

THE FOLLOWING ITEMS ARE IN REFERENCE TO PROPOSED PAVING IMPROVEMENTS TO AN

EXISTING WAREHOUSE DEVELOPMENT LOCATED AT 3768 HAWKINS STREET NE IN A PORTION OF LOT 19, INTERSTATE INDUSTRIAL TRACT UNIT IV, ALBUQUERQUE, NEW MEXICO, GRADING AND DRAINAGE PLAN ARE CONTAINED HEREON:

DRAINAGE CALCULATIONS

VICINITY MAP (D-17) FLOOD INSURANCE RATE MAP 35001C0136 G GRADING PLAN

EXISTING CONDITIONS

AS SHOWN BY THE VICINITY MAP, THE SITE CONTAINS APPROXIMATELY 7.90 ACRES AND IS LOCATED AT THE SOUTHEAST CORNER OF SNAPROLL STREET NE AND HAWKINS STREET NE (SEE ATTACHED VICINITY MAP D-17). THIS DEVELOPMENT IS CLASSIFIED AS AN INFILL SITE, PER CITY CRITERIA, SINCE THE SURROUNDING AREA IS COMPLETELY DEVELOPED.

THE SITE'S EXISTING TOPOGRAPHY SLOPES FROM AN EAST TO WEST DIRECTION AT A SLOPE OF APPROXIMATELY 1.2%. THE SITE CURRENTLY HAS EXISTING BUILDINGS THAT CONTAINS ABOUT 2.56ACRES AND EXISTING ASPHALT PAVING CONTAINING ABOUT 3,20ACRES WITH THE REMAINDER OF THE SITE CONSISTING OF COMPACTED BASECOURSE CONTAINING 2.13ACRES. THE MAJORITY OF THE LOT DRAINS IN A WESTERLY DIRECTION TO THE WESTSIDE OF THE MOST WESTERLY BUILDING THAN MEANDERS IN A NORTHERLY DIRECTION AND EVENTUALLY OUTFALLS INTO THE CUL-DE SAC AT THE INTERSECTION OF SNAPROLL STREET NE AND HAWKINS STREET NE THROUGH A 5'-6" WIDE CURB OPENING IN THE EXISTING CURB. THIS FLOW THAN GOES WEST ACROSS THE CUL-DE-SAC INTO AN EXISTING 9' WIDE CONCRETE CHANNEL THAT OUTFALLS AND CONNECTS INTO THE EAST SIDE OF THE AMAFCA NORTH DIVERSION CHANNEL.

THE MOST WESTERLY PORTION OF LOT 19 SHEETFLOWS IN A WESTERLY DIRECTION ONTO LOT 18 AS SHOWN ON BASIN "B-1".

THE SITE IS LOCATED ON FIRM MAP 35001 C0136 G AND IS NOT LOCATED WITHIN A 100-YEAR FLOODPLAIN.

PROPOSED CONDITIONS AS SHOWN BY THE GRADING PLAN PREPARED FOR THIS SITE, THE INTENT IS TO REMOVE EXISTING ASPHALT PAVING AND REGRADE THE NEW ASPHALT PAVING ALONG THE WESTERLY BUILDING IN ORDER TO FLOW IN THE SAME DIRECTION AS IT CURRENTLY EXIST TODAY. DUE TO MINIMAL SLOPES IN A SOUTH TO NORTH DIRECTION A CONCRETE VALLEY GUTTER WILL BE PROVIDED TO BETTER DIRECT THE ONSITE FLOWS FROM BASIN A-2 AND OFFSITE BASIN A-1 TO THE NORTHWEST END OF THE SITE AND DISCHARGE INTO THE EXISTING CURB CUT AT THE CUL-DE-SAC AT SNAPROLL STREET NE AND HAWKINS STREET NE.

THE CALCULATIONS WHICH APPEAR HEREON, ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6 HOUR RAINFALL RUNOFF FOR PEAK FLOWS AND STORM DURATION FOR VOLUME REQUIREMENTS. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS AS SET FORTH IN THE REVISION OF SECTION 22.7 HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993. THIS D.P.M. PROCEDURE IS USED FOR ANALYZING ONSITE FLOWS.

