CITY OF ALBUQUERQUE

Planning Department
David Campbell, Director

Mayor Timothy M. Keller

June 24, 2019

David Aube, P.E.
Hartman & Majewski Design Group
120 Vassar Dr SE, Suite 100
Albuquerque, NM, 87106

RE: Hope Christian School – Elementary
6721 Palomas Ave. NE
Grading and Drainage Plan
Engineer's Stamp Date: 06/10/19
Hydrology File: D18D009A

Dear Mr. Aube:

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

Based upon the information provided in your submittal received 06/10/2019, the Grading & Drainage Plan is approved for Building Permit.

Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Prior to approval in support of Permanent Release of Occupancy by Hydrology, Engineer Certification per the DPM checklist will be required.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Curtis Cherne, PE, ccherne@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

Also as a reminder, please provide a Drainage Covenant per Chapter 17 of the DPM for the stormwater quality ponds prior to Permanent Release of Occupancy. Please submit this on the 4th floor of Plaza de Sol. A $25 fee will be required.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov .

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: Hope Christian School Building Permit #: _____ Hydrology File #: D18D009A

DRB#: _____ EPC#: _____ Work Order#: _____

Legal Description: Tract B, Hope Christian School, Lot 23-A, Block 11, Track A Unit A, NAA, Remaining Portion of Lot A, Block11, Tract A, Unit A, NAA, and Hope Christian School

City Address: 6721 Palomas Avenue NE, 87113

Applicant: Hope Christian School Contact: Terry Heisey

Address: 6721 Palomas Avenue NE, 87113

Phone#: 505-822-8858 Fax#: _____ E-mail: tlheisey@hcsnm.org

Other Contact: Hope Christian School Contact: David Aube

Address: 120 Vassar Drive SE

Phone#: 505-998-6430 Fax#: 505-242-6881 E-mail: daube@designgroupnm.com

TYPE OF DEVELOPMENT: ____ PLAT (# of lots) ____ RESIDENCE X DRB SITE ___ ADMIN SITE

IS THIS A RESUBMITTAL? X Yes ____ No

DEPARTMENT ____ TRANSPORTATION X HYDROLOGY/DRAINAGE

Check all that Apply:

TYPE OF SUBMITTAL:

____ ENGINEER/ARCHITECT CERTIFICATION
____ PAD CERTIFICATION
____ CONCEPTUAL G & D PLAN
____ GRADING PLAN
X DRAINAGE REPORT
____ DRAINAGE MASTER PLAN
____ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
____ ELEVATION CERTIFICATE
____ CLOMR/LOMR
____ TRAFFIC CIRCULATION LAYOUT (TCL)
____ TRAFFIC IMPACT STUDY (TIS)
____ STREET LIGHT LAYOUT
____ OTHER (SPECIFY) _____
____ PRE-DESIGN MEETING?

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

____ BUILDING PERMIT APPROVAL
____ CERTIFICATE OF OCCUPANCY

____ PRELIMINARY PLAT APPROVAL
X SITE PLAN FOR SUB'D APPROVAL
____ SITE PLAN FOR BLDG. PERMIT APPROVAL
____ FINAL PLAT APPROVAL

____ SIA/ RELEASE OF FINANCIAL GUARANTEE
____ FOUNDATION PERMIT APPROVAL
____ GRADING PERMIT APPROVAL
____ SO-19 APPROVAL
____ PAVING PERMIT APPROVAL
____ GRADING/ PAD CERTIFICATION
____ WORK ORDER APPROVAL
____ CLOMR/LOMR
____ FLOODPLAIN DEVELOPMENT PERMIT
____ OTHER (SPECIFY) _____

