1. VICINITY MAP 2. FIRM FLOOD MAP 3. DRAINAGE CALCULATIONS

PASEO DEL NORTE

AS SHOWN BY THE VICINITY MAP, THE SITE CONTAINS 0.80 ACRES AND IS LOCATED SOUTH OF EL PUEBLO RD. NE. ON A PRIVATE RD. WEST OF LORRAINE CT. NE. THE SITE HAS BEEN GRADED PER THE MASTER PLAN PREPARED BY JEFF MORTENSEN AND ASSOCIATES. THE TOPOGRAPHY SLOPES FROM EAST TO WEST. ACCORDING TO THE FLOOD INSURANCE RATE MAP PANEL 0136D, DATED SEPTEMBER 20,1996, THE SITE IS NOT LOCATED WITHIN A DESIGNATED 100-YEAR FLOOD ZONE.

PROPOSED CONDITIONS

AS SHOWN BY THE DRAINAGE/GRADING PLAN, THE PROJECT WILL CONSIST OF TWO OFFICE/WAREHOUSE BUILDINGS TOTALING 10,100 SQUARE FEET ALONG WITH ASSOCIATED PAVED PARKING AND LANDSCAPE AREAS. ON-SITE RUN-OFF WILL BE LOCATED WITHIN THE COMMON ACCESS-DRAINAGE EASEMENT, FROM THAT POINT THE PROPOSED INLET WILL BE CONNECTED TO AN EXISTING "A" INLET LOCATED WITHIN THE PRIVATE ROAD. AN ALLOWABLE DISCHARGE RATE OF 1.3 CFS IS ALLOWED BY THE MASTER DRAINAGE PLAN. BOTH PARCEL B AND C WILL BE DRAINED TO THE PROPOSED DROP INLET WITH AN ALLOWABLE DISCHARGE OF 2.6 CFS. A HYDROGRAPH HAS BEEN PROVIDED INDICATING THE REQUIRED PONDING. THE CALCULATION WHICH APPEARS HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRES AND SMALLER BASINS. AS SET FORTH IN THE REVISIONS OF SECTION 22.2 HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL VOLUME 2, DESIGN CRITERIA DATED 1997, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUN-OFF GENERATED.

DOWNSTREAM CAPACITY

PER THE APPROVED MASTER PLAN FOR PASEO DEL NORTE INDUSTRIAL PARK, PARCEL "B" IS ALLOWED 1.3 CFS FREE DISCHARGE IS ALLOWED. A MEETING WITH BRAD BINGHAM ON 5/23/2000 ALLOWED 1.6 CFS.

DRAINAGE/GRADING PLAN PARCEL "C"

THE FOLLOWING ITEMS CONCERNING PARCEL "C" OF PARCELS A THRU H OF PASEO DEL NORTE INDUSTRIAL PARK SUBDIVISION, BERNALILLO COUNTY, ALBUQUERQUE, NEW MEXICO ARE CONTAINED HEREON:

1. VICINITY MAP 2. FIRM FLOOD MAP 3. DRAINAGE CALCULATIONS

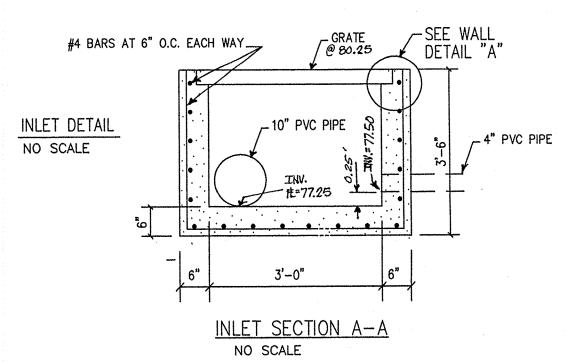
PASEO DEL NORTE AS SHOWN BY THE VICINITY MAP, THE SITE CONTAINS 0.80 ACRES AND IS LOCATED SOUTH OF EL PUEBLO RD. NE. ON A PRIVATE RD. WEST OF LORRAINE CT. NE. THE SITE HAS BEEN GRADED PER THE MASTER PLAN PREPARED BY JEFF MORTENSEN AND ASSOCIATES. THE TOPOGRAPHY SLOPES FROM EAST TO WEST. ACCORDING TO THE FLOOD INSURANCE RATE MAP PANEL 0136D, DATED SEPTEMBER 20,1996, THE SITE IS NOT LOCATED WITHIN A DESIGNATED 100-YEAR FLOOD ZONE.

PROPOSED CONDITIONS

AS SHOWN BY THE DRAINAGE/GRADING PLAN, THE PROJECT WILL CONSIST OF TWO OFFICE/WAREHOUSE BUILDINGS TOTALING 10,100 SQUARE FEET ALONG WITH ASSOCIATED PAVED PARKING AND LANDSCAPE AREAS. ON-SITE RUN-OFF WILL BE LOCATED WITHIN THE COMMON ACCESS-DRAINAGE EASEMENT, FROM THAT POINT THE PROPOSED INLET WILL BE CONNECTED TO AN EXISTING "A" INLET LOCATED WITHIN THE PRIVATE ROAD. AN ALLOWABLE DISCHARGE RATE OF 1.3 CFS IS ALLOWED BY THE MASTER DRAINAGE PLAN, BOTH PARCEL B AND C WILL BE DRAINED TO THE PROPOSED DROP INLET WITH AN ALLOWABLE DISCHARGE OF 2.6 CFS. A HYDROGRAPH HAS BEEN PROVIDED INDICATING THE REQUIRED PONDING. THE CALCULATION WHICH APPEARS HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR , 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRES AND SMALLER BASINS, AS SET FORTH IN THE REVISIONS OF SECTION 22.2 HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL VOLUME 2, DESIGN CRITERIA DATED 1997, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUN-OFF GENERATED.

