

MARCH 7, 2012

## DRAINAGE REPORT

FOR

BROADSTONE SANTA MONICA  
San Pedro Blvd. and Santa Monica Ave. NE

BY



**ISAACSON & ARFMAN, P.A.**  
*Consulting Engineering Associates*

Thomas O. Isaacson, PE (Ret.) & LS (Ret.)

Fred C. Arfman, PE

Åsa Nilsson-Weber, PE

I&A Project No. 1900

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I&A Project No. 1900

Prepared by:

  
6  
Åsa Nilsson-  
Weber  
Date

3-7-12  
Date



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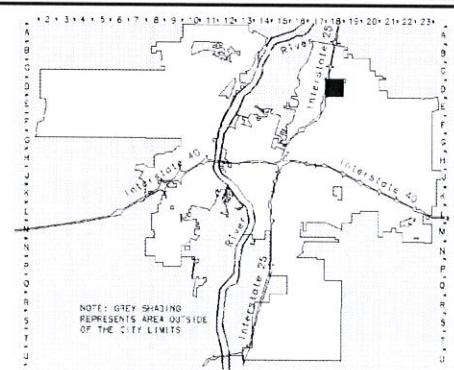
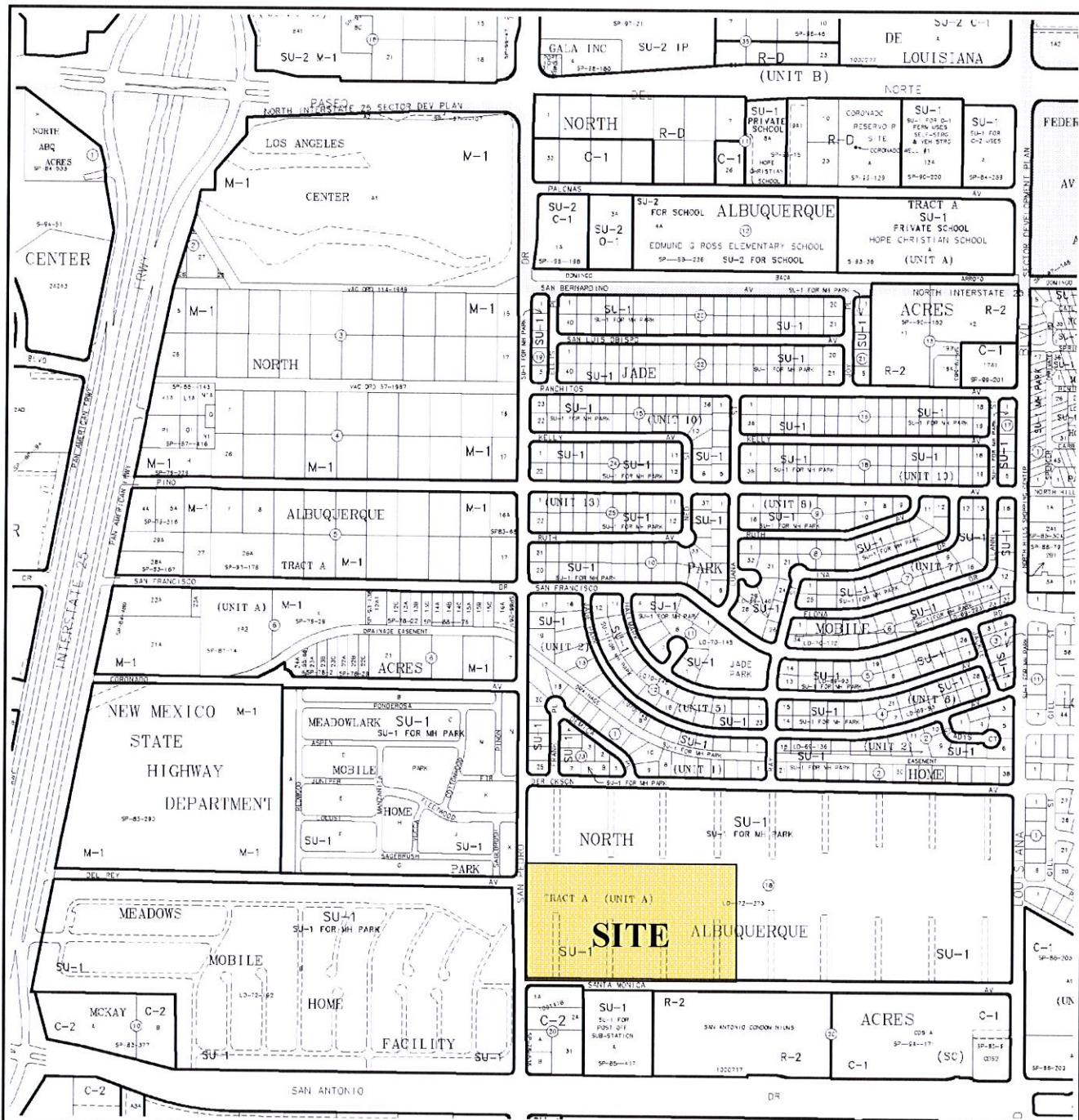
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GRAPHIC SCALE IN FEET  
250 0 750 1000

## Zone Atlas Page

# D-18-Z

Map Amended through February 01, 2005

**A**lbuquerque **G**eographic **I**nformation **S**ystem  
PLANNING DEPARTMENT

Copyright 2004

MAP SCALE 1" = 500'



PANEL 0137G

**FIRM**

**FLOOD INSURANCE RATE MAP  
BERNALILLO COUNTY,  
NEW MEXICO  
AND INCORPORATED AREAS**

PANEL 137 OF 825

(SEE MAP NUMBER FOR FULL LAYOUT)

CONTAINS	NAME	SEAL	SUPER.
ANNUAL FLOOD CHANCE CONTAINED IN CHANNEL	350002	C-37	C
ANNUAL FLOOD CHANCE CONTAINED IN CHANNEL	350007	D-37	C

NOTICE TO USER: The information shown below should be used when placing map within the Community Rating System. It should not be used as insurance information for the National Flood Insurance Program.

MAP NUMBER  
35001G0137G

MAP REVISED  
SEPTEMBER 26, 2008

Federal Emergency Management Agency



This is an official copy of a portion of the above referenced flood map. It was created using F-NET On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps, contact FIRM, Flood Map Store at www.fema.gov.

SAN ANTONIO DR

5324

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SAN ANTONIO DR

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## **I. PROJECT INFORMATION**

**LEGAL DESCRIPTION:**  
Tract 2, Santa Monica Place

**TOTAL AREA:**  
13.98 Acres

**FLOOD PLAIN:**  
Zone X  
This site lies outside the 100-year flood based on FIRM Map No. 35001C0137G  
Map Revision date: September 26, 2008

**ENGINEER:**  
Isaacson & Arfman, P.A.  
128 Monroe Street NE  
Albuquerque, NM 87108  
(505) 268-8828  
Attn: Åsa Nilsson-Weber

**SURVEYOR:**  
Surv-Tek, Inc.  
(505) 897-3366  
Attn: Russ P. Hugg, NMPLS No. 9750

**LAND OWNER:**  
Broadstone Santa Monica, LLC  
c/o Titan Development  
6300 Riverside Plaza Lane NW - #200  
Albuquerque, NM 87120-2617  
Attn: Kurt Browning

## **II. INTRODUCTION**

Broadstone Santa Monica, Tract 2 Santa Monica Place, will be developed with apartments. The site is bound by Santa Monica Ave. NE to the south and by San Pedro Blvd. NE to the west. A Master Drainage Report for Tracts 1-4, Santa Monica Place was prepared by Isaacson & Arfman, P.A. on February 21, 2012. This master report outlined the required drainage improvements for this site.

The purpose of this master drainage report is to analyze flows generated and flow capacity of onsite driving aisles, sidewalk culverts and storm drains.

## **III. EXISTING CONDITIONS**

The site discharges to the northwest and flows enter the existing public storm sewer system inlets near the intersection of Derickson Ave. and San Pedro Blvd. NE.

The existing public storm drain system at the intersection of Derickson Ave. and San Pedro Blvd. includes a 48" pipe east to an existing cattle guard inlet in Derickson Ave. and a 36" pipe south to collect street flow from four existing curb inlets in San Pedro Blvd. The flows are conveyed north in a 60" storm drain to the North Pino Arroyo. There are also four additional curb inlets in San Pedro Blvd. between Derickson Ave. and the North Pino Arroyo.

## **IV. PROPOSED CONDITIONS**

The site will be developed with apartments, drives, parking areas and landscaping. There is one entrance from San Pedro Blvd. and two entrances from Santa Monica Ave. Land treatments were calculated based on the impervious and pervious areas shown on the site plan and landscaping plan. See Appendix A for a Basin Map and a Basin Area & Land Treatment table. The 100-year, 6-hour flows were calculated in AHYMO (Appendix B).