DOWNSTREAM CAPACITY

THE OVERALL PROPOSED PEAK FLOW AND VOLUME IS EQUIVALENT TO THE EXISTING CONDITIONS AS THEY EXIST TODAY SINCE THIS PROJECT IS ONLY REPAVING AND REGRADING THE PROPOSED NEW PAVED AREA. OTHER EXISTING ASPHALT AREAS ON THE SITE WILL BE PATCHED ON AREAS THAT ARE ALLIGATOR CRACKED WITH NO CHANGES IN EXISTING GRADING AND RESEALING OF EXISTING ASPHALT. BASED ON THIS ANALYSIS THIS RE-PAVING DOES NOT AGGRAVATE THE EXISTING DOWNSTREAM CAPACITY SINCE THERE IS NO INCREASE IN FLOWS, ALSO THIS SITE IS ADJACENT TO THE AMAFCA NORTH DIVERSION CHANNEL.

BASED ON A FIELD VISIT AND TOPOGRAPHIC CONTOUR INFORMATION WE HAVE OFFSITE FLOWS THAT DRAIN FROM THE EAST ONTO THE REFERENCED LOT (SEE OFFSITE BASIN A-1). IT IS THE INTENT OF THIS DRAINAGE STUDY TO INCORPORATE ONSITE DRAINAGE IMPROVEMENTS TO ACCEPT THESE FLOWS AND ALLOW THESE FLOWS TO CONTINUE DRAINING TO THE AMAFCA NORTH DIVERSION CHANNEL AS THEY CURRENTLY DRAIN TODAY.

AT ANALYSIS POINT "A" SHOWN ON THE DRAINAGE BASIN MAP IT APPEARS THAT THE FLOWS SPLIT IN HALF AT THIS LOCATION BASED ON THE TOPOGRAPHIC INFORMATION AND FIELD VERIFICATION. FIFTY (50%) PERCENT OF OFFSITE BASIN "A-1" WILL BE ACCOMMODATED INTO BASIN "A-2". THE OTHER 50% FLOWS NORTH IN THE EXISTING ASPHALT SWALE TO THE NORTH AND DISCHARGES INTO HAWKINS STREET NE.

TEMPORARY EROSION CONTROL WILL BE REQUIRED DURING THE CONSTRUCTION PHASE TO PROTECT DOWNSTREAM DRAINAGE INFRASTRUCTURE FROM SEDIMENT AND UNCONTROLLED RUNOFF (SEE THE EROSION CONTROL PLAN). IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO PROPERLY MAINTAIN THE EROSION CONTROL RECOMMENDATIONS (BMP's) DURING THE ENTIRE CONSTRUCTION PHASE OF THE PROJECT UNTIL SUCH TIME OF CITY OR ENGINEER'S ACCEPTANCE.

DRAINAGE CALCULATIONS 1. PRECIPITATION ZONE = 2

2. <u>DESIGN STORM = DEPTH (INCHES)</u> AT 100-YEAR STORM 6-HOUR = 2.35 INCHES 24-HOUR = 2.75 INCHES 10 DAY = 3.95 INCHES

3. PEAK DISCHARGE (CFS/ACRE) FOR 100-YEAR, ZONE 2, TABLE A-9:

Q = 1.56 CFS/ACRE SOIL UNCOMPACTED "A" Q = 2.28 CFS/ACRE LAND\$CAPED "B"

Q = 3.14 CFS/AC COMPACTED SOIL "C" Q = 4.70 CFS/ACRE IMPERVIOUS AREA "D"

FOR WATERSHEDS LESS THAN OR EQUAL TO 40 ACRES

4. EXCESS PRECIPITATION, E (INCHES), FOR 100-YEAR, 6 HOUR STORM, ZONE 2, TABLE A-8: E = 0.53 INCHES SOIL UNCOMPACTED "A"

E = 0.78 INCHES LANDSCAPED "B" E = 1.13 INCHES COMPACTED SOIL "C" E = 2.12 INCHES IMPERVIOUS AREA 'D"

5. EXCESS PRECIPITATION, E (INCHES), FOR 10-YEAR, 6 HOUR STORM, ZONE 2, TABLE A-8 E = 0.13 INCHES SOIL UNCOMPACTED "

E = 0.28 INCHES LANDSCAPED "B"

E = 0.52 INCHES COMPACTED SOIL "C" E = 1.34 INCHES IMPERVIOUS AREA 'D"

