

2. PEAK DISCHARGE

B. LOT 19

1. VOLUME

 $V_{100} = (E_W/12)A_T$

2. PEAK DISCHARGE

C. LOT 20

VOLUME

2. PEAK DISCHARGE

VIII. COMPARISON

A. LOT 18

B. LOT 19

C. LOT 20

C = 2.60

L = 5.0'

 $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

 $V_{100} = (2.18/12)(38,610) = 7,010 \text{ CF}$

PAAA+QPBAB+QPCAC + QPDAD

 $= Q_{100} = (2.60)(0.11) + (5.02)(0.78) = 4.2 \text{ GFS}$

 $E_W = [(0.92)(0.11)+(2.36)(0.78)]/(0.89) = 2.18 \text{ IN.}$

 $= Q_{100} = (2.60)(0.11)+(5.02)(0.78) = 4.2 \text{ CFS}$

 $\Delta V_{100} = 7010 - 4150 = 2860 \text{ CF (INCREASE)}$

= 7010 - 5050 = 1960 CF (INCREASE)

4150 = 2860 CF (INCREASE)

 $\Delta Q_{100} = 4.2 - 3.1 = 1.1 \text{ CFS (INCREASE)}$

 $\Delta Q_{100} = 4.2 - 3.4 = 0.8 \text{ CPS (INCREASE)}$

// IX. ENTRANCE CONDITION CALCULATIONS

X. RUNDOWN CAPACITY CALCULATIONS

Q₁₀₀= 4.2 CFS PER LOT

n = 0.013 (CONCRETE)

S = 0.01 (MINIMUM)

DEPTH = 0.5 0.4

WIDTH = 2.0'

 $Q_{CAP} = CLH^{3/2}$ (BROAD CRESTED WEIR)

L = 5.0' $0.44_{3/2}$ 3.8 < $Q_{CAP} = (2.60)(5.0)(0.5)^{3/2} = \frac{3.8}{4.6} \text{ CFS} \sim Q_{100}$

3.8/4.2 = 90% 0,00

 $Q_{CAP} = (1.49/n) AR^{2/3} S^{1/2}$ (MANNING'S EQUATION)

 $Q_{CAP} = (1.49/0.013)(\frac{4.0}{1.0})(0.333)^{2/3}(\frac{6.01}{1/2})^{1/2} = \frac{4.2}{5.5} \text{CFS} > Q_{100} = 4.2 \text{ CFS}$

Q₁₀₀= 4.2 CFS PER LOT

 $H = 0.50^{\circ} 0.44$

= 4.2 - 3.1 = 1.1 CFS (INCREASE)

 $V_{100} = (2.18/12)(38,610) = 7,010 \text{ CF}$

 $Q_p = Q_{100} = (2.60)(0.11)+(5.02)(0.78) = 4.2 CFS$

 $E_W = [(0.92)(0.11) + (2.36)(0.78)]/(0.89) = 2.18 \text{ IN}.$

IV. EXISTING LAND TREATMENT

A. LOT 18

TREATMENT AREA (SF/AC) % C 38,610/0.89 100

B. LOT 19

TREATMENT AREA (SF/AC) % C 28,770/0.66 74 D 9,840/0.23 26

C. LOT 20

TREATMENT AREA (SF/AC) % C 38,610/0.89 100

V. DEVELOPED LAND TREATMENT

A. LOT 18

TREATMENT AREA (SF/AC) % B 4,680/0.11 12 D 33,930/0.78 88

B. LOT 19 TREATMENT AREA (SF/AC) %

B 4,680/0.11 12 D 33,930/0.78 88

C. LOT 20

TREATMENT AREA (SF/AC) % B 4,680/0.11 12 D 33,930/0.78 88

VI. EXISTING CONDITION

A. LOT 18

VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C E_D A_D)/A$

 $E_W = 1.29 \text{ IN}.$ $V_{100} = (E_W/12)A_T$

 $V_{100} = (1.29/12)(38,610) = 4,150 \text{ CF}$

2. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

 $Q_p = Q_{100} = (3.45)(0.89) = 3.1 CFS$ B. LOT 19

VOLUME

 $T_{W} = (1.29)(0.66) + (2.36)(0.23)/0.89 = 1.57 \text{ IN}.$

 $V_{100} = (1.57/12)(38,610) = 5,050 \text{ CF}$

2. PEAK DISCHARGE

= (3.45)(0.66) + (5.02)(0.23) = 3.4 CFS

(NA)