DOWNSTREAM CAPACITY

PER THE APPROVED MASTER PLAN FOR PASEO DEL NORTE INDUSTRIAL PARK, PARCEL "C" IS ALLOWED 1.3 CFS FREE DISCHARGE. A MEETING WITH BRAD BINGHAM ON 5/23/2000 ALLOWED 1.6 CFS.



PARCEL B & C PASEO DEL NORTE INDUS. PARK	ON).
Flow depth, y 0.50 feet	Weir flow7.95cfs
Grate open area, A 6.00 sq. ft.	Orfice flow17.10 cfs
Grate perimeter, P 10.00 feet	17.10 CFS > 6.65 CFS O
Clogging percentage 25%	

PASEO DEL NORTE AREA =0.75ac. PARCEL B ZONE 2 PRECIPITATION: 360 = 2.35 in.1440= 2.75in.

ARGE:

		EXCE:	SS PRECIP	ITATION:	PEAK	DISCHA
	TREATMENT	Α	0.53in.		1.56	cfs/ac.
	TREATMENT	В	0.78in.		2.28	cfs/ac
	TREATMENT	С	1.13in.		3.14	cfs/ac.
	TREATMENT	D	2.12in.		4.70	cfs/ac.
EXISTING CONDITIONS:		IS:	PROPOSI	ED CO	NDITIONS	
			AREA	AREA		
	TREATMENT	Α	Oac.	Oac.		
	TREATMENT	В	Oac.	Oac.		
	TOCATMENIT	^	0.7500	0.160	a	

10da= 3.95in.

EXISTING EXCESS PRECIPITATION:

TREATMENT D

Weighted E = 0.53)x0.00)+ 0.78)x0.00)+1.13)x0.75)+2.12)x0.00)0.75ac. V100-360 = 1.13)x0.75)/12 = 0.0708ac-f= 3085 cfEXISTING PEAK DISCHARGE:

Q100 = 1.56 \times 0.00)+ 2.28) \times 0.00)+3.14) \times 0.75)+4.70) \times 0.00)=2.36cfs PROPOSED EXCESS PRECIPITATION:

0.583ac.

Weighted E = 0.53)x0.00)+ 0.78)x0.00)+1.13)x0.17)+2.12)x0.58)0.75ac. $V100-360 = 1.90 \times 0.75$ / 12.0= 0.1189ac-f= 5181 cf

V100-1440 = 0.12)+0.58 $\times 2.75- 2.35$ /12 = 0.1383ac-f= 6028 cf V100-10day = 0.12+0.58x 3.95-2.35/12 = 0.1967ac-f= 8569 cf PROPOSED PEAK DISCHARGE:

Q100 = 1.56 $\times 0.00$ + 2.28 $\times 0.00$ + 3.14 $\times 0.17$ + 4.70 $\times 0.58$ = 3.30cfs

PASEO DEL NORTE AREA =0.76ac. PARCEL C

360 = 2.35 in.PRECIPITATION: 1440= 2.75in.

EXCESS PRECIPITATION: PEAK DISCHARGE:

	TREATMENT A	0.53in.	1.56 cfs/ac.	
	TREATMENT B	0.78in.	2.28 cfs/ac.	
	TREATMENT C	1.13in.	3.14 cfs/ac.	
	TREATMENT D	2.12in.	4.70 cfs/ac.	
EXISTING CONDITIONS:			PROPOSED CONDITIONS:	
		AREA	AREA	
	TREATMENT A	Oac.	Oac.	
	TREATMENT B	Oac.	Oac.	
	TREATMENT C	0.76ac.	0.143ac.	

10da= 3.95in.

EXISTING EXCESS PRECIPITATION:

Weighted $E = 0.53)\times0.00 + 0.78)\times0.00 + 1.13)\times0.76 + 2.12)\times0.00)0.76$ ac. = 1.13 $V100-360 = 1.13) \times 0.76)/12 = 0.0716 \text{ac-f} = 3120 \text{ cf}$

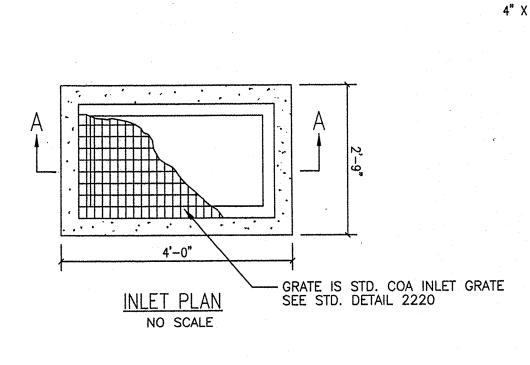
EXISTING PEAK DISCHARGE:

Q100 = 1.56 \times 0.00)+ 2.28 \times 0.00)+3.14 \times 0.76)+4.70 \times 0.00)=2.37cfs PROPOSED EXCESS PRECIPITATION:

