City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

March 2, 2004

Diane Hoelzer, P.E.
Mark Goodwin & Associates, PA
P.O. Box 90606
Albuquerque, NM 87199

Re: Paseo de Estrella Subdivision (Formerly Paseo de Estrella and Vista Estrella, in Vista del Norte Development), SIA/Financial Guarantee Release

Engineer's Stamp dated 1-30-02 (D16/D12)

Certification dated 3-01-04

Dear Ms. Hoelzer,

Based upon the information provided in your submittal received 3-02-04, the above referenced certification is approved for release of SIA and Financial Guarantee.

If you have any questions, you can contact me at 924-3981.

Sincerely,

Kristal D. Metro
Engineering Associate, Planning Dept.
Development and Building Services

C: Arlene Portillo, COA# 687181
file

DRAINAGE REPORT

FOR

Vista Estrella and Paseo de Estrella

Subdivisions
at
Vista del Norte

Mark Goodwin & Associates, PA
February 2002
DLH

I. PROJECT DESCRIPTION

This project site is identified as Tract T-2-A-1-C and is located within Vista del Norte Unit 2. The site is sandwiched between Vista Monte Drive and Vista del Norte Drive near the Las Lomitas Drive / Vista del Norte intersection. The tract of land is proposed to be developed into two separate residential subdivisions. The north portion encompassing approximately 22.77 acres (Paseo de Estrella) is proposed to be built into a 76 lot residential gated community with private streets and public utilities. The south portion encompassing approximately 17.32 acres (Vista Estrella) is proposed to be built into an 86 lot residential community.

II. DRAINAGE DESIGN CRITERIA AND PREVIOUS REPORTS

The design criteria used in this report was in accordance with Section 22.2 Hydrology of the Development Process Manual, Volume 2, Design Criteria, January 1993 edition. The 100-year 6-hour storm event was analyzed to determine street capacities using P(1 hr) = 2.00", P(6 hr)=2.30". The onsite Land Treatment values were determined using "Table A-5 Percent Treatment D" in the DPM. AHYMO printouts are provided in Appendix A. Two summary of hydrology tables are provided in the report.

III. EXISTING DRAINAGE CONDITIONS

This site was rough graded as part of the approved master grading plan for Vista del Norte Unit 1 several years ago. The site is relatively flat with a gentle fall towards the north.

IV. DEVELOPED DRAINAGE CONDITIONS

This site is part of the Vista del Norte Master Drainage Plan. A portion of the site will drain to the North Pond through an internal storm sewer that connects to the proposed storm sewer in Calle Comodo, (Vista del Norte subdivision Units 4, 5A and 5B) which discharges into the Vista del Norte North Pond. The remaining portion of the site will flow in a storm sewer that discharges into the Vista del Norte Middle Pond. The previous basin wide storm sewer analysis prepared as part of the Vista del Norte Subdivision (D-16/D-2)Drainage Plan was updated and is included in the Appendix of this report. Also the "Vista Del Norte Overview of Major Drainage Basins" and "Vista del Norte Subasin Boundaries and Street Flow" Exhibits from that report have been updated and revised to reflect this project's drainage plan. These exhibits are in the back pocket.

The northern project, Paseo de Estrella, is divided into three subasins. Runoff from two of the subasins flows to the adjacent streets, Vista del Norte Drive and Vista Monte Drive. At the Vista del Norte Drive entrance two inlets will intercept about half of the runoff reaching this point. At the Vista Monte Drive entrance, the onsite runoff will be intercepted by two inlets in Vista Monte Drive a short distance south of the entrance road under a sump inlet condition.

The southern project, Vista Estrella, is divided into three subasins. Runoff from all three subasins flows to a low area where it is intercepted by the required number of sump inlets. In each case the sump inlets were designed to intercept two times the 100 year peak discharge.

Runoff from the 4 acre park will be intercepted by one of three beehive grates located along the perimeter of the park as shown on the G&D plan and the "Subasin Boundary & Street Flow Exhibit".

The required offsite storm sewer in Vista Monte Drive, Vista del Norte Drive and Calle Comodo right-of-way is in the process of being designed for DRC submittal at the writing of this report.