LOT 20

1. VOLUME

V₁₀₀ = (1.29/12)(8,610) = 4,150 CF

2. PEAK DISCHARGE OPAAA+QPBAB+QPCAC

 $= Q_{100} = (3.45)(0.89) = 3.1$

VII. DEVELOPED CONDITION

A. LOT 18

1. VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C E_D A_D) / A_T$

 $E_W = [(0.92)(0.11) + (2.36)(0.78)]/(0.89) = 2.18 \text{ IN}$

 $V_{100} = (E_W/12)A_T$

 $V_{100} = (2.18/12)(38,610) = 7,010 \text{ CF}$

EXECUTIVE SUMMARY AND INTRODUCTION:

THIS GRADING AND DRAINAGE PLAN SUPPORTS THE CONSTRUCTION OF THREE (3) PROPOSED OFFICE/WAREHOUSE BUILDINGS WITH ASSOCIATED PAVING AND LANDSCAPING IMPROVEMENTS. THE THREE SITES WILL BE DEVELOPED BY THE SAME DEVELOPER AS THREE INDIVIDUAL BUILDINGS ON SEPARATE LOTS (LOTS 18, 19 AND 20). THE SITES WILL USE SHARED ACCESS TO CORONADO AVE. N.E. THIS SHARED ACCESS REQUIRES PRIVATE ACCESS EASEMENTS WHICH WILL BE GRANTED BY SEPARATE DOCUMENTS. THE SHARED ACCESS CONFIGURATION ALSO NECESSITATES THE GRANTING OF PRIVATE, NON-SPECIFIC DRAINAGE EASEMENTS TO ADDRESS THE RESULTING CROSS-LOT DRAINAGE. ALTHOUGH THIS GRADING AND DRAINAGE PLAN DEPICTS AND ANALYZES ALL THREE LOTS, THE INDIVIDUAL SITES WILL BE PERMITTED AND CONSTRUCTED SEPARATELY. FOR THIS REASON, THIS GRADING AND DRAINAGE PLAN WILL BE SUBMITTED TO THE CITY OF ALBUQUERQUE HYDROLOGY SECTION THREE TIMES, ONCE FOR EACH BUILDING. EACH OF THESE THREE SUBMITTALS WILL "STAND ALONE" AND ALLOW INDIVIDUAL DRAINAGE CERTIFICATIONS AND CERTIFICATES OF OCCUPANCY FOR EACH BUILDING. MINIMAL OFFSITE GRADING WILL BE REQUIRE TO ALLOW PROJECT PHASING. TWO OF THE LOTS ARE CURRENTLY UNDEVELOPED. LOT 19 IS DEVELOPED. ALL THREE LOTS ARE ZONED M-1. THE PROPOSED IMPROVEMENTS WILL CAUSE AN INCREASE IN IMPERVIOUS AREA AND WILL INCREASE THE PEAK RATE AND VOLUME OF RUNOFF FROM THE SITES AS DEMONSTRATED IN THE DRAINAGE CALCULATIONS CONTAINED HEREON. THE IMPROVEMENTS CONSIST OF MODIFICATIONS TO EXISTING SITES WITHIN AN INFILL AREA WHICH DRAIN TO THE NORTH ARROYO DEL PINO. THIS PLAN PROPOSES AND JUSTIFIES THE FREE DISCHARGE OF DEVELOPED RUNOFF FROM THESE PROPERTIES TO THE CONCRETE LINED PUBLIC DRAINAGE CHANNEL. AS INDICATED BY THE ATTACHED DRAINAGE INFORMATION SHEETS, THE PURPOSE OF THESE SUBMITTALS IS TO OBTAIN BUILDING PERMIT APPROVAL AND S.O.19 APPROVA FOR ALL THREE SITES.

