

BOARD OF COUNTY COMMISSIONERS
PATRICK J. BACA, CHAIRMAN
DISTRICT 1

JACQUELYN SCHAEFER, VICE CHAIR
DISTRICT 5

ALBERT "AL" VALDEZ, MEMBER
DISTRICT 2

EUGENE M. GILBERT, MEMBER
DISTRICT 3

PATRICIA "PAT" CASSIDY, MEMBER
DISTRICT 4

JUAN R. VIGIL, COUNTY MANAGER

County of Bernalillo

State of New Mexico

ONE CIVIC PLAZA, N.W.
ALBUQUERQUE, NEW MEXICO 87102
ADMINISTRATION (505) 768-4000
COMMISSION (505) 768-4217
FAX (505) 768-4329

February 23, 1993

RAY GALLAGHER, SHERIFF
PATRICK J. PADILLA, TREASURER
GLADYS M. DAVIS, CLERK
MARK J. CARRILLO, ASSESSOR
THOMAS J. MESCALL, PROBATE JUDGE

Mike Beyer, P.E. The B.E.A.R. Company 1311 Apex Court Rio Rancho, NM 87124

RE: DRAINAGE PLAN FOR LOT 24, BLOCK 15, TRACT 2, UNIT 1, N.A.A., (C-22/D33), ENGINEER'S STAMP DATED JANUARY 18, 1993

Dear Mr. Beyer:

Based on the information provided on the referenced submittal received February 16, 1993, the plan is acceptable for Building Permit release.

Please be advised that prior to final inspection by County Building Department, an Engineer's Certification must be submitted and approved by this office.

On future submittals, use the zone atlas for the vicinity map, also you need to check the appropriate review request on your information sheet.

If you should have any questions, please do not hesitate to contact me at 768-2650.

