CITY OF ALBUQUERQUE

March 10, 2009

Scott Medina, P.E.
Mark Goodwin & Associates, PA
P.O. Box 90606
Albuquerque, NM 87199

Re: Bosquecito Subdivision Drainage Report
Engineer's Stamp dated 2-27-09 (D13/D001)

Dear Mr. Medina,

Based upon the information provided in your submittal received 2-27-09, the above referenced report cannot be approved for Preliminary Plat action by the DRB until the following comments are addressed:

- A Grading Plan is required.
- The detention pond at the entrance road should be a chain of sedimentation ponds that add an open space quality to the area.
- The storm drain running west-east through the development should align with the street network with manholes in the street.
- Include storm water protection measures in the Grading Plan.
- Add street capacity analyses to the report.

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne

Curtis A. Cherne, P.E.
Senior Engineer, Planning Dept.
Development and Building Services

C: file
Brad Bingham

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

DRAINAGE AND TRANSPORTATION INFORMATION SHEET
(REV 12/2005)

D-13 D001

PROJECT TITLE: Bosquecito Subdivision ZONE MAP: ~~D-13~~ 13
DRB#: 100696 EPC#: _____ WORK ORDER#: _____

LEGAL DESCRIPTION: Tract C, Lee's Bosque Subdivision
CITY ADDRESS: _____

ENGINEERING FIRM: Mark Goodwin + Associates CONTACT: John Mackenzie
ADDRESS: P.O. Box 90606 PHONE: (505) 828-2200
CITY, STATE: Albuquerque, NM ZIP CODE: 87199

OWNER: JCDC, LLC CONTACT: Scott Grady
ADDRESS: P.O. Box 1443 PHONE: (505) 892-5533
CITY, STATE: [redacted] Corrales, NM ZIP CODE: 87048

ARCHITECT: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

SURVEYOR: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

CONTRACTOR: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

TYPE OF SUBMITTAL:
- X DRAINAGE REPORT
- ___ DRAINAGE PLAN 1st SUBMITTAL
- ___ DRAINAGE PLAN RESUBMITTAL
- ___ CONCEPTUAL G & D PLAN
- ___ GRADING PLAN
- ___ EROSION CONTROL PLAN
- ___ ENGINEER'S CERT (HYDROLOGY)
- ___ CLOMR/LOMR
- ___ TRAFFIC CIRCULATION LAYOUT
- ___ ENGINEER'S CERT (TCL)
- ___ ENGINEER'S CERT (DRB SITE PLAN)
- ___ OTHER (SPECIFY)

CHECK TYPE OF APPROVAL SOUGHT:
- ___ SIA/FINANCIAL GUARANTEE RELEASE
- X PRELIMINARY PLAT APPROVAL
- ___ S. DEV. PLAN FOR SUB'D APPROVAL
- ___ S. DEV. FOR BLDG. PERMIT APPROVAL
- ___ SECTOR PLAN APPROVAL
- ___ FINAL PLAT APPROVAL
- ___ FOUNDATION PERMIT APPROVAL
- ___ BUILDING PERMIT APPROVAL
- ___ CERTIFICATE OF OCCUPANCY (PERM)
- ___ CERTIFICATE OF OCCUPANCY (TEMP)
- ___ GRADING PERMIT APPROVAL
- ___ PAVING PERMIT APPROVAL
- ___ WORK ORDER APPROVAL
- ___ OTHER (SPECIFY)

WAS A PRE-DESIGN CONFERENCE ATTENDED:
- ___ YES
- ___ NO
- ___ COPY PROVIDED

$ 460.00

FEB 27 2009
HYDROLOGY SECTION

DATE SUBMITTED: 2/27/09 BY: Scott Mulvaney

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan:** Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
2. **Drainage Plans:** Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
3. **Drainage Report:** Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

November 3, 1995

Mark Goodwin
Mark Goodwin & Associates
P.O. Box 90606
Albuquerque, NM 87199

RE: LINBAUGH CAR WASH (D13-D1) GRADING AND DRAINAGE PLAN FOR BUILDING PERMIT APPROVAL. ENGINEER'S STAMP DATED 10-23-95.

Dear Mr. Goodwin:

Based on the information provided on your October 25, 1995 submittal, the above referenced project is approved for Building Permit.

Prior to Certificate of Occupancy approval, an Engineer's Certification is required.

If I can be of further assistance, please feel free to contact me at 768-3622.

Sincerely,

Lisa Ann Manwill
Engineering Assoc./Hyd.

c: Andrew Garcia
File

THE CITY OF ALBUQUERQUE IS AN EQUAL OPPORTUNITY/REASONABLE ACCOMMODATION EMPLOYER

City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

KEN SCHULTZ
MAYOR

D13/D1

July 26, 1989

RECEIVED
JUL 28 1989
HYDROLOGY SECTION

CERTIFICATE OF COMPLETION AND ACCEPTANCE

Mr. Greg Polk
Miller Brown Land Company
3411 Candelaria N.E.
Albuquerque, NM 87107

RE: PROJECT NO. 3688, BOSQUE DEL PUEBLO, (MAP NO. D-13)

Dear Mr. Polk:

This is to certify that the City of Albuquerque accepts Project No. 3688 as being completed according to approved plans and construction specifications. If all required right-of-ways and/or easements have been dedicated, the City of Albuquerque will accept for continuous maintenance all public infrastructure improvements constructed as part of Project No. 3688. If the required right-of-ways and/or easements have not been dedicated, the City of Albuquerque cannot accept the project for continuous maintenance and said maintenance will be the responsibility of the developer. When a final plat has been filed it will be the developer's responsibility to provide the Construction Management Division with a copy, at which time the City will fully accept Project No. 3688.

The project is described as follows:

- Installed 8" water main and 8" sewer main on Caminito Coors Rd. Installed 10" water main on Eagle Ranch Rd. Placed paving and curb and gutter on Caminito Coors Rd. and Eagle Ranch Rd. Storm drain improvements, channel lining and cut off wall at the AMAFCA Channel. Installed two (2) 21" RCP lines from Coors Rd. to AMAFCA Channel.

- The contractor's warranty begins the date of this letter and will be effective for a period of one (1) year.

