CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

March 16, 2022

Ronald Bohannan, P.E. Tierra West, LLC 5571 Midway Park Place NE Albuquerque, NM 87109

RE: Rutledge Office/Warehouse 9721 Rutledge Rd NE Grading Plans and Drainage Report Engineer's Stamp Date: 03/07/22 Hydrology File: D17D003AA7

Dear Mr. Bohannan:

PO Box 1293 Based upon the information provided in your submittal received 12/17/2021, the Grading Plans and Drainage Report are approved for Building Permit, Grading Permit, and for action by the DRB on Platting and Site Plan for Building Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter.

Albuquerque

www.cabq.gov

PRIOR TO CERTIFICATE OF OCCUPANCY:

 NM 87103
 Engineer's Certification, per the DPM Part 6-14 (F): Engineer's Certification Checklist For Non-Subdivision is required.

2. AMAFCA is requiring a Maintenance and Encroachment License for maintenance of the swale and concrete rundown in the AMAFCA NDC ROW. This must be executed and recorded with the County. Hydrology will need email confirmation of this from AMAFCA.

3. Please provide the Drainage Covenant with Exhibit A for the stormwater quality ponds per Article 6-15(C) of the DPM prior to Permanent Release of Occupancy. Please submit the original copies along with the \$25.00 recording fee check made payable to Bernalillo County to Marion G. Velasquez (<u>mgvelasquez@cabq.gov</u>) on the 4th floor of Plaza de Sol. Please note that Hydrology will need a pdf copy of the recorded Drainage Covenant prior to Hydrology's approval of Permanent Release of Occupancy.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, <u>jhughes@cabq.gov</u>, 924-3420) 14 days prior to any earth disturbance.

CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



Mayor Timothy M. Keller

If you have any questions, please contact me at 924-3995 or <u>rbrissette@cabq.gov</u>.

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology Planning Department

PO Box 1293

Albuquerque

NM 87103

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	Planning De	-	
Developm	nent & Buildin	ng Services Division	
DRAINAGE ANI	O TRANSPOR	TATION INFORMATION SHEET (REV 6/2018)	
Project Title: TRACT A-2-A & A-3-A JOURNAL CENTER 2	_Building Perr	nit #: Hydrology File #:	
		Work Order#:	
City Address: <u>RUTLEDGE ST NE ALBUQUE</u>		AL CENTER 2 UNIT 2 (EXCEL PORTION OUT TO R/W) CONT 2.7385 +/- AC	
Applicant:_TIERRA WEST, LLS		Contact: VINCE CARRIC	A
Address: 5571 MIDWAY PARK PLACE NE,	ALBUQUERQU	E, NM 87109	
		E-mail: VCARRICA@TIERRAWEST	LLC.CON
		Contact: ANGELO BRUN	
Address:			
Phone#: (505) 833-2928	Fax#:	E-mail:	NACIN
Chast all that Apply		ROLOGY/DRAINAGE	т.
Check all that Apply:		TYPE OF APPROVAL/ACCEPTANCE SOUGH BUILDING PERMIT APPROVAL	T:
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FEE PAID:

DRAINAGE REPORT

For

RUTLEDGE OFFICE/WAREHOUSE DEVELOPMENT 9721 RUTLEDGE ROAD, NE ALBUQUERQUE, NEW MEXICO

Prepared by

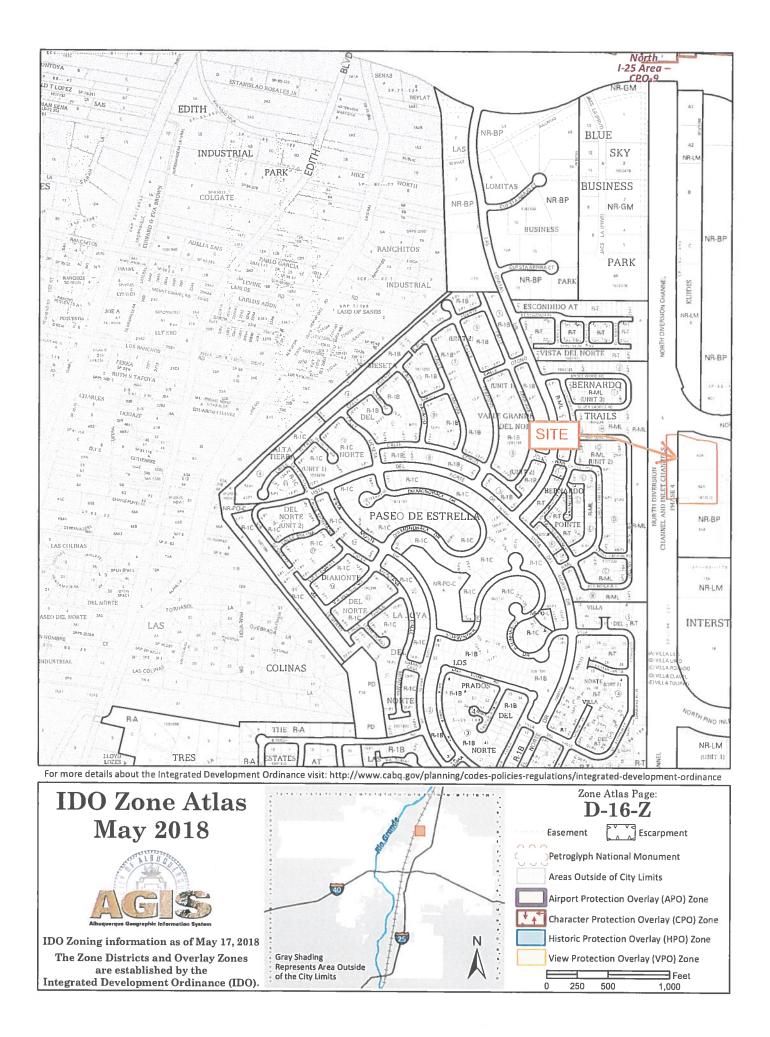
Tierra West, LLC 5571 Midway Park Place NE Albuquerque, New Mexico 87109