The site will discharge 13.0 cfs to Santa Monica Ave. and 48.1 to San Pedro Blvd. for a total of 61.1 cfs, which is less than the 61.4 cfs listed for Tract 2 in the Master Drainage Report for Tracts 1-4.

### **PUBLIC STORM DRAIN**

Per the Master Drainage Report, a public 48-inch storm drain shall be extended from the existing cattle guard inlet in Derickson Ave. onto Tract 1. A temporary inlet shall be constructed at the terminus of the 48-inch storm drain to capture undeveloped flows from Tract 1. See section on Offsite Drainage Improvements for further discussion.

## **PRIVATE STORM DRAIN**

There are two onsite private storm drain systems as shown on the Private Storm Drain Detail plan in the back pocket (Sheet CG5.1). The Storm Drain Sub-Basin Map in Appendix C shows the basins contributing flows to each inlet. Flows were calculated based on the master drainage basin cfs/acre.

The west storm drain system collects 21.5 cfs through 5 sump inlets, and the flows are discharged to San Pedro Blvd. via a stilling basin at the entrance. A 4-inch discharge pipe will convey water collected in the 0.5-foot sump in the basin to the 2-foot sidewalk culvert north of the basin. The westerly inlet in the driving aisle will accept 11 cfs at a head of 0.5 feet (see Appendix C for Inlet Capacity Chart), and the remaining flows will overflow toward the San Pedro Blvd. entrance.

The east storm drain system collects 2.8 cfs in 4 sump inlets located in Basin 1. These flows discharge through a headwall to a parking area and then surface flows northwest. Appendix C contains storm drain calculations and inlet capacity charts.

## **SIDEWALK CULVERTS**

A total of 5 sidewalk culverts will discharge between 1 and 2.2 cfs each to Santa Monica Ave. and San Pedro Blvd. The sidewalk culvert capacity calculations included in Appendix D show that a 2-foot sidewalk culvert has capacity for 2.4 cfs. There are also a series of private onsite sidewalk culverts and drain pipes that will convey minor flows under walks.

## **DRIVING AISLE FLOW CAPACITY**

The drives have capacity to contain surface flows. Capacity calculations are included in Appendix E for one critical location adjacent to Building Number 16, denoted on the Basin Map as Section A-A.

## **OFFSITE DRAINAGE IMPROVEMENTS**

Per the Master Drainage Report for Tracts 1-4, Santa Monica Place, the following improvements shall be required.

- A storm water deflection berm shall be constructed to direct the flows to a temporary inlet at the terminus of the new 48" storm drain that shall capture flows from Tract 1. The 48" storm drain and the temporary inlet shall be constructed with the public work order plans for Broadstone Santa Monica Apartments.
- A berm shall be constructed between Tracts 1 & 3 and Tracts 2 & 4, and the offsite flows from the east shall be diverted via a swale to a detention pond with weir overflow to Derickson Ave. The maximum pond storage volume at a maximum outflow of 26.8 cfs is 1.9 ac-ft.

The Offsite Grading Plan in the back pocket shows the offsite grading improvements.

## **V. SUMMARY AND CONCLUSIONS**

Broadstone Santa Monica, Tract 2 Santa Monica Place, will be developed with apartments. The drainage improvements include the following:

- A public 48" storm drain shall be installed from the existing cattle guard inlet in Derickson Ave. to onsite (by public work order).
- Private storm drain with inlets shall be constructed per the Grading Plan. A stilling basin shall be constructed at the storm drain outfall by the San Pedro Blvd. entrance.
- Five 24-inch sidewalk culverts shall be installed to convey flows to Santa Monica Ave. and San Pedro Blvd. (by public work order).
- Private sidewalk culverts and drain pipes shall be installed per the Grading Plan.
- A storm water deflection berm shall be constructed to direct the flows to a temporary inlet at the terminus of the new 48" storm drain that shall capture flows from Tract 1. The temporary inlet shall be constructed by public work order.
- A berm shall be constructed between Tracts 1 & 3 and Tracts 2 & 4, and a swale shall be constructed to direct offsite flows from the east to a detention pond. The detention pond shall be constructed in accordance with the Offsite Grading Plan with a riprap-lined weir and overflow channel to Derickson Ave.

## **APPENDIX A**

### **LAND TREATMENTS & BASIN AREAS**

**BASIN AREA & LAND TREATMENT TABLE**

PROJECT NAME Broadstone Santa Monica

PROJECT # 1900

BASIN ID	AREA (sf)	AREA (Ac)	AREA (sq. mi.)	Q100 (cfs)	LAND TREATMENTS			
					%A	%B	%C	%D
1	205,518	4.7181	0.00737	21.7	0	0	20	80
2a	19,920	0.4573	0.00071	1.9	0	0	52	48
2b	22,611	0.5191	0.00081	2.2	0	0	50	50
2c	12,601	0.2893	0.00045	1.1	0	0	50	50
2d	10,384	0.2384	0.00037	1.0	0	0	40	60
3	85,176	1.9554	0.00306	8.9	0	4	16	80
4	121,519	2.7897	0.00436	13.1	0	0	15	85
5a	107,771	2.4741	0.00387	8.8	0	25	45	30
5b	12,993	0.2983	0.00047	1.3	0	0	50	50
6	10,671	0.2450	0.00038	1.1	0	0	20	80
TOTAL AREA =	609,164	13.9845	0.02185	61.1				

**NOTE:**

Land treatments were calculated from impervious/pervious areas in site plan, and 100-yr, 6-hr flow was calculated in AHYMO.