DATE SUBMITTED: 6-10-19 By: David Aube

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

Drainage Summary

Project: Hope Christian School
Project Numbe: 2553
Date: 05/05/19
By: Dave A

Site Location 6721 Palomas Avenue NE

Precipitaion Zone 3 Per Table A-1 COA DPM Section 22.2

Existing summary

Basin Name	EX 1	EX 2	EX 3	EX 4	EX 5	
Area (sf)	41420.42	99535	37025	108444	46960.2	
Area (acres)	0.95	2.29	0.85	2.49	1.08	
%A Land treatment	0	15	0	0	0	
%B Land treatment	0	5	5	0	15	
%C Land treatment	85	55	45	30	20	
%D Land treatment	15	25	50	70	65	
Soil Treatment (acres)						
Area "A"	0.00	0.34	0.00	0.00	0.00	
Area "B"	0.00	0.11	0.04	0.00	0.16	
Area "C"	0.81	1.26	0.38	0.75	0.22	
Area "D"	0.14	0.57	0.42	1.74	0.70	
Excess Runoff (acre-feet)						
100yr. 6hr.	0.1149	0.2751	0.1280	0.4230	0.1734	acre-ft.
10yr. 6hr.	0.0596	0.1452	0.0742	0.2564	0.1036	acre-ft.
2yr. 6hr.	0.0240	0.0639	0.0381	0.1417	0.0564	acre-ft.
100yr. 24hr.	0.1209	0.2989	0.1457	0.4956	0.2026	acre-ft.
Peak Discharge (cfs)						
100 yr.	3.50	8.14	3.56	11.32	4.68	cfs
10yr.	2.10	4.78	2.26	7.40	3.00	cfs
2yr.	0.92	2.17	1.17	4.14	1.63	cfs

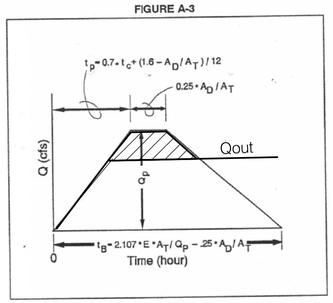
Proposed summary

Basin Name	PRO 1	PRO 2	PRO 3	PRO 4	PRO 5	EX 5	Part PRO 3 East	
Area (sf)	33815	12673	118011	13107	108763	46960.2	82153	
Area (acres)	0.776	0.291	2.709	0.301	2.497	1.08	1.89	
%A Land treatment	30	0	12	0	15	0	20	
%B Land treatment	0	10	28	15	10	15	40	
%C Land treatment	0	0	30	0	20	20	20	
%D Land treatment	70	90	40	85	55	65	20	
Soil Treatment (acres)								
Area "A"	0.23	0.00	0.33	0.00	0.37	0.00	0.38	
Area "B"	0.00	0.03	0.76	0.05	0.25	0.16	0.75	
Area "C"	0.00	0.00	0.81	0.00	0.50	0.22	0.38	
Area "D"	0.54	0.26	1.08	0.26	1.37	0.70	0.38	
Excess Runoff (acre-feet)								
100yr. 6hr.	0.1197	0.0537	0.3765	0.0538	0.3635	0.1734	0.1933	acre-ft.
10yr. 6hr.	0.0716	0.0336	0.2054	0.0333	0.2109	0.1036	0.0952	acre-ft.
2yr. 6hr.	0.0403	0.0196	0.0977	0.0192	0.1114	0.0564	0.0380	acre-ft.
100yr. 24hr.	0.1423	0.0646	0.4217	0.0644	0.4207	0.2026	0.2090	acre-ft.
Peak Discharge (cfs)								
100 yr.	3.16	1.39	10.82	1.40	9.97	4.68	5.86	cfs
10yr.	1.98	0.92	6.39	0.92	6.17	3.00	3.15	cfs
2yr.	1.11	0.54	3.00	0.53	3.24	1.63	1.22	cfs
First Flush Ponding Voulms (cf)	670.7	323.2	1337.5	315.7	NA	NA	465.5	