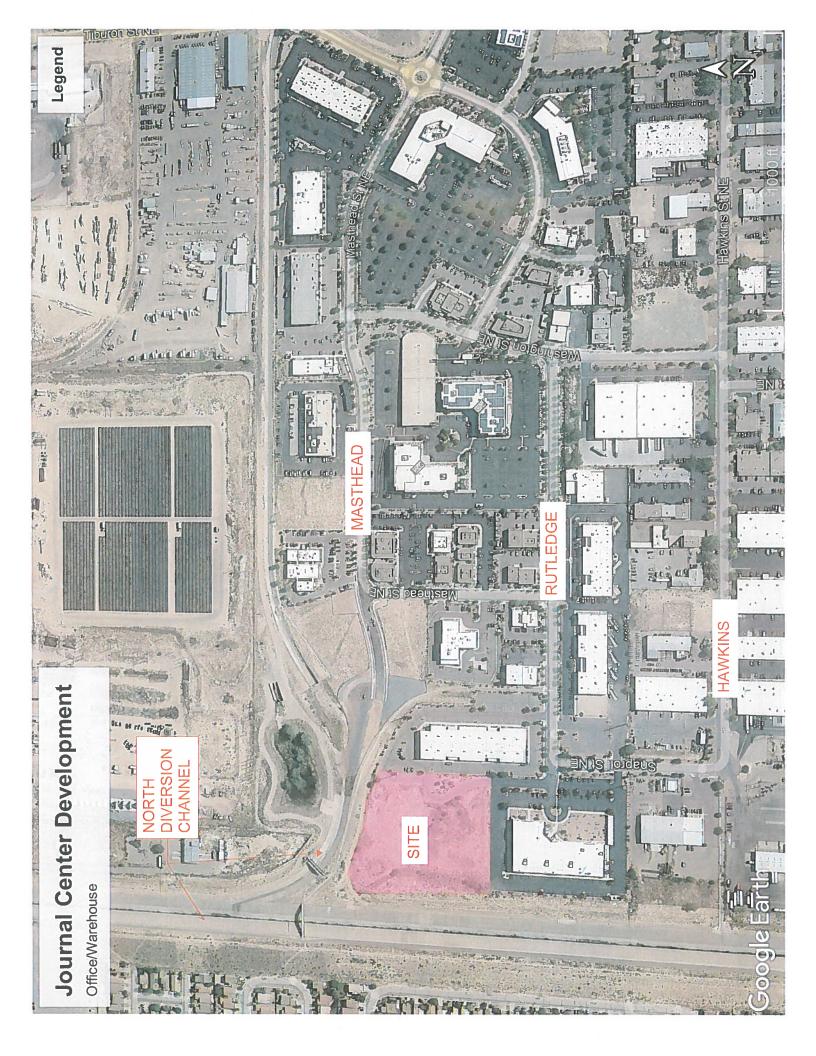
Prepared for	
Brunacini Developmer Albuquerque, NM	City of Albuquerque Planning Department Development Review Services HYDROLOGY SECTION APPROVED 03/16/22 BY: BY: HydroTrans #D17D003AA7
February 18, 2021	THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY OF ALBUQUERQUE FROM REQUIRING CORRECTION, OR ERROR OR DIMENSIONS IN PLANS, SPECIFICATIONS, OR CONSTRUCTIONS. SUCH APPROVED PLANS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.
ARLIN MET CONTRACTOR	
02/18/202	1

RONALD R BOHANNAN, PE #7868

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LOCATION

The proposed office / warehouse development is located off the west end of Rutledge Road and Masthead Road in the Journal Center in Albuquerque. It is comprised of approximately 4.7453 acres zoned NRBP. This report represents a drainage management and grading plan for approval by the City of Albuquerque, for grading and Building Permit submittal.

DRAINAGE BASIN DESIGNATION

The drainage basins for proposed conditions are as indicated on the BASIN MAP included in this report. The site is broken into 19 onsite drainage basins (including 4 first flush ponds) and three offsite upland basins.

EXISTING DRAINGE CONDITIONS

The site is currently vacant with the exception of an existing outdoor patio area in the middle portion along the east property line of the site. The site drains predominantly southeast to northwest. Runoff from the existing site is conveyed to the existing swale in AMAFCA right of way where it drains to the north to an existing surface rundown into the North Diversion Channel.

FIRM MAP

The site is not located in a designated flood plain as shown on the attached Flood Hazard Zone Map No. 35001C0136G dated 9/26/2008.

DESIGN-CRITERIA

The drainage plan presented in this report was prepared in accordance with the City of Albuquerque Drainage Ordinances and Chapter 6 of the Development Process Manual DPM. The hydrological analysis is based on the 100-year frequency, 24-hour duration storm, as Represented in Article 6-2(A), Hydrology, of the Development Process Manual. The plan will also include retention of the storm water quality in

National Flood Hazard Layer FIRMette

N"201°35 W"55°201



Legend



0.2% Annual Chance Flood Hazard, Areas depth less than one foot or with drainage of 1% annual chance flood with average areas of less than one square mile Zone X

Area with Reduced Flood Risk due to Future Conditions 1% Annual Chance Flood Hazard Zone X Levee. See Notes. Zone X

Area with Flood Risk due to Levee Zone D

OTHER AREAS OF FLOOD HAZARD

NO SCREEN Area of Minimal Flood Hazard Zone Effective LOMRs

Area of Undetermined Flood Hazard Zone D OTHER AREAS

Zone A

SITE

Channel, Culvert, or Storm Sewer GENERAL – – – – Channet, Cutvert, or Storn STRUCTURES | 1 1 1 1 1 1 Levee, Dike, or Floodwall

Cross Sections with 1% Annual Chance Base Flood Elevation Line (BFE) **Coastal Transect Baseline** Water Surface Elevation **Jurisdiction Boundary** Hydrographic Feature Digital Data Available **Coastal Transect Profile Baseline** Limit of Study (B) 20.2 17.5 mun []] mun I OTHER FEATURES

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

No Digital Data Available

Unmapped

0

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

authoritative NFHL web services provided by FEMA. This map reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or The flood hazard information is derived directly from the 31 PM and does not become superseded by new data over time. was exported on 12

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



proposed on-site landscaped areas / storm water detention ponds. See attached Weighted E Table for excess precipitation values calculated for this site.

DEVELOPED-DRAINAGE CONDITIONS

The site is proposed to be developed as a single story 62,045 SF office / warehouse development with associated parking, drive aisles, landscaping and storm drainage first flush ponds.