Weighted E = 0.53)x0.00)+ 0.78)x0.00)+1.13)x0.14)+2.12)x0.62)0.76ac. $V100-360 = 1.93) \times 0.76$ / 12.0= 0.1226ac-f= 5341 cf V100-1440 = 0.12)+0.62x 2.75- 2.35)/12 = 0.1432ac-f= 6239 cf V100-10day = 0.12)+0.62 $\times 3.95- 2.35$ /12 = 0.2050ac-f= 8931 cf

PROPOSED PEAK DISCHARGE: Q100 = 1.56 $\times 0.00$ + 2.28 $\times 0.00$ + 3.14 $\times 0.14$ + 4.70 $\times 0.62$ = 3.35cfs

THE PRIVATE ROAD AREA WILL GENERATE AN ADDITIONAL 1.63 CFS



CONCRETE FOR NEW INLET STRUCTURE SHALL BE 4000#-28-DAY STRENGTH. REBAR SHALL BE GRADE 40 BARS. 2. ALL SMALL DIAMETER PVC PIPE SHALL BE SDR 26 PIPE.

RECTANGULAR CHANNEL ANALYSIS & DESIGN OPEN CHANNEL - UNIFORM FLOW

WORK SHEET NAME : PARCEL B & C COMMENT: CONCRETE TROUGH FOR ROOF DISCHARGE FOR B SOLVE FOR BOTTOM WIDTH

GIVEN INPUT DATA: MANNING'S n.....

CHANNEL SLOPE..... DISCHARGE 1.09 CFS

COMPUTED RESULTS

FLOW AREA.... FLOW TOP WIDTH ... 0.62 FT WETTED PERIMETER.. 1.62 FT CRITICAL DEPTH..... 0.46 FT

0.0125 FT/FT CRITICAL SLOPE..... 0.88 (FLOW IS SUBCRITICAL) FROUDE NUMBER.....

CIRCULAR CHANNEL ANALYSIS & DESIGN SOLVED WITH MANNING'S EQUATION OPEN CHANNEL - UNIFORM FLOW

WORK SHEET NAME : PARCEL B & C COMMENT: ALLOWABLE DISCHARGE OF 3.2 CFS SOLVE FOR FULL FLOW DIAMETER

GIVEN INPUT DATA

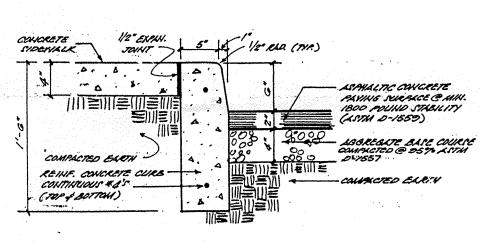
MANNING'S n..... 0.0250 FT/FT PIPE SLOPE.....

COMPUTED RESULTS

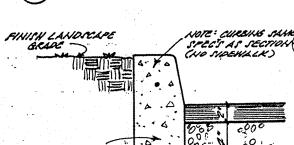
DISCHARGE

FULL FLOW DIAMETER ... 0.81 FT 0.81 FT FULL FLOW DEPTH..... 6.23 fps VELOCITY..... 0.51 FT FLOW AREA..... PERCENT FULL..... CRITICAL DEPTH... CRITICAL SLOPE...... 0.0216 FT/FT FROUDE NUMBER..... FULL

USE 10" PVC

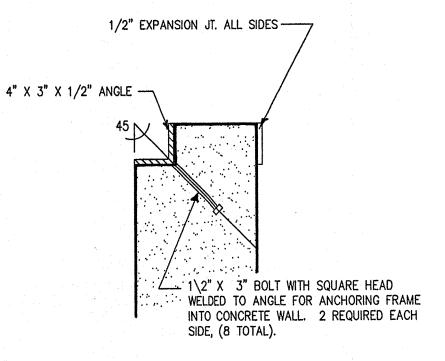


3.2 CFS



(A) CURB AT CONCRETE WALK

1/2" EXPLYION JOHT & 36'0.C. , PROVIDE /" B TYPICAL CURB



NO SCALE

RECTANGULAR CHANNEL ANALYSIS & DESIGN OPEN CHANNEL - UNIFORM FLOW

WORK SHEET NAME : PARCEL B & C COMMENT: CONCRETE TROUGH FOR ROOF DISCHARGE FOR C SOLVE FOR BOTTOM WIDTH

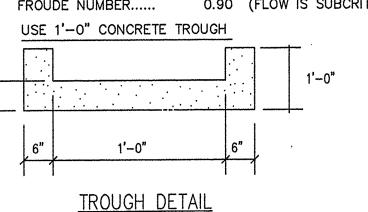
GIVEN INPUT DATA :

MANNING'S n..... CHANNEL SLOPE..... 1.21 CFS DISCHARGE

COMPUTED RESULTS:

BOTTOM WIDTH...... 0.67 FT 3.63 fps FLOW AREA..... 0.33 sf FLOW TOP WIDTH .. 0.67 FT 1.67 FT WETTED PERIMETER..

