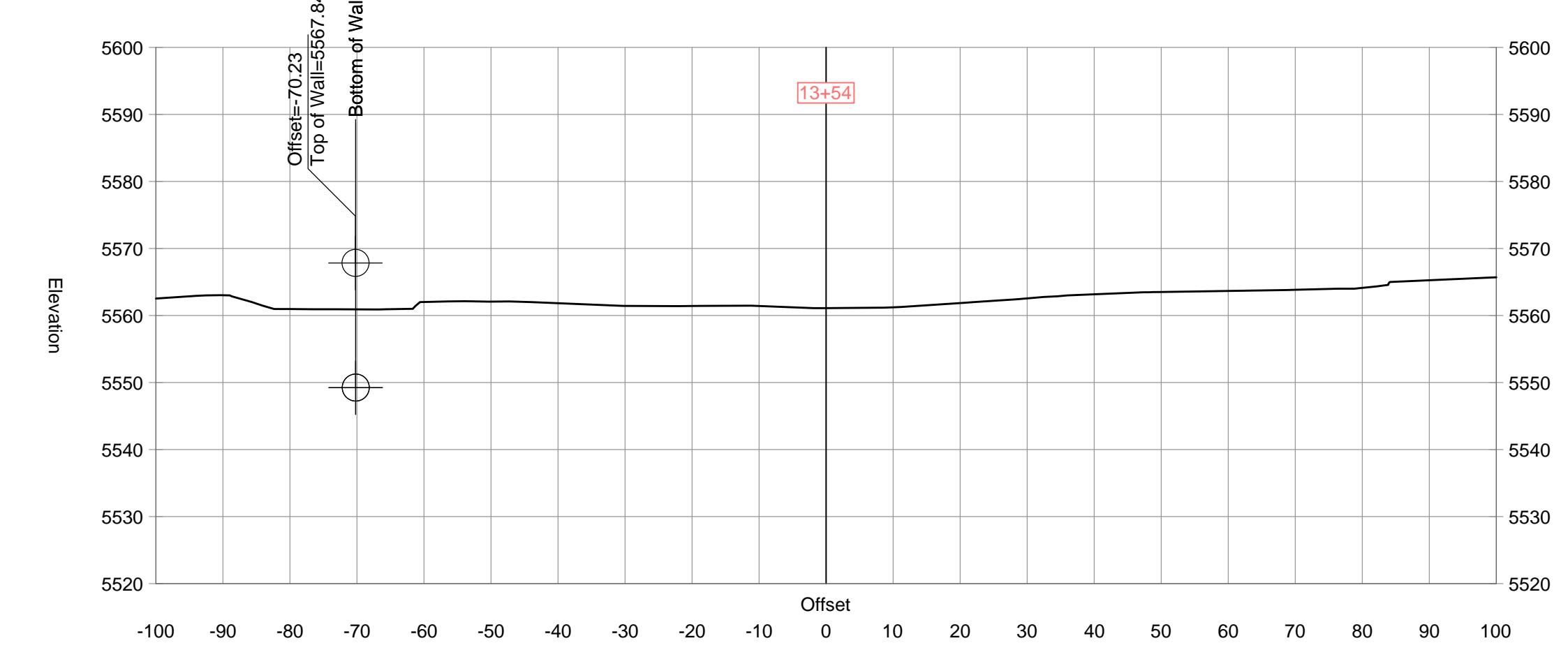
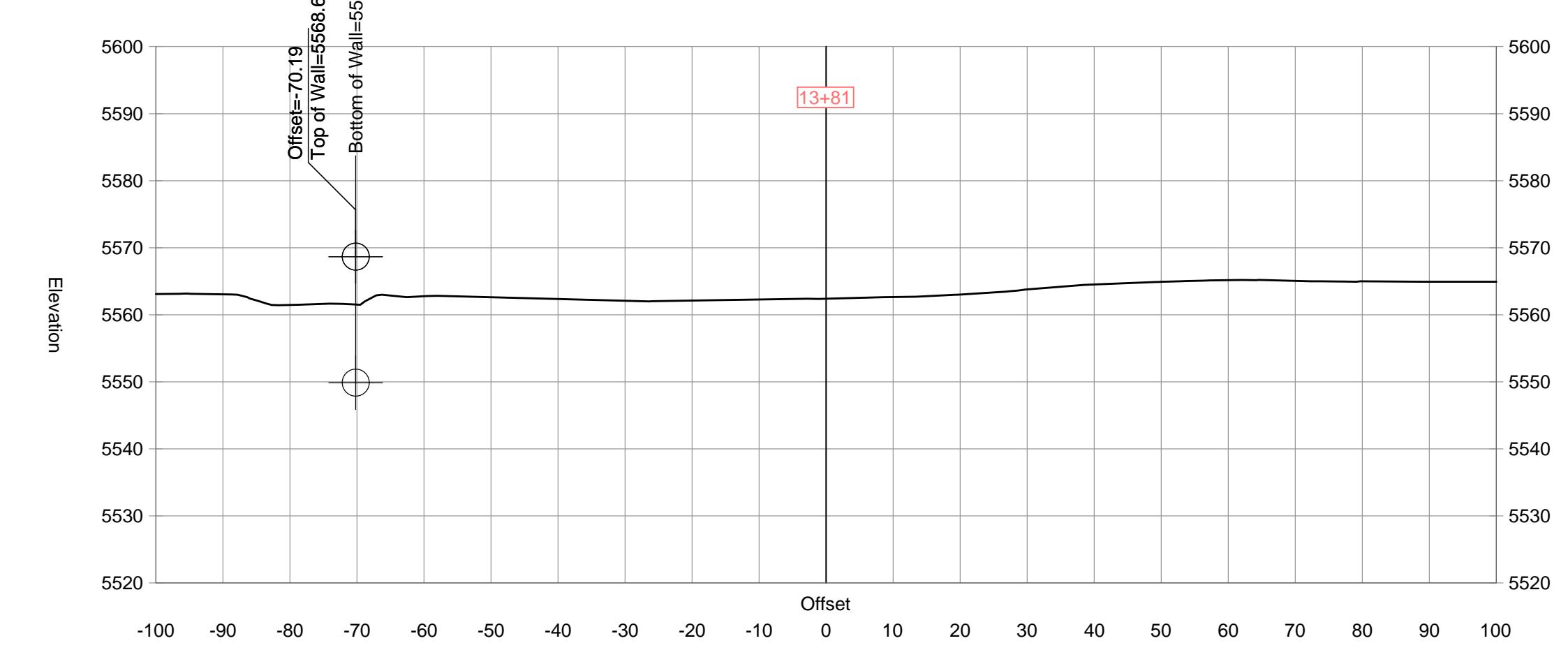
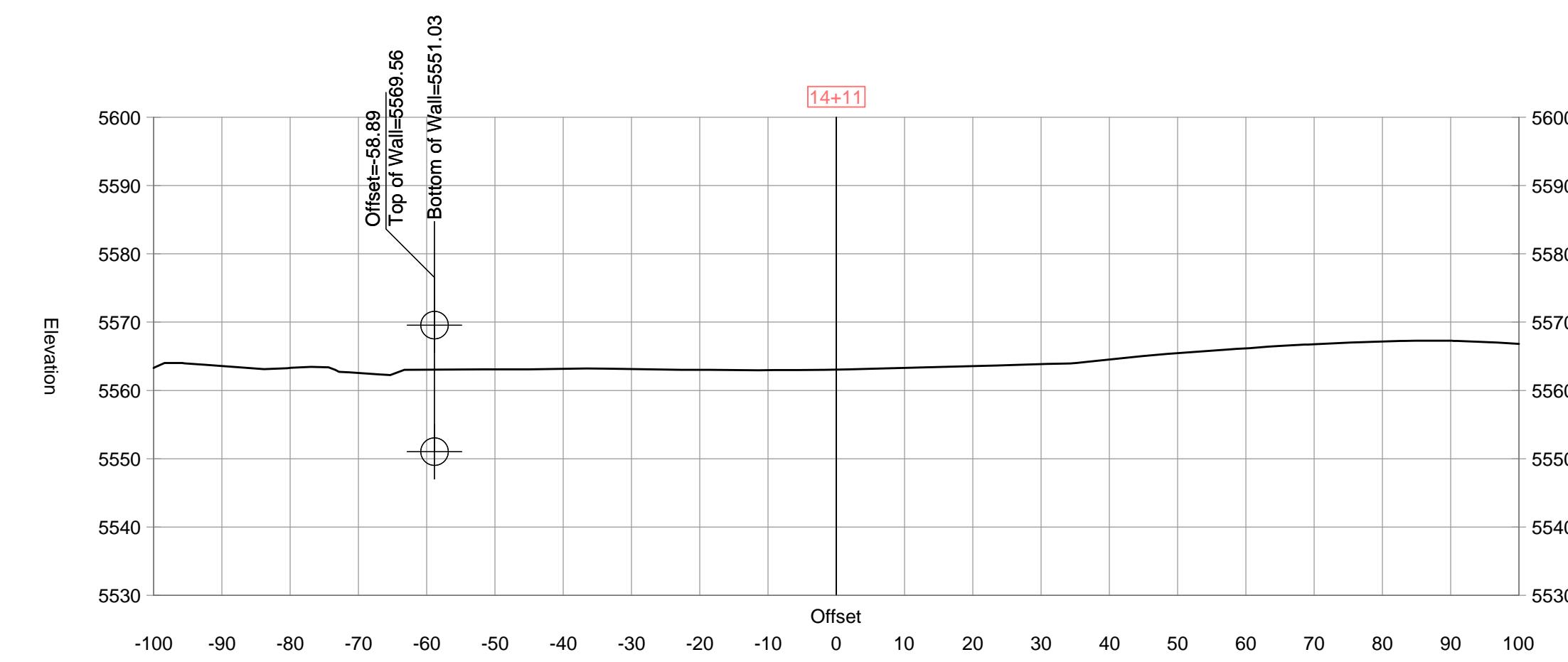
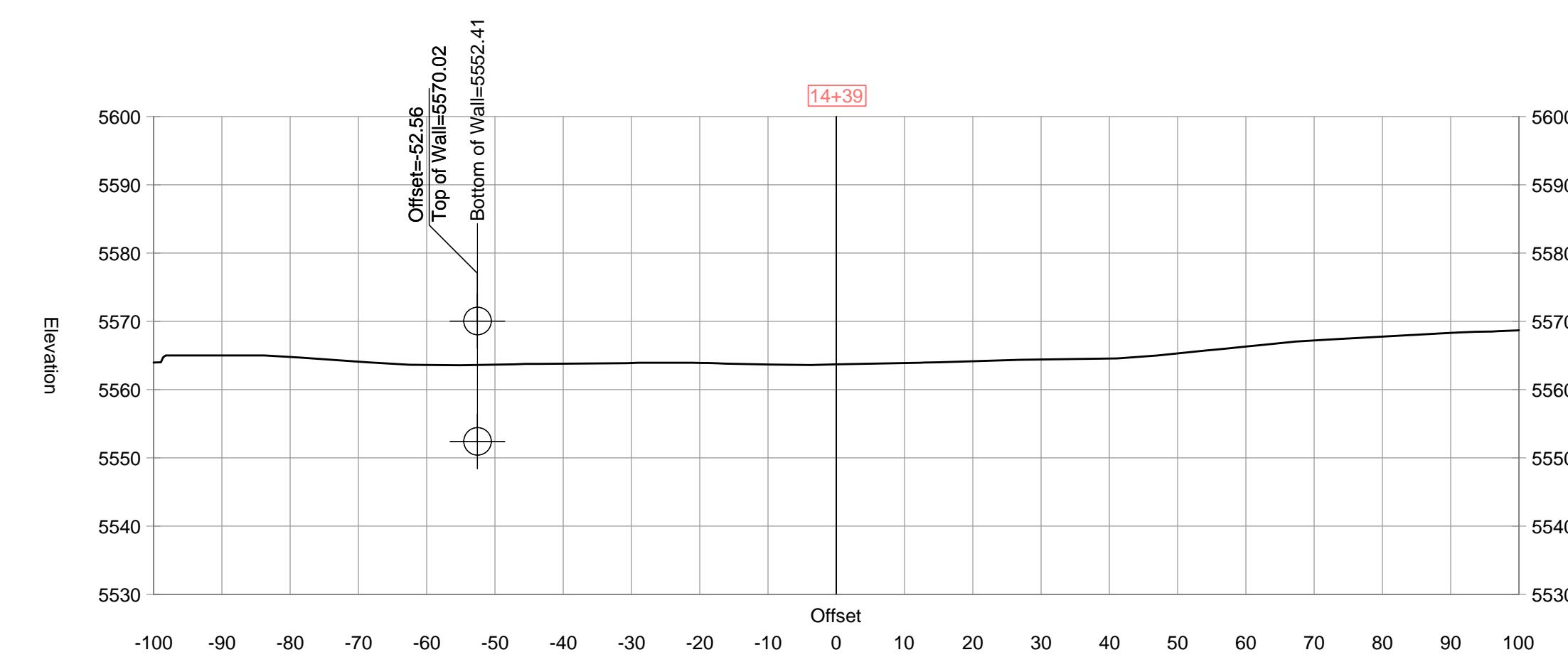
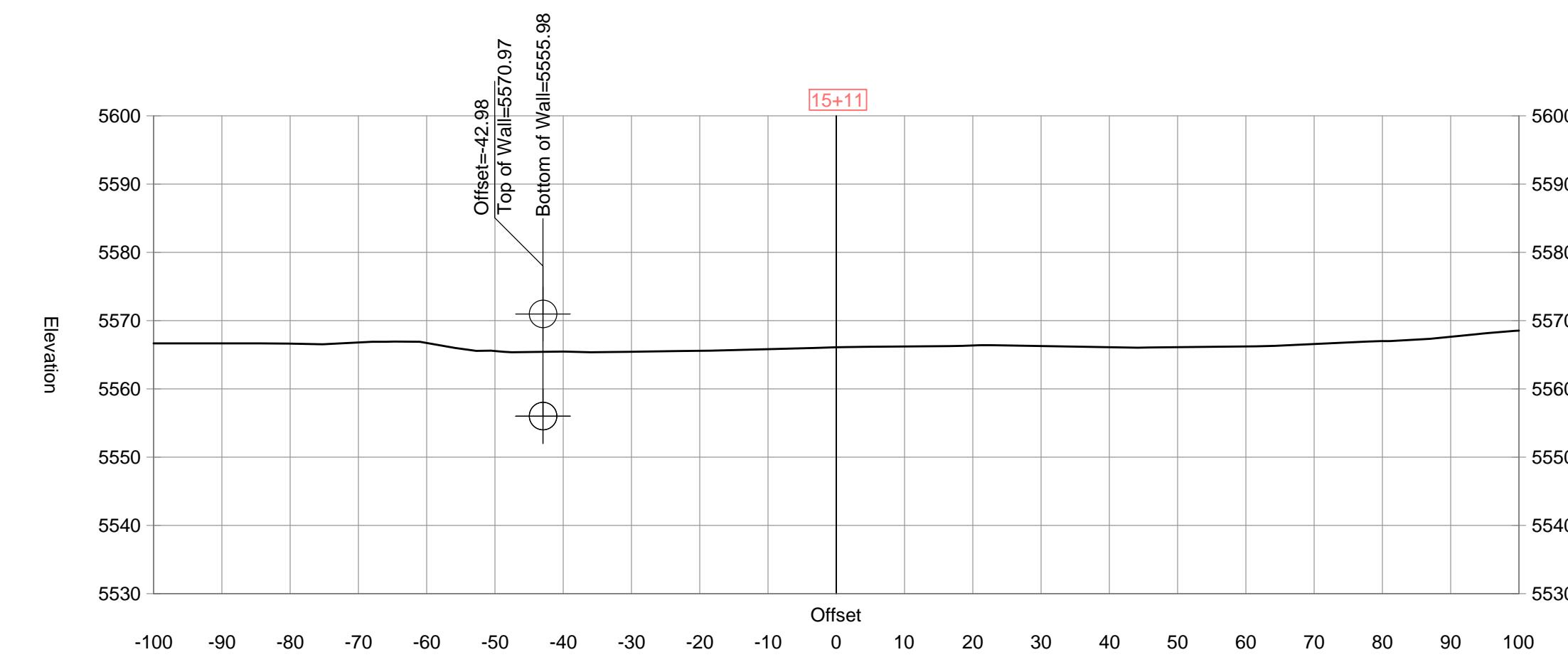
FIRST FLUSH CALCULATIONS:
 $(0.34 \text{ in} \times 12 \text{ in}/\text{ft}) \times (1.2 \text{ ac} \times 43,560 \text{ ft}^2/\text{ac} \times 0.647) = 960 \text{ cf}$