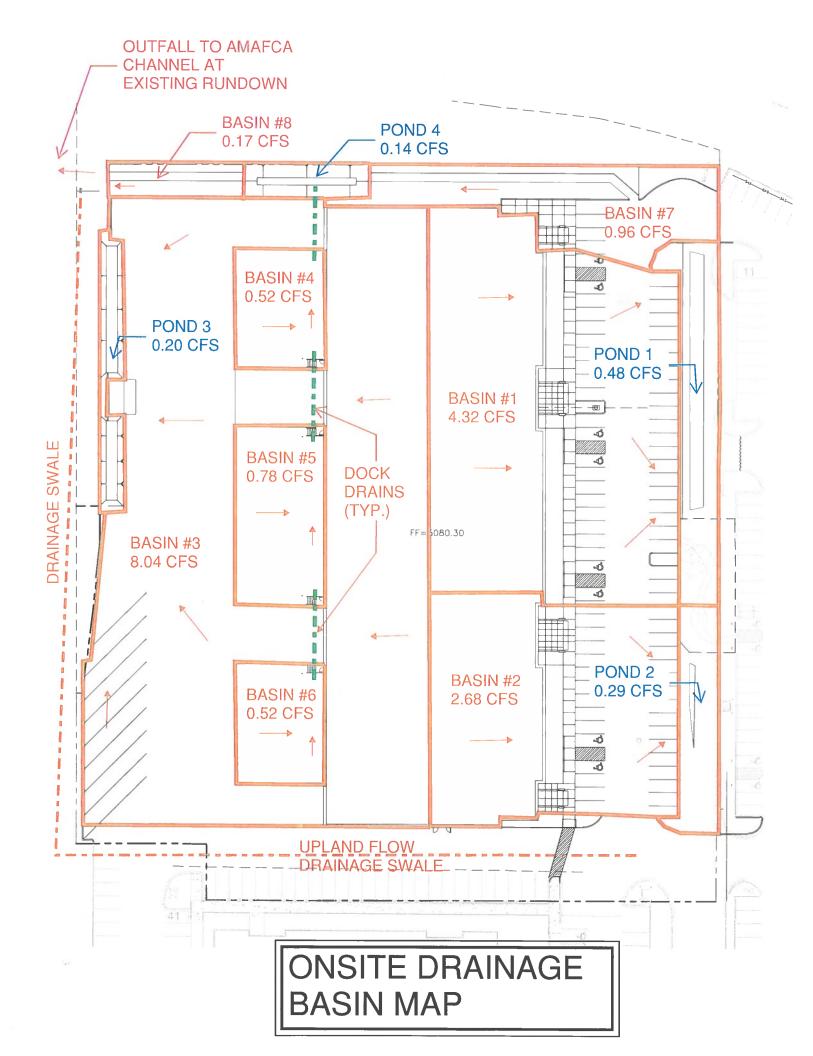
The runoff from the site is proposed to drain to onsite first flush ponds that will retain the required first flush volume and release remaining flows to the adjacent North Diversion Channel. Refer to enclosed Weighted E computation spreadsheet for developed conditions.

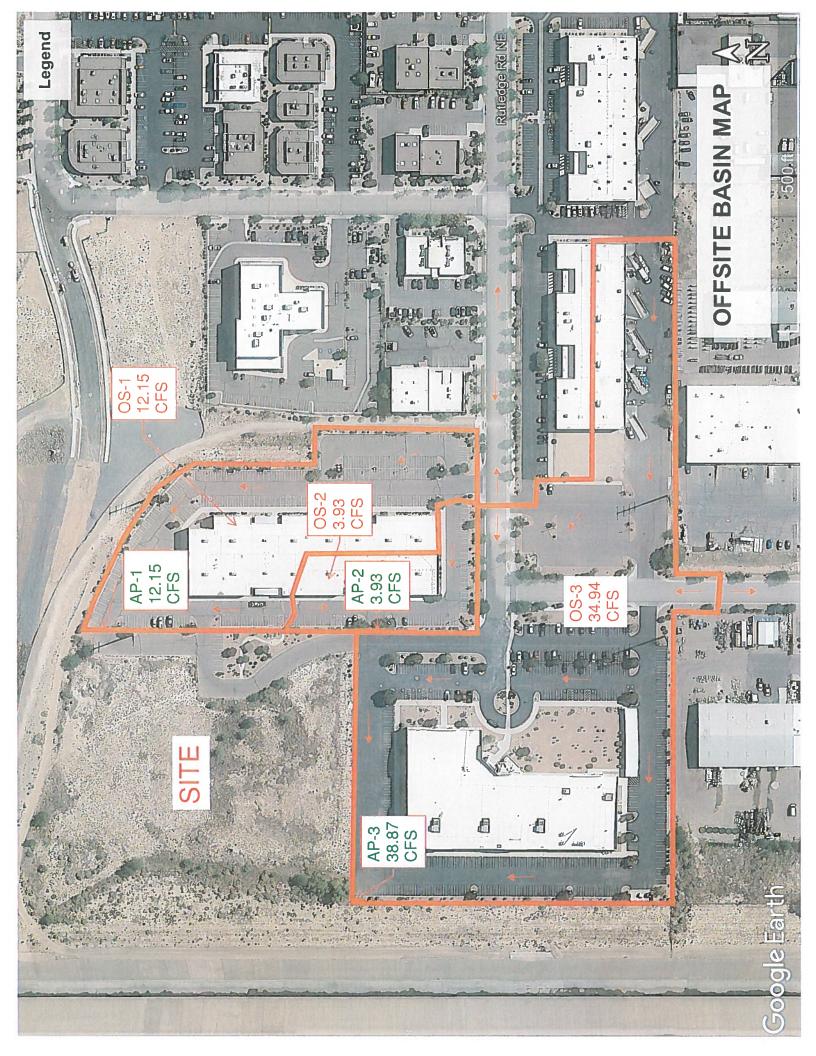
Runoff from three upland basins (currently developed) will either be routed through the subject site or be routed in existing paved swales west to the North Diversion Channel Road right of way. The upland flows from Basin OS-1 will be routed through the subject site to the northwest corner where it will be intercepted by a proposed concrete channel to the existing North Diversion Channel concrete rundown. Runoff from upland basins OS-2 and OS-3 will continue to be routed in existing paved drainage swale located in drive aisles and roadways of existing developed sites adjacent to the subject site. The upland flows will be routed via a proposed earthen drainage swale within the North Diversion Channel Road right of way and intercepted by the proposed concrete channel noted above, and to the existing North Diversion Channel concrete rundown. The earthen swale location will coincide with the proposed roadside swale noted in the attached 95% preliminary North Diversion Channel Roadway Plans prepared by TY-LIN International and provided by COA Department of Municipal Development. See attached Sheet 6 of 103 Channel Road Phase 2 Roadway Typical Sections.

SUMMARY

The proposed grading and drainage plan for the proposed development of the existing undeveloped Tracts A-2-A and A-3-A property includes surface flows of runoff to onsite water quality and ponds. The ponds will retain first flush volumes for the

developed portion of the lot and the pond will exit the site to the west where it will follow historic drainage path to the North Diversion Channel rundown via a proposed concrete rundown. Runoff from upland basins will also be routed to the proposed concrete channel which is proposed to be connected to the existing concrete rundown on the North Diversion Channel. The storm drain capacity downstream of the site is sufficient to carry the historic runoff.





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Zone #2 Onsite Developed Basins

	Flow	cfs	4.32	2.68	8.04	0.52	0.78	0.52	0.96	0.17	0.48	0.29	0.20	0.14	19.11	
100-Year	Volume	(ac-ft)	0.161	0.100	0.302	0.020	0.029	0.020	0.034	0.006	0.014	0.009	0.006	0.004	 0.704	
	Weighted E	(ac-ft)	2.051	2.080	2.120	2.120	2.120	2.120	1.675	1.457	1.130	1.130	1.130	1.130		
	Treatment D	(acres)	0.875	0.554	1.710	0.111	0.165	0.111	0.133	0.015	0.000	0.000	0.000	0.000		
	Treatn	%	93%	96%	100%	100%	100%	100%	55%	33%	%0	%0	%0	%0	 	_
	Treatment C	(acres)	0.065897	0.023083	0	0	0	0	0.108533	0.031439	0.153903	0.093549	0.064509	0.044536		
	Treati	%	%2	4%	%0	%0	%0	%0	45%	67%	100%	100%	100%	100%		
	Treatment B	(acres)	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		
	Treatr	%	%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	 	
	Freatment A	(acres)	0	0	0	0	0	0	0	0	0	0	0	0		
	Treatr	%	%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	%0	%0		
	Area	(sq miles)	0.00147	06000.0	0.00267	0.00017	0.00026	0.00017	0.00038	0.00007	0.00024	0.00015	0.00010	0.00007	0.00666	
	Area	(acres)	0.941	0.577	1.710	0.111	0.165	0.111	0.241	0.047	0.154	0.094	0.065	0.045	4.260	
	Area	(sf)	41007.00	25137.00	74496.00	4820.00	7200.00	4820.00	10506.00	2044.00	6704.00	4075.00	2810.00	1940.00	185559.00	
	Basin		-	2	m	4	5	9	2	ø	Pond 1	Pond 2	Pond 3	Pond 4	Total	

Equations:

Pond Capacity (Cu.Ft.) 6,717 1,673 2,749 2,749

 1st Flush

 POND No.
 Vol. (Cu.Ft.)