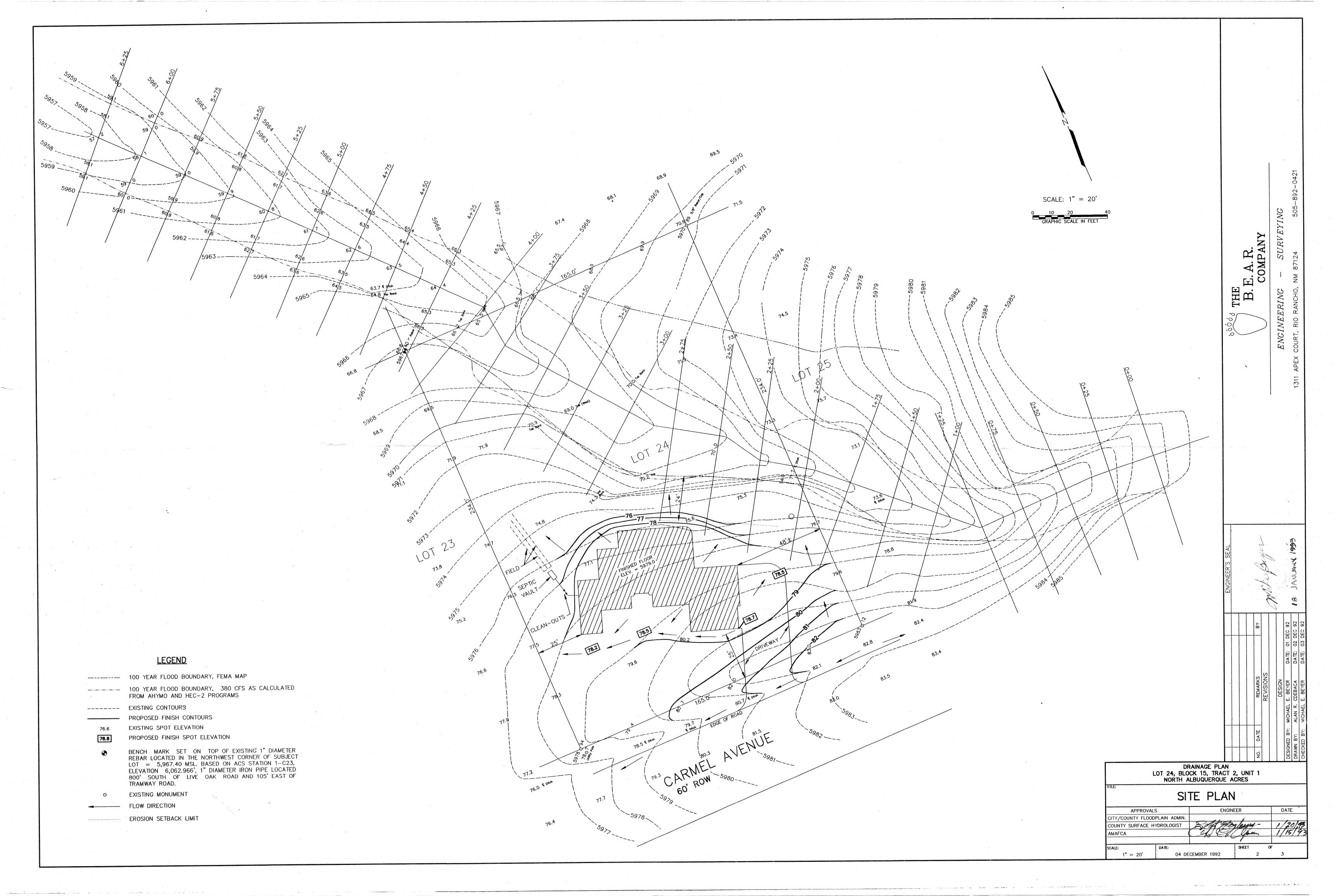
Cordially,

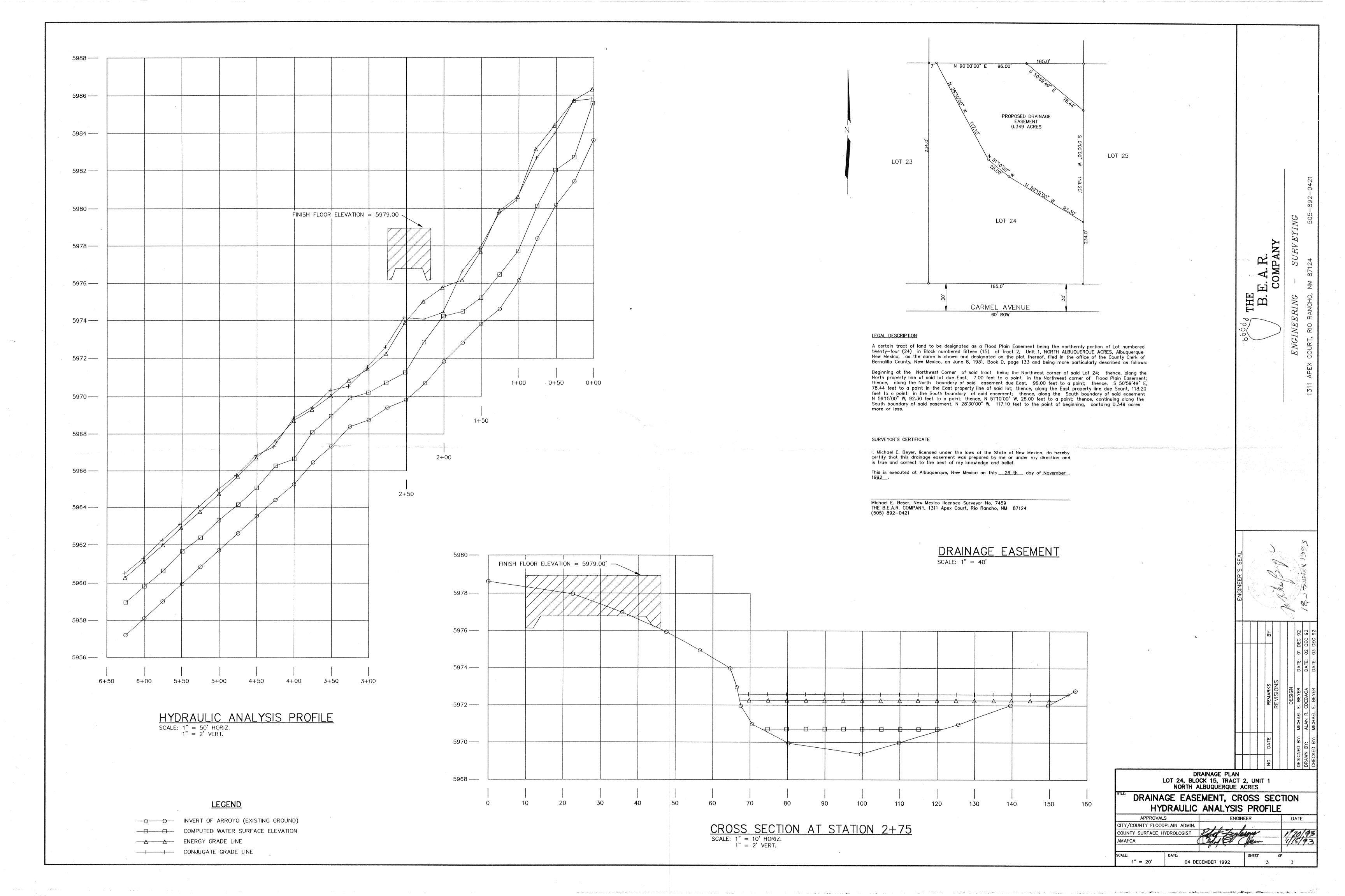
Gilbert Aldaz, P.E & PS.