EXISTING CONDITIONS FLOWS BASIN A-2 (ONSITE TO CUL-DE-SAC) TOTAL AREA = 4.90ACRES

AREA(ACRES)

3.70

TYPE "D" TREATMENT = EXISTING ROOF AREAS (27,184SF + 29,744SF + 2,991SF) + EXISTING CONCRETE AND PAVED AREAS (101,058SF) = 160,977SF = 3.70AC TYPE "B" TREATMENT = LANDSCAPED AREAS (OSF) = 0.00AC TYPE "C" TREATMENT = REMAINING COMPACTED GRAVEL AND DISTURBED AREAS COMPACTED BY HUMAN ACTIVITY = 4,90AC - 3,70AC = 1,20AC

1.20

 $Q(EXISTING-6HR) = (3.14 \times 1.20) + (4.70 \times 3.70)$ = 21.16CFS (6HR) EXISTING ONSITE FLOW INTO CUL-DE-SAC $V(EXISTING-6HR) = ((1.13 \times 1.20) + (2.12 \times 3.70))/12)$ = 0.77AC-FT = 33,396CF EXISTING VOLUME INTO CUL-DE-SAC 7. EXISTING CONDITIONS FLOWS BASIN "A-1" (OFFSITE) TOTAL AREA = 4.33ACRES TYPE "D" TREATMENT = EXISTING ROOF AREAS (13,535SF + 22,423SF + 5,103SF) + EXISTING CONCRETE AND PAVED AREAS (127,814SF) = 168,875SF = 3.88AC TYPE "B" TREATMENT = LANDSCAPED AREAS (OSF) = 0.00AC TYPE "C" TREATMENT = REMAINING COMPACTED GRAVEL AND DISTURBED AREAS COMPACTED BY HUMAN ACTIVITY = 4.33AC - 3.88AC = 0.45AC AREA(ACRES)

 $Q(EXISTING-6HR) = (3.14 \times 0.45) + (4.70 \times 3.88)$ = 19.65CFS (6HR) EXISTING ONSITE FLOW 50% INTO BASIN "A-2" = 0.50 X 19.65CFS = 9.82CFS ONTO BASIN "A-2" $V(EXISTING-6HR) = ((1.13 \times 0.45) + (2.12 \times 3.88))/12)$ = 0.73AC-FT = 31,705CF EXISTING ONSITE VOLUME 50% INTO BASIN "A-2" = 0.50 X 0.73AC-FT = 0.37AC-FT = 15,899CF

8. EXISTING CONDITIONS ONSITE FLOWS INTO CUL-DE-SAC AT HAWKINS STREET THAN INTO AMAFCA NORTH DIVERSION CHANNEL DRAINAGE BASIN A-2 (ONSITE) AND BASIN (A-1) OFFSITE; ANALYSIS POINT "C" Q = 21.16CFS + 9.82CFS = 30.98CFSV = 0.77AC-FT + 0.37AC-FT = 1.14AC-FT

9. EXISTING CONDITIONS FLOWS BASIN "B-1" (INTO ADJACENT LOT 18) TOTAL AREA = 1.26ACRES TYPE "C" TREATMENT = COMPACTED GRAVEL AND DISTURBED AREAS COMPACTED BY HUMAN ACTIVITY = 1.26AC

Q(EXISTING-6HR) = $(3.14 \times 1.26) = 3.97$ CFS (6HR) EXISTING ONSITE FLOW $V(EXISTING-6HR) = ((1.13 \times 1.26) / 12)$ = 0.12AC-FT = 5,227CF EXISTING ONSITE VOLUME

10. PROPOSED CONDITIONS FLOWS BASIN A-2 (ONSITE) TOTAL AREA = 4.90ACRES TYPE "D" TREATMENT = EXISTING ROOF AREAS (27,184SF + 29,744SF + 2,991SF) = 1.38AC PROPOSED CONCRETE AND PAVED AREAS (99,920SF) = 2.29AC TYPE "B" TREATMENT = LANDSCAPED AREAS (OSF) = 0.00AC TYPE "C" TREATMENT = REMAINING COMPACTED GRAVEL AND DISTURBED AREAS COMPACTED BY HUMAN ACTIVITY = 4.90AC - 1.38AC - 2.29AC = 1.23AC