 1
 1,303

 2
 824

 3
 2,545

 4
 773

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

Weighted E Method

Zone #2 Offsite Developed Basins

													100-Year	
Basin	Area	Area	Area	Treat	reatment A	Treat	Freatment B	Treati	Freatment C	Treat	Freatment D	Weighted E	Volume	Flow
	(sf)	(acres)	(sq miles)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(ac-ft)	(ac-ft)	cfs
OS-1	122,778	2.819		%0	0	%0	0.000	25% (0.704649	75%	2.114	1.873	0.440	12.15
OS-2	39,686	0.911		%0	0	%0	0.000	25%	0.227766	75%	0.683	1.873	0.142	3.93
OS-3	350,569	8.048	0.01257	%0	0	%0	0.000	23%	1.85103	%17	6.197	1.892	1.269	34.94
Total	513,033	11.778	0.01840										1.851	51.01

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

Rutledge Office/Warehouse POND #1 Ab - Bottom Of The Pond Surface Area At - Top Of The Pond Surface Area D - Water Depth Dt - Total Pond Depth C - Change In Surface Area / Water Depth Volume = $Ab * D + 0.5 * C * D^2$ C = (At - Ab) / DtAb =1,585.00 B.O.P.= 5075.00 At = 3,328.00 T.O.P. = 5078.00 Dt = 3.00 C = 581.00 B Elev. =5,075.00

ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
5075.00	0	0	0.000
5075.50	0.50	0.0199	0.000
5076.00	1.00	0.0431	0.000
5076.50	1.50	0.0696	0.000
5077.00	2.00	0.0994	0.000
5077.80	2.80	0.1542	0.000

 $\frac{\text{Orifice Equation}}{Q = CA \text{ SQRT}(2gH)}$

C = 0.6Diameter (in) 10 Area (ft^2)= 0.545415391 g = 32.2H (Ft) = Depth of water above center of orifice Q (CFS)= Flow

Rutledge Office/Warehouse POND #2 Ab - Bottom Of The Pond Surface Area At - Top Of The Pond Surface Area D - Water Depth Dt - Total Pond Depth C - Change In Surface Area / Water Depth Volume = $Ab * D + 0.5 * C * D^2$ C = (At - Ab) / DtAb = 142.00 B.O.P.= 5076.00 At = 1,880.00 T.O.P. = 5078.50 Dt = 2.50 C = 695.20 B Elev. = 5,076.00

ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
5076.00	0	0	0.000
5076.50	0.50	0.0036	0.000
5077.00	1.00	0.0112	0.000
5077.50	1.50	0.0228	0.000
5078.00	2.00	0.0384	0.000

 $\frac{\text{Orifice Equation}}{Q = CA SQRT(2gH)}$

C =	0.6
Diameter (in)) 10
Area (ft^2)=	0.545415391
g =	32.2
H (Ft) =	Depth of water above center of orifice
Q (CFS)=	Flow

Rutledge Office/Warehouse

POND #3

- Ab Bottom Of The Pond Surface Area
- At Top Of The Pond Surface Area
- D Water Depth
- Dt Total Pond Depth
- C Change In Surface Area / Water Depth

Volume = $Ab * D + 0.5 * C * D^2$

Dt		
1,033.00	B.O.P.=	5073.50
2,634.00	T.O.P. =	5075.00
1.50		
1067.33		
5,073.50		
	1,033.00 2,634.00 1.50 1067.33	1,033.00 B.O.P.= 2,634.00 T.O.P. = 1.50 1067.33

ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
5073.50	0	0	0.000
5074.00	0.50	0.0149	0.000
5074.50	1.00	0.0360	0.000
5075.00	1.50	0.0631	0.000

 $\frac{\text{Orifice Equation}}{Q = CA \text{ SQRT(2gH)}}$

C =	0.6
Diameter (in)	10
Area (ft^2)=	0.545415391
g =	32.2
H (Ft) =	Depth of water above center of orifice
Q(CFS)=	Flow

Rutledge Office/Warehouse POND #4 Ab - Bottom Of The Pond Surface Area At - Top Of The Pond Surface Area D - Water Depth Dt - Total Pond Depth C - Change In Surface Area / Water Depth Volume = $Ab * D + 0.5 * C * D^2$ C = (At - Ab) / DtAb =350.00 D.C.A 1,500.00 T.O.P. = 350.00 B.O.P.= 5076.00 At = 5077.50 Dt = 1.50 766.67 C = B Elev. = 5,076.00

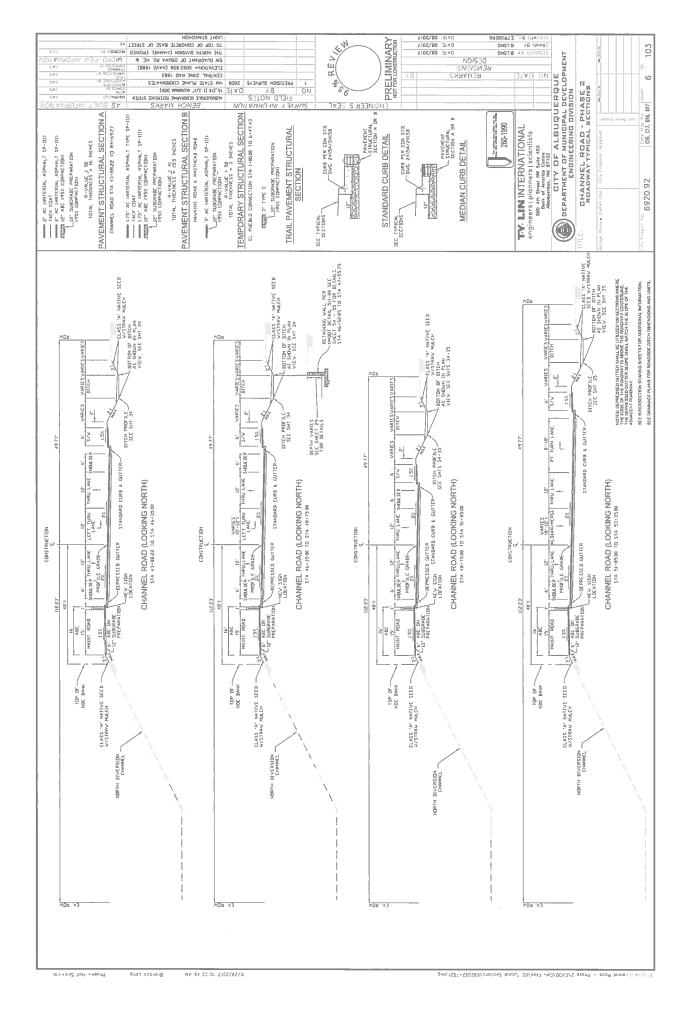
ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
5076.00	0	0	0.000
5076.50	0.50	0.0062	0.000
5077.00	1.00	0.0168	0.000
5077.50	1.50	0.0319	0.000