CRITICAL SLOPE 0.0119 FT/FT FROUDE NUMBER..... 0.90 (FLOW IS SUBCRITICAL)



NO SCALE

1 AMERICAN GYPSUM COMPANY SU-1 M-1 PERM USES JOURNAL

ZONE ATLAS D-17 FIRM PANEL 0136D

JUN 2 9 2000 HYDROLOGY SECTION 12.33 Qp=3.35 CFS---Qp=3.30 CFS----VOLUME TO POND VOLUME TO POND 1842.0 CF 1731.0 CF Qr=1.6 CFS ---Qr=1.6 CFS ----سيدي ياسومون TIME (MINUTES) TIME (MINUTES) 40.50 40.61

> REQUIRED PONDING: 3573.0 CF PONDING PROVIDED: 4035.0 CF FREEBOARD: 462.0 CF

PER A MEETING WITH BRAD BINGHAM ON MAY 23,2000 IT WAS DETERMINED THAT AN ALLOWABLE DISCHARGE RATE OF 1.6 CFS PER PARCEL WAS ACCEPTABLE COMPARED TO THE 1.3 CFS ALLOWED PER THE MASTER DRAINAGE PLAN THEREFORE EACH PARCEL WILL BE ALLOWED 1.6 CFS WITH A TOTAL OF 3.2 CFS FOR BOTH PARCELS.

<u>HYDROGRAPH FOR PARCEL "B"</u>

EROSION CONTROL MEASURES

THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR MANAGEMENT OF STORM RUN-OFF DURING CONSTRUCTION, HE SHALL ASSURE THAT THE FOLLOWING MEASURES ARE

HYDROGRAPH FOR PARCEL "C"

1. ADJACENT PROPERTY SHALL BE PROTECTED AT ALL TIMES BY TEMPORARY BERMS, DIKES, SWALES, AND OTHER TEMPORARY GRADING AS REQUIRED TO PREVENT STORM RUN-OFF FROM LEAVING THE SITE AND ENTERING ADJACENT PROPERTY.

2. ADJACENT PUBLIC RIGHT-OF-WAY SHALL BE PROTECTED AT ALL TIMES FROM STORM WATER RUN-OFF FROM THE SITE. NO SEDIMENT BEARING WATER SHALL BE PERMITTED TO ENTER THE PUBLIC STREETS.

3. THE CONTRACTOR SHALL IMMEDIATELY AND THOROUGHLY REMOVE ANY OR ALL SEDIMENT WITHIN THE PUBLIC STREETS THAT HAVE BEEN ERODED FROM THE SITE AND DEPOSITED THERE.

SYMBOL LEGEND EXISTING CONTOUR ____ 83 ____ PROPOSED CONTOUR DESIGNED SPOT ELEVATION 80.32 PROPERTY LINE EXISTING SPOT ELEVATION \$\infty 82.5 DOWN SPOUT O <u>BENCHMARK:</u> EASEMENT LINE -----