Sincerely,

Russell B. Givler, P.E.
Chief Construction Engineer
Construction Mgmt. Division
Engineering Group
Public Works Department

RBG:kt

D13/D1

R. WARD HUNNICUTT, CHAIRMAN
PAT D. HIGDON, VICE-CHAIRMAN
REX FUNK, SECRETARY-TREASURER
DANIEL W. COOK, DIRECTOR
RONALD D. BROWN, DIRECTOR

LARRY A. BLAIR
EXECUTIVE ENGINEER

Albuquerque
Metropolitan
Arroyo
Flood
Control
Authority

2600 PROSPECT N.E. - ALBUQUERQUE, N.M. 87107
TELEPHONE (505) 884-2215

RECEIVED
JUN 0 9 1989
HYDROLOGY SECTION

D13-D1

June 7, 1989

Jeffrey G. Mortenson, P.E.
Jeff Mortenson & Associates, Inc.
811 Dallas NE
Albuquerque, New Mexico 87110

RE: Site Development Plan approval for portions of Tract 2-A, Mann Tracts, also known as Lots 1 and 2, Bosque del Pueblo Subdivision, Scheduled for EPC public hearing on June 15, 1989 (Z-89-163-1).

Dear Jeff:

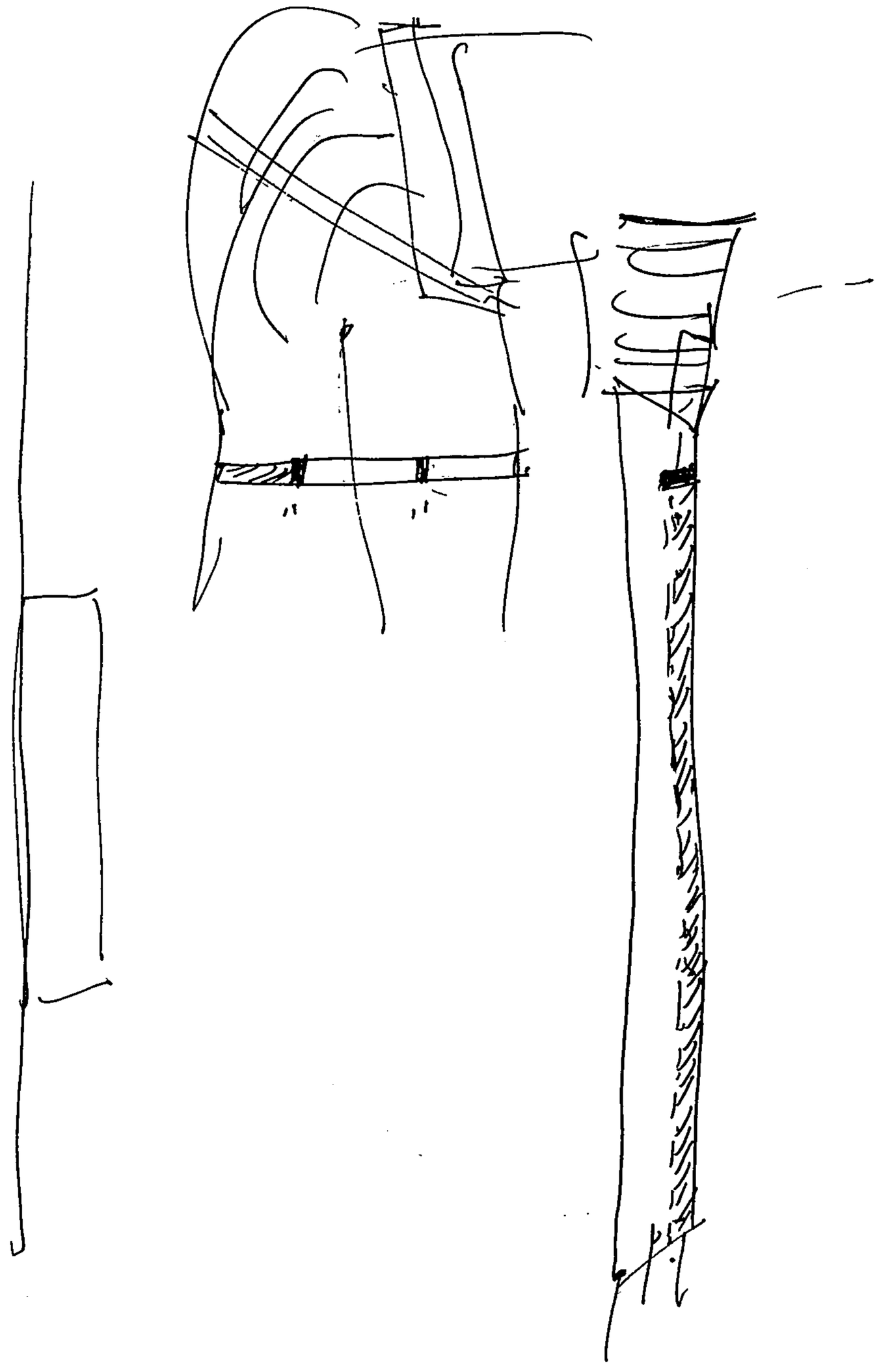
AMAFCA has reviewed the revised Conceptual Grading and Drainage Plan for the above referenced project and believe that the comments in our letter dated May 9, 1989 have been addressed by this revised submittal. Specifically, control of coarse silt by a sedimentation basin is included in the drainage computations and shown on the site plan.

The site development therefore meets the requirements of AMAFCA and allows for stormwater to be discharged into the Corrales Main Canal.

Sincerely,

Clifford E. Anderson, PE & LS
Drainage Engineer

Copy to:
Fred Aguirre, Public Works Department, Hydrology
Virginia Benavides, EPC Secretary, Planning Division
Subhas Shah, District Engineer, MRGCD
Ernest Ulibarri & Associates, 1520 Lomas NW 87102



Greiner

Greiner, Inc.
5971 Jefferson Blvd., N.E.
Suite 101
Albuquerque, New Mexico 87109
(505) 345-3999
FAX: (505) 345-8393

RECEIVED
FEB 08 1989
HYDROLOGY SECTION

DRAINAGE INFORMATION FOR

BOSQUE DEL PUEBLO

December 9, 1988
Revised January 18,1989
Revised February 6,1989

INTRODUCTION

The site falls within the area analyzed by the North Coors Drainage Management Plan (NCDMP). The site is composed of subbasins 13.2E AND 14.2E of the NCDMP. The NCDMP analysis method was that of the U.S.D.A. HYMO computer program. The NCDMP was completed on October 19, 1984. The character of the site has remained unchanged from the time of the report. Therefore, the calculations in the report are assumed to still be accurate for the existing conditions on site. Also, the NCDMP establishes the upper limits of flows for the developed condition within the study area.