Pond Routing and Volumes

		Pond 1	Pond 2	Pond 3	Pond 4	
Incoming Flow Rate	Qin	5.86	3.16	1.39	1.4	cfs
Allowable Discharge Rate	Qout	4.40	2.61	1.00	1.07	cfs
Hyrdology Zone		2	2	2	2	Figure A-1
Area Total	At	2.709	0.95	0.291	0.301	acres
Area Type A	Aa	12	30			%
Area Type B	Ab	28		10	15	%
Area Type C	Ac	30				%
Area Type D Impervious	Ad	40	70	90	85	%
Excess runoff rates	A	**0.53**	**0.53**	**0.53**	**0.53**	inches
	B	**0.78**	**0.78**	**0.78**	**0.78**	inches
	C	**1.13**	**1.13**	**1.13**	**1.13**	inches
	D	**2.12**	**2.12**	**2.12**	**2.12**	inches
Weighted E (Exces Runoff)		1.47	1.64	1.99	1.92	in
Time of Concentration		0.2	0.2	0.2	0.2	hours
Time to Peak =0.7*Tc + ((1.6-(Ad/At))/12)		0.240	0.215	0.198	0.203	hours
Time of Base =2.107*E*At/Qp-(.25*Ad/At)		1.331	0.866	0.651	0.657	hours
Duration of Peak		0.100	0.175	0.225	0.213	hours
Time for end of peak		0.340	0.390	0.423	0.415	hours
Time when storage begins		0.180	0.178	0.143	0.155	hours
Time incoming is less that discharge		0.587	0.473	0.487	0.472	hours
Volume Required during storm		0.370	0.129	0.111	0.087	acre inch
		1343	469	403	317	cubic feet

FIGURE A-3



HOPE CHRISTIAN ELEMENTARY SCHOOL, PHASE 1

I. PURPOSE AND SCOPE

The purpose of this drainage plan is to present the existing and proposed drainage management plans for the proposed Classroom Building to be located within the Hope Christian Elementary School located mid-block on Paloma Avenue NE, between San Pedro Boulevard NE, and Louisiana Boulevard NE. The site is located in Zone Atlas Page D-18-Z. The site is currently partially developed.

II. SITE DESCRIPTION AND HISTORY

The site has been previously developed with a mixture of built on site buildings and modular classroom buildings. Several of the buildings are to remain, and the portables will be removed prior beginning Phase 2 development.

III. COMPUTATIONAL PROCEDURES

Hydrologic analysis was performed utilizing the design criteria found in the COA-DPM Section 22.2 released in June 1997.

IV. PRECIPITATION

The 100-yr. 6-hr duration storm was used as the design storm for this analysis. This site is within Zone 3 as identified in the DPM Section 22.2. Tables within the section were used to establish the 6-hr precipitation, excess precipitation and peak discharge.

V. EXISTING DRAINAGE CONDITIONS OVERVIEW

The existing site contains a variety of functions from turf practice fields, buildings, pedestrian circulation paths, vehicular paths, parking areas and vacant dirt lots out to the west. The site is bounded to the east by a site for ABCWUA water tanks, to the north by Paseo Del Norte, to the south by Palomas Avenue NE, and to the West by a site currently under construction for an Assisted Care Facility.

The site generally drains from east to west. The ABCWUA water tanks to the east also generally drain from east to west, but the driveway located on the western side of the water tanks site collects the water and diverts it to the south into Palomas Avenue NE. This prevents offsite drainage from entering the project site from the east.

A majority of the southenr parts of the site (Basins Ex 4 and Ex5) drains gently to south and will discharge into Palomas Avenue NE. The middle portion of the site (Basin Ex1 and Basin Ex2) will drain historically into the neighboring property to the west. There is not a defined concentrated flow location, but more sheet flow in nature. The south western part of the site (Basin Ex 3) will drain back into Palomas. Please refer to the Drainage Summary Table for Peak Flowrates for each of these basins and Excess Runoff Volumes.

VI. DRAINAGE MANAGEMENT PLAN

The site overall drainage patterns will change slightly with the phased construction. In Phase 1 the new building will have a center ridge and drain toward the edges. For Proposed Basin 1 (PRO 1) the peak runoff will be 3.16 cfs and will still drain to the historic discharge point along the western property line. The developed discharge rate is 0.34 cfs less than the historic conditions. The site to the west was designed to accept 2.75 cfs per acres with the historic basin area of 41420 sf giving an allowable discharge of 2.61 cfs onto the property to the west. A portion (255 cf) of the required first flush volume 671 cubic feet that will be collected in a shallow pond at the north west corner of the site. The remainder would be captured within the engineer wood fibers of the playground area. The play areas is 7400sf and would only need to contain 0.056' (less that $\frac{3}{4}$") over the area.