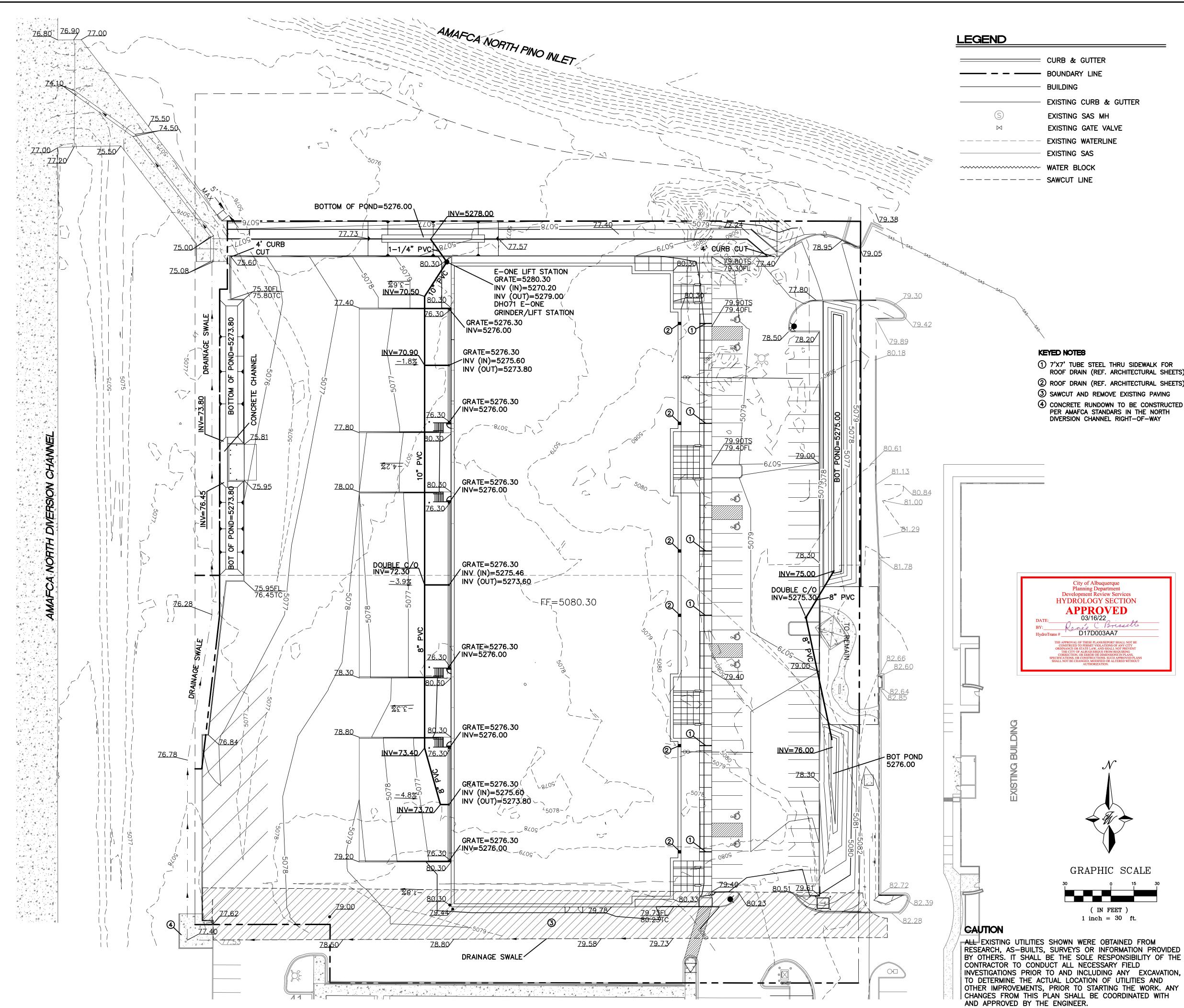
 $\frac{\text{Orifice Equation}}{Q = CA SQRT(2gH)}$

C =	0.6
Diameter (in)) 10
Area (ft^2)=	0.545415391
g =	32.2
H (Ft) =	Depth of water above center of orifice
Q (CFS)=	Flow

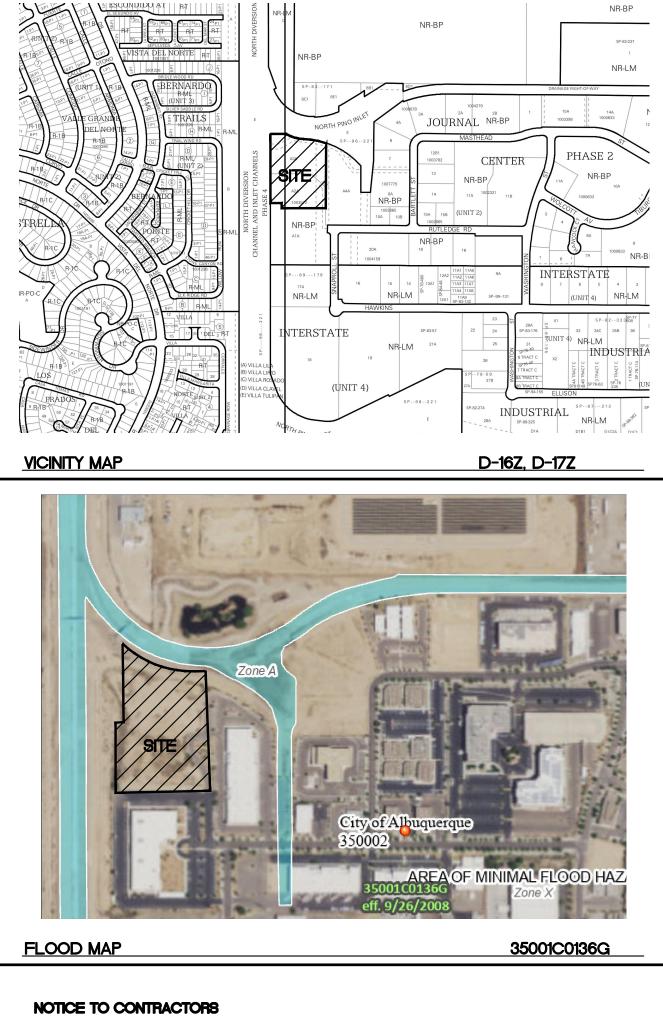
TIERRA WEST, LL	Project Rutlege offic/wAREH Project No Meeting Purpose Attendees C	
EARTHEN CHANNEL CALCS:		
L 6' L 2' L 6'		
	₫	
		-
5 = 0.50%		
N= 0.025 (EARTHEN)		
A= 16 ft 0=01.49	$\left(\frac{A}{P}\right)^{2/3} - \sqrt{5} = (16) 1.49 (16)^{2/3}$ 0.025 (14.66)	7.48cfs
P= 14.66ft M.M	(P) 0.025 (14.66)	70.005
V= 4.479	t/sec	
Θ_{-} (ΔP_{z}) - 30	3.87cfs & QCAPACITY = 7	110-0-1
~ R2Q (MI-5 J - J C). OICTS = UCAPACITY = 1	1.40675 1
• • • • • • • • • • • • • • • • • • •		