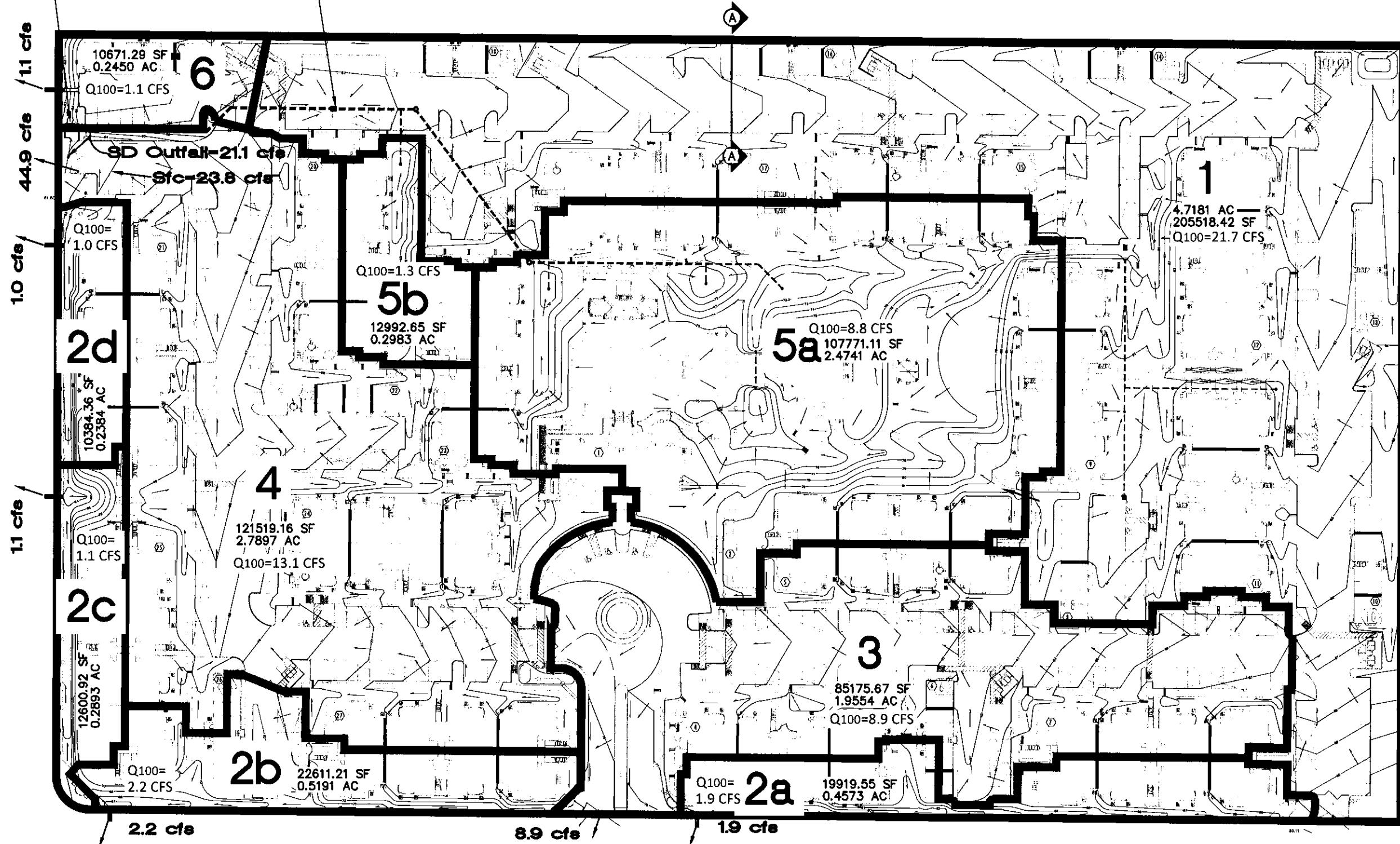
**BROADSTONE SANTA MONICA  
BASIN MAP**

**3-6-12**

STILLING BASIN WITH  
OVERFLOW TO SAN PEDRO  
VIA ENTRANCE

2'X3' INLET CAPACITY  
= 11 CFS @ 0.5' HEAD  
REMAINDER OVERFLOWS TO SAN  
PEDRO VIA ENTRANCE

TOTAL FLOWS & DISCHARGE LOCATIONS:  
SANTA MONICA 13.0 CFS  
SAN PEDRO 48.1 CFS  
TOTAL: 61.1 CFS



## **APPENDIX B**

### **DRAINAGE CALCULATIONS**

AHYMO PROGRAM SUMMARY TABLE (AHYMO\_97) -  
INPUT FILE = 1900P2.DAT

- VERSION: 1997.02d RUN DATE (MON/DAY/YR) = 02/28/2012  
USER NO. = AHYMO-S-9702dIIsa-Ariman3

```

*****
*S HYDROGRAPH FROM TO PEAK RUNOFF CFS PAGE = 1
*IDENTIFICATION ID ID DISCHARGE VOLUME PER
NO. NO. (SQ MI) (CFS) (AC-FT) PEAK
*****  

*S BROADSTONE SANTA MONICA AREA RUNOFF CFS PAGE = 1
*IDENTIFICATION TRACT 2, SANTA MONICA PLACE
NO. DEVELOPED CONDITIONS
*S 100-YR, 6-HR STORM
*S 1900P2.DAT
*S FEBRUARY 28, 2012
*S BY ASA NILSSON-WEBER
*S ISAACSON & ARFMAN, P.A.  

*S *****  

*START RAINFALL TYPE= 1  

*S
*S BASIN 1 COMPUTE NM HYD 100.00 - 1 .00737 21.68 .792 2.01525 1.500 4.597 PER IMP= 80.00
*S BASIN 2a COMPUTE NM HYD 200.00 - 2 .00071 1.87 .064 1.69538 1.500 4.122 PER IMP= 48.00
*S BASIN 2b COMPUTE NM HYD 300.00 - 3 .00081 2.15 .074 1.71538 1.500 4.151 PER IMP= 50.00
*S BASIN 2c COMPUTE NM HYD 400.00 - 4 .00045 1.12 .038 1.56363 1.500 3.891 PER IMP= 41.67
*S BASIN 2d COMPUTE NM HYD 500.00 - 5 .00037 1.03 .036 1.81533 1.500 4.353 PER IMP= 60.00
*S BASIN 3 COMPUTE NM HYD 600.00 - 6 .00306 8.94 .326 1.99861 1.500 4.565 PER IMP= 80.00
*S BASIN 4 COMPUTE NM HYD 700.00 - 7 .00436 13.06 .480 2.06522 1.500 4.680 PER IMP= 85.00
*S BASIN 5a COMPUTE NM HYD 800.00 - 8 .00387 8.84 .292 1.41397 1.500 3.568 PER IMP= 30.00
*S BASIN 5b COMPUTE NM HYD 900.00 - 9 .00047 1.26 .043 1.71538 1.500 4.179 PER IMP= 50.00
*S BASIN 6 COMPUTE NM HYD 110.00 - 10 .00038 1.14 .041 2.01525 1.500 4.674 PER IMP= 80.00
*****  

*FINISH

```

```

*S*****BROADSTONE SANTA MONICA
*S      TRACT 2, SANTA MONICA PLACE
*S      DEVELOPED CONDITIONS
*S      100-YR, 6-HR STORM
*S      1900P2.DAT
*S      FEBRUARY 28, 2012
*S      BY ASA NILSSON-WEBER
*S      ISAACSON & ARFMAN, P.A.
*S*****START          RAINFALL BEGINS AT 0.0 HRS
RAINFALL        TYPE=1 RAIN QUARTER=0 RAIN ONE=2.1
                  RAIN SIX=2.45 RAIN DAY=2.85 DT=0.03333HR
*S
*S  BASIN 1
COMPUTE NM HYD    ID=1 HYD NO=100 AREA=0.00737 SQ MI
                  PER A=0 PER B=0 PER C=20 PER D=80
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=1 CODE=1
*S  BASIN 2a
COMPUTE NM HYD    ID=2 HYD NO=200 AREA=0.00071 SQ MI
                  PER A=0 PER B=0 PER C=52 PER D=48
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=2 CODE=1
*S  BASIN 2b
COMPUTE NM HYD    ID=3 HYD NO=300 AREA=0.00081 SQ MI
                  PER A=0 PER B=0 PER C=50 PER D=50
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=3 CODE=1
*S  BASIN 2c
COMPUTE NM HYD    ID=4 HYD NO=400 AREA=0.00045 SQ MI
                  PER A=0 PER B=20 PER C=50 PER D=50
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=4 CODE=1
*S  BASIN 2d
COMPUTE NM HYD    ID=5 HYD NO=500 AREA=0.00037 SQ MI
                  PER A=0 PER B=0 PER C=40 PER D=60
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=5 CODE=1
*S  BASIN 3
COMPUTE NM HYD    ID=6 HYD NO=600 AREA=0.00306 SQ MI
                  PER A=0 PER B=4 PER C=16 PER D=80
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=6 CODE=1
*S  BASIN 4
COMPUTE NM HYD    ID=7 HYD NO=700 AREA=0.00436 SQ MI
                  PER A=0 PER B=0 PER C=15 PER D=85
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=7 CODE=1
*S  BASIN 5a
COMPUTE NM HYD    ID=8 HYD NO=800 AREA=0.00387 SQ MI
                  PER A=0 PER B=25 PER C=45 PER D=30
                  TP=-0.1333 HR MASS RAIN=-1
PRINT HYD         ID=8 CODE=1

```

```
*S  BASIN 5b  
COMPUTE NM HYD          ID=9 HYD NO=900 AREA=0.00047 SQ MI  
                      PER A=0 PER B=0 PER C=50 PER D=50  
                      TP=-0.1333 HR MASS RAIN=-1  
PRINT HYD             ID=9 CODE=1  
*S  BASIN 6  
COMPUTE NM HYD          ID=10 HYD NO=110 AREA=0.00038 SQ MI  
                      PER A=0 PER B=0 PER C=20 PER D=80  
                      TP=-0.1333 HR MASS RAIN=-1  
PRINT HYD             ID=10 CODE=1  
FINISH
```