Proposed Basin 2 (PRO 2) will also receive some roof runoff, as well as drainage from a base course fire lane. The basin will generate a peak runoff of 1.39 cfs. The site to the west was designed to accept 2.75 cfs per acre onto the property to the west. When the neighboring drainage study was completed the offsite flows permitted was identified as 6.08 cfs. This would reduce the allowable from Basin 2 to (6.08-2.61=3.47cfs). Due to the size of the basin, the peak runoff will be less than 1.39 cfs generated in the basin due to the routing through the pond and retaining the first flush volume. The proposed runoff is well below the amount that was anticipated onto the parcel to the west. The first flush volume is 323 cubic feet and will be completely contained in a shallow pond (with capacity for 400cf) at the south west corner of the basin. The outfall from this basin will be through openings in the wall that will allow the 1.00 cfs (discharge rate after the 400 cubic feet is removed) to flow into the neighbors property.

VI. DRAINAGE MANAGEMENT PLAN (CONTINUED)

Proposed Basin 3 (PRO 3) contains portions of the new building, the Kindergarten Playground area, existing turf fields and an area that currently contains portable buildings that will be removed once the new building is constructed and the classroom functions are transferred from the portables to the permanent facility. This basin was previous contained within Existing Basin 2 and 3 but with the new building blocking the historic drainage path, the kindergarten playground and loading area will divert the water so the basins are now combined.

The new peak runoff rate from PRO 3 is 10.82 cfs. Historically EX 2 and EX 3 combined created a peak runoff of 11.70 cfs. This basin has also been divided to allow for the eastern portion that drains into the kindergarten play area to be studied. this will generate 5.86 cfs and the outfall from the play area is set at 4.40 cfs. This will set the ponding volume equal to the first flush volume for the entire basin. The lower portion of the basin will discharge directly into Palomas, but an attempt has been made to offset and contain the first flush volume in the upper portion of the basin.

A portion of the roof and paving will flow toward the shallow ponding areas that is created by the Kindergarten Playgound area. The basin will be filled with engineered wood fibers but will still contain some of the first flush volume. This depression will also accept runoff from the eastern portion of the basin. The ponding volume is 1,560 and will be able to contain the first flush volume of 1338 cf.

Once the ponding volume is reached, an overflow drain (sidewalk culvert) will discharge the excess runoff into the drive lane along the east side of the building and then flowing down toward Palomas Avenue NE. The proposed discharge point for PRO 3 is into Palomas Avenue NE as there is a drive lane that diverts the runoff into the South Domingo Baca Arroyo that is located just to the south of Edmund G Ross Elementary School. The peak runoff from this basin will be 10.36 cfs.

Proposed Basin 4 (PRO 4) is a small parking area located along Palomas Avenue NE. This basin creates a peak runoff of 1.40 cfs. Runoff will be routed through the landscaping areas (two internal islands with 1421 sf of surface area) where possible to harvest excess runoff and to contain the First Flush (316 cf). To contain the first flush volume the landscaping areas would need to be depressed an average of .22' (3") . Due to the slopes within the parking area and landscaping areas, we proposed to depress the area 8" on the low side so that the volume is achevied even though the area is sloping. A sidewalk culvert will be required to discharge the runoff under the sidewalk and into Palomas Avenue NE.

Proposed Basin 5 (PRO 5) and Existing Basin (EX 5) are unchanged in the proposed conditions from existing. The peak flow rate and discharge point remain. Retention of First Flush is not required in these basins.

VII. CONCLUSIONS

The site generally drains from east to west, and historic patterns have been maintained. Drainage onto the adjacent parcels has decreased from the historic that will benefit the two properties to the west.

Because the new two story classroom facility is replacing the single story modular buildings, the overall impervious roof surface are is decreasing. There is an increase in parking areas that will keep the overall impervious area approximately the same in the existing and proposed conditions.

Proposed Basin 1 through 4 have been designed to retain the first flush volumes. This is accomplished by directing excess runoff to shallow ponds prior to the overflow that directs runoff toward the western property line or out into Palomas Avenue NE. Proposed Basin 5 and Existing Basin 5 do not contain any new impervious surface and therefore is not affected by the first flush requirements.