REFERENCES:

THE FOLLOWING IS A LIST OF PREVIOUSLY APPROVED GRADING AND DRAINAGE PLANS RELATIVE TO THIS SITE AND/OR REFERENCED WITHIN THIS DRAINAGE PLAN. THIS LIST MAY NOT BE INCLUSIVE, HOWEVER, REPRESENTS A SUMMARY OF THOSE PLANS WHICH ARE KNOWN TO THIS PREPARER.

1. GRADING AND DRAINAGE PLAN FOR COLONY METALS PREPARED BY WILSON AND COMPANY, DATED 7/22/92 (D-18/D25). THIS PLAN WAS PREPARED FOR THE EXISTING IMPROVEMENTS ON LOT 19. THIS PLAN PROVIDED FOR FREE DISCHARGE OF DEVELOPED RUNOFF FROM THIS LOT TO CORONADO AVE. N.E. AND THE PUBLIC STORM DRAIN CONTAINED THEREIN.

2. CONSTRUCTION PLANS AND DRAINAGE REPORT FOR S.A.D. 221 (CITY PROJECT 3824) PREPARED BY WILSON AND COMPANY, DATED 05/12/95. THIS PROJECT CONSTRUCTED PUBLIC IMPROVEMENTS WITHIN SEVERAL CITY OF ALBUQUERQUE STREETS INCLUDING PERMANENT PAVING AND STORM DRAIN IMPROVEMENTS IN CORONADO AVE. N.E. THIS STORM DRAIN DISCHARGES DIRECTLY TO THE NORTH ARROYO DEL PINO WHICH IS CONCRETE LINED. THE DRAINAGE REPORT FOR CORONADO AVE. N.E. SHOWS DEVELOPED RUNOFF FROM THE SOUTH HALF OF LOTS 18-20 DISCHARGING DIRECTLY TO THE STREET, LEAVING THE NORTH HALF OF THE LOTS TO DRAIN DIRECTLY TO THE CONCRETE LINED ARROYO.

3. GRADING AND DRAINAGE PLAN FOR THE HOLY CROSS LUTHERAN CHURCH (E-19/D21) PREPARED BY JEFF MORTENSEN AND ASSOCIATES DATED 03/05/92. SECTIONS AND DETAILS FROM THE CHURCH PLAN ARE REFERENCED AND REPRODUCED HEREIN.

PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP ON SHEET 1, THE SITES ARE LOCATED ON THE NORTH SIDE OF CORONADO AVE. N.E., WEST OF SAN PEDRO BOULEVARD, N.E. THE LOTS ARE ADJACENT TO AND SOUTH OF THE NORTH ARROYO DEL PINO, A CONCRETE LINED PUBLIC DRAINAGE CHANNEL OWNED, OPERATED AND MAINTAINED BY THE CITY OF ALBUQUERQUE. LOTS 18 AND 20 ARE UNDEVELOPED AND CONTAIN BARE SOIL WITH MINIMAL VEGETATION. LOT 20 CONTAINS A FEW SMALL TREES. L'IT 19 IS DEVELOPED WITH TWO BUILDINGS AND ASSOCIATED PAVING AND LANDSCAPING THE LEGAL DESCRIPTIONS OF THE SUBJECT LOTS ARE SHOWN ON SHEET 1. THE SITES ARE ZONED M-1 AND LIE WITHIN AN INFILL AREA.

AS SHOWN BY PANEL 137 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS FOR BERNALILLO COUNTY, NEW MEXICO, AND INCORPORATED AREAS DATED SEPTEMBER 20, 1996, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE. THE PANEL IDENTIFIES 100-YEAR FLOODING CONFINED TO THE NORTH ARROYO DEL PINO CONSTRUCTED CHANNEL.

