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PRECIPITATION: 10 days = 3.95 in.
RAINFALL INTENSITY = 5.05
RUNOFF COEFFICIENTS:
TREATMENT A
                0.31
TREATMENT B
                 0.45
                0.62
TREATMENT C
TREATMENT D
                0.93
EXISTING CONDITIONS:
                                 PROPOSED CONDITIONS:
                                 AREA
TREATMENT A
                0.00 ac.
                                  0.00 ac.
TREATMENT B
                0.00 ac.
                                  0.00 ac.
TREATMENT C
               2.79 ac.
                                  2.65 ac.
TREATMENT D
               0.59 ac.
                                  0.73 ac.
                                  3.38
                 3.38
EXISTING RUNOFF VOLUME:
 Weighted C = (0.31)x(0.00)+(0.45)x(0.00)+(0.62)x(2.79)+(0.93)x(0.59)/(0.59)/(0.67)
  V100-360 = (0.67)x(3.95)x(3.38)/ 12 = 0.7500 ac-ft = 32670 cf
EXISTING PEAK DISCHARGE:
     Q100 = (0.67)x(5.05)x(3.38) = 11.51 cfs
PROPOSED EXCESS PRECIPITATION:
 Weighted C = (0.31)x(0.00)+(0.45)x(0.00)+(0.62)x(2.65)+(0.93)x(0.73)/(3.38 ac. = 0.69)
  V100-360 = (0.69)x(0.3.95)x(0.3.38)/ 12 = 0.7643 ac-ft = 33293 cf
PROPOSED PEAK DISCHARGE:
     Q100 = (0.69)x(5.05)x(3.38) = 11.73 cfs
RESULTS:
                33293 - 32670 = 622 cf
                                               Increase in runoff volume
                11.73 - 11.51 = 0.22 \text{ cfs}
                                               Increase in peak discharge
SIDEWALK CULVERT CALCULATION:
   Q = CA \sqrt{2gh}
                                      C = 0.50
                                       A= 1.34 FOR A SINGLE 24" SW CULVERT
                                       h= 0.80
                                      g= 32.20
Q for single 24" SW Culvert= 4.809 cfs
Required # of SW culverts based on Exist Peak Discharge= 2.39 Round to 3-24" SW Culverts
DRAINAGE NOTES
THE EXISTING SITE GENERALLY DRAINS FROM EAST TO WEST AND
CURRENTLY HAS A FEW STRUCTURES ONSITE.
 THE PROPOSED IMPROVEMENTS FOR THE SITE INCLUDE AN ADDITIONAL
BUILDING FOR MAINTENANCE PURPOSES.
THE HYDROLOGY CALCULATION USED WERE TAKEN FROM THE CITY OF
ALBUQUERQUE DESIGN PROCESS MANUAL. THE PRECIPITATION ZONE IS
ZONE 2. SEE THE DRAINAGE CALCULATIONS (THIS SHEET) FOR MORE
INFORMATION.
THE PROPOSED ON-SITE RETENTION/DETENTION POND IS REQUIRED TO
CAPTURE THE 100-YR 10 DAY FLOOD EVENT (622 CF) AND THE POND
DESIGN IS 826 CF. THE DETENTION PORTION OF THE POND DRAINS WITHIN
24 HOURS.
AS SHOWN ON THE FEMA FLOODPLAIN MAP (THIS SHEET) THERE IS NO
FLOODPLAIN IMPACTING THE SITE. THERE IS A FLOODPLAIN, ZONE AO,
ADJACENT TO THE SITE WITHIN SAN FRANCISCO RD.
 THE 3-24" SIDEWALK CULVERTS WERE SIZED USING A FLOW OF 11.51 CFS
 (EXISTING FLOWRATE SINCE THERE IS A PROPOSED POND). CHAPTER 22
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SECTION 3.A.2 (ORIFICES) WAS USED TO CALCULATE THE SIDEWALK CULVERT

CAPACITY. THE C VALUE ASSUMED WAS 0.5.

PROJECT:

NW CORNER SAN PEDRO/SAN FRANCI

DRAINAGE CALCULATIONS

100 year

AREA = 3.38 ac.