PROJECT LOCATION:
SOUTHWEST CORNER OF PASEO DEL NORTE AND LOUISIANA BOULEVARD NE

LEGAL DESCRIPTIONS:
TRACT B, HOPE CHRISTIAN SCHOOL,
LOT 23-A, BLOCK 11, TRACT A, UNIT A, NORTH ALBUQUERQUE ACRES,
REMAINING PORTION LOT A, BLOCK 11, TRACK A, UNIT A NORTH ALBUQUERQUE ACRES,
LOT 13A, BLOCK 11, TRACK A, UNIT A, NORTH ALBUQUERQUE ACRES, AND
HOPE CHRISTIAN SCHOOL

ZONE ATLAS PAGE:
D-18-Z

TOTAL ACREAGE:
ELEMENTARY SCHOOL = 6.752 ACRES

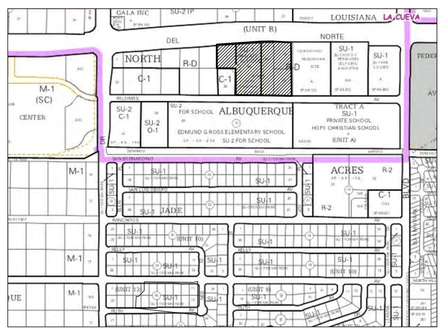
EXISTING ZONING:
ELEMENTARY SCHOOL IS DESIGNATED MX-M
ADDITIONAL PARCEL TO BE ADDED TO ELEMENTARY SCHOOL IS DESIGNATED PD , BUT WILL BE REVIEWED UNDER VOLUNTARY ZONING CONVERSION TO CHANGE TO MX-M

MIDDLE SCHOOL IS DESIGNATED MX-M

HIGH SCHOOL IS DESIGNATED MX-L

PROPOSED USES:
PRIVATE SCHOOL

B5 FIRMETTE PAGE
SCALE: NOT TO SCALE 35001C0137H



A5 ZONE ATLAS PAGE
SCALE: NOT TO SCALE

DAVID A. AUBE
STATE OF NEW MEXICO
14221
REGISTERED PROFESSIONAL ENGINEER
6-10-19

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HOPE CHRISTIAN ELEMENTARY

REVISIONS	PHASE	DATE	
	CD	5.6.19	DRAINAGE PLAN NARRATIVE

CD-1

PROJECT CONTROL POINT
PK Nail and aluminum disc stamped "Surv-Tek, Inc- Control"
Modified project surface coordinates:
N= 1,518,913.00
E= 1,543,386.00
ELEV= 5265.25 feet (NAVD 88)

PASEO DEL NORTE N.E.

EX 1
Q_{100} = 3.50 CU-FT/SEC
Q_2 = 0.92 CU-FT/SEC
AREA=41,420 sf

EX 2
Q_{100} = 8.14 CU-FT/SEC
Q_2 = 2.17 CU-FT/SEC
AREA=99,535 sf

EX 3
Q_{100} = 3.56 CU-FT/SEC
Q_2 = 1.17 CU-FT/SEC
AREA=37,025 sf

EX 4
Q_{100} = 11.32 CU-FT/SEC
Q_2 = 4.14 CU-FT/SEC
AREA=108,444 sf

EX 5
Q_{100} = 4.68 CU-FT/SEC
Q_2 = 1.63 CU-FT/SEC
AREA=46,960 sf

AP 1
Q_{100} = 3.50 CU-FT/SEC

AP 2
Q_{100} = 8.14 CU-FT/SEC

AP 3
Q_{100} = 3.56 CU-FT/SEC

AP 4
Q_{100} = 3.61 CU-FT/SEC

NOTES

THE PARCELS TO THE WEST OF THE SUBJECT PROPERTY HAVE BEEN DESIGNED TO ACCEPT OFF-SITE FLOWS FROM THIS PROJECT IN THE AMOUNT OF 2.75 CFS/ACRE.
THIS ALLOWS FOR A TOTAL OF 6.08 CFS TO BE DISCHARGED ACROSS THE WESTERN PROPERTY LINE OF THE HOPE CHRISTIAN SCHOOL PROPERTY.