City/County Floodplain Administrator

xc: Clifford E. Anderson, AMAFCA
Bob Foglesong, County PWD
Tom Burlison, County Building Dept.
John Mechembier, Mechembier Construction

wp+3802







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ONE CIVIC PLAZA, N.W.
ALBUQUERQUE, NEW MEXICO 87102
ADMINISTRATION (505) 768-4000
COMMISSION (505) 768-4217
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January 13, 1993

RAY GALLAGHER, SHERIFF
PATRICK J. PADILLA, TREASURER
GLADYS M. DAVIS, CLERK
MARK J. CARRILLO, ASSESSOR
THOMAS J. MESCALL, PROBATE JUDGE

Mike Beyer, P.E. The B.E.A.R. Company 1311 Apex Court Rio Rancho, NM 87124

RE: DRAINAGE PLAN FOR LOT 24, BLOCK 15, TRACT 2, UNIT 1, N.A.A., RECEIVED DECEMBER, 1992 FOR BUILDING PERMIT APPROVAL, (C-22/D33)

Dear Mr. Beyer:

Prior to Building Permit release please address the attached letter dated December 21, 1992 by AMAFCA regarding the floodplain easement. In addition please secure AMAFCA's and County Public Works signature on the plan.

In the future, for your vicinity maps, use the zone atlas so we can locate the site.

The above comments include AMAFGA and County Public Works review. If you should have any questions, please do not hesitate to contact me at 768-2650.

Cordially,

Gilbert Aldaz, P.E. & P.S.

City/County Floodplain Administrator

xc: Clifford E. Anderson, AMAFCA Bob Foglesong, County PWD

wp+3802



MICHAEL E. BEYER

PROFESSIONAL ENGINEER License No. 7902

PROFESSIONAL SURVEYOR License No. 7459

ENGINEERING ● *SURVEYING*

1311 Apex Court • Rio, Rancho, New Mexico 87124 • 505-892-0421

DRAINAGE PLAN CALCULATIONS FOR

LOT 24, BLOCK 15, TRACT 2, UNIT 1 WITHIN NORTH ALBUQUERQUE ACRES

BERNALILLO COUNTY, NEW MEXICO



MICHAEL E. BEYER

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DRAINAGE DESIGN CALCULATIONS FOR

LOT 24, BLOCK 15, TRACT 2, UNIT 1 NORTH ALBUQUERQUE ACRES

ONSITE CONDITIONS EXISTING: All property is East of Eubank and North of Interstate 40 yielding Zone 4. Subject lot contains 0.89 acres and is a type "A" land treatment. Drainage primarily sheet flow in developmental area with a designated 100 Year Flood Boundary area across Northern half of property.

Using Table 9 from Page 9 of Interim Criteria:

Peak Drainage = 2.26 cfs/acre x 0.89 acres = 2.01 cfs.

Using rational method:

Q = CIA = (0.37)(6.03)(0.89) = 2.00 cfs

ONSITE CONDITIONS - PROPOSED: Using 50% or 0.45 acres as type "D" from
Pate 5 of criteria.

After improvements:

Q = 2.26 (0.45 acres) + 5.74 (0.45 acres) = 3.6 cfs

NOTE: An increase of 1.6 cfs.

OFFSITE FLOWS ONTO SUBJECT PROPERTY: Designated 100 Year Flood Boundaries are indicated on drainage plan.

