

City of Albuquerque P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

December 19,1997

Shahab Biazar 10209 Snowflake Ct. NE Albuquerque, New Mexico 87114

RE: DRAINAGE PLAN FOR DAN & ANGELA ARAGON RESIDENCE (D12-D5) ENGINEER'S STAMP DATED 12/15/97

Dear Mr. Biazar:

Based on the information provided on your December 15,1997 submittal, the above referenced site is approved for Building Permit.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

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Also, prior to the final, Engineer Certification per the DPM checklist will be required.

If I can be of further assistance, please feel free to contact me at 924-3985.

C: Andrew Garcia
File

Sincerely

Bernie J. Montoya ČE

Associate Engineer

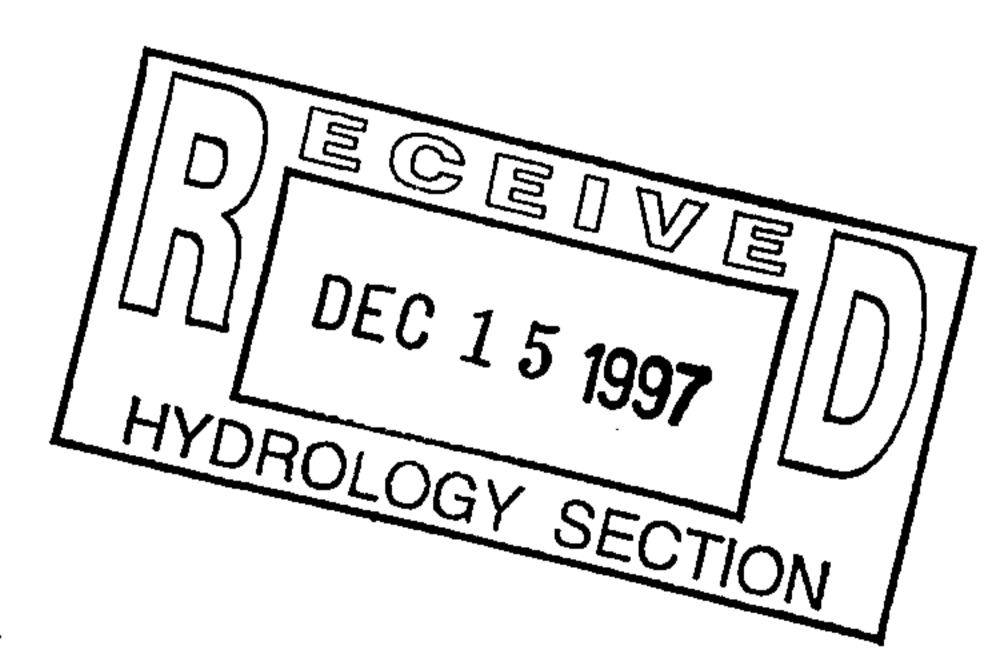
DRAINAGE REPORT FOR

Lot 85
OF

Rancho Sereno Subdivision

Prepared by:

Shahab Biazar 10209 Snowflake Ct. NW Albuquerque, New Mexico 87114



December, 1997

I certify that this report was prepared under my supervision, and I am a registered professional engineer in the state of New Mexico in good standing.

Shahab Biazar PE NO. 13479

Location

Lot 85 of the Rancho Sereno Subdivision is a 2.006 acre lot which was plated as part of the Rancho Sereno subdivision. This lot is located at east end of the Rancho Grande Place. See attached Zone Atlas page number D-12 for location. We are proposing to build a two story house and creating an impervious area of ±8000 sf.

Purpose

I, Shahab Biazar, on behalf of Dan and Angela Aragon is preparing this drainage report to present a grading and drainage solution for their proposed house. We are requesting grading and drainage and building permit approval.

Existing Drainage Conditions

The site falls within the Basin D-2 of Rancho Sereno Subdivision Drainage

Management Plan (D12/C12 - D1M). See Plate 2 of Rancho Sereno Subdivision Drainage

Management Plan for Basin layout (See map pocket for the Basin Layout). According to Plate

2 there are no offsite runoff entering the site. There is a small offsite basin which drains east

and then north to an existing v-ditch, and from there it drains to Piedras Marcadas Dam at a

flow rate of 1.48 cfs. The on site runoff drains west to an existing v-ditch and then north to

Piedras Marcadas Dam at a flow rate of 1.48 cfs.