1 SITE DRAINAGE PLAN - EXISTING CONDITIONS
1" = 30'-0"

DAVID A. AUBE
STATE OF NEW MEXICO
14321
REGISTERED PROFESSIONAL ENGINEER
6-10-19

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HOPE CHRISTIAN ELEMENTARY

REVISIONS	PHASE	DATE
	CD	5.6.19

DRAINAGE PLAN
EXISTING CONDITIONS

CD-2

PROJECT CONTROL POINT
PK Nail and aluminum disc stamped "Surv-Tek, Inc- Control"
Modified project surface coordinates:
N= 1,518,913.00
E= 1,543,386.00
ELEV= 5265.25 feet (NAV 88)

PASEO DEL NORTE N.E.

POND 2
Vol=255cf

POND 3
Vol=400cf

POND 1
Vol=1340cf

POND 4
Vol=316cf

PRO 1
Q_{100} = 3.16 CU-FT/SEC
Q_E = 1.11 CU-FT/SEC
AREA=33,815 sf
area 33814.86

PRO 2
Q_{100} = 1.39 CU-FT/SEC
Q_E = 0.54 CU-FT/SEC
AREA=12,673 sf
area 12673.07

PRO 3
Q_{100} = 10.82 CU-FT/SEC
Q_E =3.00 CU-FT/SEC
AREA=118,011 sf
area 108763.01

PRO 4
Q_{100} = 1.40 CU-FT/SEC
Q_E =0.53 CU-FT/SEC
AREA=13,107 sf
area 13106.92

PRO 5
Q_{100} = 9.97 CU-FT/SEC
Q_E =3.24 CU-FT/SEC
AREA=108,763 sf

EX 5
Q_{100} = 4.68 CU-FT/SEC
Q_E = 1.63 CU-FT/SEC
AREA=46,960 sf
area 46960.20

AP 1
Q_{100} = 3.16 CU-FT/SEC
Q (allowed) = 2.61 CU-FT/SEC

AP 2
Q_{100} = 1.39 CU-FT/SEC

AP 3
C_{100} in = 5.86 CU-FT/SEC
Cout = 4.40 cfs

AP 4
Q_{100} = 10.36 CU-FT/SEC

AP 5
Q_{100} = 1.07 CU-FT/SEC
NEW SIDEWALK CULVERT

EXISTING SIDEWALK CULVERT

EXISTING SIDEWALK CULVERT

EXISTING SIDEWALK CULVERT

NOTES

THE PARCELS TO THE WEST OF THE SUBJECT PROPERTY HAVE BEEN DESIGNED TO ACCEPT OFF-SITE FLOWS FROM THIS PROJECT IN THE AMOUNT OF 2.75 CFS/ACRE.
THIS ALLOWS FOR A TOTAL OF 6.08 CFS TO BE DISCHARGED ACROSS THE WESTERN PROPERTY LINE OF THE HOPE CHRISTIAN SCHOOL PROPERTY.
PROPOSED BASIN 2 AND 3 WILL GENERATE A COMBINED EXCESS RUNOFF RATE OF 4.53 CFS.

BP: 0+00.00
EP: 9+24.12
PALOMAS AVENUE N.E.