LEGAL DESCRIPTION

PARCEL "B" AND "C" OF PARCELS "A" THRU "H" OF PASEO DEL NORTE INDUSTRIAL PARK SUBDIVISION, ALBUQUERQUE, NEW MEXICO

ABBREVIATION LEGEND

TOP OF CON. PAD TOP OF CURB - TC TOP OF ASPHALT TA - FL FLOWLINE

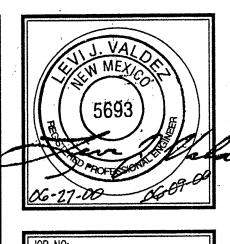
TOP OF WALL

ROOF FLOWS

- TW

— RF

A SQUARE CUT CHISLED ON TOP OF CURB SEE DRAINAGE PLAN: ELEVATION 5080.70



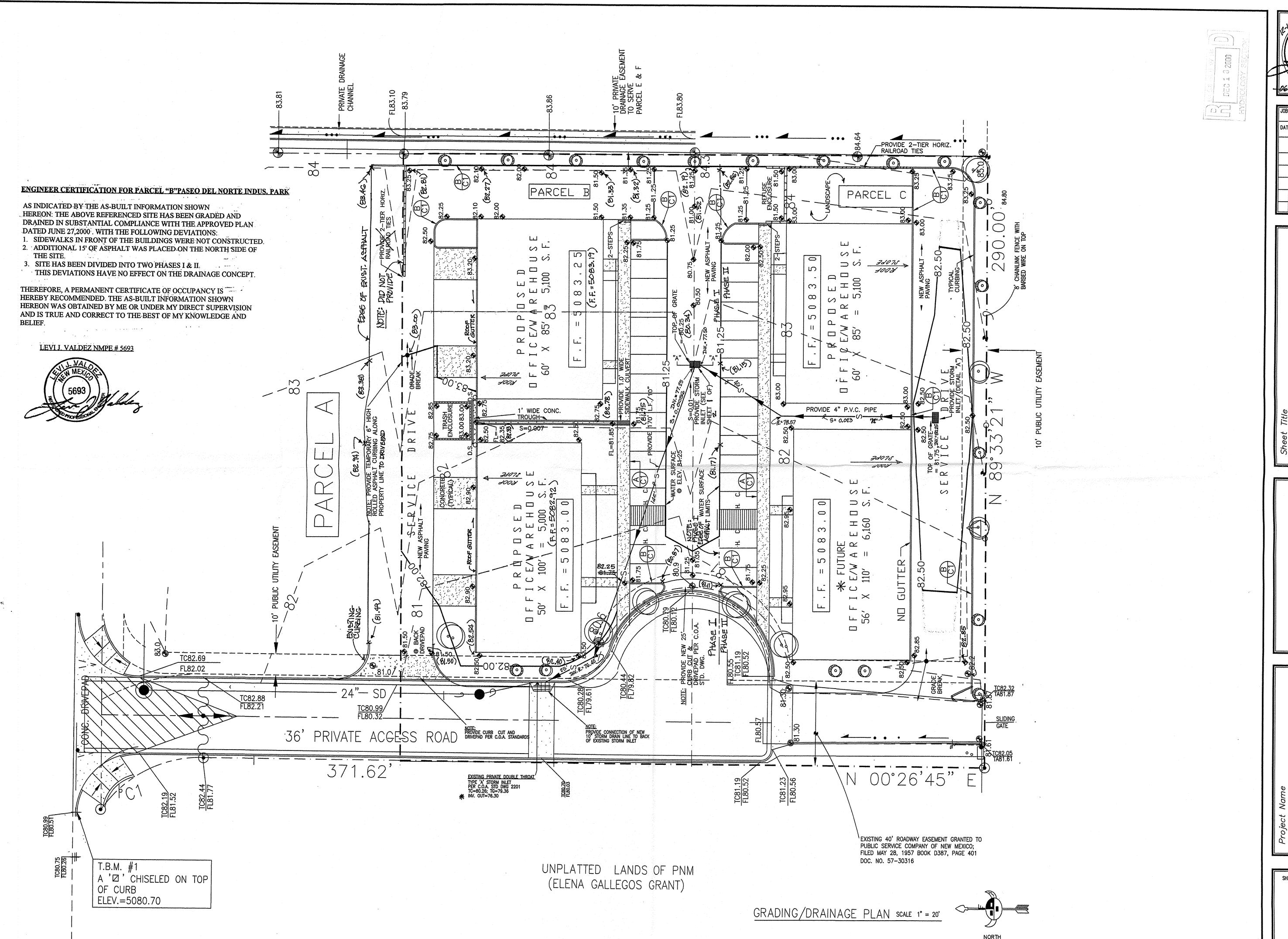
DATE: MAY 13,2000 REVISIONS

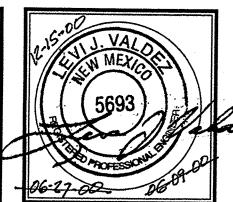
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9 TINSNOO

OPMENT DEVEL \leq \triangle

SHEET NO. 1 Of 2





JOB NO:

DATE: JUNE,2000

REVISIONS

IG & DRAINAGE PLAN

BJM DEVELOPMENT CONSULTANT
DESIGN - PLANNER
Albuquerque, New Mexico

USTRIAL PARK

SHEET NO. 2 OF 2

1. VICINITY MAP 2. FIRM FLOOD MAP 3. DRAINAGE CALCULATIONS

EXISTING CONDITIONS

- PASEO DEL NORTE

AS SHOWN BY THE VICINITY MAP, THE SITE CONTAINS 0.80 ACRES AND IS LOCATED SOUTH OF EL PUEBLO RB. NE. ON A PRIVATE RD. WEST OF LORRAINE CT. NE. THE SITE HAS BEEN GRADED PER THE MASTER PLAN PREPARED BY JEFF MORTENSEN AND ASSOCIATES. THE TOPOGRAPHY SLOPES FROM EAST TO WEST. ACCORDING TO THE FLOOD INSURANCE RATE MAP PANEL 0136D, DATED SEPTEMBER 20,1996, THE SITE IS NOT LOCATED WITHIN A DESIGNATED 100-YEAR FLOOD ZONE.

PROPOSED CONDITIONS

AS SHOWN BY THE DRAINAGE/GRADING PLAN, THE PROJECT WILL CONSIST OF TWO OFFICE/WAREHOUSE BUILDINGS TOTALING 10,100 SQUARE FEET ALONG WITH ASSOCIATED PAVED PARKING AND LANDSCAPE AREAS. ON-SITE RUN-OFF WILL BE LOCATED WITHIN THE COMMON ACCESS-DRAINAGE EASEMENT, FROM THAT POINT THE PROPOSED INLET WILL BE CONNECTED TO AN EXISTING "A" INLET LOCATED WITHIN THE PRIVATE ROAD. AN ALLOWABLE DISCHARGE RATE OF 1.3 CFS IS ALLOWED BY THE MASTER DRAINAGE PLAN. BOTH PARCEL B AND C WILL BE DRAINED TO THE PROPOSED DROP INLET WITH AN ALLOWABLE DISCHARGE OF 2.6 CFS. A HYDROGRAPH HAS BEEN PROVIDED INDICATING THE REQUIRED PONDING. THE CALCULATION WHICH APPEARS HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRES AND SMALLER BASINS, AS SET FORTH IN THE REVISIONS OF SECTION 22.2 HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL VOLUME 2, DESIGN CRITERIA DATED 1997, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUN-OFF GENERATED.

DOWNSTREAM CAPACITY

PER THE APPROVED MASTER PLAN FOR PASEO DEL NORTE INDUSTRIAL PARK, PARCEL "B" IS ALLOWED 1.3 CFS FREE DISCHARGE IS ALLOWED. A MEETING WITH BRAD BINGHAM ON 5/23/2000 ALLOWED 1.6 CFS.