SIGNAL VILLAGE		GRADING AND DRAINAGE PLAN	
DATE	REVISION	DATE	REVISION

CITY/COUNTY REVIEW		SIGN-OFF	
DEPARTMENT	DATE	DEPARTMENT	DATE
WASTEWATER MGT. DIV.		WATER SERVICES	
WATER SERVICES		SUBDIVISION ENG.	
SUBDIVISION ENG.		STREETS	
STREETS		TRAFFIC	
TRAFFIC		FOR CITY/COUNTY USE ONLY	

CITY/COUNTY REVIEW										
BASIN #	AREA (acre)	LAND TREATMENT				100-YEAR PRECIPITATION				Q (cfs)
		A (%)	B (%)	C (%)	D (%)	E (in)	V (6-hr) (acre-ft)	V (6-hr) (cu-ft)	V (24-hr) (acre-ft)	
EXISTING CONDITIONS										
SITE	1.2020	100.00	0.00	0.00	0.00	0.66	0.07	2,880	0.07	2,880
TOTAL RUNOFF	1.20						0.07	2,880	0.07	2,880
PROPOSED CONDITIONS										
SITE	1.2020	0.00	17.60	17.70	64.70	1.92	0.19	8,365	0.22	9,777
TOTAL RUNOFF	1.20						0.19	8,365	0.22	9,777
EXCESS PRECIP.	0.66	0.92	1.29	2.36	E (in)					