FLOODWAY ESTABLISHMENT: The AHYMO392 computer program (AMAFCA Hydrologic Model) was implemented for 100 year storm flow determination across subject property. Velocity and depth of flow was calculated by HEC-2, Water Surface Profiles Program. See Attachment No. 4.

COMPARISON OF DRAINAGE PLAN CALCULATIONS: A review of document entitled "North and South Domingo Baca Arroyos and Paseo Del Norte Corridor Drainage Management Plan" by Resource Technology, Inc., Dated December 1991, indicates a 329 cfs 100 Year peak discharge at analysis point 910.20, this point is downstream of subject lot on Lowell Street. The 376 cfs 100 Year peak discharge used in this analysis is very conservative. Difference is in the drainage areas.

EARTHWORK VOLUMES:

Cut = 112 cubic yards Fill = 64 cubic yards

<u>EROSION LIMITS</u>: City of Albuquerque checklist states that building setback requirements for 100 Year Flood Plain should be six feet per 100 cfs, or fraction thereof; at a flow of 376 cfs the setback would be as follows:

$$(4)$$
 $(6') = 24'$

Northeast corner of proposed residence meets this minimum setback requirement. See drainage plan.

<u>Tp CALCULATIONS</u>: Time of concentration, t_c , equation for subbasins with reach lengths between 4,000 and 12,000 feet.

$$t_c = [(12,000-L)/(120(K)S^{0.5})] + [(L-4,000)(K_N)(L_{CA}/L)^{0.33}/(4.305)(S^{0.165})]$$

where,

L = Subbasin length = 1,300 + 600 + 1,400 + 1,800 = 5,100 ft.

K = 3, from Table 12, Conveyance Factors Page 14, D.P.M., Section 22.2

S = Slope, from topo 4.7%

 $K_N = 0.025$, from Table 13

 $L_{CA} = 0.60(5,100) = 3,060$

This equation is from page 14 of D.P.M., Section 22.2

$$t_c = [(12,000 - 5,100)/(120(3)(4.7)^{0.5})] + [(5,100-4,000)(0.025)(3,060/5,100)^{0.33}/(4.305)(4.7)^{0.165})] = 4.661$$

 $t_p = 2/3(t_c) = 0.66(4.661) = 3.076 \text{ min} = 0.051 \text{ hours}$

FINISH FLOOR ELEVATION: A review of the D.P.M. for freeboard analysis (Section 22.3, Pages 61, 61.1 and 91 Trapezoidal Channel and Associated Types and Berms and Levees) requires a minimum of 3.0 feet of freeboard. The finish floor elevation of 5,979.00 feet meets this requirement as shown on the profile of hydraulic analysis of the drainage plan.

NOTES:

- (1) The insignificant increase of 1.6 cfs will not cause additional erosion downstream.
- (2) Additional grading will be required for the driveway as shown on the drainage plan. Slight swale will be used for crossing Carmel Avenue onto subject lot. Driveway ditchline crossing is near apex of ditch. See drainage plan.
- (3) No development or improvements will infringe on any designated 100 year flood boundaries.
- (4) This is to certify that I, Michael E. Beyer, a duly qualified professional engineer and surveyor, licensed under the laws of the State of New Mexico, have on April 18, 1992 visited the subject site and that it appears no grading, filling, or excavation has taken place.