 $Q(PROPOSED-6HR) = (3.14 \times 1.23) + (4.70 \times 3.67)$ = 21.11CFS (6HR) PROPOSED ONSITE FLOW INTO CUL-DE-SAC $V(EXISTING-6HR) = ((1.13 \times 1.23) + (2.12 \times 3.67))/12)$ = 0.76AC-FT = 33,106CF EXISTING VOLUME INTO CUL-DE-SAC

11. PROPOSED CONDITIONS ONSITE FLOWS INTO CUL-DE-SAC AT HAWKINS STREET THAN INTO AMAFCA NORTH DIVERSION CHANNEL DRAINAGE BASIN A-2 (ONSITE) AND BASIN (A-1) OFFSITE; ANALYSIS POINT "C" Q = 21.11CFS + 9.82CFS = 30.93CFS V = 0.76AC-FT + 0.37AC-FT = 1.13AC-FT

12. IMPACT OF THIS PAVING ON DOWNSTREAM CAPACITY (100-YEAR, 6 HOUR STORM) Q (EXISTING-6HR) RELEASE RATE FOR SITE = 21.16CFS Q (PROPOSED-6HR) RELEASE RATE FOR SITE = 21.11CFSQ (DIFFERENCE-6HR) = 21.16CFS - 21.11CFS = 0.05CFS DECREASE TO DOWNSTREAM

V (EXISTING-6HR) RUNOFF VOLUME FOR SITE = 33,396CF V (PROPOSED-6HR) RUNOFF VOLUME FOR SITE = 33,106CF V (DIFFERENCE-6HR) = 33,396CF - 33,106CF = 290CF DECREASE TO DOWNSTREAM CAPACITY

DRAINAGE LEGEND

DRAINAGE BASIN ID NUMBER

DRAINAGE BASIN BOUNDARY

DRAINAGE ANALYSIS POINT

DRAINAGE BASIN FLOW DIRECTION

13. SEE HYDRAULIC ANALYSIS SHEET CI02.

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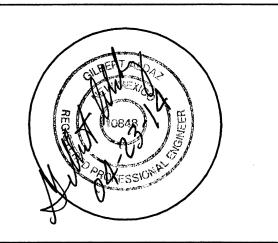
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3768 HAWKINS **PAVING IMPROVEMENTS** ALBUQUERQUE, NEW MEXICO

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MARK	DATE	DESCRIPTION

PROJECT NUMBER: **140101 HAWKINS** DRAWING FILE: DRAWN BY CHECK BY: COPYRIGHT: APRIL 21, 201

> PROPOSED **DRAINAGE PLAN**

> > SHEET NUMBER

C101

Hydraulic Analysis Report

Channel Analysis: Channel Analysis South Cross Section

 (ANALYSIS POINT "B")

 Input Parameters

 Channel Type: Cross Section at South End

 Cross Section Data

 Station ft
 Manning's n

 0.00
 89.50
 0.0170

 25.00
 88.90
 0.0170

 75.00
 89.20
 0.0170

 75.10
 93.20

 Longitudinal Slope: 0.0039 ft/ft

 Flow: 23.4800 cfs

Depth: 0.3527 ft

Depth: 0.3527 ft
Area of Flow: 12.7251 ft^2
Wetted Perimeter: 64.7526 ft
Hydraulic Radius: 0.1965 ft
Average Velocity: 1.8452 ft/s
Top Width: 64.6961 ft
Froude Number: 0.7332
Critical Depth: 0.3140 ft
Critical Velocity: 2.2892 ft/s
Critical Slope: 0.0077 ft/ft

Critical Top Width: 63.0856 ft

Calculated Max Shear Stress: 0.0858 lb/ft^2

Calculated Avg Shear Stress: 0.0478 lb/ft^2

Composite Manning's n Equation: Lotter method

Channel Analysis: Channel Analysis North Cross Section

(ANALYSIS POINT "C")
Input Parameters
Channel Type: Triangular

Manning's n: 0.0170

Side Slope 1 (Z1): 41.6700 ft/ft
Side Slope 2 (Z2): 31.6500 ft/ft
Longitudinal Slope: 0.0039 ft/ft
Manning's n: 0.0170
Flow: 30.9300 cfs