PEAK DISCHARGE	1.87	2.6	3.45	5.02	Qn (cfs)					
WEIGHTED E (in) = (Ea)(Aa) + (Eb)(Bb) + (Ec)(Cc) + (Ed)(Dd)										
V6-HR (acre-ft) = (WEIGHTED E)(AREA)/12										
V1-DAY (acre-ft) = V6-HR + (Aa)(P1-DAY - P6-HR)/12										
Q (cfs) = (QPA)(Aa) + (Qb)(Bb) + (Qc)(Cc) + (Qd)(Dd)										
ZONE = 3										
P6-HR (in.) = 2.60										
P1-DAY (in.) = 3.10										
P10-DAY (in.) = 4.90										

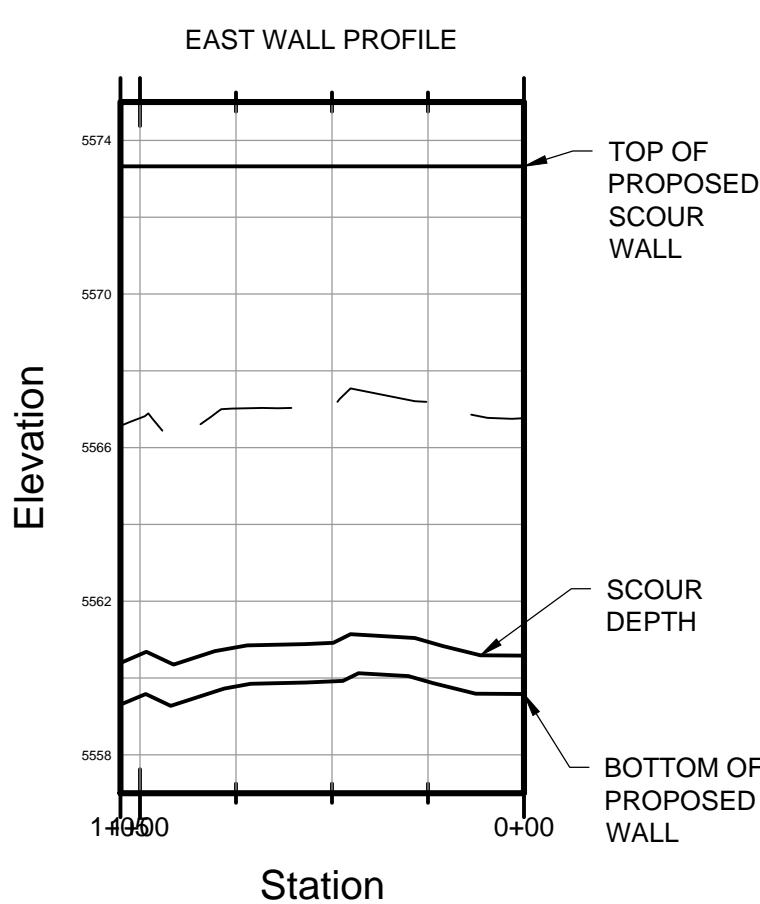
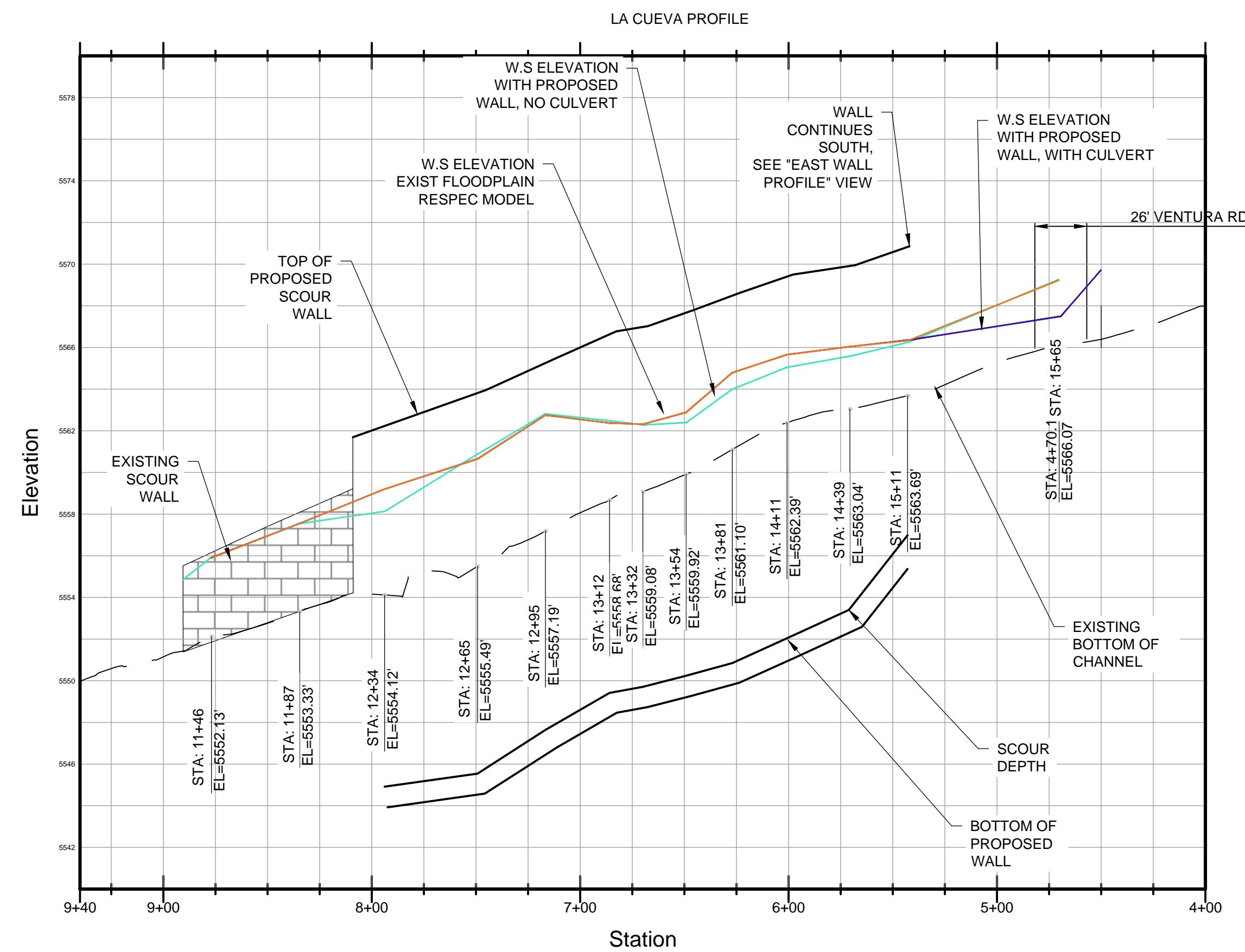
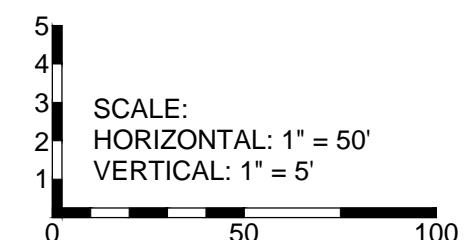
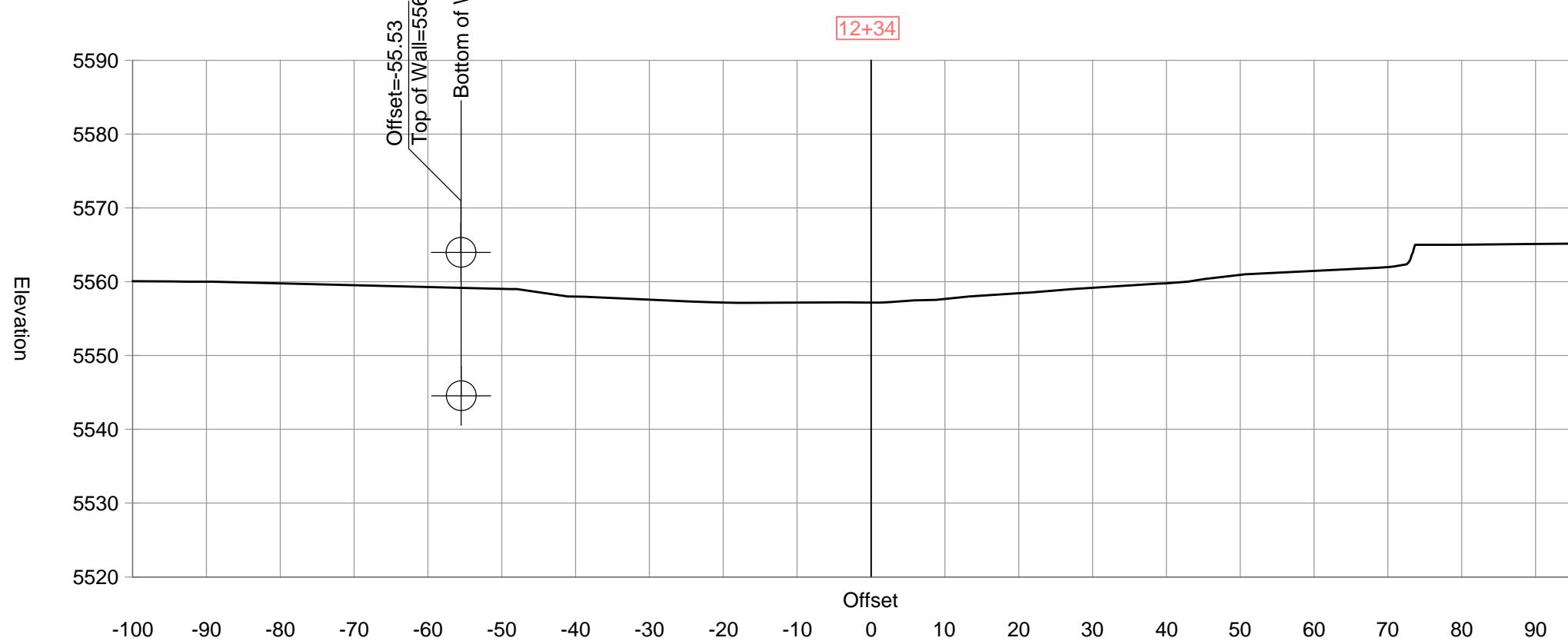
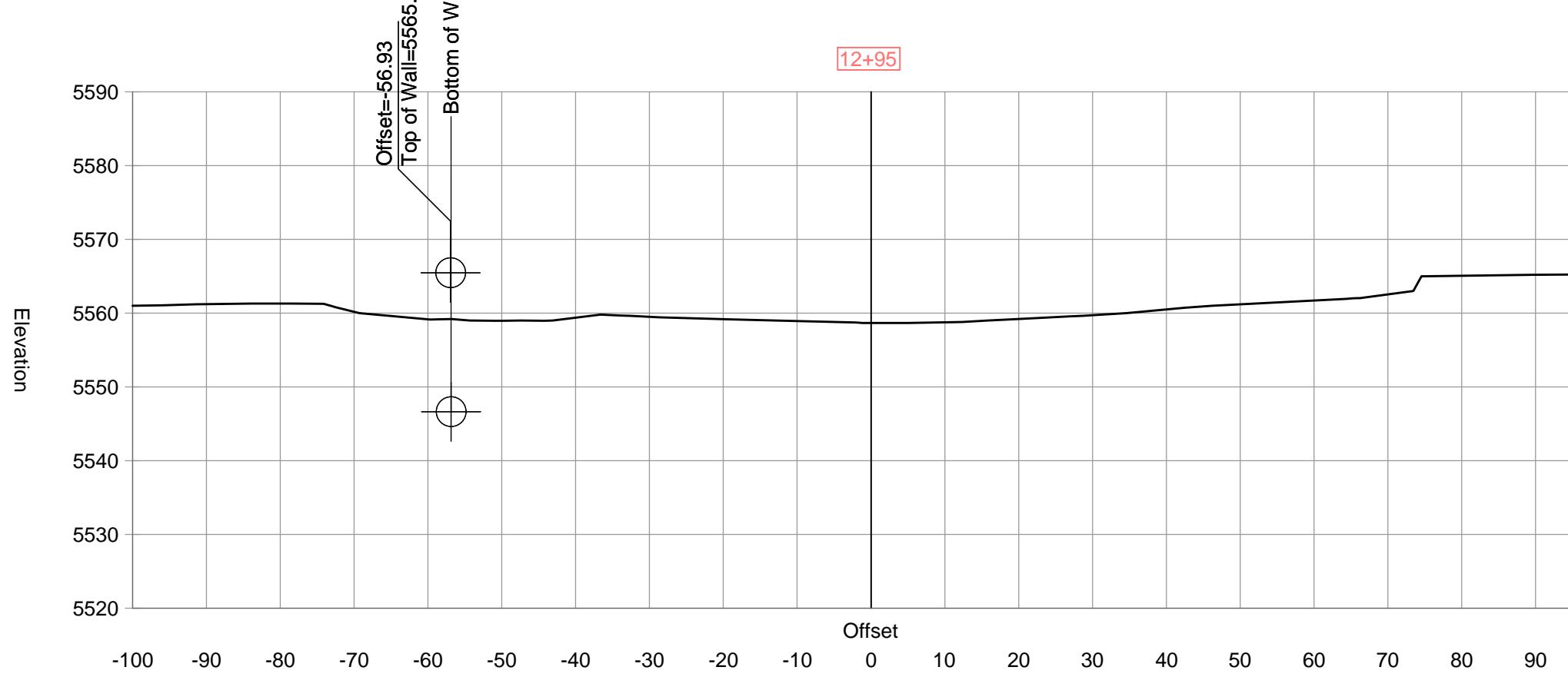
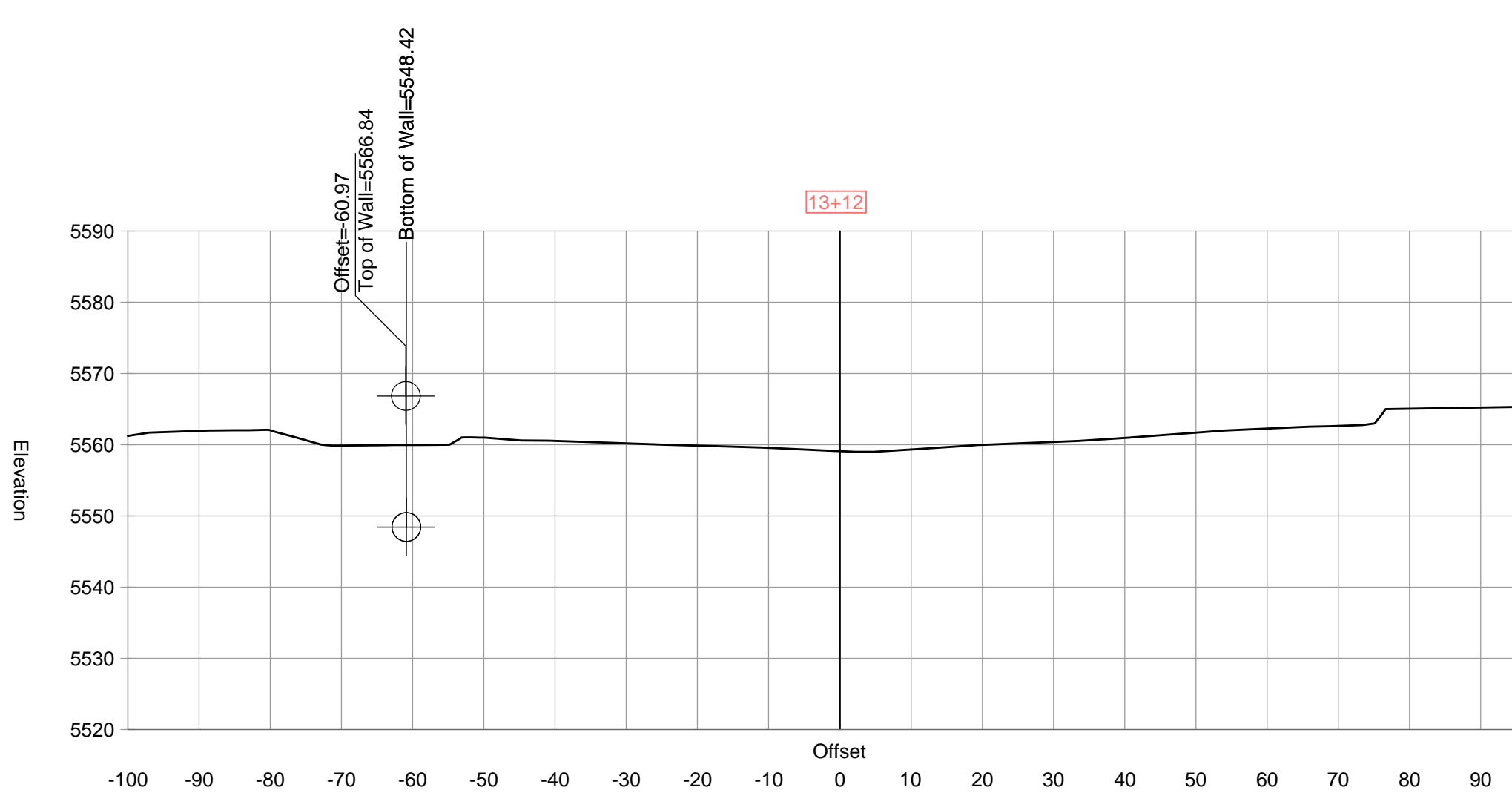
Scour Wall Cross Sections



THOMPSON ENGINEERING CONSULTANTS, INC.	
1030 S. 45TH ST., APT. 103 OMAHA, NE 68102 PHONE: (402) 271-1000 FAX: (402) 645-6246	
E-mail: info@thompsonengineering.com	
PROJECT: LA CUEVA	DRAWN BY: DEM
DATE: 09/10/2018	CHECKED BY:
HORZ. SCALE:	APPROVED BY:
VERT. SCALE:	FILE:
 9677 9-10-18 PROFESSIONAL ENGINEER	
CROSS SECTIONS 15+11 to 13+32	
SHEET No. 3 OF 5	

CITY/COUNTY REVIEW	
DEPARTMENT	SIGN-OFF
WASTEWATER MGMT. DIV.	DATE
WATER SERVICES	
SUBDIVISION ENG.	