EXISTING CONDITION:

Subbasin 13.1W is the off-site area which currently flows onto the site in Subbasin 13.2E. The runoff flows identified in the NCDMP for Subbasin 13.1E are Existing Q100 = 3.9 cfs. and a maximum allowable Developed Q100 = 20.5 cfs.

The runoff flows identified in the NCDMP for Subbasin 13.2E are Existing Q100 = 3.1 cfs and a maximum allowable Developed Q100 = 29.1 cfs.

Subbasin 14.1W is the off-site area which currently flows onto the site in Subbasin 14.2E. The runoff flows identified in the NCDMP for Subbasin 14.1E are Existing Q100 = 8.4 cfs. and a maximum allowable Developed Q100 = 13.4 cfs.

The runoff flows identified in the NCDMP for Subbasin 14.2E are Existing Q100 = 1.7 cfs and a maximum allowable Developed Q100 = 16.7 cfs.

PROPOSED CONDITIONS:

Due to desilting requirements, site drainage and constructability some minor rerouting of the flows within subbasins 13.2E and 14.2E are proposed. Therefore the two subbasins have been redefined as drainage areas A through E (see plan). The original subbasins were analyzed with the HYMO computer model. In order to be consistent with that analysis the new drainage areas were analyzed by the SCS Method using the same parameters as in the HYMO study. For the developed condition in subbasins 13.2E AND 14.2E the NCDMP assumed an impervious area of 75% and a previous curve number of 44.0. For this analysis a composite curve number of 85 was extrapolated from plate 22.2 C-3 of the DPM. The point rainfall (P) of 2.2" was identified from plate 22.2 D-1 of the DPM. The runoff volume (R) of 1.00 was taken from plate 22.2 C-4 of the DPM. Due to the small areas a Tc of 10 min. is used.

Drainage Area A
area = 1.87 ac.

$$qp = \frac{45.4 * 1.87}{10} = 8.45$$

$Q_{100peak}$ = R * qp = 8.45 cfs on site, see computation for pond routing and Q leaving site

Drainage Area B
area = 1.97 ac.

$$qp = \frac{45.4 * 1.97}{10} = 8.94$$

$Q_{100peak}$ = R * qp = 8.94 cfs on site, see computation for pond routing and Q leaving site

Drainage Area C
area = 0.97 ac.

$$qp = \frac{45.4 * 0.97}{10} = 4.40$$

$Q_{100peak}$ = R * qp = 4.40 cfs on site, see computation for pond routing and Q leaving site

Drainage Area D
area = 4.70 ac.

$$qp = \frac{45.4 * 4.7}{10} = 21.34$$

$Q_{100peak}$ = R * qp = 21.34 cfs on site, see computation for pond routing and Q leaving site

Drainage Area E
area = 1.18 ac.

$$qp = \frac{45.4 * 1.18}{10} = 5.31$$

$Q_{100peak}$ = R * qp = 2.4 cfs
Due to final development in this area actual values were calculated i.e. 50% impervious, Cn = 72, R = 0.45

The flows from subbasin 13.1W are to be diverted by plugging the two 30" pipes passing under Coors Blvd. and flow in an earthen channel southerly along Coors Blvd. The total developed flows to be controlled would be 20.5 cfs; the current Q100 flows to control is 3.9 cfs. The cross section for this channel is detailed on the plan. The channel hydraulics were calculated with the following independent variables:

n = .03
cross section as shown on plan
channel slope = 0.5%
Q = 3.9 cfs

The resulting flow data is as follows:
normal depth = 0.83′
velocity = 1.88 ft./sec

The flows from subbasin 13.1W and 14.1W are to be desilted on the west side of Coors Blvd. prior to entering the pipes crossing Coors Blvd. The NCDMP and MRGCD License require the Q100/2 flow be desilted; following is the desilting requirements based on removal of coarse silt:

Min. pond storage area = .0024 acft/acre upstream area
Velocity of sediment fall = .002 ft/sec
Max. horizontal velocity .5 ft/sec
Retention time in pond = depth (ft) / .002

The flow from subbasin 13.1W is to be desilted in a pond just north of the 60" pipes crossing Coors Blvd. The pond hydraulics were calculated with the following independent variables:
n = .03
5′ wide bottom w/ 3:1 side slopes
pond bottom slope = .025%
Q = 1.95 cfs

The resulting flow data is as follows:
normal depth = 0.61′
horz velocity = 0.47 ft/sec

With a pond length of 144′ the retention time is 306.4 sec.
The sediment storage volume = .004 acft

These flows join with the flows from subbasin 14.1W at the 60" RCPs crossing Coors Blvd. However, the desilting storage volume for subbasin 14.1W is currently full and overflowing. Maintenance must be performed to re-establish the storage volume of 0.7 acft at the 60′ RCPs. The total (developed) flows to be controlled at this point would be 33.9 cfs; the current flows to controlled are 12.3 cfs. These flows are transported across Coors Blvd to a junction box on the east side of Coors Blvd by two 60" RCPs. The pipe hydraulics for these pipes were calculated with the following independent variables:
n = .013
60" diameter pipe
slope = 1.77%
Q = 16.95 cfs

The outlet for the pond would be a sumped drop inlet. The inlet is large enough to function as a weir rather than an orifice. The Q100/2 flow results in a .36′ depth flowing over the weir. The Q100 flow would result in a 0.57′ depth flowing over the weir; elevations on the plan indicate adequate freeboard.

Drainage area B sheet flows across Lots C, D & E and is collected at the northeast corner of Lot D. At this point the flow enters a desilting pond before dumping into the Corrales Main Canal. The desilting requirement are the same as above.

The cross section for this pond is detailed on appendix B. The pond hydraulics were calculated with the following independent variables:
n = .03
cross section as shown on appendix B
pond bottom slope = 0.025%
Q = 4.47 cfs Q100 = 8.94 cfs

The resulting flow data is as follows:
normal depth = 0.55′
horz velocity = 0.49 ft/sec

With a pond length of 135′ the retention time is 275.5 sec.
The sediment storage volume = .012 acft
Routing the hydrograph through the site and detention pond the resulting Q100 = 6.08 cfs with the allowable from the NCDMP being 6.9 cfs.