Flood Plain

The site is located on FEMA Map No. 350002 panel 8 as shown on the attached excerpt. The map shows that the site does not lie within any 100 year flood plains. There is an existing flood to the North of the lot (Piedras Marcadas Dam).

Proposed Conditions and On-Site Drainage Management Plan

The site will maintain its natural drainage patterns. It will be graded in such way to allow the runoff drain around the house and then to exiting v-ditches on the west and north side of the site. This building will increase the runoff by 0.62 cfs, a total runoff of 4.70 cfs. Since the Lot is next to Piedras Marcadas Dam, will not have any impact over the drainage capacity of the existing storm sewer structures. The runoff from this site will be in and out of the existing v-ditches before the peak discharge at the ditches occur. Only \pm 5000 sf of the structures will be built at this time, and the rest of them at a later time.

Calculations

City of Albuquerque, Development Process Manuel, Section 22.2, Hydrology Section, revised January, 1993 was used for runoff calculations. See section runoff calculations of this report for Summary Table for runoff results. See also AHYMO input and summery output file for runoff and ponding calculations.

RUNOFF SUMMARY TABLE

PROPOSED

DRAINAGE BASINS

BASIN	AREA (SF)	AREA (AC)	AREA (MI ²)
Lot 85	87422.35	2.0069	0.003136
Off-Site	31645.75	0.7265	0.001135

BASINS RUNOFF CALCULATION RESULTS UNDER EXISTING CONDITIONS

		
BASIN	Q-100	Q-10
	CFS	CFS
Lot 85	4.7	2.07
Off-Site	1.48	0.56

BASINS RUNOFF CALCULATION RESULTS UNDER EXISTING CONDITIONS

BASIN	Q-100	Q-10
	CFS	CFS
Lot 85	4.08	1.54
Off-Site	1.48	0.56

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RUNOFF CALCULATIONS

The site is @ Zone 1

LAND TREATMENT

Proposed

D = 10.00 %

B = 80.00 %

C = 10.00 %

Existing

B = 100 %

DEPTH (INCHES) @ 100-YEAR STORM

 $P_{60} = 1.87$ inches

 $P_{360} = 2.20 \text{ inches}$

 $P_{1440} = 2.66 \text{ inches}$

DEPTH (INCHES) @ 10-YEAR STORM

 $P_{60} = 1.87 \times 0.667$ = 1.25 inches

 $P_{360} = 1.47$

 $P_{1440} = 1.77$

See the summary output from AHYMO calculations.

Also see the following summary tables.

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) INPUT FILE = LOT85

- VERSION: 1997.02a

RUN DATE (MON/DAY/YR) =12/14/1997 USER NO.= AHYMO-I-9702a0100011K-SH

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COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE =	
START								•		TIME=	.00
RAINFALL TYP	E= 1									RAIN6=	2.200
COMPUTE NM HY	D 101.00	-	1	.00314	4.70	.138	.82472	1.500	2.342	PER IMP=	10.00
COMPUTE NM HY	D 102.00	-	1	.00114	1.48	.040	.66738	1.533	2.037	PER IMP=	.00
START										TIME=	.00
RAINFALL TYP	E= 1									RAIN6=	1.470
COMPUTE NM HY	D 101.10	-	1	.00314	2.07	.057	.34277	1.533	1.031	PER IMP=	10.00
COMPUTE NM HY	D 102.10	-	1	.00114	.56	.014	.22437	1.533	.770	PER IMP=	.00
START										TIME=	.00
RAINFALL TYP	E= 1	•								RAIN6=	2.200
COMPUTE NM HY	D 111.00	-	1	.00314	4.08	.112	.66738	1.533	2.031	PER IMP=	-00
START						•				TIME=	.00
RAINFALL TYP	E= 1							•		RAIN6=	1.470
COMPUTE NM HY FINISH	D 111.10	-	1	.00314	1.54	.038	.22437	1.533	.768	PER IMP=	.00