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AHYMO-I-9702a01000150-SH
          INPUT FILE = G:\Proj\SdvlCnty\2007CO~1\temp\SANPED~1\duggers.txt
    *S Data File: windev7.dat
   *S Compute 100-Year Flow
   *S Use 6 Hour Storm
   START
                      TIME=0.0 CODE 0 LINES -6
*$
   *S USE COA DPM ZONE 2 RAINFALL
RAINFALL
                      TYPE=-1 RAIN ONE=2.01
                        RAIN SIX=2.35 RAIN DAY=2.75 DT=0.050
               COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 -
PEAK AT 1.40 HR.
               DT = .033333 HOURS END TIME = 5.999940 HOURS
*S COMPUTE BASIN
    COMPUTE NM HYD
                      ID=1 HYD NO=BAS.1 DA=0.00528 SQ MI
                      PER A=0.00 PER B=0.00 PER C=78 PER D=22 TP=-.133
                      MASS RAINFALL=-1
       K = .072485HR TP = .133000HR K/TP RATIO = .545000 SHAPE
CONSTANT, N = 7.106420
        UNIT PEAK = 4.5964 CFS UNIT VOLUME = .9970 B = 526.28
P60 = 2.3500
        AREA = .001162 \text{ SQ MI} IA = .10000 \text{ INCHES} INF = .04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT
= .033333
       K = .111031HR TP = .133000HR K/TP RATIO = .834817 SHAPE
CONSTANT, N = 4.271263
        UNIT PEAK = 11.546 CFS UNIT VOLUME = .9992 B = 372.87
        AREA = .004118 SQ MI IA = .35000 INCHES INF = .83000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT
= .033333
    PRINT HYD
                      ID=1 CODE=1
                                    HYDROGRAPH FROM AREA BAS.1
       RUNOFF VOLUME = 1.69680 INCHES = .4778 ACRE-FEET
       PEAK DISCHARGE RATE = 14.11 CFS AT 1.500 HOURS BASIN AREA =
    *S ROUTE THROUGH ONSITE POND
    *S NEW RATING CURVE
                      ID=20 HYD=POND INFLOW=1 CODE=10
    ROUTE RESERVOIR
                             OUTFLOW STORAGE ELEV
                                           5229.0
                                       0.004 5229.5
                             0.020
                                      0.013
                                              5230.0
                            8.835
                                      0.027
                                              5230.5
                                     0.045 5231.0
                            14.427
     TIME
               INFLOW ELEV
                              VOLUME
                                      OUTFLOW
       (HRS)
               (CFS)
                      (FEET)
                               (AC-FT)
                                      (CFS)
                      5229.00
                     5229.00
                     5229.00
                                 .000
                      5229.00
                                 .000
                                 .010
        1.33
                2.62 5229.85
        1.67
                7.70 5230.54
                                        9.25
                     5230.17
                                 .018
                                        2.95
        2.33
                 .65 5230.04
                                 .014
                                         .69
        2.67
                     5230.01
                                 .013
                     5230.01
                                 .013
                                         .11
        3.33
3.67
                     5230.00
                                 .013
                 .03
                     5230.00
                                 .013
                                         .03
        4.00
                     5230.00
                                 .013
                                 .013
        4.33
                     5230.00
        4.67
                     5230.00
                                         .03
                                 .013
        5.00
                     5230.00
        5.33
5.67
                                 .013
                                         .03
                     5230.00
                     5230.00
        6.00
                                 .013
                     5230.00
                                 .013
.012
.012
                     5229.99
                                         .02
        6.67
                     5229.96
        7.00
                     5229.93
                      5229.90
                                 .011
                                 .011
        7.67
                     5229.88
                                         .02
        8.00
                     5229.85
                                         .02
                      5229.82
                                 .009
                                         .02
        8.67
                      5229.80
        9.00
                     5229.78
                                 .009
                     5229.75
                                 .008
.008
.007
        9.67
                     5229.73
                                         .01
       10.00
                     5229.71
       10.33
                     5229.69
       10.67
                     5229.67
                                         .01
                                 .007
       11.00
                 .00 5229.65
       11.33
                     5229.63
                                 .006
                     5229.61
                                         .01
       12.00
12.33
                                 .006
                     5229.59
                 .00 5229.57
                                 .005
       12.67
                  .00 5229.55
                                         .01
       13.00
                     5229.54
       13.33
                                 .004
                 .00 5229.52
       13.67
                 .00 5229.51
       14.00
                 .00 5229.48
                                 .004
                                 .004
       14.33
                 .00 5229.45
       14.67
                 .00 5229.42
       15.00
                 .00 5229.39
                                 .003
                                         .01
                 .00 5229.36
.00 5229.34
       15.33
15.67
                                 .003
                                         .01
                     5229.32
                                 .003
       16.33
                 .00 5229.29
                                 .002
                                         .01
                 .00 5229.28
       16.67
                                         .01
       17.00
                 .00 5229.26
                                 .002
                                         .01
       17.33
                 .00 5229.24
     PEAK DISCHARGE = 13.248 CFS - PEAK OCCURS AT HOUR 1.53
     MAXIMUM WATER SURFACE ELEVATION = 5230.895
                       .0412 AC-FT INCREMENTAL TIME= .033333HRS
     MAXIMUM STORAGE =
    FINISH
       NORMAL PROGRAM FINISH
                               END TIME (HR:MIN:SEC) = 11:06:11
 □(s0p10h4099T□&16D
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AHYMO OUTPUT FILE

AHYMO PROGRAM (AHYMO 97) -

RUN DATE (MON/DAY/YR) = 05/24/2013

START TIME (HR:MIN:SEC) = 11:06:11 USER NO.=

- Version: 1997.02c

☐(s16.67h8.5v0T☐&18D

MAY 3 0 2013
LAND DEVELOPMENT SECTION

GRADING & DRAINAGE PLA

DUGGER'S

NORTHWEST CORNER OF

SAN PEDRO & SAN FRANCE

DESIGNED BY: DPLW
DRAWN BY: RA

DRAWN BY: RA
CHECKED BY: RA

PROJECT NO. XXXX

DATE: May 29, 2013

DEMEULE CONSULTING

SHEET:

G2