DRAINAGE/GRADING PLAN PARCEL "C"

THE FOLLOWING ITEMS CONCERNING PARCEL "C" OF PARCELS A THRU H OF PASEO DEL NORTE INDUSTRIAL PARK SUBDIVISION, BERNALILLO COUNTY, ALBUQUERQUE, NEW MEXICO ARE CONTAINED HEREON:

1. VICINITY MAP 2. FIRM FLOOD MAP 3. DRAINAGE CALCULATIONS

EXISTING CONDITIONS

PASEO DEL NORTE

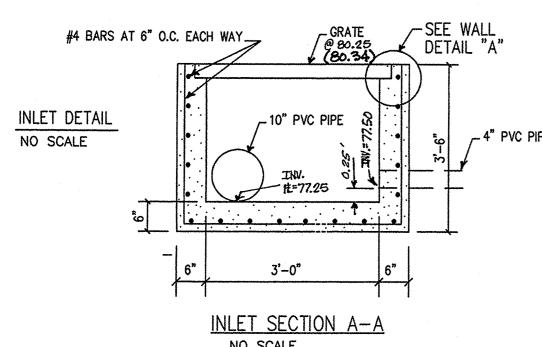
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DOWNSTREAM CAPACITY

PER THE APPROVED MASTER PLAN FOR PASEO DEL NORTE INDUSTRIAL PARK, PARCEL "C" IS ALLOWED 1.3 CFS FREE DISCHARGE. A MEETING WITH BRAD BINGHAM ON 5/23/2000 ALLOWED 1.6 CFS.



NO SCALE RECTANGULAR GRATE DROP INLET (SUMP CONDITION) PARCEL B & C PASEO DEL NORTE INDUS. PARK Weir flow...7.95cfs Flow depth, y..... 0.50 feet Grate open area, A.... 6.00 sq. ft. Orfice flow..17.10 cfs 17.10 CFS > 6.65 CFS OK Grate perimeter, P... 10.00 feet Clogging percentage.... 25%

PASEO DEL NORTE AREA =0.75ac. PARCEL B ZONE 2

PRECIPITATION: 360= 2.35in. 1440= 2.75in. 10da= 3.95in.

EXCESS PRECIPITATION: PEAK DISCHARGE:

TREATMENT A	0.53in.	1.56	cfs/ac	
TREATMENT B	0.78in.	2.28	cfs/ac	
TREATMENT C	1.13in.	3.14	cfs/ac	
TREATMENT D	2.12in.	4.70	cfs/ac	
EXISTING CONDITIONS:		PROPOSED CONDITION		
TREATMENT A	AREA Oac.	AREA Oac.		
TREATMENT B	Oac.	Oac.		
TREATMENT C	0.75ac.	0.169ac.		
TREATMENT D	Oac,	0.583ac.		

EXISTING EXCESS PRECIPITATION:

Weighted E = 0.53)x0.00)+ 0.78)x0.00)+1.13)x0.75)+2.12)x0.00)0.75ac. $V100-360 = 1.13)\times0.75$ /12 = 0.0708ac-f= 3085 cf

EXISTING PEAK DISCHARGE:

Q100 = 1.56 \times 0.00)+ 2.28 \times 0.00)+3.14 \times 0.75)+4.70 \times 0.00)=2.36cfs PROPOSED EXCESS PRECIPITATION:

Weighted E = 0.53)x0.00)+ 0.78)x0.00)+1.13)x0.17)+2.12)x0.58)0.75ac. V100-360 = 1.90)x0.75 / 12.0= 0.1189ac-f= 5181 cf

V100-1440 = 0.12+0.58 x 2.75-2.35/12 = 0.1383ac-f= 6028 cf $V100-10day = 0.12+0.58 \times 3.95-2.35 / 12 = 0.1967ac-f= 8569 cf$ PROPOSED PEAK DISCHARGE:

 $Q100 = 1.56 \times 0.00 + 2.28 \times 0.00 + 3.14 \times 0.17 + 4.70 \times 0.58 = 3.30 cfs$

PASEO DEL NORTE AREA =0.76ac. PARCEL C ZONE 2

PRECIPITATION: 360= 2.35in. 1440= 2.75in 10da= 3.95in.

EXCESS PRECIPITATION: PEAK DISCHARGE:

TREATMENT A TREATMENT B TREATMENT C TREATMENT D	0.53in. 0.78in. 1.13in. 2.12in.	1.56 2.28 3.14 4.70	cfs/ac. cfs/ac. cfs/ac. cfs/ac.	
EXISTING CONDITIONS: AREA		PROPOSED CONDITIONS:		
TREATMENT A	Oac.	Oac.		
TREATMENT B	Oac.	Oac.		
TREATMENT C	0.76ac.	0.143ac.		
TREATMENT D	Oac.	0.618ac.		