VISTA DEL NORTE
To Middle Pond

TABLE 1A: SUMMARY OF HYDROLOGY

SUBASIN ID	AREA (acres)	# Lots	N value	Land Treat. D	Land Treat.B/C	Q(100) cfs	Runoff Volume (Ac.Ft.)
TRACT B Units 1A,1B,2							
B1	4.46390	15	3.4	37	31.5	15.02	0.540
B2	6.38110	27	4.2	44	28.0	22.36	0.827
B3	2.23313	9	4.0	42	29.0	7.75	0.284
B4	3.04049	13	4.3	44	28.0	10.66	0.394
B5	1.76108	7	4.0	42	29.0	6.11	0.224
B6	3.90157	15	3.8	41	29.5	13.46	0.492
B7	1.82050	8	4.4	45	27.5	6.41	0.238
B8	3.72812	14	3.8	40	30.0	12.77	0.465
B9	3.55313	14	3.9	42	29.0	12.31	0.452
B10	1.07287	5	4.7	47	26.5	3.83	0.143
B11	3.28183	13	4.0	42	29.0	11.38	0.417
E1.1	3.60627	17	4.7	47	26.5	12.85	0.480
E1.3	5.09469	22	4.3	44	28.0	17.85	0.660
E2.2	2.61663	10	3.8	41	29.5	9.02	0.330
E3	3.99604	0	park	0	30/70	11.22	0.329
V4	1.36085	0	road	80	20.0	5.85	0.240
Los Prados Subd.	18.45000			55	22.5	68.57	2.637
SUB TOTAL	**35.23772**	**189**				**247.42**	**9.152**

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VISTA DEL NORTE

To North Pond

TABLE 1B: SUMMARY OF HYDROLOGY

SUBASIN ID	AREA (acres)	# Lots	N value	Land Treat. D	Land Treat.B/C	Q(100) cfs	Runoff Volume (Ac.Ft.)
TRACT D Units 4A,4B,5A,5B							
D1	4.18410	22	5.3	51	24.5	15.24	0.578
D2	4.51190	26	5.8	55	22.5	16.78	0.645
D3	1.61074	7	4.3	44	28.0	5.66	0.209
D4	2.60643	11	4.2	44	28.0	9.14	0.338
D5	4.48173	24	5.4	52	24.0	16.41	0.624
D6	4.69405	26	5.5	53	23.5	17.27	0.659
D7	6.86337	39	5.7	55	22.5	25.51	0.980
D8	1.26515	5	4.0	42	29.0	4.40	0.161
D9	5.19326	25	4.8	48	26.0	18.60	0.698
D10	2.12068	10	4.7	47	26.5	7.56	0.283
D11	2.94873	15	5.1	50	25.0	10.68	0.404
D12	1.07444	5	4.7	47	26.5	3.84	0.143
D13	2.00810	11	5.5	53	23.5	7.40	0.282
D14	2.83001	12	4.2	44	28.0	9.92	0.367
D15	3.23216	16	5.0	49	25.5	11.65	0.438
D16	1.36359	8	5.9	56	22.0	5.11	0.197
V3	1.91705	0		80	20.0	8.23	0.338
E2.1	4.47180	18	4.0	42	29.0	15.50	0.569
V2	1.68459	0		80	20.0	7.24	0.297
V1	3.25785	0		78	22.0	13.88	0.567
E1.2	9.60630	47	4.9	49	25.5	34.59	1.303
E2.3	10.46263	50	4.8	48	26.0	37.46	1.407
Bernardo Trails BT1	4.46080	26		60	40/0	16.27	0.641
Bernardo Trails BT2	6.65600	44		60	40/0	24.27	0.956
Bernardo Trails BT3	13.68320	92		60	40/0	49.87	1.966
AMAFCA slope BT4A	1.16992			0	0/100	3.59	0.108
AMAFCA slope BT5A	3.57760			0	0/100	10.98	0.329
AMAFCA slope BT6A	2.09380			0	0/100	6.42	0.193
TRACT N	5.05000			90	10.0	22.01	0.929
TRACT T-1 (*)	11.50000			58	42.5/0	20.50	1.652