Drainage area C sheet flows across Lots F and is collected at the northeast corner of Lot F. At this point the flow enters a desilting pond before dumping into the Corrales Main Canal. The desilting requirement are the same as above.

The cross section for this pond is detailed on appendix C. The pond hydraulics were calculated with the following independent variables:
n = .03
cross section as shown on appendix C
pond bottom slope = 0.025%
Q = 2.20 cfs

The resulting flow data is as follows:
normal depth = 0.45′
horz velocity = 0.43 ft/sec

With a pond length of 115′ the retention time is 267.4 sec.
The sediment storage volume = .006 acft

Routing the hydrograph through the site and detention pond the resulting Q100 = 2.98 cfs with the allowable from the NCDMP being 2.98 cfs.

The combined pond configuration (6.67cfs) requires outlet control to maintain the maximum velocity for desilting. A rectangular weir having a 5.1′ width with the .55′ head was chosen for this purpose. The Q100 flow (9.06cfs) was checked for a worse case scenario where the weir notch was blocked and the flow breached the sides of the control weir. This resulted in a 0.32′ depth flowing over the control weir; elevations on the plan indicate adequate freeboard.

Drainage area D includes Eagle Ranch Road, the private street and Lots G, H, & I. Site inspection indicates that the area north of Eagle Ranch Rd. primarily drains to the north, a negligible area does pond and ultimately drain to the south via an existing CMP culvert in the MRGCD R.O.W. Lots G, H, & I sheet flow to the northeast corner of each lot. At this point the flow must be desilted before dumping into the private street.

The drainage area for lot G is 0.918 ac. using the hydrologic parameters from above the resulting Q100 is 4.38 cfs. The desilting requirement are the same as above. The cross section for this pond is detailed on appendix E. The pond hydraulics were calculated with the following independent variables:
n = .03
cross section as shown on appendix E
pond bottom slope = 0.025%
Q = 2.19 cfs

The resulting flow data is as follows:
normal depth = 0.45′
horz velocity = 0.43 ft/sec

With a pond length of 97′ the retention time is 225.6 sec.
The sediment storage volume = .006 acft

This pond configuration requires outlet control to maintain the maximum velocity for desilting. A sidewalk culvert (functioning as a rectangular weir) having a 2.2′ width with the .45′ head was chosen for this purpose.

The drainage area for lot H is 0.911 ac. using the hydrologic parameters from above the resulting Q100 is 4.34 cfs. The desilting requirement are the same as above. The cross section for this pond is detailed on appendix E. The pond hydraulics were calculated with the following independent variables:

The resulting flow data is as follows:
normal depth = 0.75′
velocity = 9.13 ft./sec

Drainage area E is collected by a landscaped swale which drains to a sump inlet connected to the junction box on the east side of Coors Blvd. The total developed flow entering the junction box would be 36.3 cfs. This total (developed) flow is proposed to be transported from the junction box to the Corrales Canal by two 21" RCPs. The resulting head water in the junction box would be 3.85′. The conduit hydraulics were calculated with the following independent variables:
n = .013
21" diameter pipe
slope = 1.33%
Q = 18.15 cfs

The resulting flow data is as follows:
normal depth = 1.42′
velocity = 8.66 ft/sec

Site inspection and aerial topo reveals that the area south of drainage area A drains to the south. Drainage area A sheet flows across Lots A & B and is collected at the southeast corner of Lot A. At this point the flow enters a desilting pond before dumping into the Corrales Main Canal. The desilting requirements are the same as above.

The cross section for this pond is detailed on appendix A. The pond hydraulics were calculated with the following independent variables:
n = .03
cross section as shown on appendix A
pond bottom slope = 0.025%
Q = 4.23 cfs

The resulting flow data is as follows:
normal depth = 0.53′
horz velocity = 0.48 ft/sec

With a pond length of 128′ the retention time is 266.7 sec.
The sediment storage volume = .011 acft
Routing the hydrograph through the site and detention pond the resulting Q100 = 6.59 cfs with the allowable from the NCDMP being 7.33 cfs.

n = .03
cross section as shown on appendix E
pond bottom slope = 0.025%
Q = 2.17 cfs

The resulting flow data is as follows:
normal depth = 0.45′
horz velocity = 0.42 ft/sec

With a pond length of 97′ the retention time is 231 sec.
The sediment storage volume = .006 acft

This pond configuration requires outlet control to maintain the maximum velocity for desilting. A sidewalk culvert (functioning as a rectangular weir) having a 2.2′ width with the .45′ head was chosen for this purpose.

The drainage area for lot I is 1.25 ac. using the hydrologic parameters from above the resulting Q100 is 5.96 cfs. The desilting requirement are the same as above. The cross section for this pond is detailed on appendix E. The pond hydraulics were calculated with the following independent variables:
n = .03
cross section as shown on appendix E
pond bottom slope = 0.025%
Q = 2.98 cfs

The resulting flow data is as follows:
normal depth = 0.54′
horz velocity = 0.47 ft/sec

With a pond length of 127′ the retention time is 270.2 sec.
The sediment storage volume = .007 acft

This pond configuration requires outlet control to maintain the maximum velocity for desilting. A sidewalk culvert (functioning as a rectangular weir) having a 3′ width with a .45′ head was chosen for this purpose.

Routing the hydrograph through the site and detention ponds the resulting Q100 = 13.76 cfs with the allowable from the NCDMP being 15.04 cfs. Drainage area D flows are collected at the east end of Eagle Ranch Road by a modified type "B" inlet 3.5′ X 20′. The flow is then transported from the inlet to the Corrales Main Canal by a small concrete channel.