EXISTING EXCESS PRECIPITATION: Weighted E = 0.53)x0.00)+ 0.78)x0.00)+1.13)x0.76)+2.12)x0.00)0.76ac. V100-360 = 1.13x0.76/12 = 0.0716ac-f= 3120 cf

EXISTING PEAK DISCHARGE:

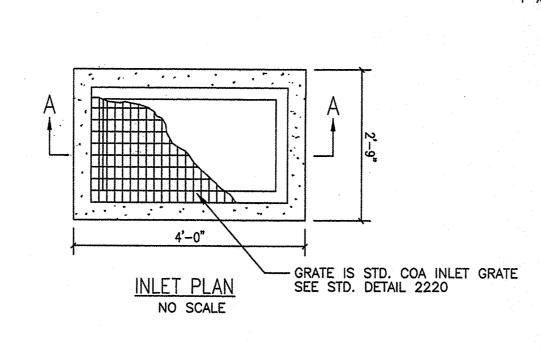
 $Q100 = 1.56 \times 0.00 + 2.28 \times 0.00 + 3.14 \times 0.76 + 4.70 \times 0.00 = 2.37 cfs$ PROPOSED EXCESS PRECIPITATION:

Weighted E = 0.53(x)(0.00) + 0.78(x)(0.00) + 1.13(x)(0.14) + 2.12(x)(0.62)(0.76ac) $V100-360 = 1.93) \times 0.76$ / 12.0= 0.1226ac-f= 5341 cf

V100-1440 = 0.12)+0.62 x 2.75-2.35 /12 = 0.1432 ac-f= 6239 cf V100-10day = 0.12+0.62x 3.95-2.35/12 = 0.2050ac-f= 8931 cf PROPOSED PEAK DISCHARGE:

 $Q100 = 1.56 \times 0.00 + 2.28 \times 0.00 + 3.14 \times 0.14 + 4.70 \times 0.62 = 3.35 cfs$

THE PRIVATE ROAD AREA WILL GENERATE AN ADDITIONAL 1.63 CFS



CONCRETE FOR NEW INLET STRUCTURE SHALL BE 4000#-28-DAY STRENGTH. REBAR SHALL BE GRADE 40 BARS." 2. ALL SMALL DIAMETER PVC PIPE SHALL BE SDR 26 PIPE.

RECTANGULAR CHANNEL ANALYSIS & DESIGN OPEN CHANNEL - UNIFORM FLOW

WORK SHEET NAME : PARCEL B & C COMMENT: CONCRETE TROUGH FOR ROOF DISCHARGE FOR B

1.09 CFS

SOLVE FOR BOTTOM WIDTH

GIVEN INPUT DATA: MANNING'S n..... CHANNEL SLOPE..... 0.0100 FT/FT 0.50 FT

DISCHARGE COMPUTED RESULTS

0.62 FT 3.52 fps BOTTOM WIDTH..... VELOCITY.... 0.31 sf FLOW AREA..... FLOW TOP WIDTH .. 0.62 FT 1.62 FT

WETTED PERIMETER... CRITICAL DEPTH...... 0.46 FT CRITICAL SLOPE..... 0.0125 FT/FT FROUDE NUMBER..... 0.88 (FLOW IS SUBCRITICAL)

CIRCULAR CHANNEL ANALYSIS & DESIGN SOLVED WITH MANNING'S EQUATION

OPEN CHANNEL - UNIFORM FLOW WORK SHEET NAME : PARCEL B & C

COMMENT: ALLOWABLE DISCHARGE OF 3.2 CFS SOLVE FOR FULL FLOW DIAMETER

GIVEN INPUT DATA: MANNING'S n..... 0.013

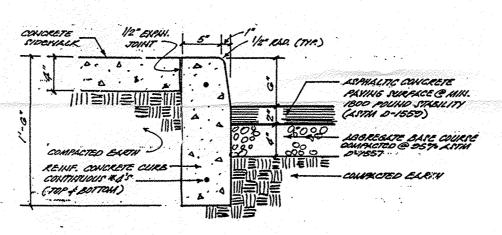
PIPE SLOPE..... 0.0250 FT/FT DISCHARGE 3.2 CFS

COMPUTED RESULTS

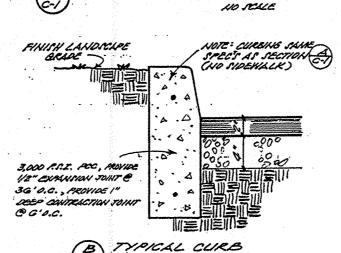
FULL FLOW DIAMETER......
FULL FLOW DEPTH..... 0.81 FT VELOCITY... 6.23 fps FLOW AREA..... 0.51 FT PERCENT FULL.... CRITICAL DEPTH... 100.00% 0.70 FT CRITICAL SLOPE..... 0.0216 FT/FT

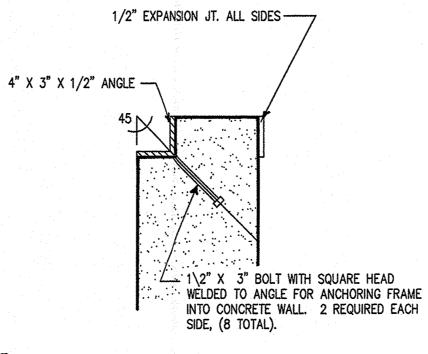
USE 10" PVC

FROUDE NUMBER.....