EXISTING CONDITIONS:

AS SHOWN BY THE GRADING PLAN, THE PROJECT CONSISTS OF THREE SEPARATE SITES LOTS 18 AND 20 ARE UNDEVELOPED. LOT 19 IS DEVELOPED WITH TWO (2) EXISTING BUILDINGS AND ASSOCIATED PAVED PARKING AND LANDSCAPING IMPROVEMENTS. ALL THREE LOTS GENERALLY DRAIN FROM EAST TO WEST IN A POORLY DEFINED SHEETFLOW MANNER WHICH IS PARTIALLY OBSTRUCTED BY SMALL BERMS AT THE PROPERTY LINES WHICH APPEAR TO DIRECT LIMITED AMOUNTS OF RUNOFF TO THE NORTH ARROYO DEL PINO, AND TO CORONADO AVE. N.E. SOME EXISTING RUNOFF FROM LOT 20 DRAINS TO THE WEST TO LOT 21 WHICH IS NOT PART OF THIS PLAN AND LIES TOPOGRAPHICALLY LOWER. A CONCRETE WALL BLOCKS OFFSITE FLOWS FROM LOT 17 TO THE EAST. OFFSITE FLOWS DO NOT ENTER THE SITES FROM THE CONCRETE LINED PUBLIC DRAINAGE CHANNEL TO THE NORTH, OR FROM CORONADO AVE. N.E. WHICH IS DEVELOPED WITH HALF WIDTH PAVING AND CURB AND GUTTER IMPROVEMENTS (NORTH HALF). CORONADO AVE. N.E. ALSO CONTAINS A PUBLIC STORM DRAIN THAT DISCHARGES DIRECTLY TO THE NORTH ARROYO DEL PINO. THE CORONADO AVE. N.E. PAVING AND STORM DRAIN IMPROVEMENTS WERE CONSTRUCTED AS PART OF S.A.D. 221 AND ARE SIZED FOR DEVELOPED RUNOFF PER THE DRAINAGE REPORT FOR S.A.D. 221. WITH THE EXCEPTION OF THE AFOREMENTIONED RUNOFF TO LOT 21, THESE SITES ALL CURRENTLY DRAIN TO EXISTING PUBLIC STORM DRAINAGE FACILITIES. THE EXISTING GRADES, AS INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'0" INTERVALS SUPPORT THESE

CERTIFICATION

AS INDICATED BY THE AS-BUILT INFORMATION SHOWN HEREON THIS PROJECT HAS BEEN GRADED AND DRAINED IN SUBSTANTIAL COMPLIANCE WITH THE APPROVED PLAN. WITH THE EXCEPTION OF THE CURB AND GUTTER THAT WERE BUILT FLATTER THAN DESIGNED. THE PROPERTY OWNER HAS ACKNOWLEDGED THIS CONDITION AND HAS ACCEPTED RESPONSIBILITY FOR THIS DEVIATION (SEE SHEET 1). IT IS BASED UPON THIS EVALUATION OF AS-CONSTRUCTED CONDITIONS THAT ISSUANCE OF A PERMANENT CERTIFICATE OF OCCUPANCY IS HEREBY RECOMMENDED. THE AS-BUILT INFORMATION SHOWN HEREON HAS BEEN OBTAINED BY ME OR UNDER MY DIRECT SUPERVISION AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

JEFFREY G. MORTENSEN. NMPE 8547

DEVELOPED CONDITIONS:

DRAINAGE PLAN

AS PREVIOUSLY INDICATED, THE PROPOSED IMPROVEMENTS CONSIST OF THE CONSTRUCTION OF THREE (3) SEPARATE OFFICE/WAREHOUSE BUILDINGS WITH ASSOCIATED PAVING AND LANDSCAPING IMPROVEMENTS. THE THREE SITES WILL BE DEVELOPED BY THE SAME DEVELOPER. THESE LOTS WILL BE CONSTRUCTED AND PERMITTED SEPARATELY. THESE LOTS WILL USE SHARED ACCESS TO CORONADO AVE N.E. THE MAJORITY OF THE RUNOFF OF EACH SITE WILL DRAIN TO THE NORTH TO THE BACK OF EACH LOT AND DIRECTLY TO THE CONCRETE LINED PUBLIC DRAINAGE CHANNEL VIA CONCRETE RUNDOWNS. CALCULATIONS SHOWN HEREON DEMONSTRAT THAT THE ENTRANCE CONDITIONS AND CARRYING CAPACITIES OF THE RUNDOWNS ARE SUFFICIENT TO ACCEPT AND CONVEY THE 100-YEAR PEAK FLOW RATES. BASED UPON THE PROXIMITY OF THE SITE TO THE PUBLIC LINED DRAINAGE CHANNEL. THE FACT THAT THE SURROUNDING AREA IS DEVELOPED MAKING THIS AN INFILL SITE, AND THE LACK OF A DOWNSTREAM FLOODING HAZARD, THE FREE DISCHARGE OF DEVELOPED RUNOFF FROM THIS SITE IS APPROPRIATE.