The cross section for this channel is detailed on appendix D. The channel hydraulics were calculated with the following independent variables:

n = .013
cross section as shown on appendix D
channel slope = 0.5%
Q = 21.34 cfs

The resulting flow data is as follows:
normal depth = 1.01'
velocity = 6.02 ft/sec

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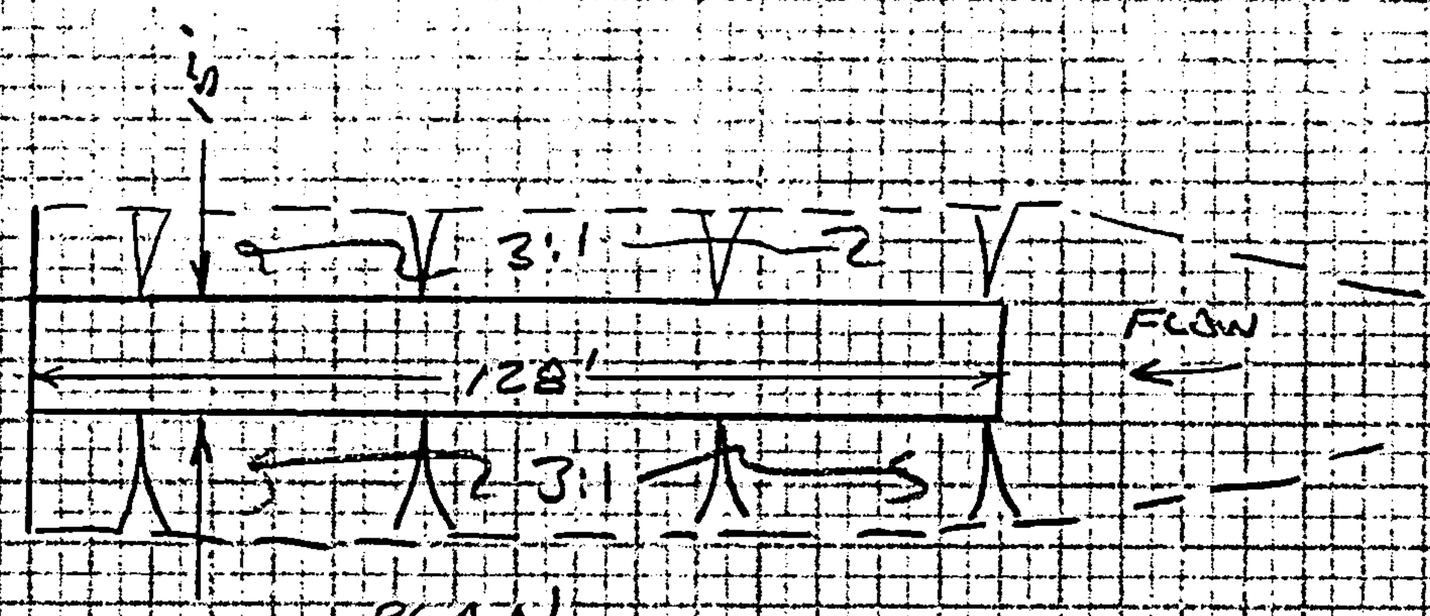
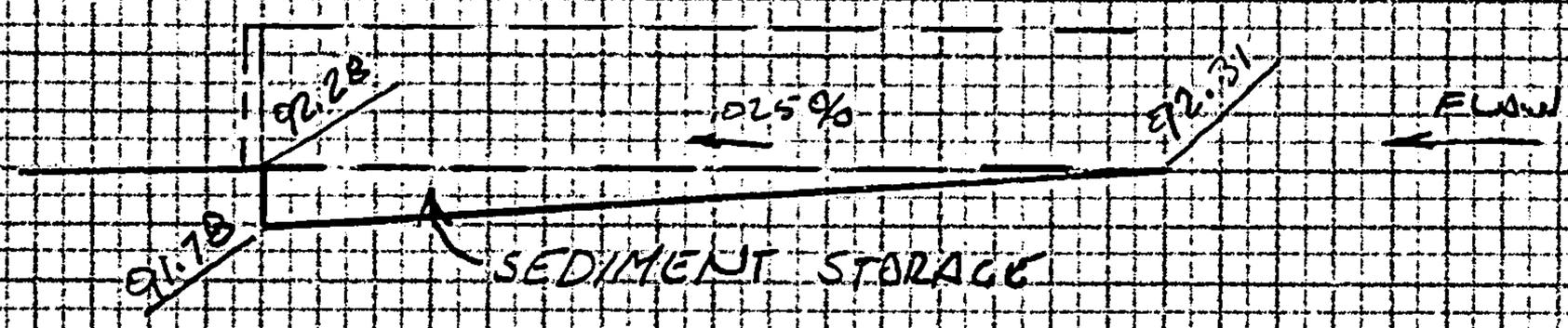
☐ ALBUQUERQUE, NEW MEXICO
☐ KEMMERER, WYOMING

PROJECT BOSQUE DEL PUEBLO
JOB NUMBER 06040656 SHEET ____ OF ____
CALCULATED BY JJB DATE ____
CHECKED BY ____ DATE ____

APPENDIX "A"



PLAN



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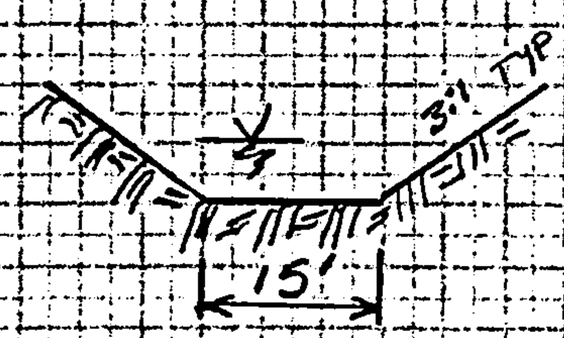