## **APPENDIX C**

### **PRIVATE STORM DRAIN & INLET CALCULATIONS**

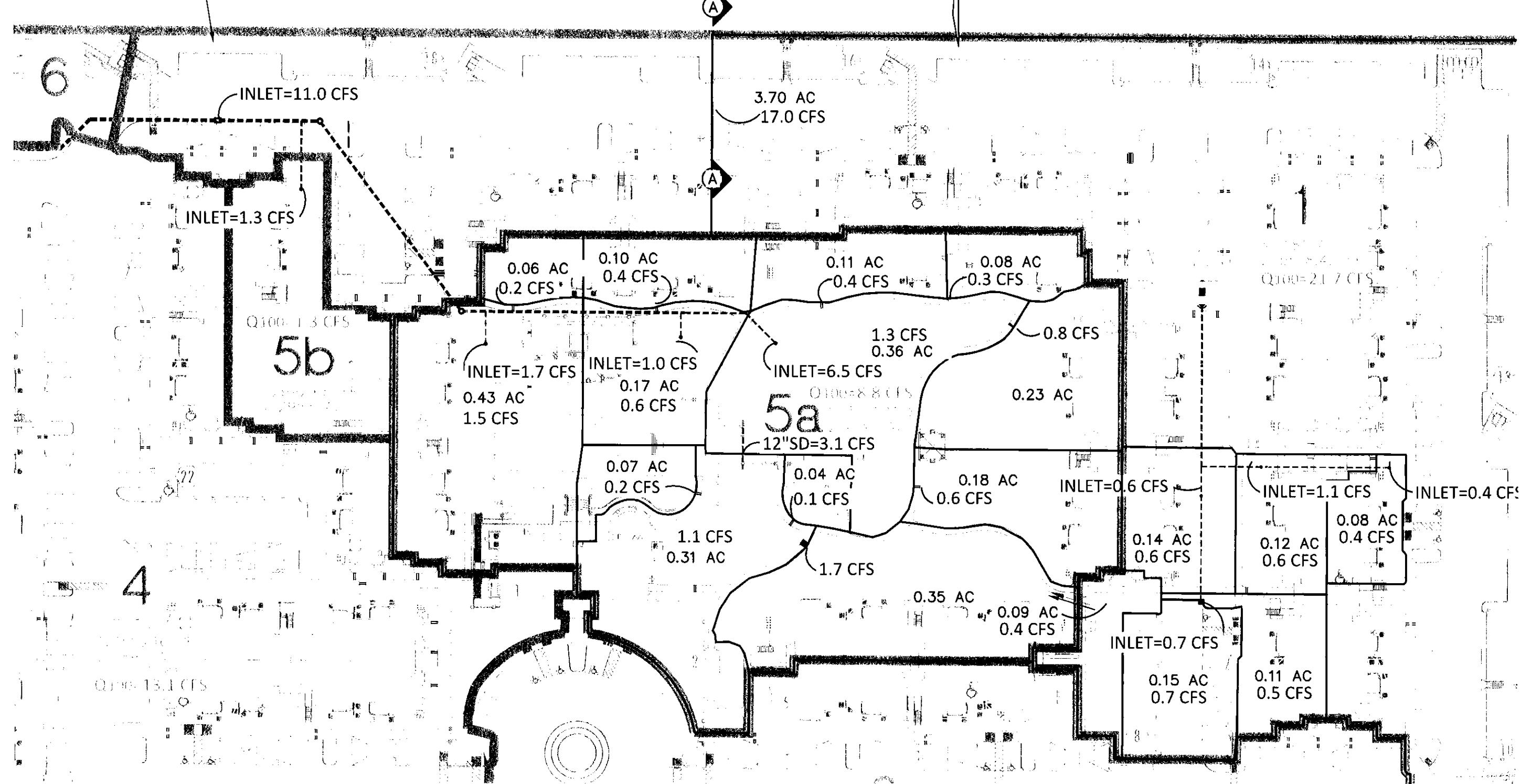
BROADSTONE SANTA MONICA  
STORM DRAIN  
SUB-BASIN MAP

3-6-12

2'X3' INLET CAPACITY  
= 11 CFS @ 0.5' HEAD  
REMAINDER OVERFLOWS TO SAN  
PEDRO VIA ENTRANCE

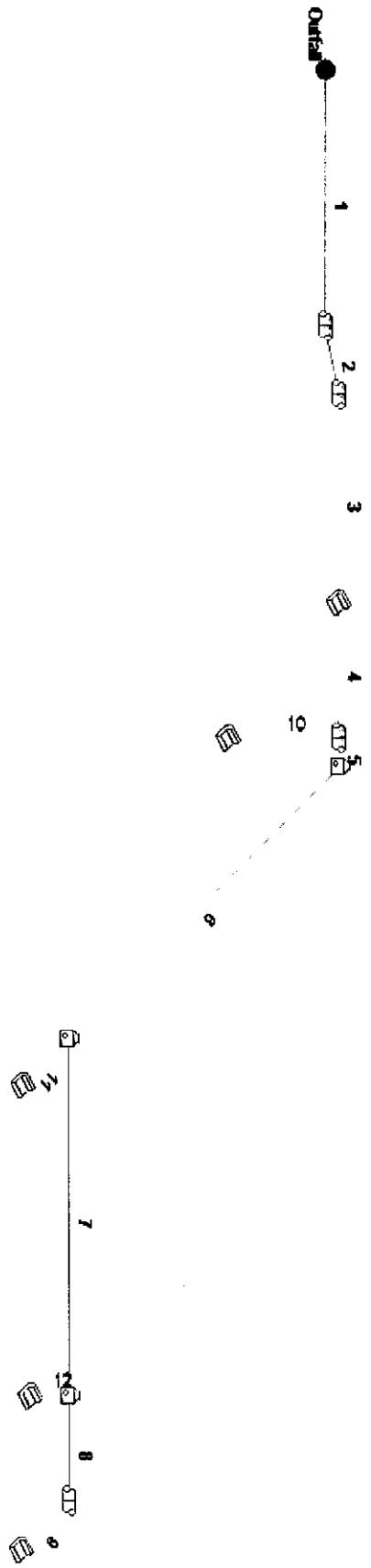
NOTE: FLOW RATES FOR SUB-BASINS  
CALCULATED BASED ON MASTER  
BASIN CFS/ACRE.

1"=60'



**BROADSTONE SANTA MONICA**

**WEST STORM DRAIN**



# Storm Sewer Inventory Report

Page 1

Line No.	Alignment				Flow Data				Physical Data				Line ID		
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/Rim El (ft)
1	End	98.000	0.000	None	0.00	0.00	0.00	0.0	61.50	0.56	62.05	24	Cir	0.012	0.23
2	1	26.200	-11.250	None	0.00	0.00	0.00	0.0	62.05	0.57	62.20	24	Cir	0.012	0.23
3	2	78.600	11.250	Genr	11.00	0.00	0.00	0.0	62.20	0.57	62.65	24	Cir	0.012	0.50
4	3	51.400	0.000	None	0.00	0.00	0.00	0.0	62.65	0.91	63.12	24	Cir	0.012	1.00
5	4	11.200	0.000	MH	0.00	0.00	0.00	0.0	63.12	0.98	63.23	18	Cir	0.012	0.75
6	5	145.400	45.000	MH	0.00	0.00	0.00	0.0	63.23	1.60	65.56	18	Cir	0.012	0.75
7	6	135.100	-45.000	MH	0.00	0.00	0.00	0.0	65.56	1.60	67.72	18	Cir	0.012	1.00
8	7	40.000	0.000	None	0.00	0.00	0.00	0.0	67.72	1.60	68.36	12	Cir	0.012	0.75
9	8	26.000	45.000	Genr	6.50	0.00	0.00	0.0	68.36	1.62	68.78	12	Cir	0.012	1.00
10	4	42.000	90.000	Genr	1.30	0.00	0.00	0.0	63.12	0.90	63.50	12	Cir	0.012	1.00
11	6	25.000	0.000	Genr	1.70	0.00	0.00	0.0	65.56	1.76	66.00	8	Cir	0.012	1.00
12	7	15.000	90.000	Genr	1.00	0.00	0.00	0.0	67.72	1.87	68.00	8	Cir	0.012	1.00

1900 PRVT SD WEST

Number of lines: 12

Date: 3/6/2012

Storm Sewers v8.00

# Storm Sewer Summary Report

Page 1

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
1		21.50	24	Cir	98.000	61.50	62.05	0.561	64.20*	64.95*	0.17	65.12	End	None
2		21.50	24	Cir	26.200	62.05	62.20	0.573	65.12*	65.32*	0.17	65.49	1	None
3		21.50	24	Cir	78.600	62.20	62.65	0.573	65.49*	66.10*	0.36	66.46	2	Generic
4		10.50	24	Cir	51.400	62.65	63.12	0.914	66.46*	66.56*	0.17	66.73	3	None
5		9.20	18	Cir	11.200	63.12	63.23	0.982	66.73*	66.80*	0.32	67.12	4	Manhole
6		9.20	18	Cir	145.400	63.23	65.56	1.602	67.12*	68.07*	0.32	68.39	5	Manhole
7		7.50	18	Cir	135.100	65.56	67.72	1.599	68.39	68.88	0.40	69.29	6	Manhole
8		6.50	12	Cir	40.000	67.72	68.36	1.600	69.29*	70.42*	0.80	71.22	7	None
9		6.50	12	Cir	26.000	68.36	68.78	1.615	71.22*	71.96*	1.06	73.03	8	Generic
10		1.30	12	Cir	42.000	63.12	63.50	0.905	66.73*	66.78*	0.04	66.82	4	Generic
11		1.70	8	Cir	25.000	65.56	66.00	1.760	68.39*	68.81*	0.37	69.18	6	Generic
12		1.00	8	Cir	15.000	67.72	68.00	1.867	69.29*	69.38*	0.13	69.50	7	Generic
1900 PRVT SD WEST														Number of lines: 12

NOTES: Return period = 2 Yrs. ; \*Surcharged (HGL above crown).