Result Parameters

Depth: 0.5905 ft

Depth: 0.5905 ft
Area of Flow: 12.7821 ft^2
Wetted Perimeter: 43.3104 ft
Hydraulic Radius: 0.2951 ft
Average Velocity: 2.4198 ft/s
Top Width: 43.2940 ft
Froude Number: 0.7848
Critical Depth: 0.5380 ft
Critical Velocity: 2.9154 ft/s
Critical Slope: 0.0064 ft/ft
Critical Top Width: 40.1937 ft
Calculated Max Shear Stress: 0.1437 lb/ft^2
Calculated Avg Shear Stress: 0.0718 lb/ft^2

Channel Analysis: Channel Analysis at North Turn (ANALYSIS POINT "D")

Input Parameters
Channel Type: Triangular
Side Slope 1 (Z1): 31.2500 ft/ft
Side Slope 2 (Z2): 27.8600 ft/ft
Longitudinal Slope: 0.0039 ft/ft
Manning's n: 0.0170
Flow: 30.9000 cfs

Result Parameters

Depth: 0.6400 ft

Area of Flow: 12.1043 ft^2

Wetted Perimeter: 37.8499 ft
Hydraulic Radius: 0.3198 ft
Average Velocity: 2.5528 ft/s
Top Width: 37.8282 ft
Froude Number: 0.7953
Critical Depth: 0.5843 ft
Critical Velocity: 3.0621 ft/s
Critical Slope: 0.0063 ft/ft

Critical Top Width: 34.6532 ft

Calculated Max Shear Stress: 0.1557 lb/ft^2

Calculated Avg Shear Stress: 0.0778 lb/ft^2

Channel Analysis: Channel Analysis at Cul-De-Sac (ANALYSIS POINT "E")

Channel Type: Trapezoidal
Side Slope 1 (Z1): 2.2400 ft/ft
Side Slope 2 (Z2): 2.2400 ft/ft
Channel Width: 10.5000 ft
Longitudinal Slope: 0.0039 ft/ft
Manning's n: 0.0170
Depth: 0.6700 ft

Result Parameters

Flow: 30.6381 cfs
Area of Flow: 8.0405 ft^2
Wetted Perimeter: 13.7871 ft
Hydraulic Radius: 0.5832 ft
Average Velocity: 3.8105 ft/s
Top Width: 13.5016 ft
Froude Number: 0.8702
Critical Depth: 0.6132 ft
Critical Velocity: 4.2083 ft/s

Critical Depth: 0.6132 ft
Critical Velocity: 4.2083 ft/s
Critical Slope: 0.0053 ft/ft
Critical Top Width: 13.2469 ft
Calculated Max Shear Stress: 0.1631 lb/ft^2

Calculated Avg Shear Stress: 0.1419 lb/ft^2

Dezoidal

...2400 ft/ft

...2400 ft/ft

...2500 ft

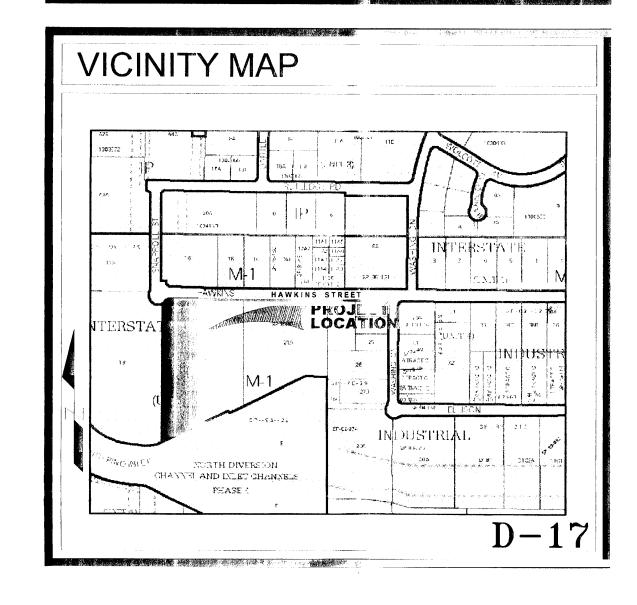
...2500 ft/ft

...2500 ft

...2500 ft/ft

..

35001C0136G



FLOOD INSURANCE RATE MAP

APPLIED ENGINEERING AND SURVEYING.