5 HOVEMBER 1992

AHYMO FROGRAM (AHYMO392) - AMAFCA VERSION OF HYMO - MARCH. 1992

RUN DATE (MON/DAY/YR) = 08/09/1992

START TIME (HR:MIN:SEC) = 14:09:26 USER NO.= BEYER_NM.IO1

INPUT FILE = GOAT

START RAINFALL TIME=0 PUNCH CODE=0
TYPE=1 QUARTER=0
ONE=2.23 SIX=2.90
DAY=3.65 DT=0.033333HR

```
COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK
DT = .033333 \text{ HOURS} END TIME =
                                        5.999940 HOURS
                                        .0284 .0345
                         .0167
                                .0225
   ..0000
          .0055 .0110
                                .0668 .0738
                                                .0809
   .0406
          ..0470
                 .0534 .0600
          .0958 .1035 .1115 .1198 .1283
                                               .1370
   .0882
         .1555 .1653 .1755 .1860
.2335 .2469 .2531 .2596
.3679 .4428 .5450 .6793
                                        .1971
                                               .2036
   .1461
                                       .2667
                                               .2819
   .2207
                                       .8505
                                              1.0637
  .3158
 1.3239 1.3646 1.6653 1.7504 1.8260 1.8948 1.9582
 2.0171 2.0720 2.1236 2.1721 2.2178 2.2611
                                               2.3020
 2.3407 2.3775 2.4124 2.4455 2.4769 2.4850
                                              2.4928
 2.5002 2.5074 2.5143 2.5210 2.5275 2.5339
                                               2.5400
                               2.5685 2.5738
                                               2.5790
 2.5460 2.5518 2.5575 2.5631
                                               2.6127
 2.5841 2.5891 2.5940 2.5989
                               2.6036
                                       2.6083
 2.6174 2.6218 2.6262 2.6305 2.6348 2.6390 2.6431
 2.6472 2.6512 2.6552 2.6591 2.6630 2.6668
                                               2.6706
 2.6744 2.6781 2.6817 2.6854 2.6890 2.6925
                                              2.6960
                               2.7131 2.7164 2.7197
 2.6995 2.7029 2.7064 2.7097
 2.7230 2.7262 2.7294 2.7326 2.7357 2.7388 2.7419
                                               2.7630
 2.7450 2.7481 2.7511 2.7541 2.7571 2.7600
 2.7659 2.7688 2.7716 2.7745 2.7773 2.7802 2.7829
 2.7357 2.7885 2.7912 2.7939 2.7967 2.7993
                                               2.8020
                                               2.8203
 2.8047 2.8073 2.8099 2.8125 2.8151 2.8177
 2.8228 2.3254 2.8279 2.8304 2.8329 2.8354 2.8378
                        2.8476 2.8500 2.8524 2.8547
                2.8451
 2.8403 2.8427
 2.8571 2.8595 2.8618 2.8641 2.8664 2.8689 2.8711
                                2,8824 2,8846 2,8868
 2.8733 2.8756 2.8779 2.3801
 2.8891 2.8913 2.8935 2.8957 2.8978 2.9000
```

COMPUTE NM HYD

ID=2 HYD NO=401.0 AREA=0.107 SQ MI A=11 AC B= 7 AC C=41 AC D=9 AC

TP=0.051 HRS MASS RAIN=-1

*****WARNING**** SUM OF TREATMENT TYPES DOES NOT EQUAL 100 PERCENT OR TOTAL

K = .027893HR TP = .051000HR K/TP RATIO = .546930 SHAPE CONSTAUNIT PEAK = 145.72 ·CFS UNIT VOLUME = 1.015 B = 524.76 F AREA = .014162 SQ MI IA = .10000 INCHES INF = .04000 INCHES FRUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .03

K = .045581HR TP = .051000HR K/TP RATIO = .893750 SHAPE CONSTA

UNIT PEAK = 642.54 CFS UNIT VOLUME = 1.005 B = 352.98 F AREA = .092838 SQ MI IA = .42373 INCHES INF = 1.03644 INCHES F FUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .03644 INCHES F