THE USE OF THE TYPE OF RUNDOWN PROPOSED HEREIN WAS SUGGESTED BY MR. GLENN JURGENSON OF CITY STORM DRAIN MAINTENANCE IN A TELEPHONE CONVERSATION DATED 03-08-99. BECAUSE THERE IS NO VEHICULAR ACCESS ALONG THE CHANNEL TO DISRUPT, MR. JURGENSON INDICATED THAT "OVER THE TOP" DISCHARGE TO THE LINED CHANNEL WOULD BE ALLOWED VIA CONCRETE RUNDOWN SIMILAR TO THAT USED AT THE HOLY CROSS LUTHERAN CHURCH ON WYOMING BLVD N.E. SOUTH OF SAN ANTONIO DRIVE N.E. (E-19/D21). THE SECTIONS AND DETAILS OF THE PROPOSED CONCRETE RUNDOWN SHOWN HEREON ARE ADAPTED FROM THOSE SHOWN IN THAT PLAN AND ARE CONSISTENT WITH CITY OF ALBUQUERQUE STANDARDS FOR DRAINAGE FACILITY CONSTRUCTION. AS INDICATED IN CONSTRUCTION NOTE #7 LOCATED ON SHFFT 1 OF THIS SUBMITTAL, THE CONTRACTOR IS REQUIRED TO HAVE EACH RUNDOWN INSPECTED BY CITY OF ALBUQUERQUE STORM DRAIN MAINTENANCE. MR. JURGENSON INDICATED THAT THE PORTIONS OF THE RUNDOWNS WITHIN NORTH PINO RIGHT OF WAY WILL BE OWNED AND MAINTAINED BY THE CITY OF ALBUQUERQUE. THIS CONSTRUCTION WITHIN CHANNEL RIGHT-OF-WAY WILL REQUIRE AN EXCAVATION PERMIT FROM THE CITY OF ALBUQUERQUE THROUGH THE S.O. 19 PROCESS.

SITE GRADING CONSTRAINTS COMBINED WITH THE GEOMETRY OF THE AFOREMENTIONED SHARED ENTRANCES WILL REQUIRE THAT A SMALL PORTION OF LOT 18 WILL DRAIN TO LOT 19. SIMILARLY, A PORTION OF LOT 19 WILL DRAIN TO LOT 20. A PORTION OF LOT 20 WILL DRAIN SOUTH TO CORONADO AVE. N.E. VIA PROPOSED DRIVEPAD, WITH THE REMAINDER OF THE LOT DRAINING NORTH TO THE PUBLIC DRAINAGE CHANNEL. THE STORM DRAIN IN CORONADO AVE. N.E. OUTFALLS TO THE NORTH ARROYO DEL PINO DRAINAGE CHANNEL. NO OFFSITE IMPROVEMENTS OTHER THAN THE AFOREMENTIONED CONCRETE RUNDOWNS ARE REQUIRED OR PROPOSED IN CONJUNCTION WITH THESE PROJECTS. TO LEGALLY ALLOW THE DISCHARGE OF DEVELOPED RUNOFF FROM LOT 18 TO LOT 19, AND FROM LOT 19 TO LOT 20, THE OWNER OF THE PROPERTIES WILL GRANT NON-SPECIFIC DRAINAGE EASEMENTS TO BENEFIT THE APPROPRIATE LOTS. THESE EASEMENTS WILL IDENTIFY THAT LOTS 19 AND 20 ARE OBLIGATED TO ACCEPT AND CONVEY RUNOFF FROM LOTS 18 AND 19. THESE EASEMENTS MAY BE PREPARED IN CONJUNCTION WITH THE REQUIRED SHARED ACCESS EASEMENTS. THE FILING OF THESE EASEMENTS WILL BE A REQUIREMENT OF BUILDING PERMIT APPROVAL.