1 SITE DRAINAGE PLAN - PROPOSED CONDITIONS
1" = 30'-0"

DAVID A. AUBE
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6-10-19

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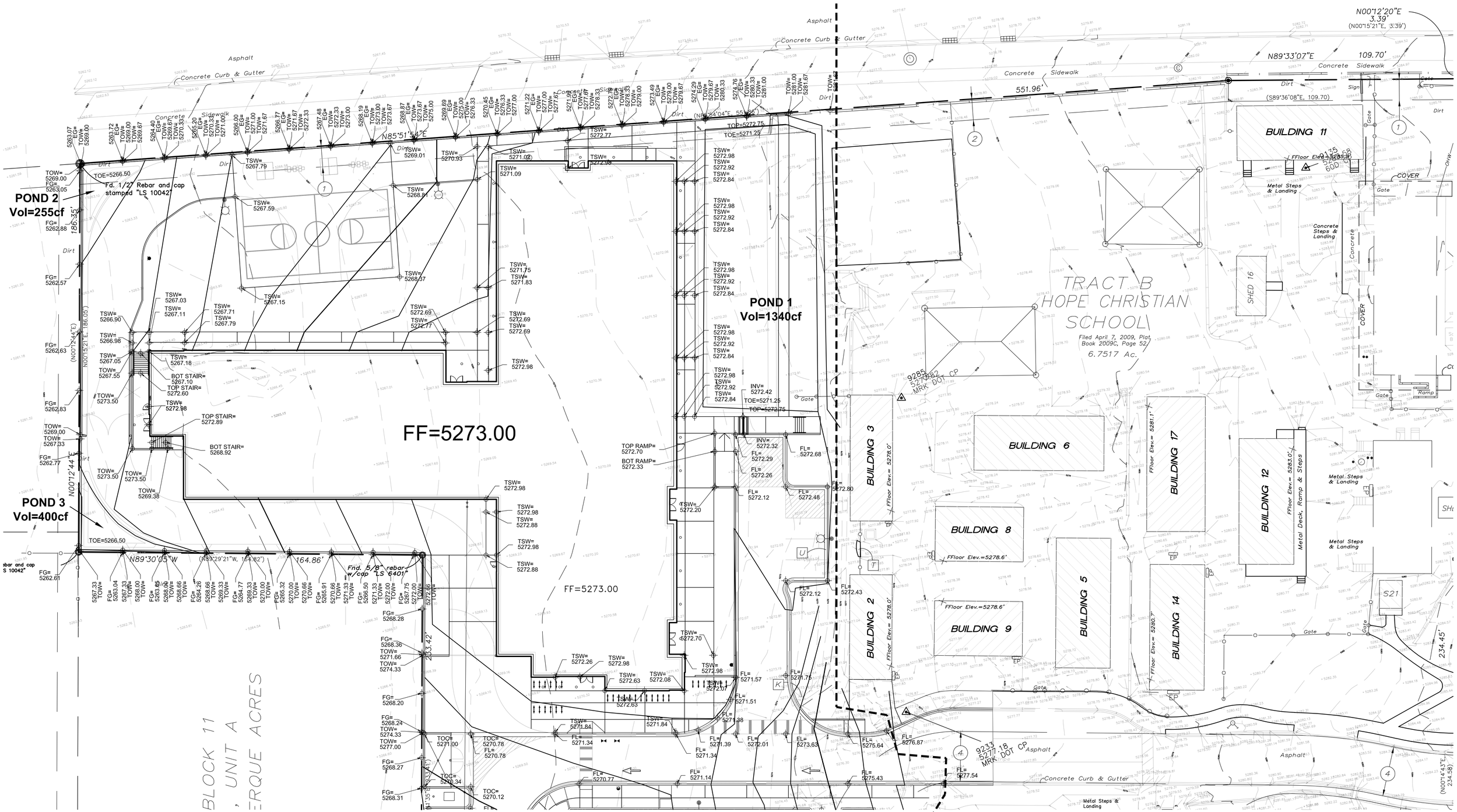
HOPE CHRISTIAN ELEMENTARY