RIHT HYD

ID=2 CODE=1

HYDROGRAFH FROM AREA 401.00

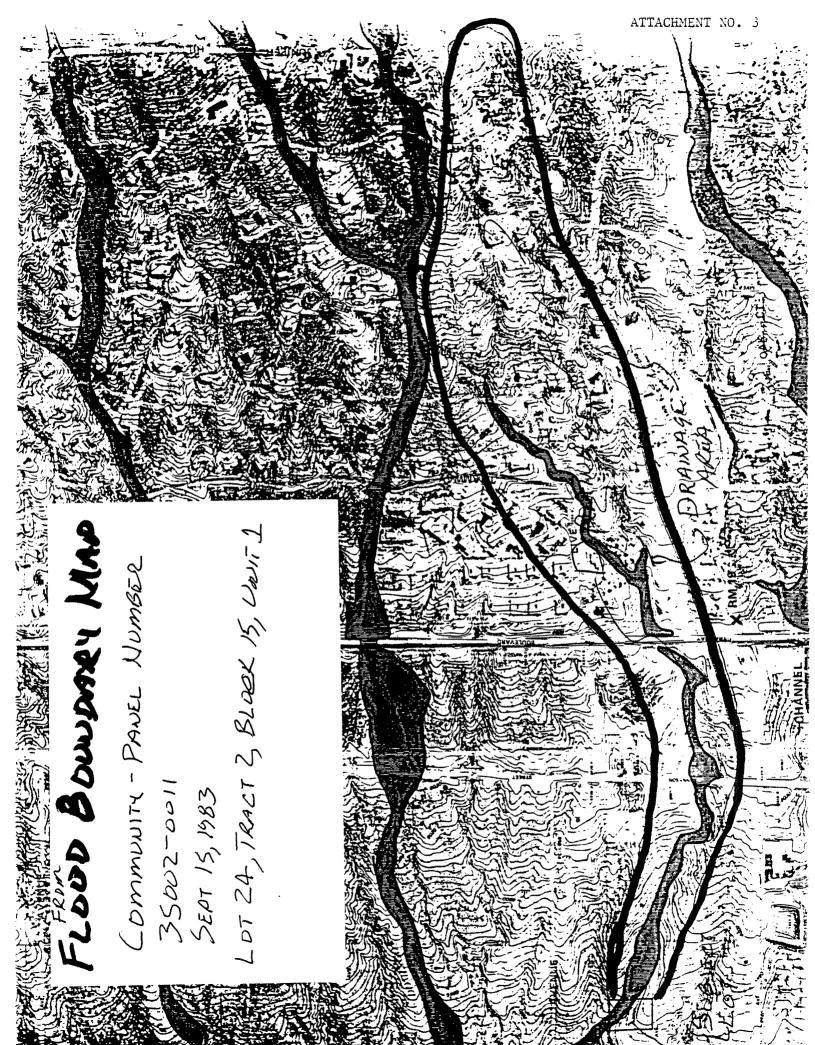
RUNOFF VOLUME = 1.43795 INCHES = 8.2058 ACRE-FEET PEAK DISCHARGE RATE = 376.10 CF8 AT 1.433 HOURS BASIN AREA = .1070

INISH

CARMIL LOT 24

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 14:09:28



	CARMBL	LOT 24								
T2 T3	C	DWEL TOT	24 RBV	ITCRN 10	/31/02					
J1		() ועם הפשעו			0	0	0	380	5986	0
•	v	V	v	'	•	v	v	300	3,00	•
J2	-1	1	0	0	0	0	-1		0	0
J3	1	2	3	4	43	8	14	26	25	68
J3	42	50	51	150						
J5	-1	-10								
	0.035		0.030	0	0					
X 1	0.00	11	32		25		26		0	0
GR.	5988.0	10	5987.0	20	5986.0		5985.0	32		37
GR	5983.6		5984.0	60	5985.0	65	5986.0	80	5987.0	87
GR	5988.0	93								
X1	0.25	12	31	72	24		28		0	0
GR	5987.0	10	5986.0	20	5985.0		5984.0		5983.0	33
GR	5982.0	37	5981.4	50	5982.0	62	5983.0	66	5984.0	72
GR	5985.0	80	5986.0	90						
X1	0.50	11	33	64	25	25	25	0	0	0
GR	5985.0	18	5984.0	23	5983.0	28	5982.0		5981.0	41
GR	5980.2	50	5981.0	59	5982.0	64	5983.0	70	5984.0	84
GR	5985.0	96								
X1	0.75	12	33		25		25	-	0	0
GR	5984.0	11	5983.0	16	5982.0	24	5981.0	28	5980.0	33
GR	5979.0	44	5978.4	50	5979.0	57	5980.0	62	5981.0	66
GR	5982.0	75	5983.0	96						
X1	1.00	10	40	80	31	25	7	0	0	0
GR	5982.0	20	5981.0	25	5980.0	30	5979.0	37	5978.0	40
GR	5977.0	45	5976.1	50	5977.0	70	5978.0	80	5979.0	95
X1	1.25	11	35	72	25	25	25	0	0	0
GR	5979.8	20	5979.0	28	5978.0	35	5977.0	38	5976.0	40
GR	5975.0	45	5974.6	50	5975.0		5976.0		5977.0	72
GR	5978.0	78								