CIVIL ENGINEERING, LAND PLANNING AND SURVEYING

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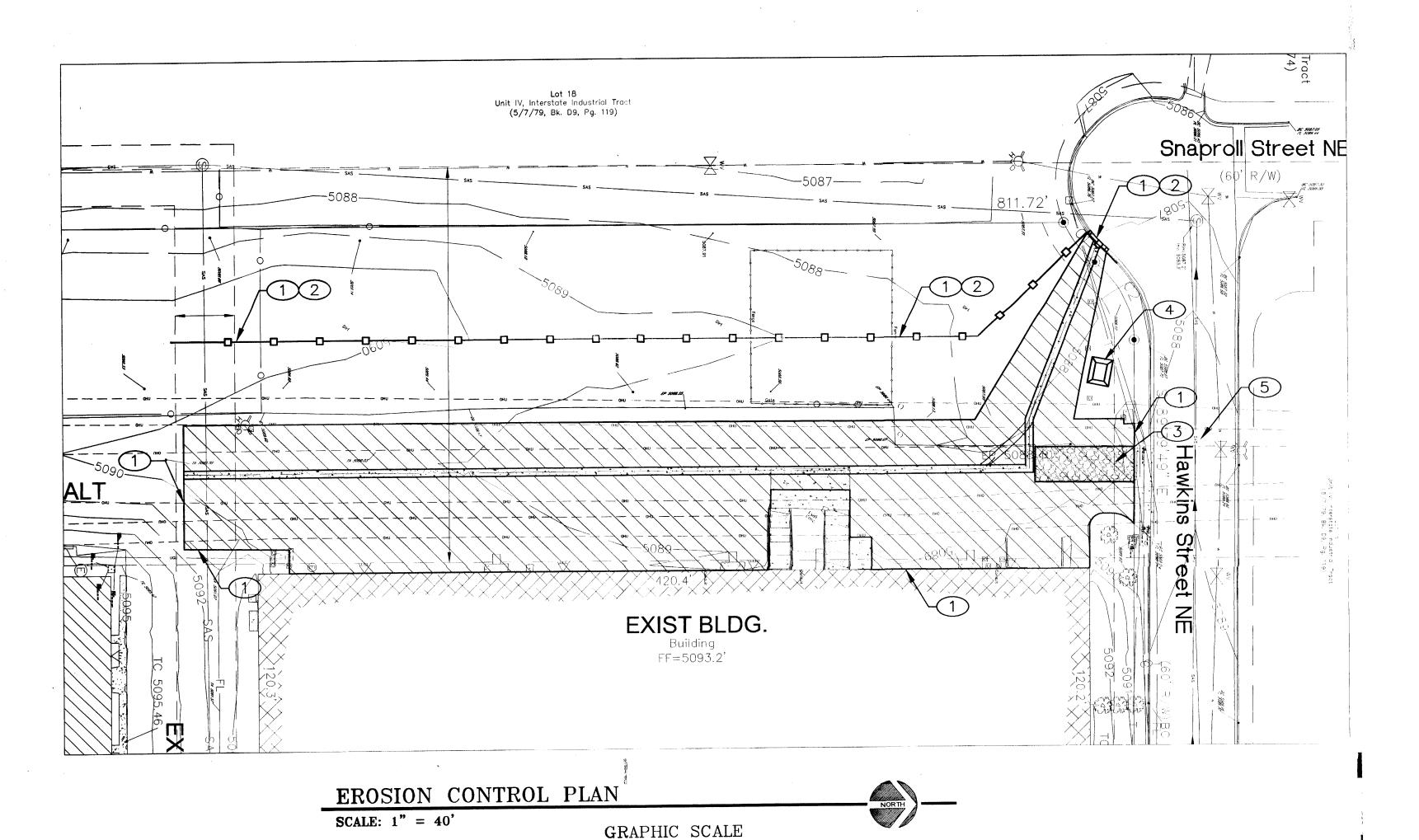
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SHEET TITLE