Run Date: 3/6/2012

# Storm Sewer Tabulation

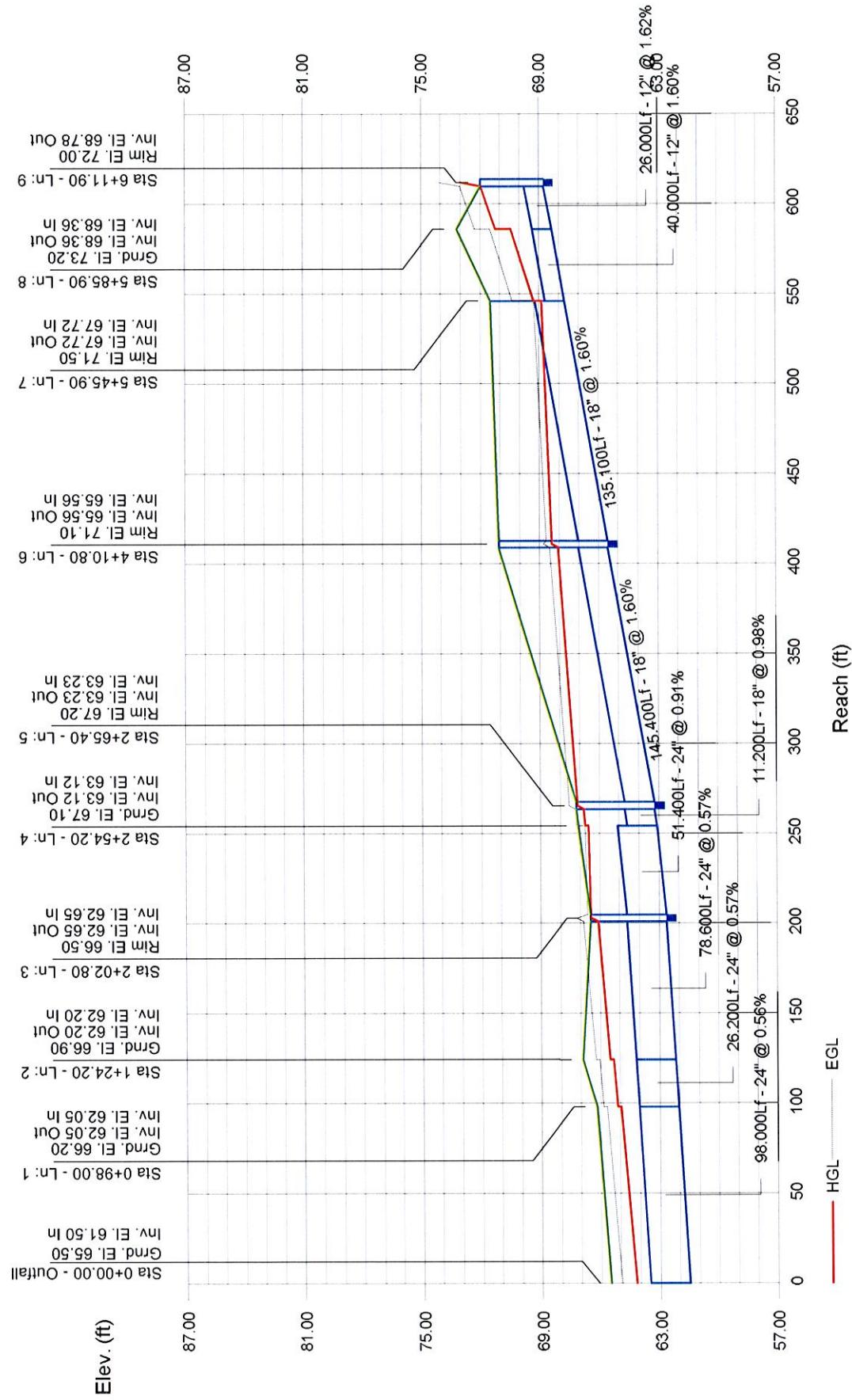
Page 1

Station	Len	Drng Area	Rnoff coeff	Area x C		Tc		Rain (I)	Total flow	Cap full	Vel	Pipe	Invert Elev		HGL Elev		Gnd / Rim Elev		Line ID		
				Incr	Total	Inlet	Syst						(min)	(in/hr)	(cfs)	(ft/s)	(in)	(%)	Dn	Up	
1	End	98.000	0.00	0.00	0.00	0.00	0.00	0.0	1.7	0.0	21.50	18.36	6.84	24	0.56	61.50	62.05	64.20	64.95	65.50	66.20
2	1	26.200	0.00	0.00	0.00	0.00	0.00	0.0	1.6	0.0	21.50	18.54	6.84	24	0.57	62.05	62.20	65.12	65.32	66.20	66.90
3	2	78.600	0.00	0.00	0.00	0.00	0.00	0.0	1.4	0.0	21.50	18.54	6.84	24	0.57	62.20	62.65	65.49	66.10	66.90	66.50
4	3	51.400	0.00	0.00	0.00	0.00	0.00	0.0	1.2	0.0	10.50	23.43	3.34	24	0.91	62.65	63.12	66.46	66.56	66.50	67.10
5	4	11.200	0.00	0.00	0.00	0.00	0.00	0.0	1.1	0.0	9.20	11.27	5.21	18	0.98	63.12	63.23	66.73	66.80	67.10	67.20
6	5	145.400	0.00	0.00	0.00	0.00	0.00	0.0	0.7	0.0	9.20	14.40	5.21	18	1.60	63.23	65.56	67.12	68.07	67.20	71.10
7	6	135.100	0.00	0.00	0.00	0.00	0.00	0.0	0.1	0.0	7.50	14.38	4.67	18	1.60	65.56	67.72	68.39	68.88	71.10	71.50
8	7	40.000	0.00	0.00	0.00	0.00	0.00	0.0	0.1	0.0	6.50	4.88	8.28	12	1.60	67.72	68.36	69.29	70.42	71.50	73.20
9	8	26.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	6.50	4.90	8.28	12	1.62	68.36	68.78	71.22	71.96	73.20	72.00
10	4	42.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	1.30	3.67	1.66	12	0.90	63.12	63.50	66.73	66.78	67.10	66.50
11	6	25.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	1.70	1.74	4.87	8	1.76	65.56	66.00	68.39	68.81	71.10	69.50
12	7	15.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	1.00	1.79	2.87	8	1.87	67.72	68.00	69.29	69.38	71.50	70.50
<b>1900 PRVT SD WEST</b>																		<b>Number of lines: 12</b>	<b>Run Date: 3/6/2012</b>		

NOTES: Intensity = 69.87 / (Inlet time + 13.10) ^ 0.87; Return period = Yrs. 2 ; c = cir e = ellip b = box