COLECT								2	- E1
X1 1.50	12	32	82	25	25	25	0	0	0
GR5979.0				5977.0		5976.0	34	5975.0	39
GR5974.0		5973.8	50	5974.0		5975.0	68	5976.0	82
GR5977.0		5978.0	96						
X1 1.75		36	81	26	25	23	0	0 5974.0	0
GR5977.3		5977.0	26	5076 D	31	5975 N	36	5974.0	43
GR5973.0		5972.8	50	5973.0	52	5074 0	76	5975.0	81
GR5976.0		5977.0							
X1 2.00		44	60	24	25	27	٥	0 5973.0	0
GR5976.6		5976.0	24	5075 N	11	5974 0	44	5973.0	46
GR5970.0		5971.8	50	5972.0	56	5973.0	60	5974.0	76
GR5975.0									
X1 2.25	11	34	62	22	25	23	0	0 5972.0	0
GR5975.8	20	5975.0	34	5974.0	39	5973.0	41	5972.0	42
	48	5970.7	50	5971.0	54	5972.0	62	5973.0	68
GR5974.0	105	371011	30	371114	•	37.4.0	•••	,	
		23	62	25	25	25	0	0	0
			23	5973.0	24	5972.0	27	5971.0	31
			50	5970.0	53	5971.0	62	5972.0	73
		5974.0	98						
X1 2.75		13	89	34	25	19	0	0	0
		5975.0	8	5974.3	13	5974.0	16	5973.0	17
							50	5970.0	
			. 89			5971.0	170		
X1 3.00	12	14	82	25	25	25	0	0	0
GR5974.0	0	5974.3	1	5974.0	2	5973.0	3	5972.0	7
			22	5969.0	32	5968.7	50	5969.0	56
GR5970.0	64	5971.0	82						
X1 3.25	9	18	62	25	25	25	0	0	0
				5970.9				5969.0	30
		5969.0				5970.0			_
		23		27				0	
				5970.0				5968.0	35
		5968.0	59	5969.0	80	5969.0	111		
				25					0
GR5970.0								5966.4	50
GR5967.0	59			5968.1		5968.0	103		0
X1 4.00	5	34	61	21	25	32	0		0
GR5967.0	21	5966.0		5965.0			61		73
X1 4.25	8	34	65	25	25	25	0		0
GR5967.0	16	5966.3	28			5965.3	34	5965.0	39
GR5965.0	63	5965.3	65		72	25	۸	٥	Λ
X1 4.50	8	33	69	25		25		0	0
GR5966.0	7		33			5963.5	J U	5964.0	60
GR5964.4	65		69		72	25	Λ	0	0
X1 4.75	11	32	68 30	25 5964.0	25	5963.5		5963.0	45
GR5965.0	26 50	5964.5	58			5964.0	68		73
GR5962.6 GR5965.0	77	5963.0	00	3703.0	0.)	J7 U4 • U	UU	3704.3	,,
X1 5.00	12	24	75	25	25	25	n	0	0
GR5964.0	24	5963.6	27.5			5962.6		5962.0	43
GR5961.7	50		56		65	5963.0	68		73
GR5964.0	75	5965.0	87	J.V.C. U	V.J	3703.0	00	370310	1.3
X1 5.25	12	23	75	25	25	25	n	0	0
GR5963.0	23					5961.7		5961.0	44
GR5960.8	50		55			5962.0			73
38370010	30	270110	,,	3,0111	• • •	m·V	٠,		

GR5963.0	75	5964.0	82						
X1 5.50	12	23	75	25	25	25	0	0	0
GR5962.0	23	5961.8	27	5961.0	33	5960.8	40	5960.0	46
GR5959.9	50	5960.0	52	5960.8	65	5961.0	67	5961.8	73
GR5962.0	75	5963.0	101						
II 5.75	11	26	74	25	25	25	0	0	0
GR5961.0	26	5960.9	27	5960.0	33	5959.9	35	5959.0	48
GR5959.0	51	5959.9	65	5960.0	66	5960.9	72	5961.0	74
GR5962.0	84								
X1 6.00	7	28	72	25	25	25	0	0	0
GR5961.0	19	5960.0	28	5959.0	35	5958.1	50	5959.0	64
GR5960.0	72	5961.0	83						
X1 6.25	11	29	71	25	25	25	0	0	0
GR5960.0	18	5959.1	27	5959.0	29	5958.1	35	5958.0	38
GR5957.2	50	5958.0	60	5958.1	65	5959.0	71	5959.1	72
GR5960.0	81								
BJ									
BR									
ISUMMARY OF	BRRORS								
٥									

T1 CARNEL LOT 24

T2 T3

CARMBL LOT 24 REVISED 10/31/92

S U M P O

Interactive Summary Printout
for MS/PC-DOS micro computers

May 1991

NOTB - Asterisk (*) at left of profile number indicates message in summary of errors list