REVISIONS	PHASE	DATE
	CD	5.6.19

DRAINAGE PLAN
PROPOSED CONDITIONS

CD-3



1 SITE GRADING PLAN - ENLARGED
1" = 20'-0"

GRADING PLAN GENERAL NOTES

1. SEE SHEET CG01 FOR COMPLETE LIST OF GENERAL NOTES AND SYMBOL/LINETYPE LEGEND THAT APPLY TO ALL SHEETS.

DAVID A. AUBE
STATE OF NEW MEXICO
14221
REGISTERED PROFESSIONAL ENGINEER
6-10-19

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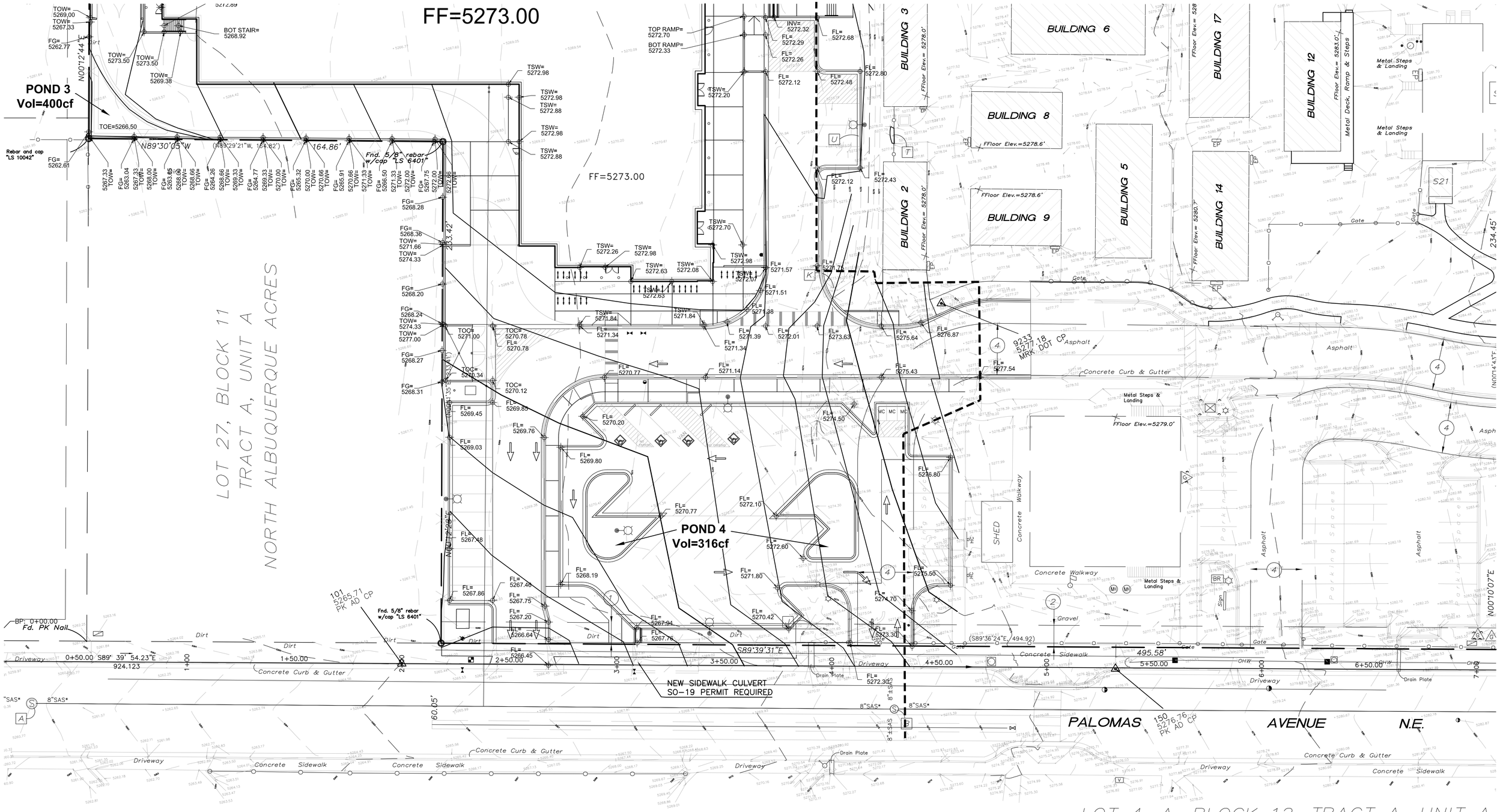
HOPE CHRISTIAN ELEMENTARY

REVISIONS	PHASE	DATE	
	CD	5.6.19	ENLARGED SITE GRADING PLAN

C-202



1 SITE GRADING PLAN - ENLARGED
1" = 20'-0"

GRADING PLAN GENERAL NOTES

1. SEE SHEET CG01 FOR COMPLETE LIST OF GENERAL NOTES AND SYMBOL/LINETYPE LEGEND THAT APPLY TO ALL SHEETS.

DAVID A. AUBE
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REVISIONS	PHASE	DATE
	CD	5.6.19

ENLARGED SITE GRADING PLAN

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