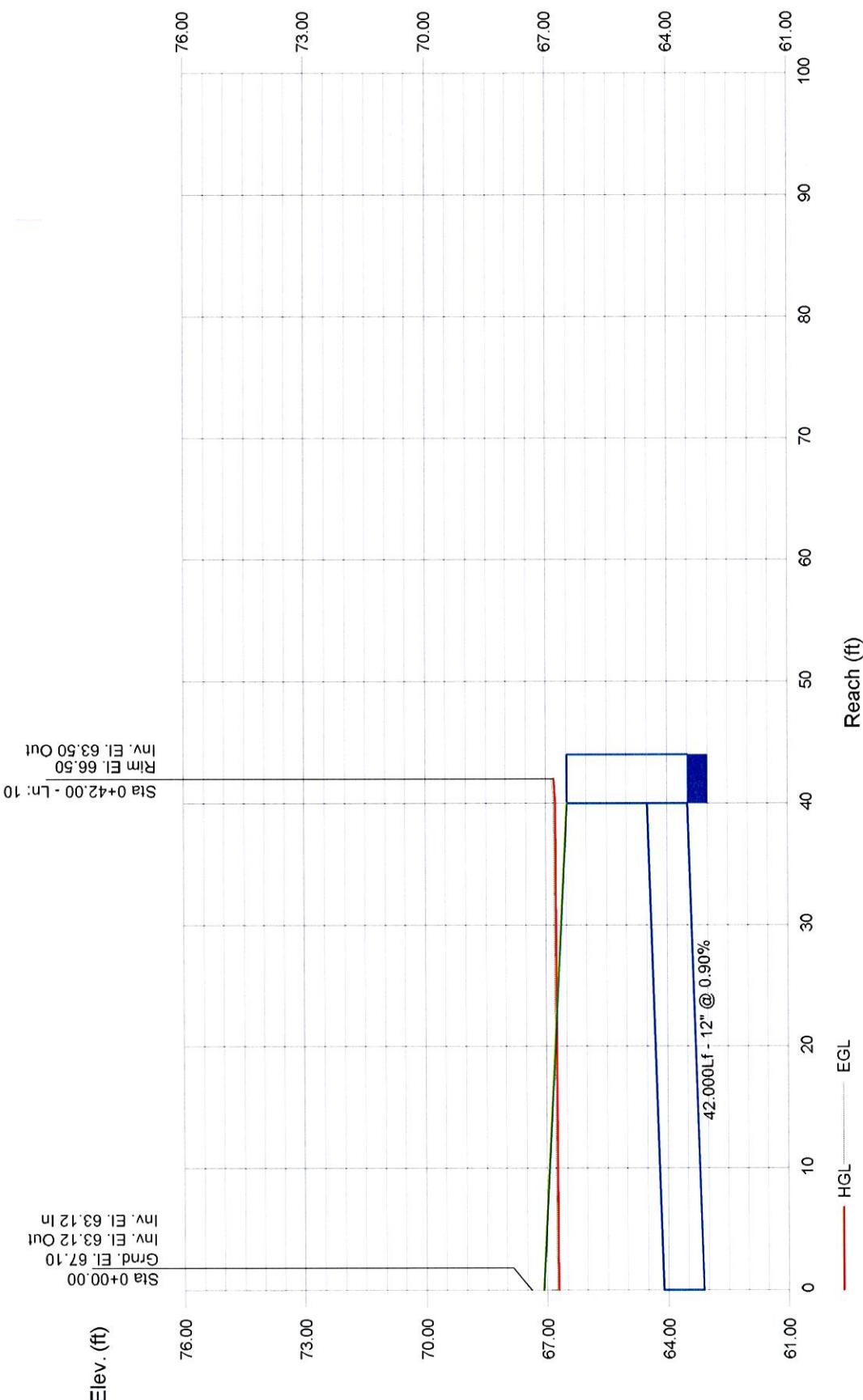
# Storm Sewer Profile

Proj. file: 1900-PRVT SD WEST.stm



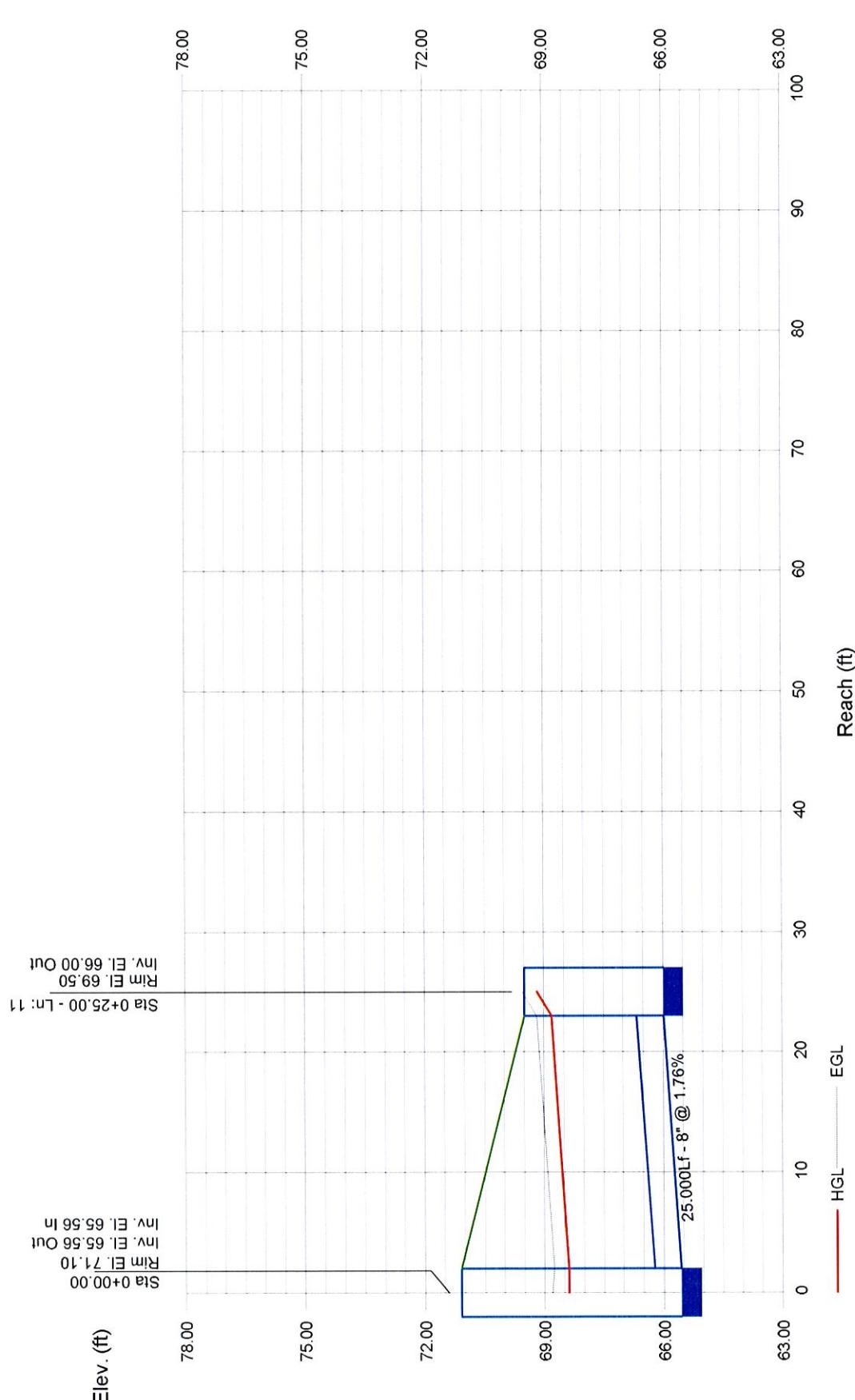
# Storm Sewer Profile

Proj. file: 1900-PRVT SD WEST.stm



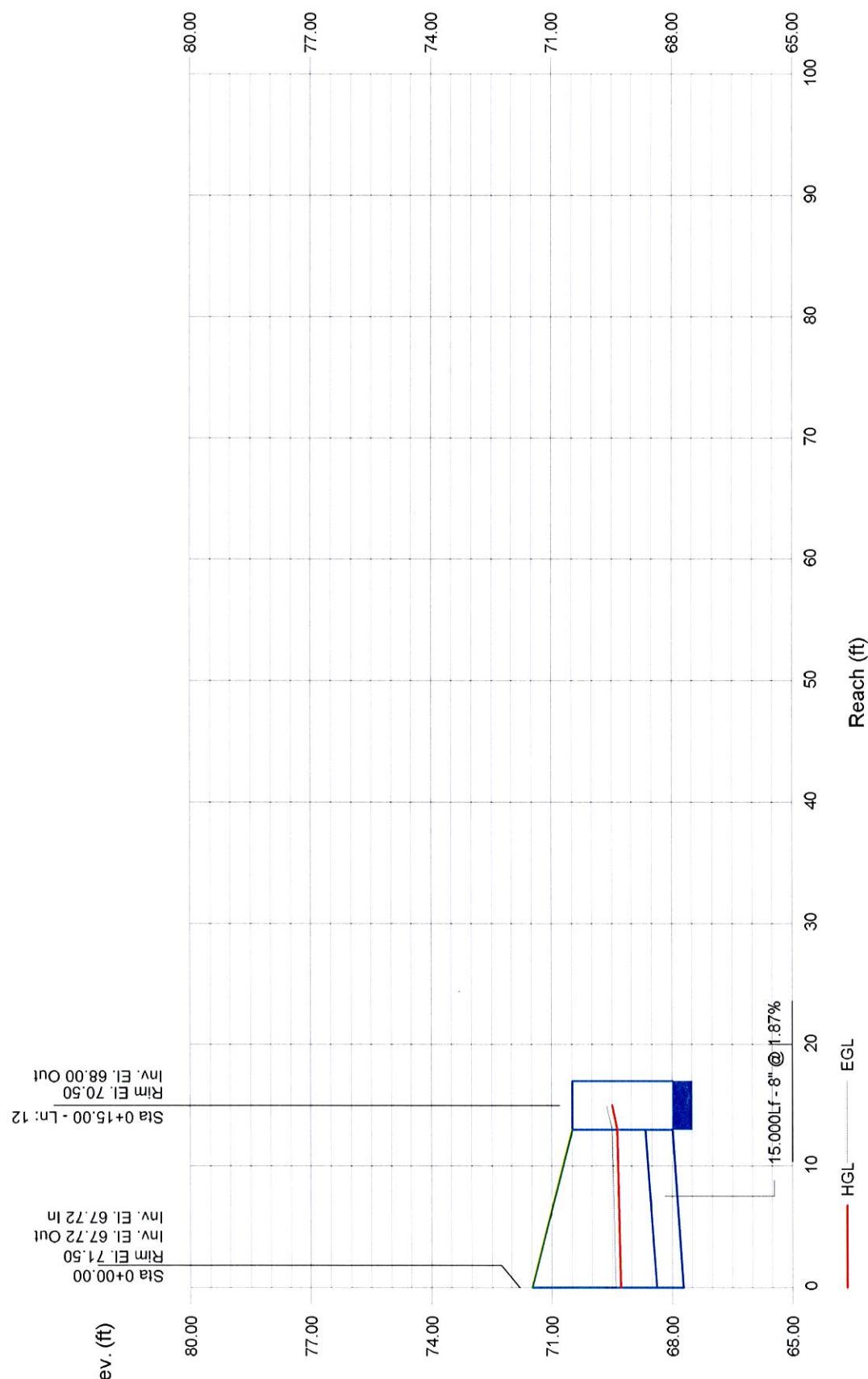
# Storm Sewer Profile

Proj. file: 1900-PRVT SD WEST.stm



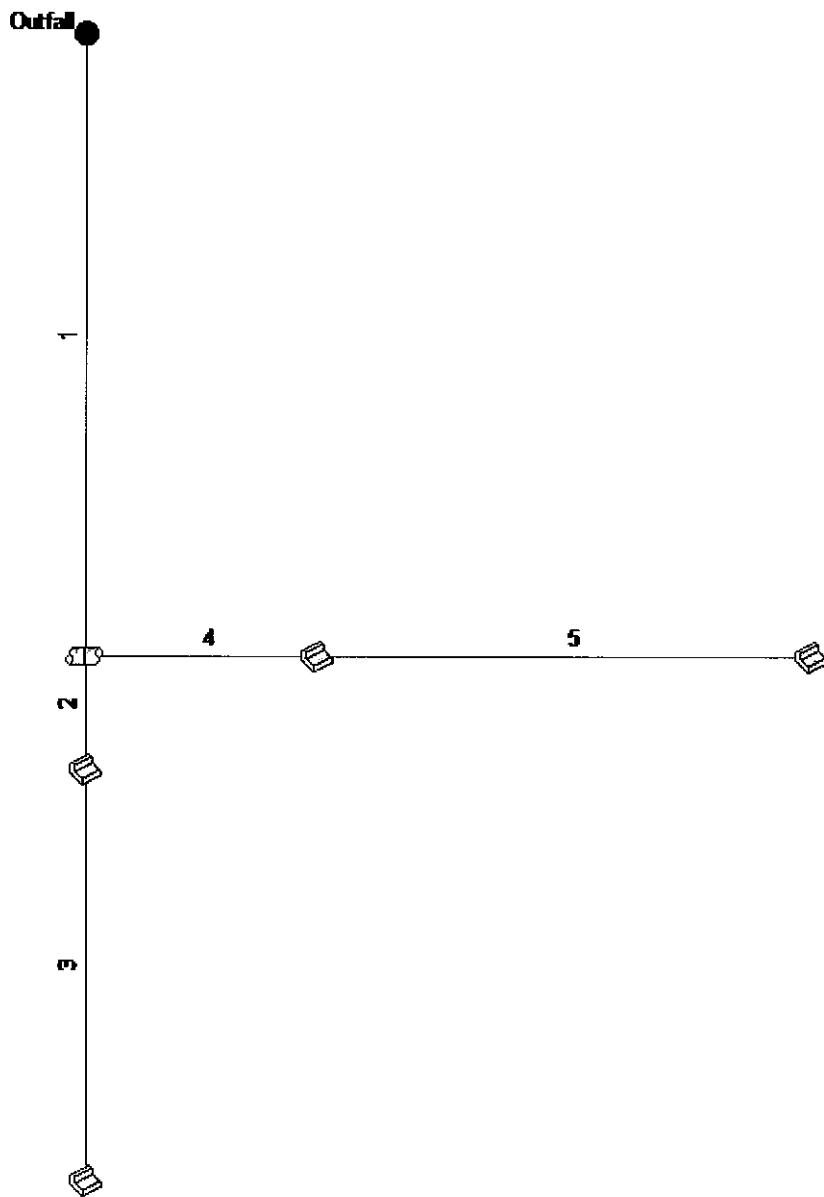
# Storm Sewer Profile

Proj. file: 1900-PRVT SD WEST.stm



**BROADSTONE SANTA MONICA**

**EAST STORM DRAIN**



# Storm Sewer Inventory Report

Page 1

Line No.	Alignment				Flow Data				Physical Data				Line ID			
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Line Size (in)	N Value (n)	J-Loss Coeff (K)			
1	End	99.300	90.000	None	0.00	0.00	0.00	0.0	78.20	0.70	78.90	12	Cir	0.012	1.00	82.50
2	1	18.000	0.000	Genr	0.60	0.00	0.00	0.0	78.90	0.56	79.00	8	Cir	0.012	0.50	82.00
3	2	65.500	0.000	Genr	0.70	0.00	0.00	0.0	79.00	1.53	80.00	8	Cir	0.012	1.00	83.00
4	1	36.800	-90.000	Genr	1.10	0.00	0.00	0.0	78.90	0.68	79.15	8	Cir	0.012	0.50	82.00
5	4	79.100	0.000	Genr	0.40	0.00	0.00	0.0	79.15	0.70	79.70	8	Cir	0.012	1.00	82.70
1900 PRVT SD EAST														Number of lines: 5	Date: 3/6/2012	Storm Sewers v9.00

# Storm Sewer Summary Report

Page 1

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
1		2.80	12	Cir	99.300	78.20	78.90	0.705	79.00	79.61	n/a	79.61 j	End	None
2		1.30	8	Cir	18.000	78.90	79.00	0.556	79.61*	79.79*	0.11	79.90	1	Generic
3		0.70	8	Cir	65.500	79.00	80.00	1.527	79.90	80.39	n/a	80.39 j	2	Generic