FULL





INLET WALL DETAIL "A" NO SCALE

RECTANGULAR CHANNEL ANALYSIS & DESIGN OPEN CHANNEL - UNIFORM FLOW

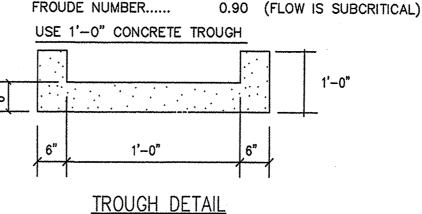
WORK SHEET NAME : PARCEL B & C COMMENT: CONCRETE TROUGH FOR ROOF DISCHARGE FOR C

SOLVE FOR BOTTOM WIDTH

GIVEN INPUT DATA MANNING'S n..... 0.014 CHANNEL SLOPE..... DISCHARGE 1.21 CFS COMPUTED RESULTS

0.67 FT 3.63 fps FLOW AREA..... 0.33 sf FLOW TOP WIDTH ... 0.67 FT

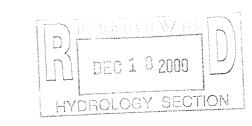
WETTED PERIMETER.. 1.67 FT CRITICAL DEPTH..... 0.47 FT CRITICAL SLOPE..... 0.0119 FT/FT

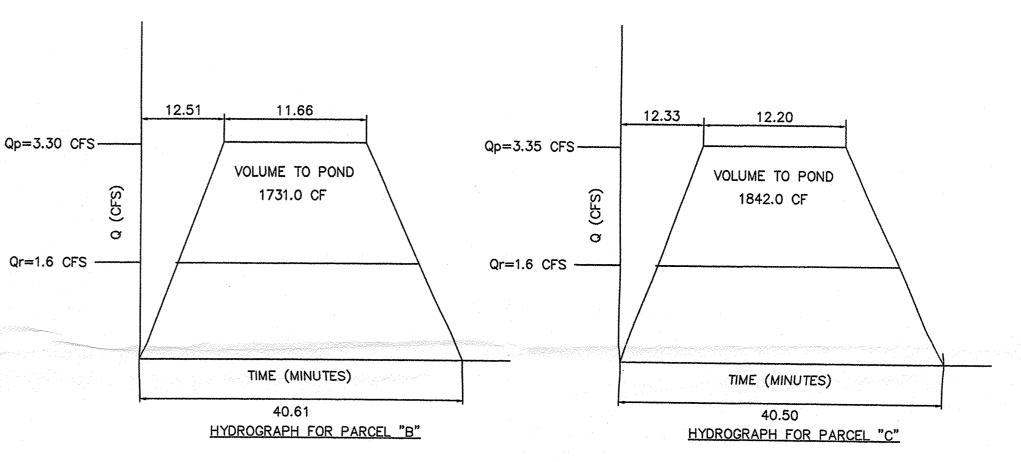


NO SCALE

AMERICAN GYPSUM COMPANY SU-1 M-1 PERM USES JOURNAL

ZONE ATLAS D-17 FIRM PANEL 0136D





REQUIRED PONDING: 3573.0 CF PONDING PROVIDED: 4035.0 CF FREEBOARD: 462.0 CF

NOTE:

PER A MEETING WITH BRAD BINGHAM ON MAY 23,2000 IT WAS DETERMINED THAT AN ALLOWABLE DISCHARGE RATE OF 1.6 CFS PER PARCEL WAS ACCEPTABLE COMPARED TO THE 1.3 CFS ALLOWED PER THE MASTER DRAINAGE PLAN THEREFORE EACH PARCEL WILL BE ALLOWED 1.6 CFS WITH A TOTAL OF 3.2 CFS FOR BOTH PARCELS.

EROSION CONTROL MEASURES

THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR MANAGEMENT OF STORM RUN-OFF DURING CONSTRUCTION, HE SHALL ASSURE THAT THE FOLLOWING MEASURES ARE

- 1. ADJACENT PROPERTY SHALL BE PROTECTED AT ALL TIMES BY TEMPORARY BERMS, DIKES, SWALES, AND OTHER TEMPORARY GRADING AS REQUIRED TO PREVENT STORM RUN-OFF FROM LEAVING THE SITE AND ENTERING ADJACENT PROPERTY.
- 2. ADJACENT PUBLIC RIGHT-OF-WAY SHALL BE PROTECTED AT ALL TIMES FROM STORM WATER RUN-OFF FROM THE SITE. NO SEDIMENT BEARING WATER SHALL BE PERMITTED TO ENTER THE PUBLIC STREETS.
- 3. THE CONTRACTOR SHALL IMMEDIATELY AND THOROUGHLY REMOVE ANY OR ALL SEDIMENT WITHIN THE PUBLIC STREETS THAT HAVE BEEN ERODED FROM THE SITE AND DEPOSITED THERE.

EXISTING CONTOUR PROPOSED CONTOUR DESIGNED SPOT ELEVATION PROPERTY LINE EXISTING SPOT ELEVATION \$82.5

LEGAL DESCRIPTION

PARCEL "B" AND "C" OF PARCELS "A" THRU "H" OF PASEO DEL NORTE INDUSTRIAL PARK SUBDIVISION, ALBUQUERQUE, NEW MEXICO

DOWN SPOUT O

SYMBOL LEGEND

EASEMENT LINE

ABBREVIATION LEGEND

TOP OF CON. PAD TOP OF CURB - TC TOP OF ASPHALT - TA FLOWLINE - FL TOP OF WALL - TW ROOF FLOWS

BENCHMARK: ----

A SQUARE CUT CHISLED ON TOP OF CURB SEE DRAINAGE PLAN: ELEVATION 5080.70

NOTE: "AS-BUILT" ELEVATIONS SHOWN THUS (80.34)

MAY 13,2000 REVISIONS

N V \ll \triangleleft

A H CONSUL OPMENT DEVEL $\stackrel{\cap}{\mathrm{m}}$

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SHEET NO. 1 Of 2