TRACT T-4-A	21.43000			90	10.0	46.52	3.942
TRACT O-1	3.82000				0/100	11.73	0.352
TRACT O-2 (*)	3.66860			90	10/O	8.00	0.675
TRACT P (*)	23.98000			90	10.0	52.00	4.411
LL	4.73000			80	0/20.4	19.45	0.804
SUB TOTAL	**188.20858**	**539**				**593.68**	**28.445**

PASEO DE ESTRELLA AND VISTA ESTRELLA

TABLE 2 SUMMARY OF STREET CAPACITIES

LOCATION	CURB	CROWN	WIDTH ft.	SLOPE %	Q cfs	DEPTH ft	EG ft.
Diamondback/Sidewinder	Mtb	Y	28' FF	0.006	9.0	0.33	0.39
Diamondback/Sidewinder	Mtb	Y	28' FF	0.0066	10.0	0.33	0.40
Sidewinder	Std	Y	28' FF	0.006	19.85	0.48	0.59
Diamondback	Std	Y	28'FF	0.006	25.11	0.52	0.66

Mtb = Mountable Curb
Std = Standard Curb

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AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2002
INPUT FILE = VDN_N&MP.DAT USER NO.= M_GOODWN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.600
COMPUTE NM HYD	100.D12	-	1	.00324	7.43	.278	1.61327	1.500	3.587	PER IMP= 48.00
COMPUTE NM HYD	100.D13	-	2	.00485	11.31	.429	1.65683	1.500	3.642	PER IMP= 51.00
ADD HYD	101.10	1& 2	3	.00809	18.74	.707	1.63933	1.500	3.620	
COMPUTE NM HYD	100.D11	-	1	.00461	10.68	.404	1.64231	1.500	3.623	PER IMP= 50.00
ADD HYD	101.10	1& 3	2	.01270	29.42	1.111	1.64039	1.500	3.621	
COMPUTE NM HYD	100.D10	-	1	.00331	7.56	.283	1.59875	1.500	3.567	PER IMP= 47.00
ADD HYD	101.10	1& 2	3	.01601	36.99	1.393	1.63175	1.500	3.610	
COMPUTE NM HYD	100.D9	-	1	.01130	26.18	.990	1.64231	1.500	3.620	PER IMP= 50.00
ADD HYD	101.10	1& 3	2	.02731	63.17	2.383	1.63611	1.500	3.614	
COMPUTE NM HYD	100.D7	-	1	.00898	21.38	.822	1.71492	1.500	3.718	PER IMP= 55.00
ADD HYD	101.10	1& 2	3	.03629	84.55	3.205	1.65561	1.500	3.640	
COMPUTE NM HYD	100.D8	-	1	.00198	4.40	.161	1.52615	1.500	3.474	PER IMP= 42.00
ADD HYD	101.10	1& 3	40	.03827	88.94	3.365	1.64892	1.500	3.631	
COMPUTE NM HYD	100.D16	-	1	.00219	5.21	.200	1.71492	1.500	3.727	PER IMP= 55.00
COMPUTE NM HYD	100.V3	-	2	.00335	9.19	.377	2.11559	1.500	4.294	PER IMP= 80.00
ADD HYD	101.10	1& 2	3	.00553	14.41	.577	1.95712	1.500	4.070	
COMPUTE NM HYD	100.D14	-	1	.00420	9.48	.352	1.56971	1.500	3.526	PER IMP= 45.00
ADD HYD	101.10	1& 3	2	.00973	23.89	.929	1.78983	1.500	3.835	
COMPUTE NM HYD	100.D15	-	1	.00212	4.80	.178	1.56971	1.500	3.532	PER IMP= 45.00
COMPUTE NM HYD	100.E21	-	3	.00696	15.36	.561	1.51163	1.500	3.446	PER IMP= 41.00
ADD HYD	101.10	1& 3	4	.00909	20.16	.739	1.52513	1.500	3.466	
ADD HYD	101.10	2& 4	3	.01882	44.05	1.668	1.66203	1.500	3.657	
COMPUTE NM HYD	100.V2	-	1	.00263	7.24	.297	2.11559	1.500	4.297	PER IMP= 80.00
ADD HYD	101.10	1& 3	2	.02145	51.28	1.965	1.71767	1.500	3.735	
COMPUTE NM HYD	100.V1	-	1	.00509	13.88	.567	2.09032	1.500	4.261	PER IMP= 78.00
COMPUTE NM HYD	100.E12	-	3	.01493	34.02	1.273	1.59875	1.500	3.561	PER IMP= 47.00
ADD HYD	101.10	1& 3	4	.02002	47.90	1.840	1.72372	1.500	3.739	
ADD HYD	101.10	2& 4	3	.04147	99.19	3.805	1.72059	1.500	3.737	
COMPUTE NM HYD	100.E23	-	1	.01647	37.33	1.392	1.58423	1.500	3.541	PER IMP= 46.00
ADD HYD	101.10	1& 3	2	.05794	136.52	5.197	1.68182	1.500	3.682	
COMPUTE NM HYD	100.D4	-	1	.00389	8.73	.323	1.55519	1.500	3.508	PER IMP= 44.00
ADD HYD	101.10	1& 2	3	.06183	145.25	5.520	1.67385	1.500	3.671	
COMPUTE NM HYD	100.D6	-	1	.00733	17.27	.659	1.68588	1.500	3.680	PER IMP= 53.00
ADD HYD	101.10	1& 3	2	.06916	162.52	6.179	1.67512	1.500	3.672	
COMPUTE NM HYD	100.D5	-	1	.00700	16.41	.624	1.67136	1.500	3.661	PER IMP= 52.00
ADD HYD	101.10	1& 2	3	.07617	178.93	6.803	1.67477	1.500	3.671	
ADD HYD	101.10	40& 3	41	.11443	267.87	10.169	1.66612	1.500	3.658	
COMPUTE NM HYD	100.D2	-	1	.00705	16.78	.645	1.71492	1.500	3.719	PER IMP= 55.00
COMPUTE NM HYD	100.D3	-	2	.00288	6.47	.239	1.55519	1.500	3.510	PER IMP= 44.00
ADD HYD	101.10	1& 2	3	.00993	23.25	.884	1.66854	1.500	3.658	
COMPUTE NM HYD	100.D1	-	1	.00654	15.24	.578	1.65683	1.500	3.641	PER IMP= 51.00
ADD HYD	101.10	1& 3	2	.01647	38.48	1.461	1.66387	1.500	3.651	