ROOF DRAIN (REF. ARCHITECTURAL SHEETS) 2 ROOF DRAIN (REF. ARCHITECTURAL SHEETS) (4) CONCRETE RUNDOWN TO BE CONSTRUCTED PER AMAFCA STANDARS IN THE NORTH

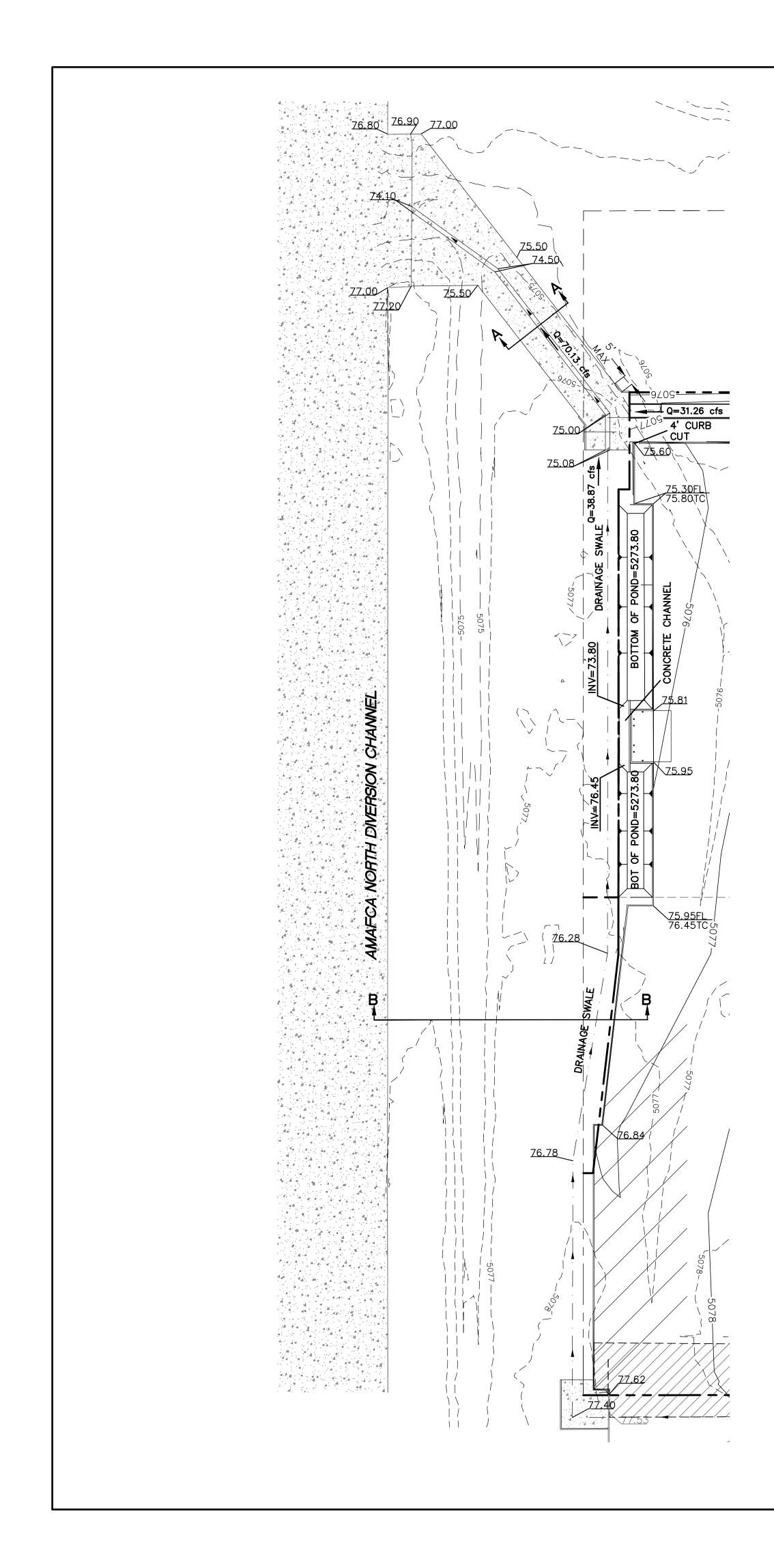


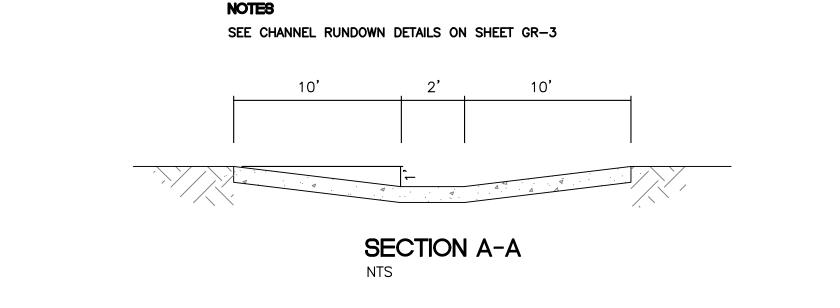
- 1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY.
- 2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HERON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF RIO RANCHO STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
- 3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES.
- 4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONNECTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
- 5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE.
- 6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.
- 7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.