4		1.50	8	Cir	36.800	78.90	79.15	0.679	79.61*	80.09*	0.14	80.24	1	Generic
5		0.40	8	Cir	79.100	79.15	79.70	0.695	80.24	80.30	0.02	80.33	4	Generic
													Number of lines: 5	
1900 PRVT SD EAST													Run Date: 3/6/2012	

NOTES: Return period = 2 Yrs. ; \*Surcharged (HGL above crown). ; j - Line contains hyd. jump.

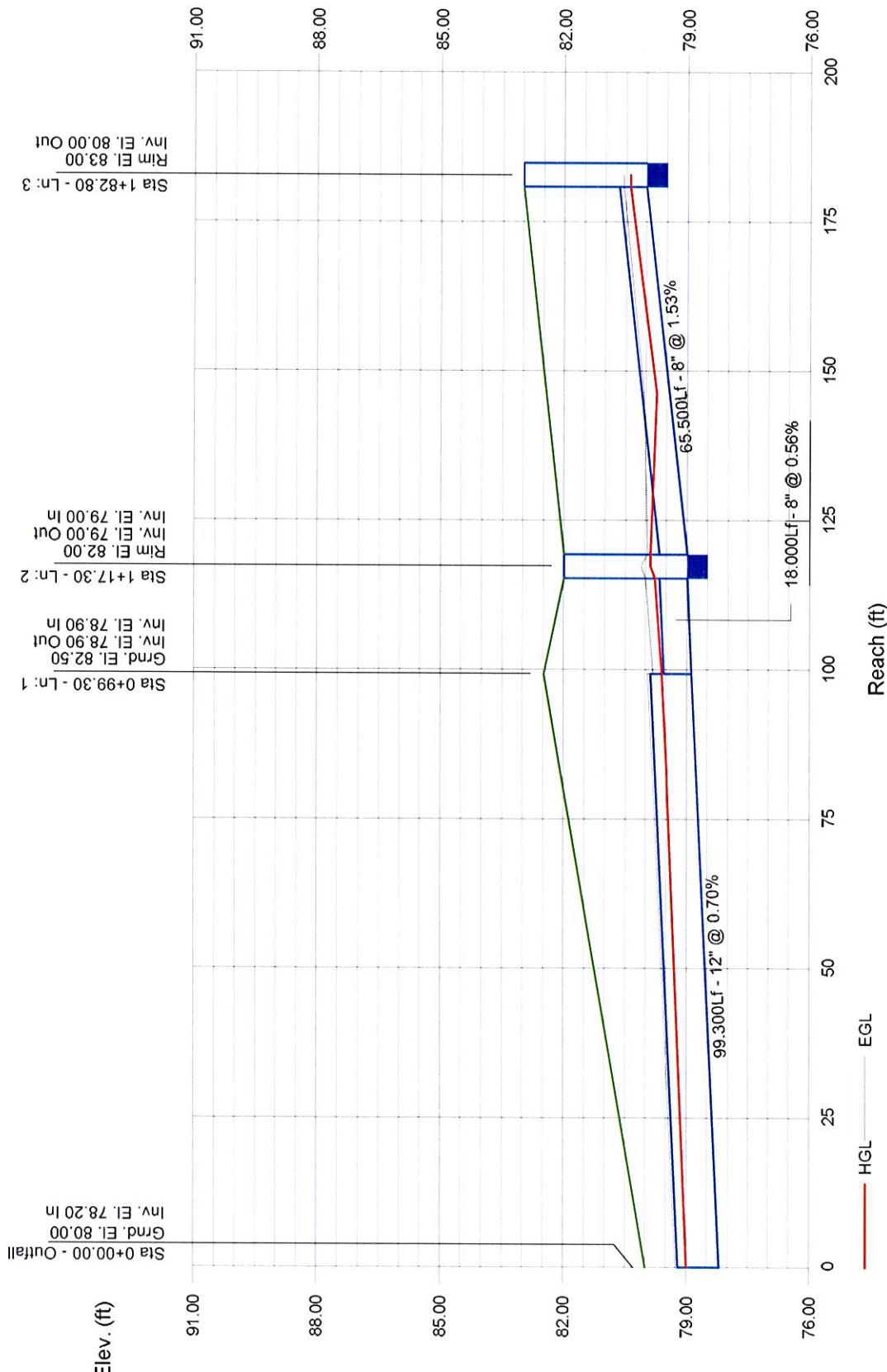
Storm Sewers v9.00

# Storm Sewer Tabulation

Station	Len	Drgn Area	Rnoff coeff	Area x C		Tc		Rain (I) (in/hr)	Total flow (cfs)	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Line ID
				Incr	Total	Inlet	Total					Size	Slope	Dn	Up	Dn	Up	
Line	To Line	(ft)	(ac)	(ac)	(C)		(min)	(min)	(ft/s)	(ft/s)	(ft/s)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
1	End	99.300	0.00	0.00	0.00	0.00	0.00	0.0	3.24	4.43	12	0.70	78.20	78.90	79.00	79.61	80.00	82.50
2	1	18.000	0.00	0.00	0.00	0.00	0.00	0.5	0.98	3.72	8	0.56	78.90	79.00	79.61	79.79	82.50	82.00
3	2	65.500	0.00	0.00	0.00	0.00	0.00	0.0	0.70	1.62	2.63	8	1.53	79.00	80.00	79.90	80.39	82.00
4	1	36.800	0.00	0.00	0.00	0.00	0.00	1.2	0.0	1.50	4.30	8	0.68	78.90	79.15	79.61	80.09	82.50
5	4	79.100	0.00	0.00	0.00	0.00	0.00	0.0	0.40	1.09	1.17	8	0.70	79.15	79.70	80.24	80.30	82.00
1900 PRVT SD EAST																		
Number of lines: 5																		
Run Date: 3/6/2012																		
NOTES: Intensity = 69.87 / (Inlet time + 13.10) ^ 0.87; Return period = Yrs. 2 ; c = cir e = ellip b = box																		