DRAINAGE

SHEET NUMBER
C102

CALCULATIONS



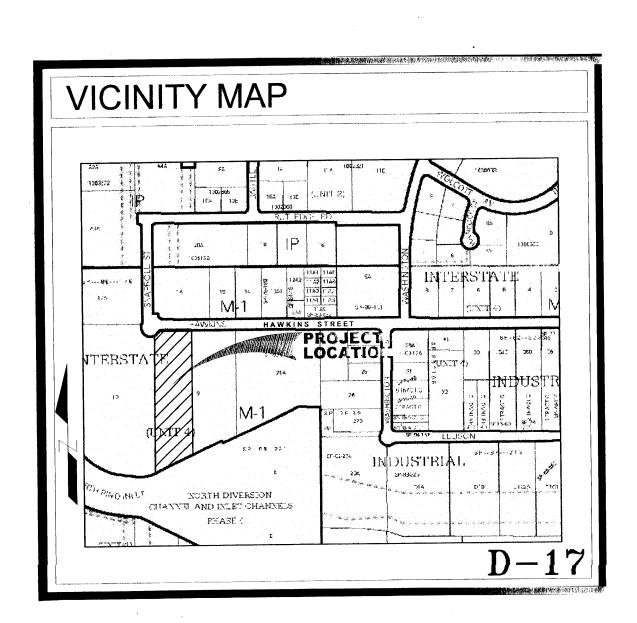
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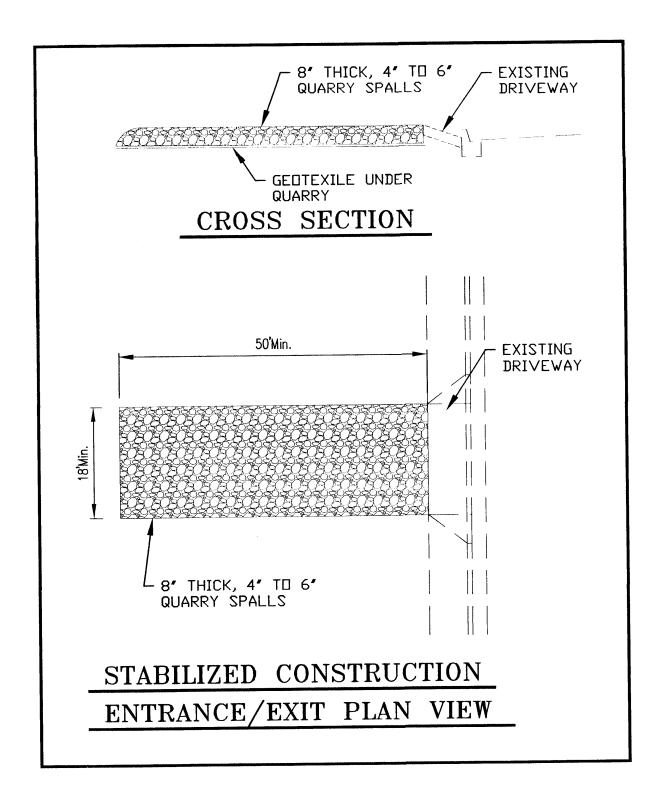
1 inch = 40 ft.

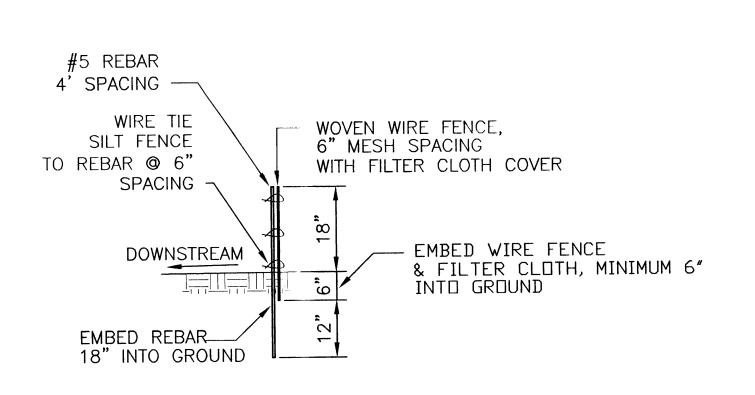
UTILITY NOTE:

IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THE DRAWING, THEY ARE SHOWN IN APPROXIMATE MANNER ONLY. UTILITY LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE UTILITY OR PIPELINE COMPANY, THE OWNER, OR BY OTHERS. THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES.

THE ENGINEER HAS UNDERTAKEN NO FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE OR TYPE OF EXISTING ABOVE AN UNDERGROUND UTILITIES, OR EXISTING PIPELINES. THE ENGINEER MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM HIMSELF OF THE LOCATION OF ANY EXISTING ABOVE AND UNDERGROUND UTILITIES, AND EXISTING PIPELINES, IN AND NEAR THE AREA OF THE WORK, IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY HIS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING ABOVE AND UNDERGROUND UTILITIES, AND EXISTING PIPELINES. THE CONTRACT SHALL COMPLY WITH STATE STATUES PERTAINING TO THE LOCATION OF THESE LINES IN PLANNING AND CONDUCTING EXCAVATION WORK.