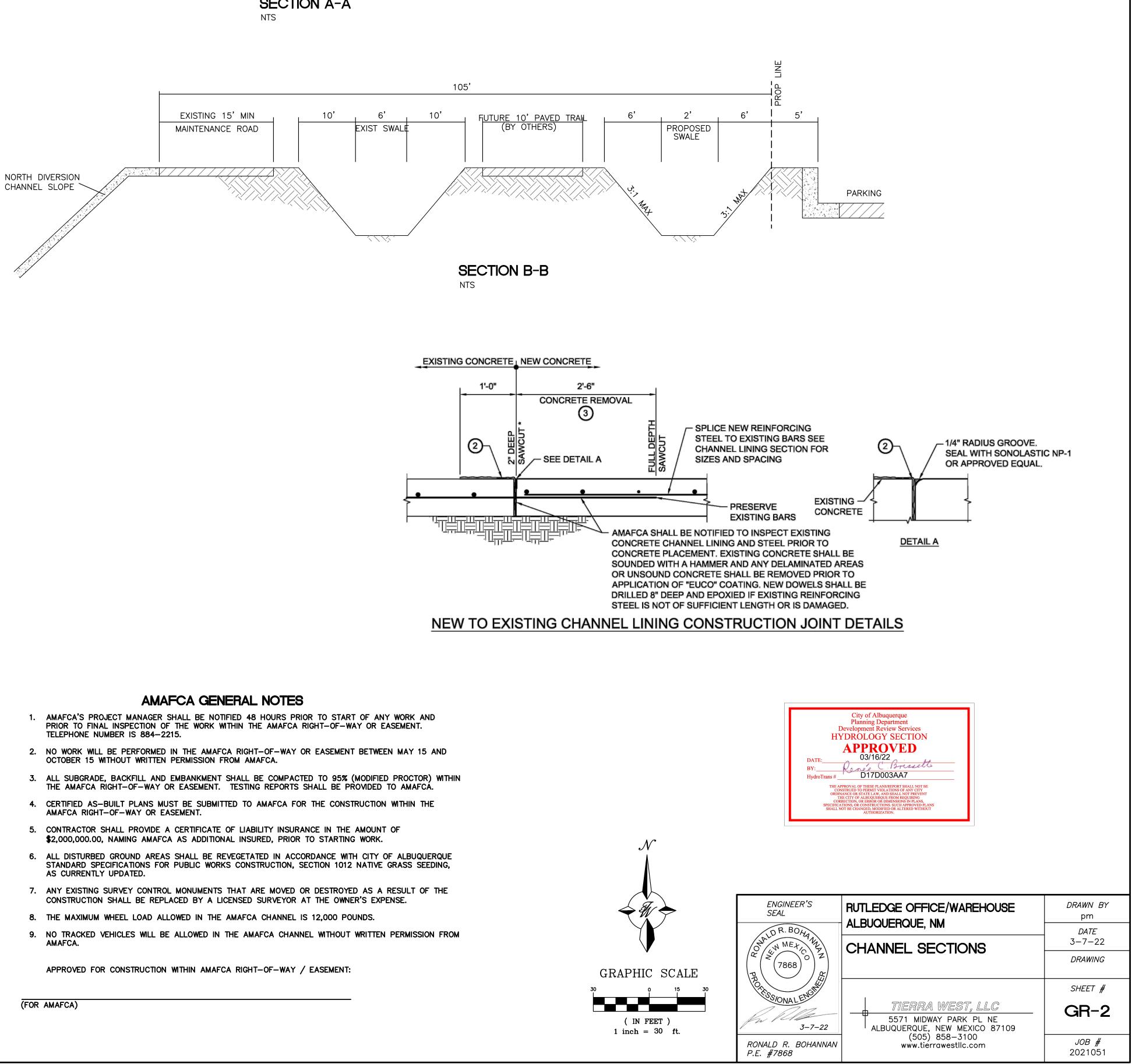
EROSION CONTROL NOTES:

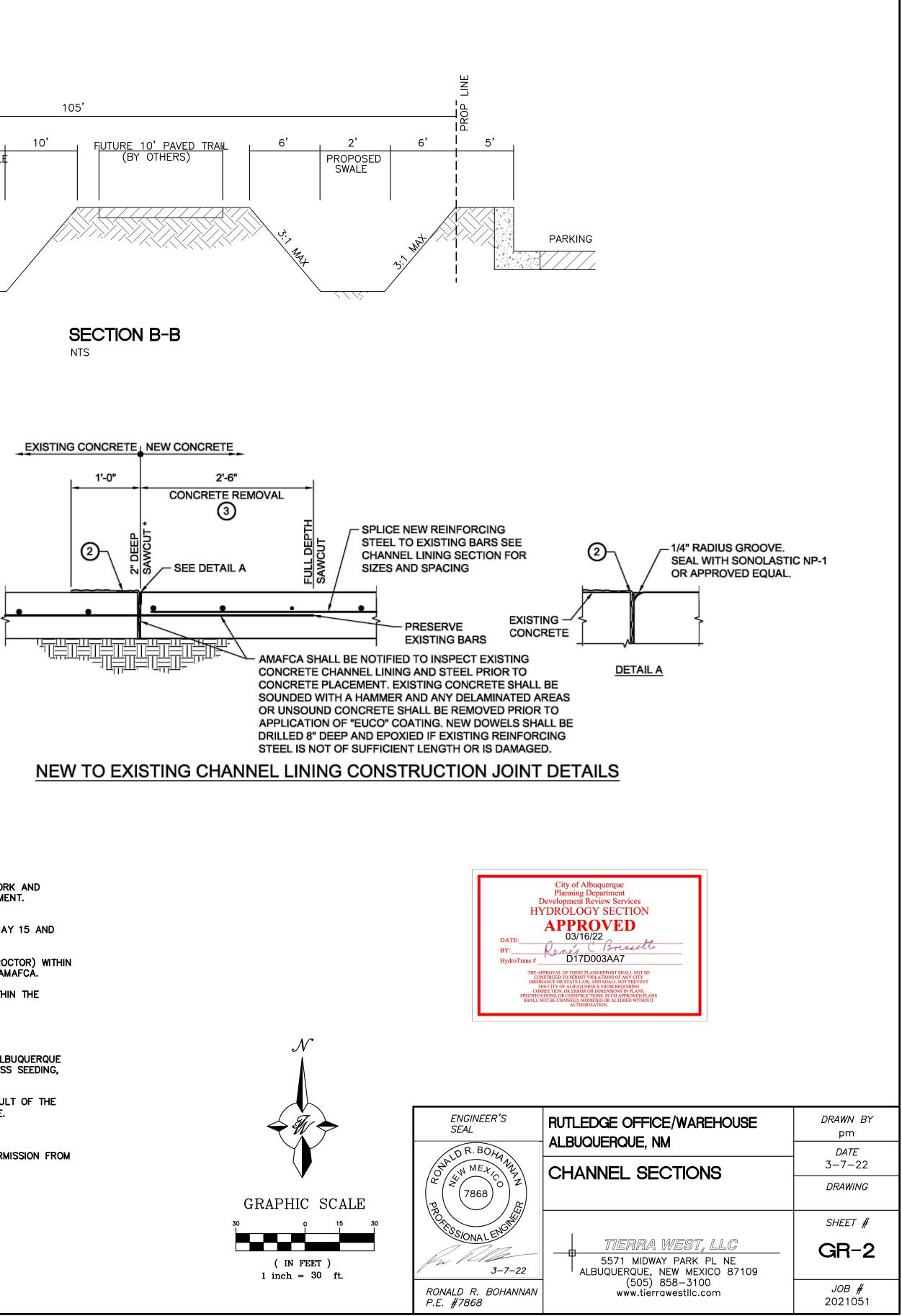
- 1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
- 2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
- 3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
- 4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL (CITY) ACCEPTANCE OF ANY PROJECT.
- 6. ALL SLOPES NOT STABILIZED AT THE END OF THE PROJECT SHALL BE STABILIZED IN ACCORDANCE WITH COA SPECS OR ₹" GRAVEL