ADD HYD	101.10	41& 2	43	.13090	306.35	11.630	1.66584	1.500	3.657			
COMPUTE NM HYD	100.BT1	-	1	.00697	16.27	.641	1.72413	1.500	3.647	PER IMP=	60.00	
COMPUTE NM HYD	100.BT4A	-	2	.00183	3.59	.108	1.10462	1.500	3.073	PER IMP=	.00	
ADD HYD	101.10	1& 2	3	.00880	19.86	.749	1.59535	1.500	3.528			
COMPUTE NM HYD	100.TN	-	1	.00789	22.01	.929	2.20728	1.500	4.358	PER IMP=	90.00	
ADD HYD	101.10	1& 3	2	.01669	41.87	1.677	1.88466	1.500	3.921			
COMPUTE NM HYD	100.BT2	-	1	.01040	24.27	.956	1.72413	1.500	3.646	PER IMP=	60.00	
ADD HYD	101.10	1& 2	3	.02709	66.14	2.634	1.82301	1.500	3.815			
COMPUTE NM HYD	100.BT3	-	1	.02138	49.87	1.966	1.72413	1.500	3.645	PER IMP=	60.00	
COMPUTE NM HYD	100.BT5A	-	4	.00559	10.98	.329	1.10462	1.500	3.068	PER IMP=	.00	
ADD HYD	101.10	1& 4	2	.02697	60.85	2.295	1.59570	1.500	3.525			
COMPUTE NM HYD	100.BT6A	-	1	.00327	6.42	.193	1.10462	1.500	3.070	PER IMP=	.00	
ADD HYD	101.10	1& 2	4	.03024	67.27	2.488	1.54260	1.500	3.476			
ADD HYD	101.10	3& 4	44	.05733	133.41	5.122	1.67509	1.500	3.636			
COMPUTE NM HYD	100.T1	-	1	.01797	41.92	1.652	1.72413	1.500	3.645	PER IMP=	60.00	
ROUTE RESERVOIR	POND.T1	1	20	.01797	20.51	1.652	1.72411	1.700	1.784	AC-FT=	.408	
COMPUTE NM HYD	100.P	-	1	.03747	104.45	4.411	2.20728	1.500	4.356	PER IMP=	90.00	
ROUTE RESERVOIR	POND.P	1	21	.03747	52.02	4.411	2.20727	1.650	2.169	AC-FT=	.844	
COMPUTE NM HYD	100.T4A	-	1	.03348	93.35	3.942	2.20728	1.500	4.356	PER IMP=	90.00	
ROUTE RESERVOIR	POND.T4A	1	22	.03348	46.52	3.942	2.20727	1.650	2.171	AC-FT=	.769	
ADD HYD	101.10	20&21	30	.05544	72.53	6.063	2.05065	1.700	2.044			
ADD HYD	101.10	30&22	31	.08892	119.05	10.005	2.10963	1.700	2.092			
COMPUTE NM HYD	100.02	-	1	.00573	15.99	.675	2.20728	1.500	4.359	PER IMP=	90.00	
ROUTE RESERVOIR	POND.O2	1	23	.00573	8.00	.675	2.20722	1.650	2.181	AC-FT=	.201	
ADD HYD	101.10	31&23	32	.09465	127.05	10.680	2.11553	1.650	2.097			
COMPUTE NM HYD	100.LL	-	1	.00739	19.45	.804	2.03979	1.500	4.112	PER IMP=	79.60	
ADD HYD	101.10	32& 1	33	.10204	143.89	11.484	2.11004	1.500	2.203			
COMPUTE NM HYD	100.O1	-	1	.00597	11.73	.352	1.10462	1.500	3.068	PER IMP=	.00	
ADD HYD	101.10	33& 1	34	.10802	155.62	11.836	2.05444	1.500	2.251			
ADD HYD	101.10	34&43	35	.23892	461.97	23.465	1.84153	1.500	3.021			
ADD HYD	101.10	35&44	36	.29625	595.38	28.587	1.80932	1.500	3.140			
ROUTE RESERVOIR	NORTH.POND	36	12	.29625	4.14	9.361	.59250	3.350	.022	AC-FT=	24.345	
COMPUTE NM HYD	100.B1	-	1	.00697	15.02	.540	1.45355	1.500	3.368	PER IMP=	37.00	
COMPUTE NM HYD	100.B2	-	2	.00997	22.36	.827	1.55519	1.500	3.504	PER IMP=	44.00	
ADD HYD	101.10	1& 2	3	.01694	37.38	1.367	1.51334	1.500	3.448			
COMPUTE NM HYD	100.B3	-	1	.00349	7.75	.284	1.52615	1.500	3.469	PER IMP=	42.00	
ADD HYD	101.10	1& 3	2	.02043	45.13	1.651	1.51551	1.500	3.452			
COMPUTE NM HYD	100.B4	-	1	.00475	10.66	.394	1.55519	1.500	3.506	PER IMP=	44.00	
ADD HYD	101.10	1& 2	3	.02518	55.79	2.045	1.52298	1.500	3.462			
COMPUTE NM HYD	100.B5	-	1	.00275	6.11	.224	1.52615	1.500	3.472	PER IMP=	42.00	
ADD HYD	101.10	1& 3	2	.02793	61.90	2.269	1.52328	1.500	3.463			
COMPUTE NM HYD	100.B6	-	1	.00610	13.46	.492	1.51163	1.500	3.446	PER IMP=	41.00	
ADD HYD	101.10	1& 2	3	.03403	75.35	2.761	1.52119	1.500	3.460			
COMPUTE NM HYD	100.B7	-	1	.00284	6.41	.238	1.56971	1.500	3.529	PER IMP=	45.00	
ADD HYD	101.10	1& 3	2	.03687	81.77	2.999	1.52491	1.500	3.465			
COMPUTE NM HYD	100.B8	-	1	.00582	12.77	.465	1.49711	1.500	3.427	PER IMP=	40.00	
ADD HYD	101.10	1& 2	3	.04269	94.53	3.463	1.52112	1.500	3.460			
COMPUTE NM HYD	100.B9	-	1	.00555	12.31	.452	1.52615	1.500	3.467	PER IMP=	42.00	
ADD HYD	101.10	1& 3	2	.04824	106.85	3.915	1.52169	1.500	3.461			
COMPUTE NM HYD	100.B10	-	1	.00168	3.83	.143	1.59875	1.500	3.574	PER IMP=	47.00	
ADD HYD	101.10	1& 2	3	.04992	110.68	4.058	1.52427	1.500	3.465			