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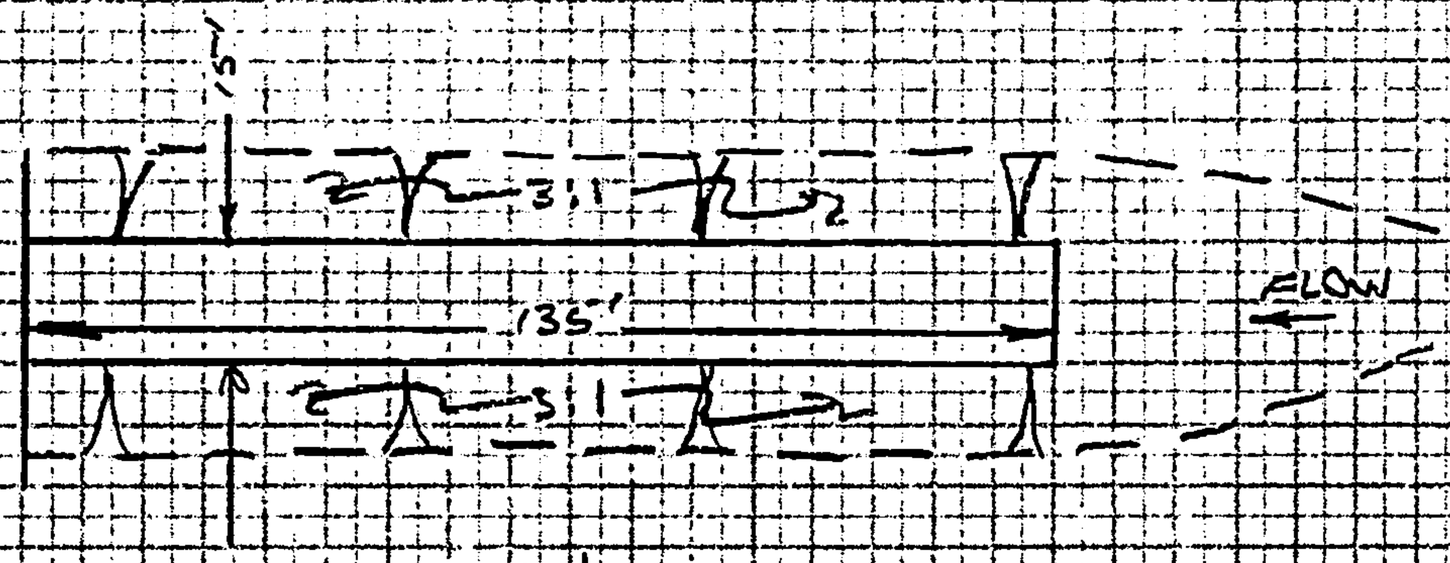
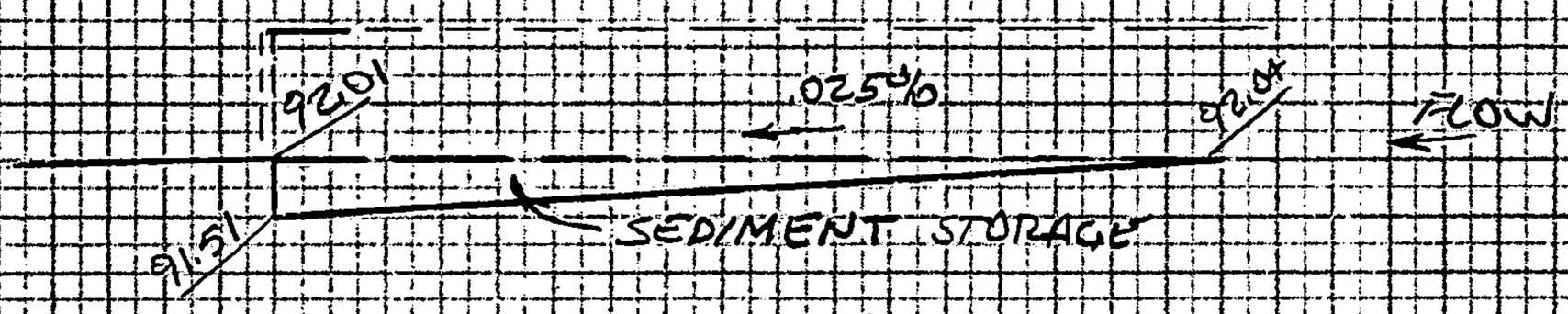
☐ KEMMERER, WYOMING

PROJECT BOSQUE DEL PUEBLO
JOB NUMBER 06400056 SHEET ____ OF ____
CALCULATED BY JJB DATE ____
CHECKED BY ____ DATE ____

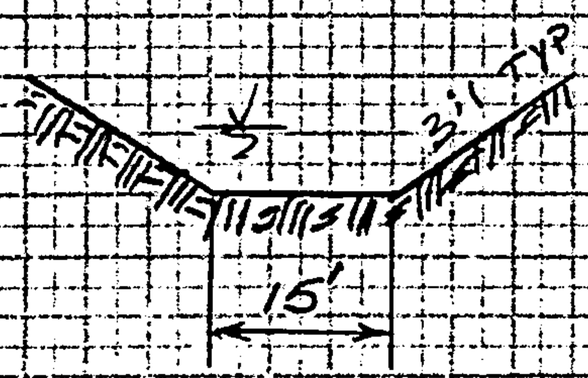
APPENDIX "B"



PLAN



SECTION



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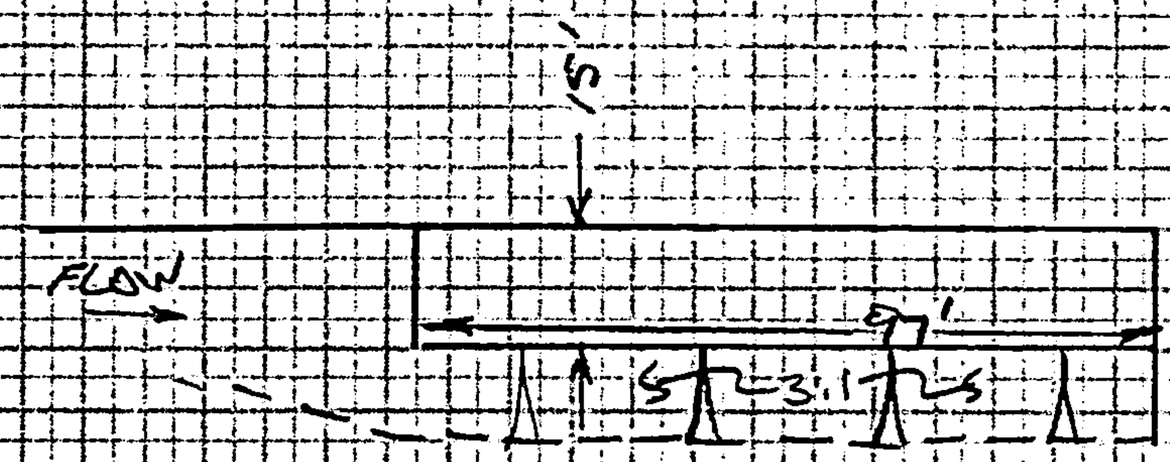
PROJECT BOSQUE DEL PUEBLO
JOB NUMBER 06900656 SHEET ____ OF ____
CALCULATED BY JB DATE ____
CHECKED BY ____ DATE ____

APPENDIX "C"