THIS GRADING AND DRAINAGE PLAN ADDRESSES CONSTRUCTION ON THREE SEPARATI SITES BY THE SAME DEVELOPER. INDIVIDUAL SUBMITTALS FOR BUILDING PERMIT APPROVAL WILL BE MADE FOR EACH LOT TO ALLOW SEPARATE PERMITTING AND SUBSEQUENT DRAINAGE CERTIFICATION FOR CERTIFICATE OF OCCUPANCY. AT THIS TIME, IT IS PLANNED TO CONSTRUCT THE THREE LOTS SEQUENTIALLY, WITH LOT 18 FIRST, FOLLOWED BY LOTS 20 AND 19, RESPECTIVELY. DEVELOPMENT OF LOT 18 WILL REQUIRE FULL CONSTRUCTION OF THE PRIVATE ENTRANCE SHARED WITH LOT 19. SIMILARLY, DEVELOPMENT ON LOT 20 WILL REQUIRE FULL CONSTRUCTION OF THE ENTRANCE SHARED WITH LOT 19. THE AFOREMENTIONED ACCESS AND DRAINAGE EASEMENTS WILL ALLOW THIS CONSTRUCTION TO OCCUR. MINIMAL GRADING WILL BE REQUIRED TO PROVIDE FOR CONTINUITY BETWEEN ADJACENT DEVELOPED AND UNDEVELOPED LOTS. AS SHOWN ON THE CRADING PLAN, THE DIFFERENCE BETWEEN EXISTING AND PROPOSED GRADES AT THE COMMON LOT LINES IS LESS THAN ONE (1) VERTICAL FOOT. TO PROVIDE CONTINUITY BETWEEN ADJACENT SITES, MINIMAL OFFSITE GRADING WILL BE REQUIRED WHICH EXTENDS A MAXIMUM OF THREE (3) FEET INTO THE ADJACENT LOT AT A MAXIMUM SLOPE OF 3:1 (H:V). SECTION D-D CONTAINED HEREON SHOWS THE TYPICAL SECTION FOR THIS GRADING.

GRADING PLAN:

THE GRADING PLAN ON SHEET 1 SHOWS: 1) EXISTING SPOT ELEVATIONS AND CONTOURS AT 1' O" INTERVALS AS DETERMINED FROM A TOPOGRAPHIC SURVEY PREPARED BY JEFF MORTENSEN AND ASSOCIATES, INC. DATED FEBRUARY, 1999, 2) THE LIMIT AND CHARACTER OF EXISTING IMPROVEMENTS, 3) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 4) PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1' 0" INTERVALS, AND 5) CONTINUITY BETWEEN EXISTING AND PROPOSED

CALCULATIONS

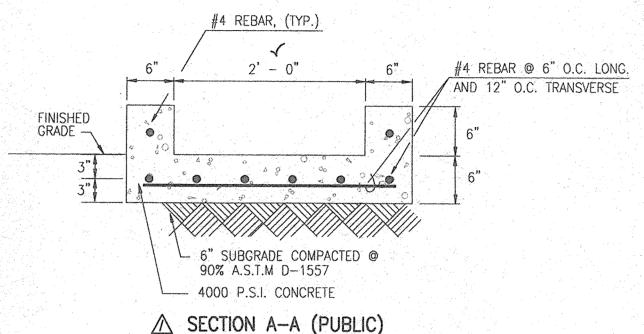
THE CALCULATIONS, WHICH APPEAR HEREON, ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40-ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS SHOWN BY THESE CALCULATIONS, A MINOR INCREASE IN THE PEAK RATE AND VOLUME OF DISCHARGE IS ANTICIPATED. BASED UPON THE PROXIMITY OF THE SITES TO THE ARROYO, THE FACT THAT THE SURROUNDING AREA IS DEVELOPED MAKING THIS AN INFILL SITE, AND THE LACK OF A DOWNSTREAM FLOODING HAZARD, THE FREE DISCHARGE OF RUNOFF FROM THESE SITES TO THE NORTH ARROYO DEL PINO IS APPROPRIATE.