SILT PERIMETER FENCE DETAIL

INSTALL SILT FENCE ALONG THE BACK OF SIDEWALK ALONG RID GRANDE BLVD. TO KEEP SEDIMENT OUT OF THE EXISTING STREET AND ADJACENT BOUNDARY LINES TO KEEP SEDIMENT FROM WASHING INTO ADJACENT PROPERTIES. IF THE FABRIC BECOMES CLOGGED, IT SHOULD BE CLEANED OR, IF NECESSARY, REPLACED. SEDIMENT SHOULD BE REMOVED WHEN IT REACHES APPROXIMATELY ONE—HALF THE HEIGHT OF THE FENCE..

CONSTRUCTION NOTES:

- 1 PROJECT BOUNDARY AND LIMITS OF CONSTRUCTION DISTURBANCE.
- (2) LIMITS OF SEDIMENT PERIMETER FENCE INSTALLATION.
- CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE/EXIT. WHEN SEDIMENT HAS SUBSTANTIALLY CLOGGED THE VOID AREA BETWEEN THE ROCKS, THE AGGREGATE MAT MUST BE WASHED DOWN OR REPLACED. PERIODIC RE-GRADING AND TOP DRESSING WITH ADDITIONAL STONE MUST BE DONE TO KEEP THE FERICIENCY OF THE ENTRANCE FROM DIMINISHING.
- 4 CONSTRUCT CONCRETE WASHOUT AREA 8'X8'X3'DEEP. THE CONCRETE WASHOUT AREA SHALL BE REPAIRED, CLEANED, OR ENLARGED AS NECESSARY TO MAINTAIN CAPACITY FOR CONCRETE WASH. CONCRETE MATERIALS, ACCUMULATED IN PIT, SHALL BE REMOVED ONCE THE MATERIALS HAVE REACHED A DEPTH OF 2'.
- 5 CONTRACTOR SHALL CLEAN DIRT/DEBRIS ON HAWKINS STREET AT THE END OF EACH DAY.

SWPPP GENERAL NOTES:

- THE CONTRACTOR SHALL BE IN COMPLIANCE WITH THIS EROSION AND SEDIMENT CONTROL PLAN AT A MINIMUM ALONG WITH COMPLIANCE OF THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM MANUAL STORM WATER MANAGEMENT GUIDELINES FOR CONSTRUCTION AND INDUSTRIAL ACTIVITIES REVISION 2, DATED AUGUST 2012
- 2 ONCE A SWPPP HAS BEEN APPROVED AND AN NOTICE OF INTENT (NO) HAS BEEN FILED AND ACKNOWLED THE CONTRACTOR SHALL IMPLEMENT ALL BMPs SHOWN (SEDIMENT PERIMETER FENCE, STABILIZED CONSTRUCTION ENTRANCE/EXIT AND CONCRETE WASHOUT AREA SHALL BE CONSTRUCTED BEFORE ANY GRADING ACTIVITIES BEGIN ON THIS PROJECT
- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THORUGHLY.
- A COPY OF THE SWPPP, A COPY OF THE PERMIT, THE NOI, AND ACKNOWLEDGEMENT LETTER MUST BE KEPT AT THE CONSTRUCTION SITE FROM THE TIME CONSTRUCTION BEGINS UNTIL THE SITE IS FINALLY STABILIZED.
- 6 ONCE CONSTRUCTION IS COMPLETE THE CONTRACTOR SHALL PROVIDE TEMPORARY SEEDING/VEGETATION AFTER A PERIOD OF 21 DAYS OR MORE.

TOTAL SITE ACREAGE = 1.37AC
TOTAL DISTURBED ACREAGE = 1.37AC

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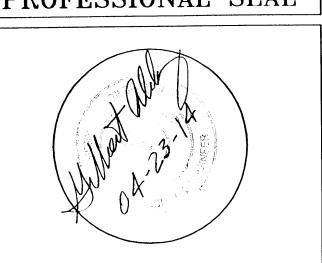
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MARK	DATE	DESCRIPTION
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PROPOSED EROSION CONTROL PLAN

C103