CARNEL LOT 24 REVISED 1

Summary Printout

	SBCNO	CWSEL	TOPWID	ARBA	Q	DBPTH	QCH	VCH	CRIWS	FRCH	BLMIN	DIFWSX	KRATIO
*	.00	5985.65	46.05	58.38	380.00	2.05	371.47	6.86	5985.65	.94	5983.60	.00	.00
*	.25	5982.71	30.62	27.04	380.00	1.31	380.00	14.05	5983.52	2.64	5981.40	-2.94	3.09
ż	.50	5982.08	31.86	34.15	380.00	1.88	380.00	11.14	5982.67	1.87	5980.20	63	.68
	.75	5980.18	30.56	30.08	380.00	1.78	379.98	12.69	5980.90	2.20	5978.40	-1.90	1.19
	1.00	5977.54	33.08	26.89	380.00	1.44	380.00	14.13	5978.32	2.76	5976.10	-2.63	1.30
	1.25	5976.23	25.50	24.71	380.00	1.63	380.00	15.38	5977.18	2.75	5974.60	-1.31	.97
	1.50	5975.16	32.02	29.56	380.00	1.36	380.00	12.86	5975.91	2.36	5973.80	-1.07	.36
	1.75	5974.46	38.51	35.93	380.00	1.66	380.00	10.58	5974.99	1.93	5972.80	70	.82
*	2.00	5974.33	36.96	46.80	380.00	2.53	316.33	9.68	5974.69	1.19	5971.80	13	.56
*	2.25	5972.84	25.91	33.29	380.00	2.14	370.53	11.89	5973.63	1.71	5970.70	-1.49	1.49
*	2.50	5971.29	35.34	29.89	380.00	1.49	378.52	12.86	5972.03	2.37	5969.80	-1.55	1.48
	2.75	5970.76	48.14	38.53	380.00	1.36	380.00	9.86	5971.25	1.94	5969.40	54	. 84
	3.00	5970.10	44.88	41.41	380.00	1.40	380.00	9.18	5970.54	1.68	5968.70	65	.85
	3.25	5969.92	73.40	58.08	380.00	1.52	320.80	7.64	5970.20	1.29	5968.40	19	.74
	3.50	5968.99	51.63	45.72	380.00	1.69	380.00	8.31	5969.34	1.56	5967.30	93	1.25
	3.75	5968.09	78.37	44.50	380.00	1.69	363.19	9.13	5968.49	1.53	5966.40	90	.95
	4.00	5966.63	42.87	35.68	380.00	1.63	358.41	11.70	5967,25	1.94	5965.00	-1.45	1.26
	4.25	5966.25	43.42	43.71	380.00	1.25	351.90	9.38	5966.64	1.50	5965.00	39	.77
	4.50	5965.05	37.64	37.00	380.00	1.55	380.00	10.28	5965.58	1.79	5963.50	-1.20	1.23
	4.75	5964.15	38.24	38.28	380.00	1.55	379.80	9.97	5964.65	1.71	5962.60	90	.95
	5.00	5963.32	40.49	39.70	380.00	1.62	380.00	9.57	5963.76	1.70	5961.70	84	1.01
	5.25	5962.39	41.17	40.15	380.00	1.59	380.00	9.46	5962.83	1.69	5960.80	92	.99
	5.50	5961.61	43.13	42.07	380.00	1.71	380.00	9.03	5962.01	1.61	5959.90	78	.95
	5.75	5960.61	41.05	40.37	380.00	1.61	380.00	9.41	5961.03	1.67	5959.00	-1.00	1.04
	6.00	5959.82	41.28	41.82	380.00	1.72	380.00	9.09	5960.20	1.59	5958.10	78	.95
	6.25	5958.94	41.19	41.28	380.00	1.74	380.00	9.21	5959.34	1.62	5957.20	88	1.02

Mike Beyer, P.E. The B.E.A.R. Company 1311 Apex Court Rio Rancho, NM 87124

The above comments include AMAFCA and County Public Works review. If you should have any questions, please do not hesitate to contact me at 768-2650.

Cordially,

Gilbert Aldaz, P.E. & P.S.

City/County Floodplain Administrator

xc: Clifford E. Anderson, AMAFCA Bob. Foglesong, County PWD

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