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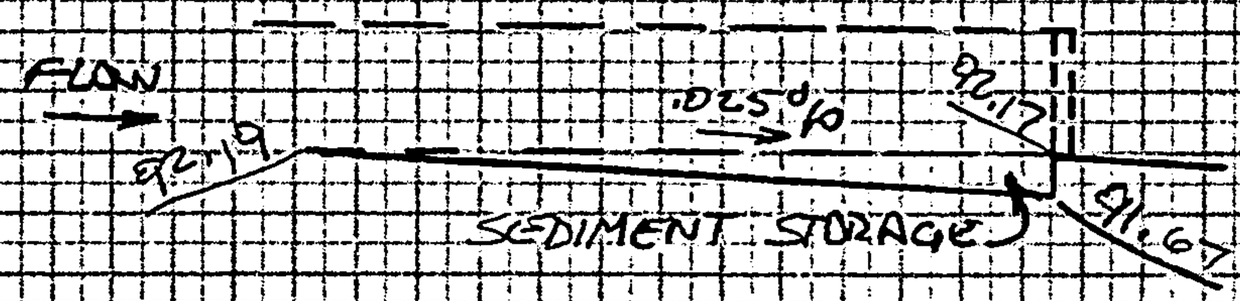

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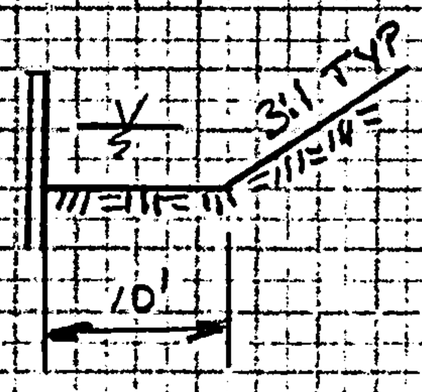

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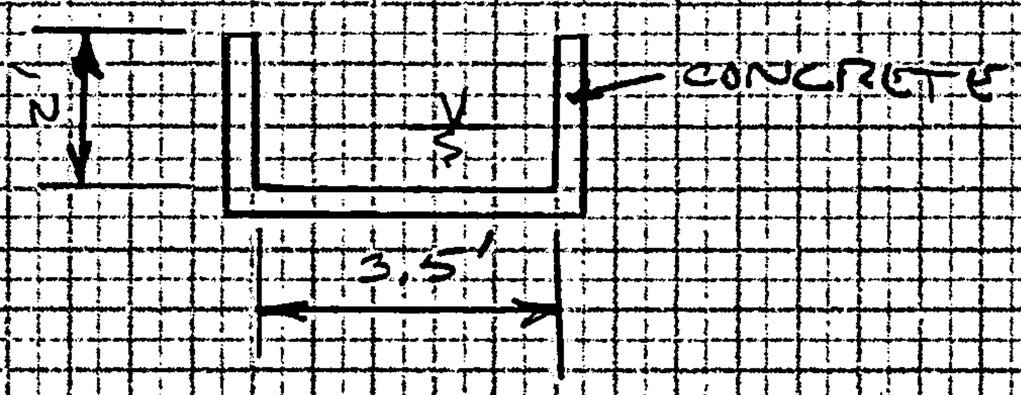
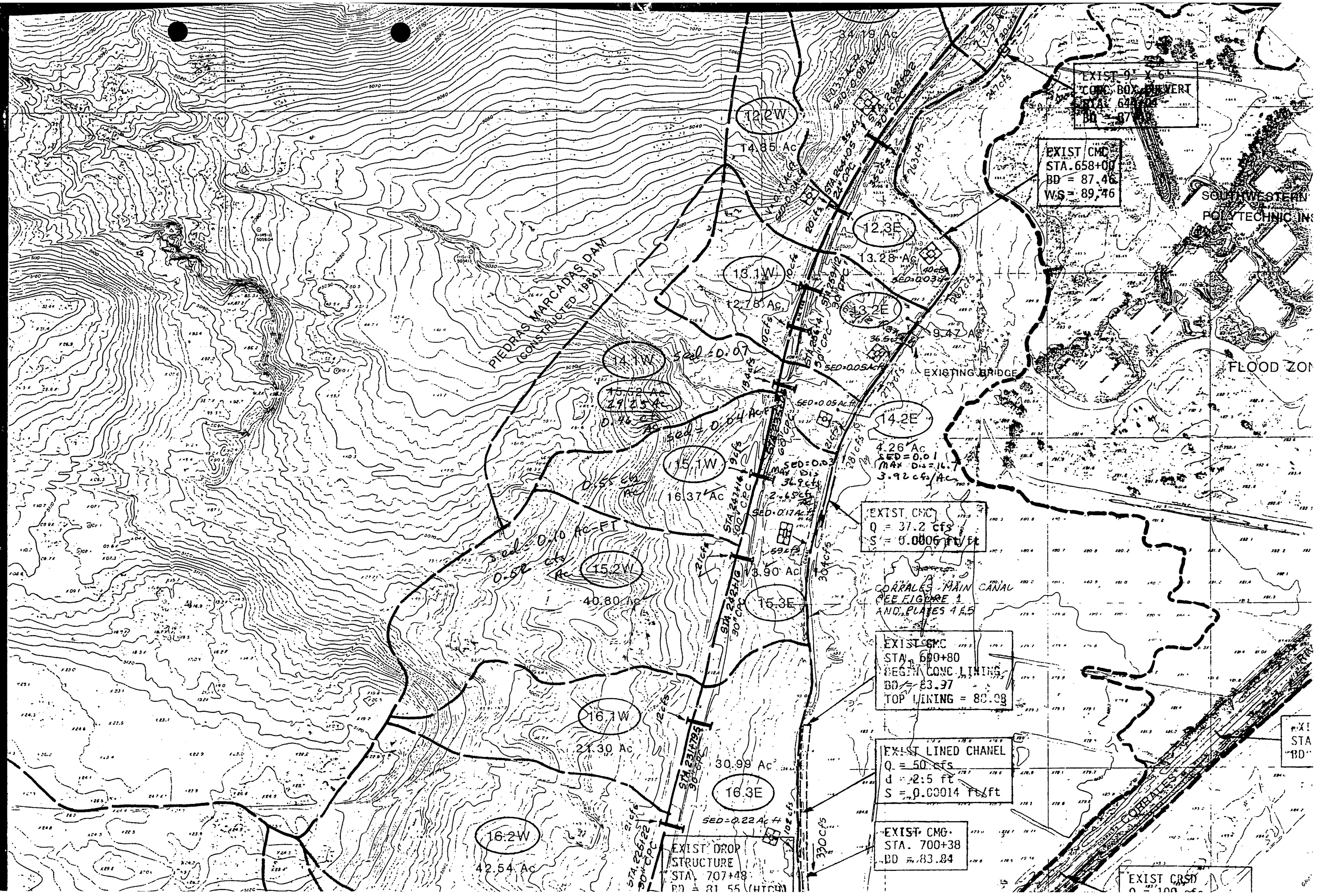
☐ KEMMERER, WYOMING