CONCLUSION:

THE PROPOSED GRADING AND DRAINAGE PLAN FOR LOTS 18-20 OF THE LABEN OFFICE COMPLEX PROPOSES A RESPONSIBLE APPROACH TO MANAGING THE STORM WATER RUNOFF ASSOCIATED WITH THE PROPOSED CONSTRUCTION. THIS PLAN PROVIDES FOR THE PHASED CONSTRUCTION OF THE THREE SITES AS "STAND ALONE" PROJECTS. FREE DISCHARGE FROM THESE SITES IS JUSTIFIED BASED UPON THE CLOSE PROXIMITY OF THE SITES TO THE NORTH ARROYO DEL PINO, THE FACT THAT THE SURROUNDING AREA IS DEVELOPED MAKING THE PROPOSED IMPROVEMENTS CONSIST OF MODIFICATIONS TO EXISTING SITES WITHIN AN INFILL AREA, AND THE LACK OF A DOWNSTREAM FLOODING HAZARD. THE INTRODUCTION OF IMPERVIOUS AREA TO BOTH BASINS WILL CAUSE A MINOR INCREASE IN THE PEAK RATE AND VOLUME OF RUNOFF DRAINING TO EXISTING PUBLIC STORM DRAINAGE FACILITIES. PRIVATE DRAINAGE EASEMENTS WILL BE REQUIRED TO ALLOW THE CROSS-LOT DISCHARGE OF DEVELOPED RUNOFF. NO DRAINAGE COVENANTS OR VARIANCES ARE REQUESTED AS PART OF THIS PLAN.

DESIGNED BY

2' - 0" -1' - 0" 1' - 0" - 3000 PSI CONCRETE - 6" SUBGRADE COMPACTED @ 90% ASTM D-1557 SCALE: 1" = 6"



- INSTALL 1/4" LOW DENSITY POLYETHYLENE BEARING PLATE. – #4 REBAR @ 6" O.C. LONG. & 12" O.C. TRANSVERSE TOP OF CHANNEL (BEYOND) _#4 REBAR 'Z' BAR @ 6" O.C. 1/4"x1/4" ETHYLENE FLOWLINE VINYL ACETATE FOAM SEALANT, EVA SEAL OR APPROVED 6" SUBGRADE COMPACTED @ 90% PER A.S.T.M. D-1557. 8" EXISTING CONCRETE CHANNEL LINING LINSTALL POLYETHYLENE FOAM FILLER AS SHOWN, PLASTAZONE OR APPROVED EQUAL DO NOT ANCHOR WITH NAILS OR BONDING AGENT, KEEP IN PLACE WITH FRESH CONCRETE WHEN POURING UPSTREAM SECTION. DO NOT ALLOW FRESH CONCRETE BETWEEN FILLER AND PREVIOUS CONCRETE. - INSPECTED BY CITY STORM DRAIN

MAINTENANCE (SO #19)

SCALE: 1" = 1' - 0"

ALBUQUERQUE STANDARD DRAWING 2265. SECTION B-B (CHANNEL JOINT WITH SLEEPER) SCALE: 1" = 1' - 0"WIDTH VARIES FROM 2' @ P TO 5' @ ENTRANCE

NOTE: THIS SECTION DERIVED FROM CITY OF

└ 6" SUBGRADE COMPACTED @ 90% A.S.T.M D-1557 - 3000 P.S.I. CONCRETE △ SECTION C-C (PRIVATE) SCALE: 1" = 1' - 0"

INTERIM SLOPE TIE @ 3.1 IF REQUIRED FOR PHASING I MAX. FINAL SLOPE @ 2:1 ELEVATION PER PLAN WITH GRAVEL MULCH 1' MAX. PAVEMENT DIFFERENCE INTERIM SLOPE TIE @ 3:1 IF REQUIRED FOR PHASING TOP OF CURB ELEVATION PER PLAN EXISTING GRADE

△ SECTION D-D (PHASING AT LOT LINES) SCALE: 1" = 4"

DRAINAGE PLAN, CALCULATIONS, SECTIONS AND DETAILS LOT 18 LABEN OFFICE COMPLEX 6105 CORONADO AVENUE NE

REVISIONS JOB NO. NO. DATE BY 990112 1/00 JGM AS-BUILT & CERTIFY 10/00 J.G.M. RECERTIFY CORRECTIONS 03-199 APPROVED BY J.G.M.

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