STREETS	
TRAFFIC	
FOR CITY/COUNTY USE ONLY	

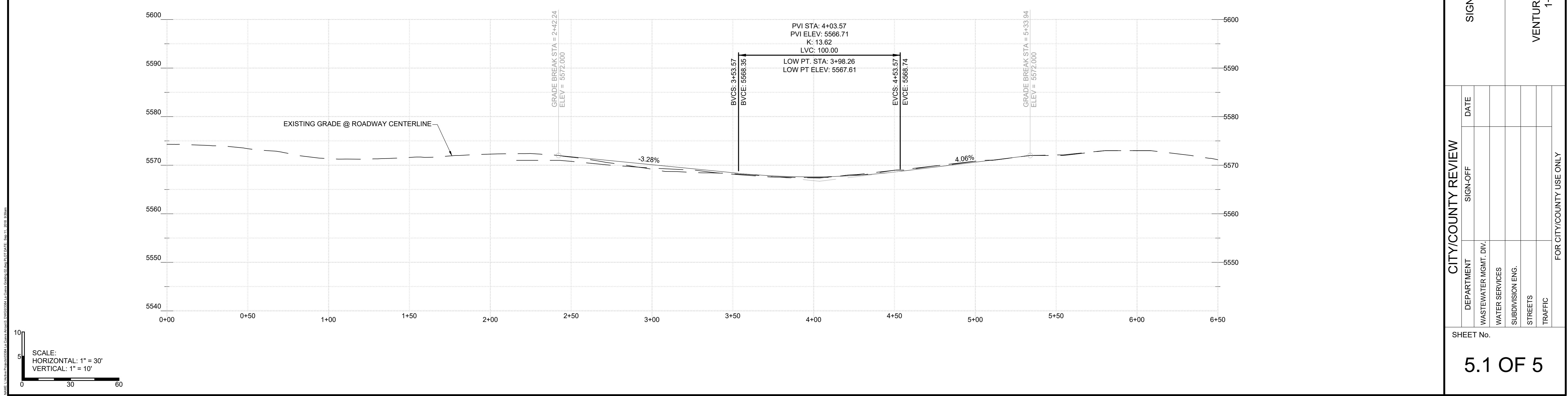
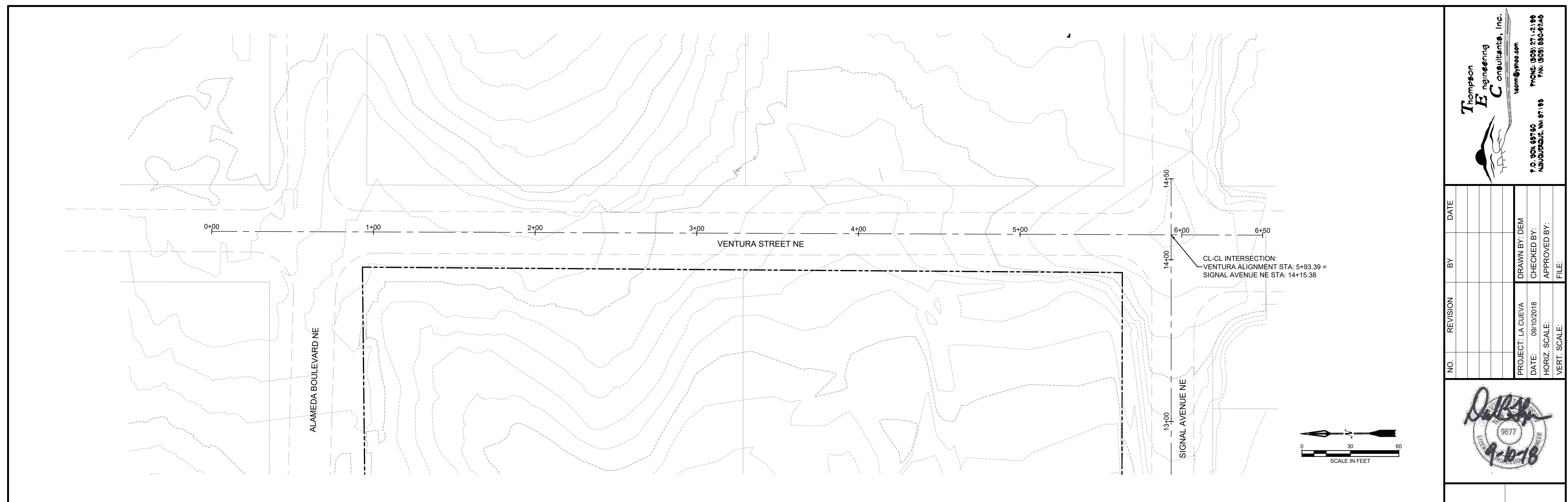
HORZ 0
VERT 0
SCALE IN FEET 20

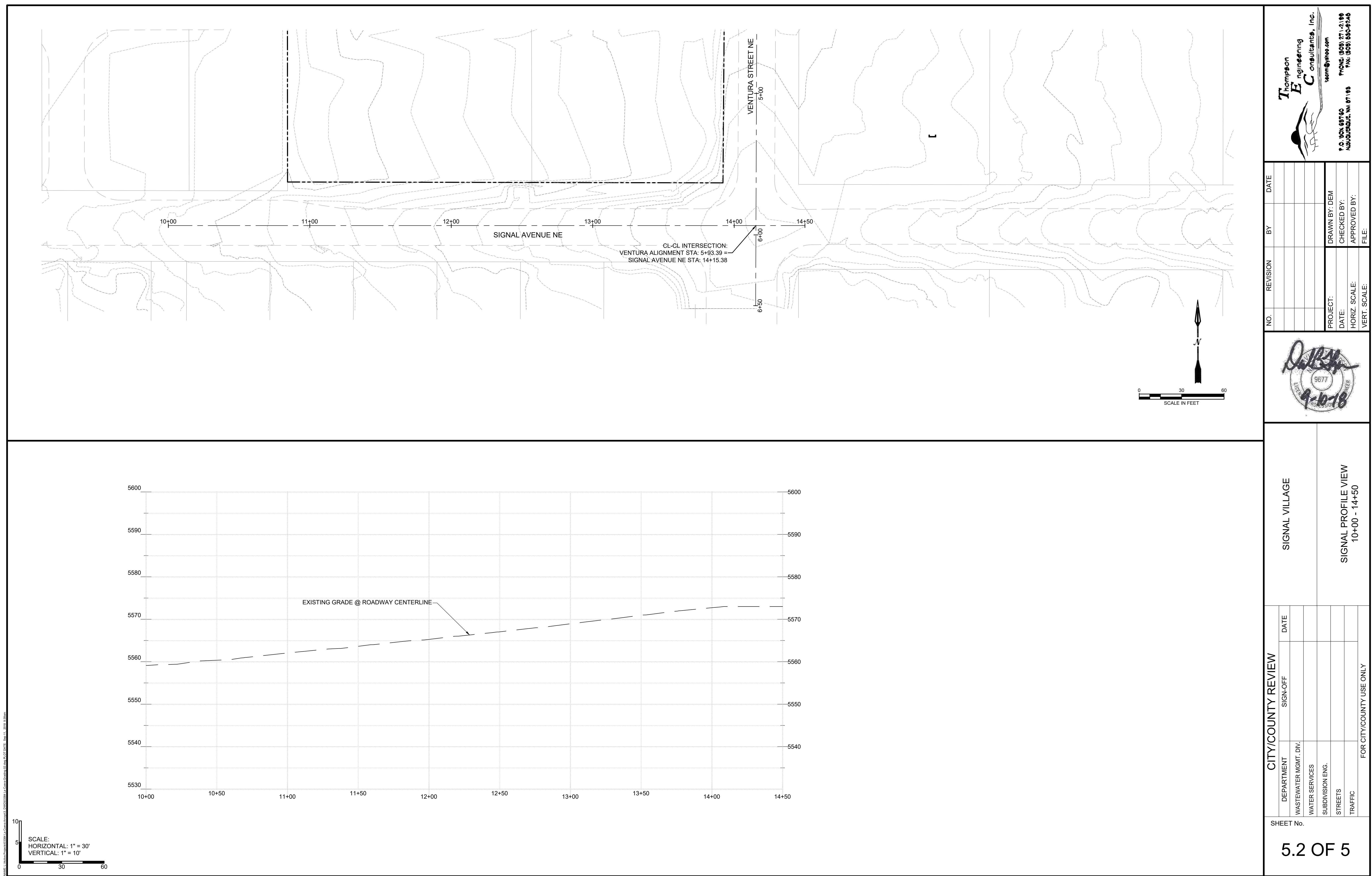


CITY/COUNTY REVIEW					
DEPARTMENT	SIGN-OFF	DATE	SIGNAL VILLAGE		
WASTEWATER MGMT. DIV.					