PROJECT BOSQUE DEL PUEBLO
JOB NUMBER 06901656 SHEET ____ OF ____
CALCULATED BY JJB DATE ____
CHECKED BY ____ DATE ____

APPENDIX "D"




PIEDRAS MARCADAS DAM
(CONSTRUCTED 1984)
12.2W
14.85 Ac
34.19 Ac
EXIST 9' X 6'
CONC BOX CULVERT
EXIST CMC
STA. 658+00
BD = 87.46
W.S = 89.46
SOUTHWESTERN
POLYTECHNIC IN
12.3E
13.28 Ac
13.1W
12.78 Ac
13.2E
9.47 Ac
EXISTING BRIDGE
FLOOD ZON
14.1W
Sed = 0.07
15.12 Ac
29.25 Ac
0.46 cfs
Sed = 0.04 Ac-F
14.2E
4.26 Ac
SED = 0.01
MAX DIS = 16.7
3.92 cfs/Ac
15.1W
16.37 Ac
0.55 cfs/Ac
Sed = 0.10 Ac-FT
0.52 cfs/Ac
15.2W
40.80 Ac
3.90 Ac
15.3E
EXIST CMC
Q = 37.2 cfs
S = 0.0006 ft/ft
CORRALES MAIN CANAL
SEE FIGURE 1
AND PLATES 4 & 5
EXIST CMC
STA. 690+80
BEGIN CONC LINING
BD = 83.97
TOP LINING = 88.08
16.1W
21.30 Ac
30.99 Ac
16.3E
SED = 0.22 Ac ft
EXIST LINED CHANEL
Q = 50 cfs
d = 2.5 ft
S = 0.00014 ft/ft
16.2W
42.54 Ac
EXIST DROP
STRUCTURE
STA. 707+48
BD = 81.55 (HIGH)
EXIST CMC
STA. 700+38
BD = 83.84
EXIST CRSD

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/28/2003rd)

D-13/D1

PROJECT TITLE: Bosquecito Subdivision
DRB #: 100696 EPC#:____

ZONE MAP/DRG. FILE #: ~~D-12~~
WORK ORDER#:_____

LEGAL DESCRIPTION: Tract C, Lee's Bosque Subdivision
CITY ADDRESS:

ENGINEERING FIRM: Mark Goodwin & Associates
ADDRESS: PO Box 90606
CITY, STATE: Albuquerque, NM

CONTACT: Scott Davis
PHONE: 828-2200
ZIP CODE: 87199

OWNER: ICDC
ADDRESS: P.O. Box 1443
CITY, STATE: Corrales, NM

CONTACT: Norm Gregory
PHONE: 991-0977
ZIP CODE: 87048

ARCHITECT: _____
ADDRESS: _____
CITY, STATE: _____

CONTACT: _____
PHONE:
ZIP CODE: _____

SURVEYOR: Aldrich Land Surveying
ADDRESS: PO Box 30701
CITY, STATE: Albuquerque, NM

CONTACT: Tim Aldrich
PHONE: 884-1990
ZIP CODE: 87190-0701

CONTRACTOR: _____
ADDRESS: _____
CITY, STATE: _____

CONTACT: _____
PHONE: _____
ZIP CODE: _____

CHECK TYPE OF SUBMITTAL:

- ☒ DRAINAGE REPORT
- ☐ DRAINAGE PLAN 1st SUBMITTAL, ***REQUIRES TCL or equal***
- ☐ DRAINAGE PLAN RESUBMITTAL
- ☐ CONCEPTUAL GRADING & DRAINAGE PLAN
- ☒ GRADING PLAN
- ☐ EROSION CONTROL PLAN
- ☐ ENGINEER'S CERTIFICATION (HYDROLOGY)
- ☐ CLOMR/LOMR
- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
- ☐ ENGINEERS CERTIFICATION (TCL)
- ☐ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
- ☐ OTHER

CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SIA / FINANCIAL GUARANTEE RELEASE
- ☒ PRELIMINARY PLAT APPROVAL
- ☐ S. DEV. PLAN FOR SUB'D. APPROVAL
- ☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
- ☐ SECTOR PLAN APPROVAL
- ☐ FINAL PLAT APPROVAL
- ☐ FOUNDATION PERMIT APPROVAL
- ☐ BUILDING PERMIT APPROVAL
- ☐ CERTIFICATE OF OCCUPANCY (PERM.)
- ☐ CERTIFICATE OF OCCUPANCY (TEMP.)
- ☒ GRADING PERMIT APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ WORK ORDER APPROVAL
- ☐ OTHER (SPECIFY)

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
- ☒ NO
- ☐ COPY PROVIDED

RECEIVED
NOV 07 2005
HYDROLOGY SECTION

DATE SUBMITTED: 11-7-05

BY: Scott Davis

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature , location and scope ot the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plan**: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
2. **Drainage Plans**: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
3. **Drainage Report**: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

CITY OF ALBUQUERQUE

THREE HUNDRED YEARS
1706 • 2006
ALBUQUERQUE
Making History

November 15, 2005

John M. McKenzie, P.E.
Mark Goodwin & Associates
PO Box 90606
Albuquerque, NM 87199

Re: Bosquecito Subdivision, Lee's Bosque Subdivision
Grading and Drainage Plan
Engineer's Stamp dated 11-07-05 (D13-D1)

Dear Mr. McKenzie,

Based upon the information provided in your submittal received 11-07-05, the above referenced plan is approved for Preliminary Plat Action by DRB. Once the Board has approved the plan, please submit a Mylar copy of the grading plan for my signature in order to obtain a Grading Permit.

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. If you have any questions regarding this permit please feel free to call the DMD Storm Drainage Design section at 768-3654 (Charles Caruso).

If you have any questions, you can contact me at 924-3695.

Sincerely,

Rudy E. Rael, Associate Engineer
Planning Department.
Development and Building Services

C: Charles Caruso, DMD Storm Drainage Design
File

P.O. Box 1293
Albuquerque
New Mexico 87103
www.cabq.gov