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COMPUTE NM HYD       100.B11    -      1       .00320      6.95      .251     1.46807   1.500    3.392 PER IMP=  38.00
ADD HYD              101.10   1& 3     2       .05312    117.63     4.309     1.52088   1.500    3.460
COMPUTE NM HYD       100.E11    -      1       .00564     13.00      .489     1.62779   1.500    3.603 PER IMP=  49.00
ADD HYD              101.10   1& 2     3       .05876    130.64     4.798     1.53113   1.500    3.474
COMPUTE NM HYD       100.E13    -      1       .00814     18.37      .682     1.56971   1.500    3.524 PER IMP=  45.00
ADD HYD              101.10   1& 3     2       .06690    149.00     5.480     1.53582   1.500    3.480
COMPUTE NM HYD       100.E22    -      1       .00396      8.79      .322     1.52615   1.500    3.468 PER IMP=  42.00
ADD HYD              101.10   1& 2     3       .07086    157.80     5.802     1.53528   1.500    3.479
COMPUTE NM HYD       100.E3     -      1       .00624     11.22      .329      .98689   1.500    2.808 PER IMP=    .00
ADD HYD              101.10   1& 3     2       .07711    169.02     6.131     1.49087   1.500    3.425
COMPUTE NM HYD       100.V4     -      1       .00213      5.85      .240     2.11559   1.500    4.299 PER IMP=  80.00
ADD HYD              101.10   1& 2     3       .07923    174.87     6.371     1.50763   1.500    3.448
COMPUTE NM HYD       100.00     -      1       .02883     68.57     2.637     1.71492   1.500    3.716 PER IMP=  55.00
ADD HYD              101.10   1& 3    43       .10806    243.43     9.008     1.56293   1.500    3.520
ROUTE RESERVOIR     POND.100    43     13       .10806     14.62     9.006     1.56267   2.250     .211 AC-FT=    6.654
COMPUTE NM HYD         100.Q    -      1       .00373     10.42      .440     2.20728   1.500    4.362 PER IMP=  90.00
COMPUTE NM HYD         100.R    -      1       .00309      8.64      .364     2.20728   1.500    4.363 PER IMP=  90.00
COMPUTE NM HYD         100.S    -      1       .00088      2.45      .103     2.20728   1.500    4.382 PER IMP=  90.00
FINISH
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Paseo de Estrella and Vista Estrella Subdivision
Vista del Norte