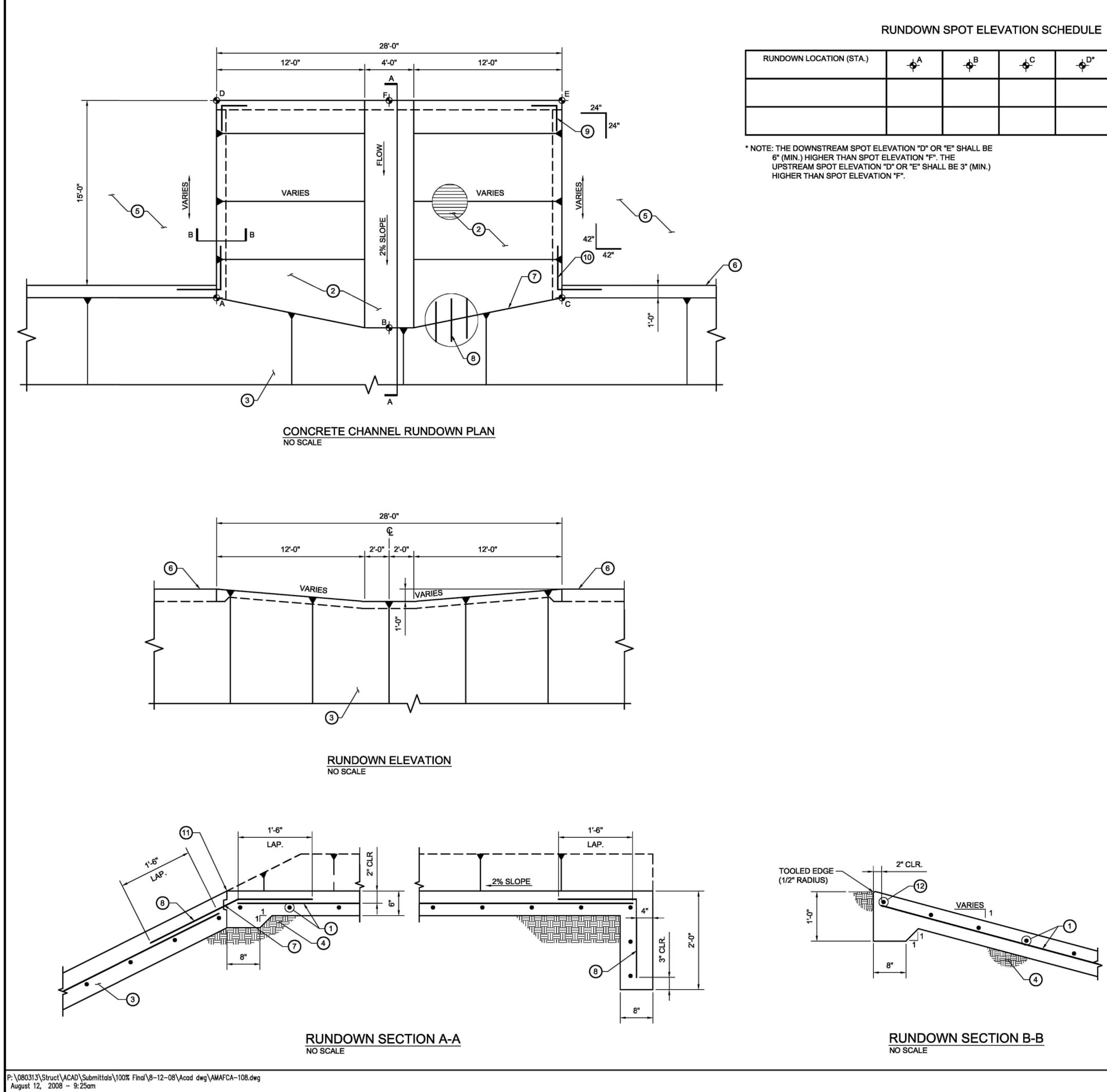
ENGINEER'S SEAL	RUTLEDGE OFFICE/WAREHOUSE ALBUQUERQUE, NM	<i>DRAWN BY</i> pm
O HALD R. BOHAN	RUTLEDGE SPEC BUILDING	<i>DATE</i> 3–7–22
	GRADING AND DRAINAGE PLAN	DRAWING
PROFILESSIONAL ENGINE		SHEET #
JUNALO Juli 3-7-22	5571 MIDWAY PARK PL NE ALBUQUERQUE, NEW MEXICO 87109	GR-1
RONALD R. BOHANNAN P.F. #7868	(505) 858-3100 www.tierrawestllc.com	<i>JOB </i>













RUNDOWN LOCATION (STA.)	- \$ ^	·◆ ^B	 • <mark>•</mark> •	• ^{E*}	÷

OKEYED NOTES

1. RUNDOWNS SHALL BE CONSTRUCTED OF 6" THICK CONCRETE WITH #5 BARS AT 15" O.C. CONCRETE SHALL BE TINTED TO MATCH CHANNEL COLOR.

- 2. EXPOSED CONCRETE SURFACE OF THE RUNDOWN SHALL BE FINISHED WITH A TINE FINISH TRANSVERSE TO RUNDOWN FLOW
- 3.8" CHANNEL LINING
- 4. COMPACTED SUBGRADE 95% MODIFIED PROCTOR, SAME CONDITIONING AS UNDER CHANNEL LINING
- 5. MAINTENANCE ROAD
- 6. CHANNEL SILL
- 7. ALLOWABLE CONSTRUCTION JOINT WITH KEYWAY
- 8. #5 DOWEL @ 15" O.C.

9. #5 CORNER BAR, TYP.

- 10. #8 CORNER BAR, TYP.
- 11. TOOLED 1/4" RADIUS BOTH SIDES WITH NP-1 SEALANT OR APPROVED EQUAL

12. #8 ON 6" SAND SHAIR

DESIGN DATA

F'C = 3000 PSI

B. REINFORCING STEEL:

A. CAST-IN-PLACE CONCRETE:

C. MOISTURE AND DENSITY CONTROL:

ASTM A615 GRADE 60

ASTM D-1557 ALL FILL MATERIAL SHALL BE PLACED IN HORIZONTAL LIFTS OF 8" MAXIMUM AND COMPACTED TO 95 PERCENT OF MAXIMUM DRY DENSITY. THE MOISTURE CONTENT SHALL BE OPTIMUM TO ±2 PERCENT.

	ALBUQUERQUE METROPOLITAN ALBUQUERQUE METROPOLITAN ARROYO FLOOD CONTROL AUTHORITY AMAFCA STANDARD DETAILS CHANNEL RUNDOWN PLAN AND DETAILS					
	REV	DATE		DESCRIPTION		CHKD
ISSUE DATE: 8-12-2008	DRAV NO.		108	MAP NO.	SHEET	OF