WATER SERVICES					
SUBDIVISION ENG.					
STREETS			CROSS SECTIONS 13+12 to 12+34		
TRAFFIC			PROFILE VIEW 3+00 - 9+00		
FOR CITY/COUNTY USE ONLY					

4 OF 5

4 OF 5





Appendix F

LA CUEVA SCOUR

LA CUEVA ARROYO SCOUR ANALYSIS INCLUDING CHANNEL EXCAVATION WITH VERTICAL CONCRETE SCOUR WALL WITH VENTURA CULVERT																									
COL 1	COL 2	COL 3	COL 4	COL 5	COL 6	COL 7	COL 8	COL 9	COL 10	COL 11	COL 12	COL 13	COL 14	COL 15	COL 16	COL 17	COL 18	COL 19	COL 20	COL 21	COL 22	COL 23	COL 24	COL 25	COL 26
SEC. NO.	HEC-RAS STA.		Q100 (CFS)	QD (CFS)	SC (FT/FT)	SAVG (FT)	WD (FT)	LAMDA (FT)	LV (FT)	DELTA MAX (FT)	EROS. SET. (FT)	VEL (FPS)	FROUDE (SF)	AREA (FT)	TOP WID (FT)	HYD DEP (FT)	UVW (FT)	UVW/WD (FT)	YS/Y (FT)	YS (FT)	HECRAS DEPTH (FT)	HECRAS FROUDE (FT)	SEQ DEP (FT)	FRBRD (FT)	WALL HT (FT)
1	1565.71	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	14.07	2.20	233.68	183.28	1.27	481.1	8.00	4.85	6.18	2.24	1.91	3.83	1.72	13.74	
2	1511.28	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	17.78	2.66	173.81	125.66	1.38	481.1	8.00	4.85	6.71	1.47	2.60	4.53	1.75	14.99	
3	1438.58	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	14.39	1.80	214.67	108.17	1.98	481.1	8.00	4.85	9.63	2.09	1.80	4.26	1.72	17.61	
4	1410.93	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	14.58	1.76	211.95	99.23	2.14	481.1	8.00	4.85	10.36	2.22	1.76	4.44	1.73	18.53	
5	1380.76	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	14.42	1.75	214.23	101.38	2.11	481.1	8.00	4.85	10.25	2.65	1.75	4.82	1.75	18.81	
6	1354.24	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	14.68	1.83	210.44	105.42	2.00	481.1	8.00	4.85	9.68	2.89	1.83	5.16	1.77	18.60	
7	1332.33	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	15.61	1.98	198.00	102.53	1.93	481.1	8.00	4.85	9.37	2.64	1.98	5.24	1.78	18.38	
8	1311.52	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	15.75	2.01	196.22	102.81	1.91	481.1	8.00	4.85	9.26	2.73	2.01	5.38	1.79	18.42	
9	1295.47	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	15.63	1.96	197.64	100.38	1.97	481.1	8.00	4.85	9.55	2.89	1.96	5.48	1.79	18.82	
10	1264.68	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	16.09	1.98	193.52	94.32	2.05	481.1	8.00	4.85	9.95	2.93	1.98	5.69	1.80	19.44	
11	1233.92	3090	618.0	0.0157	0.0348	60.1	841.9	421.0	210.5	240.6	17.80	2.28	173.63	91.49	1.90	481.1	8.00	4.85	9.20	2.13	2.28	5.48	1.80	18.48	
COL 1 CHANNEL CROSS-SECTION NUMBER																									
COL 2 HEC-RAS CHANNEL STATION																									
COL 3 NOT USED																									
COL 4 100-YEAR PEAK DISCHARGE, Q (CFS)																									
COL 5 DOMINANT DISCHARGE, QD = 0.2 Q100 (CFS)																									
COL 6 CRITICAL SLOPE, SC = 0.037 QD (^0.133) (FT/FT)																									
COL 7 AVERAGE SLOPE OF THE CHANNEL (FT/FT)																									
COL 8 DOMINANT CHANNEL WIDTH, WD = 4.6QD(^0.4) (FT)																									
COL 9 MEANDER WAVELENGTH, LAMDA = (014)WD (FT) FOR QD > 2000 CFS																									
COL 10 DOWN-VALLEY LENGTH, LV = LAMDA/2 (FT)																									
COL 11 MAXIMUM LATERAL EROSION, DELTA MAX = (16.1)QD(^0.4) (FT) FOR QD > 2000 CFS																									
COL 12 EROSION SETBACK = WD/2 + DELTA MAX (FT)																									
COL 13 CHANNEL VELOCITY FROM HECRAS (FT/SEC)																									
COL 14 FROUDE NUMBER CALCULATED USING HYDRAULIC DEPTH																									
COL 15 CROSS-SECTIONAL AREA FROM HECRAS (SQ. FT)																									
COL 16 TOP WIDTH FROM HECRAS (FT)																									
COL 17 HYDRAULIC DEPTH, HD = A/TW (FT)																									
COL 18 UNCONSTRAINED VALLEY WIDTH, UVW = 2 X EROSION SETBACK (FT)																									
COL 19 RATIO OF UNCONSTRAINED VALLEY WIDTH TO DOMINANT CHANNEL WIDTH, UVW/WD																									
COL 20 RATIO OF SCOUR DEPTH TO FLOW DEPTH, YS/Y (FIGURE 3.31 AMAFCA SEDIMENT AND EROSION DESIGN GUIDE)																									
COL 21 ESTIMATED SCOUR (FT)																									
COL 22 DEPTH OF FLOW FROM HECRAS (FT)																									
COL 23 FROUDE NUMBER FROM HECRAS																									
COL 24 SEQUENT DEPTH (FT)																									
COL 25 FREEBOARD (FT)																									
COL 26 SCOUR WALL HEIGHT = SCOUR DEPTH + SEQUENT DEPTH +FREEBOARD+ TWO FOOT FACTOR OF SAFETY (FT)																									