CALCULATIONS FOR SUMP INLETS

Capacity is measured by the weir equation at the lip of the gutter assuming an allowable ponding elevation equal to the lowest adjacent right of way elevation. The length of the double grate facing the street is 6.0' and the maximum depth is 0.722' at the lip of the gutter. The sides are each 2' long and the average depth is 0.899'. These depths assume an 8" curb with right of way 9' behind the curb for an additional depth of 0.18' above the top of curb. From the weir equation:

For Double 'C' Inlet

*Front Q cap = (3.0) x (6.0') x (0.722)**1.5 = 11.04 cfs*

*Sides Q cap = (3.0) x (4.0') x (0.899) **1.5 = 10.23 cfs*

Total Q cap = 11.04 cfs + 10.23 cfs = 21.27 cfs (per double inlet)

For Single 'C' Inlet

*Front Q cap = (3.0) x (3.00') x (0.722) **1.5 = 5.52 cfs*

*Sides Q cap = (3.0) x (4.0') x (0.899) **1.5 = 10.22 cfs*

Total Q cap = 16.56 cfs + 10.23 cfs = 15.74 cfs (per single inlet)

When there is no overflow to the onsite retention pond the sump inlets are designed for approximately 2 times the 100 year peak flow.

Paseo de Estrella Subdivision

Peak Q(100year) = 37.33 cfs

Use (2) Double 'A' inlets and (2) Single 'A' inlets (Intercepts 74 cfs)

Vista Estrella Subdivision

Peak Q (100 year) = 34.02 cfs

Use (1) Double 'A' inlets and (3) Single 'A' inlets (intrecepts 68.5 cfs)