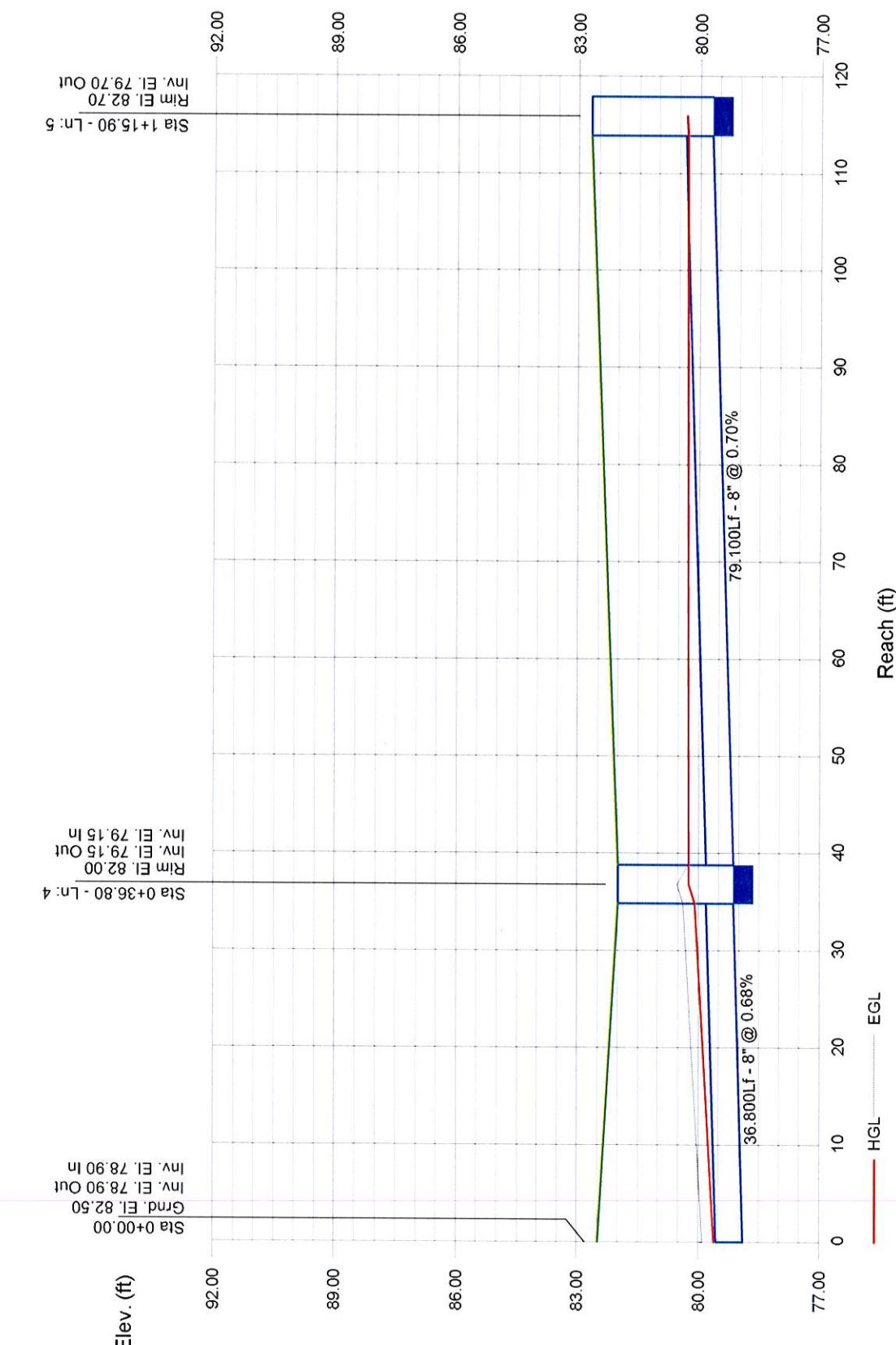
# Storm Sewer Profile

Proj. file: 1900-PRVT SD EAST.stm

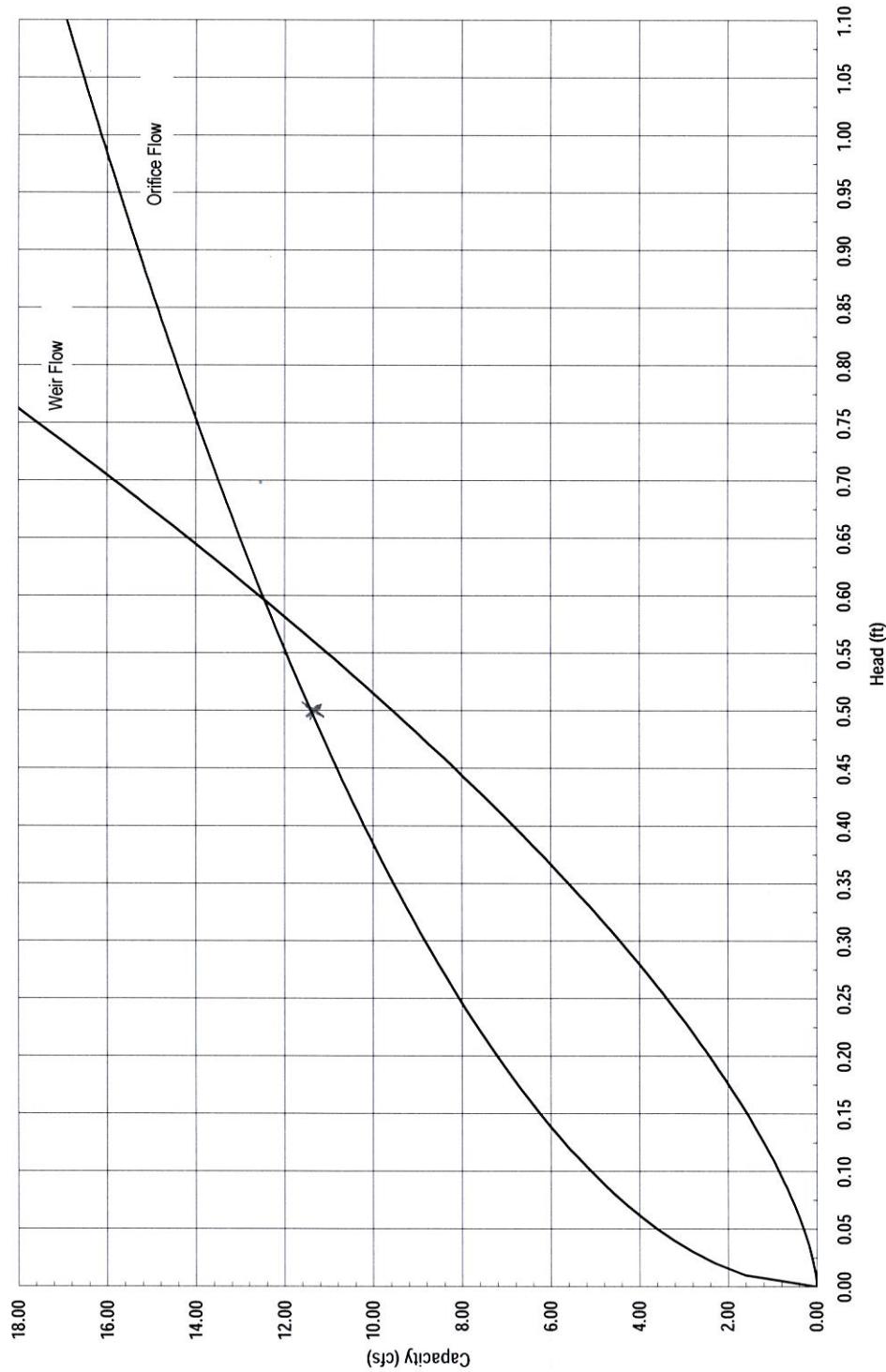


# Storm Sewer Profile

Proj. file: 1900-PRVT SD EAST.stm



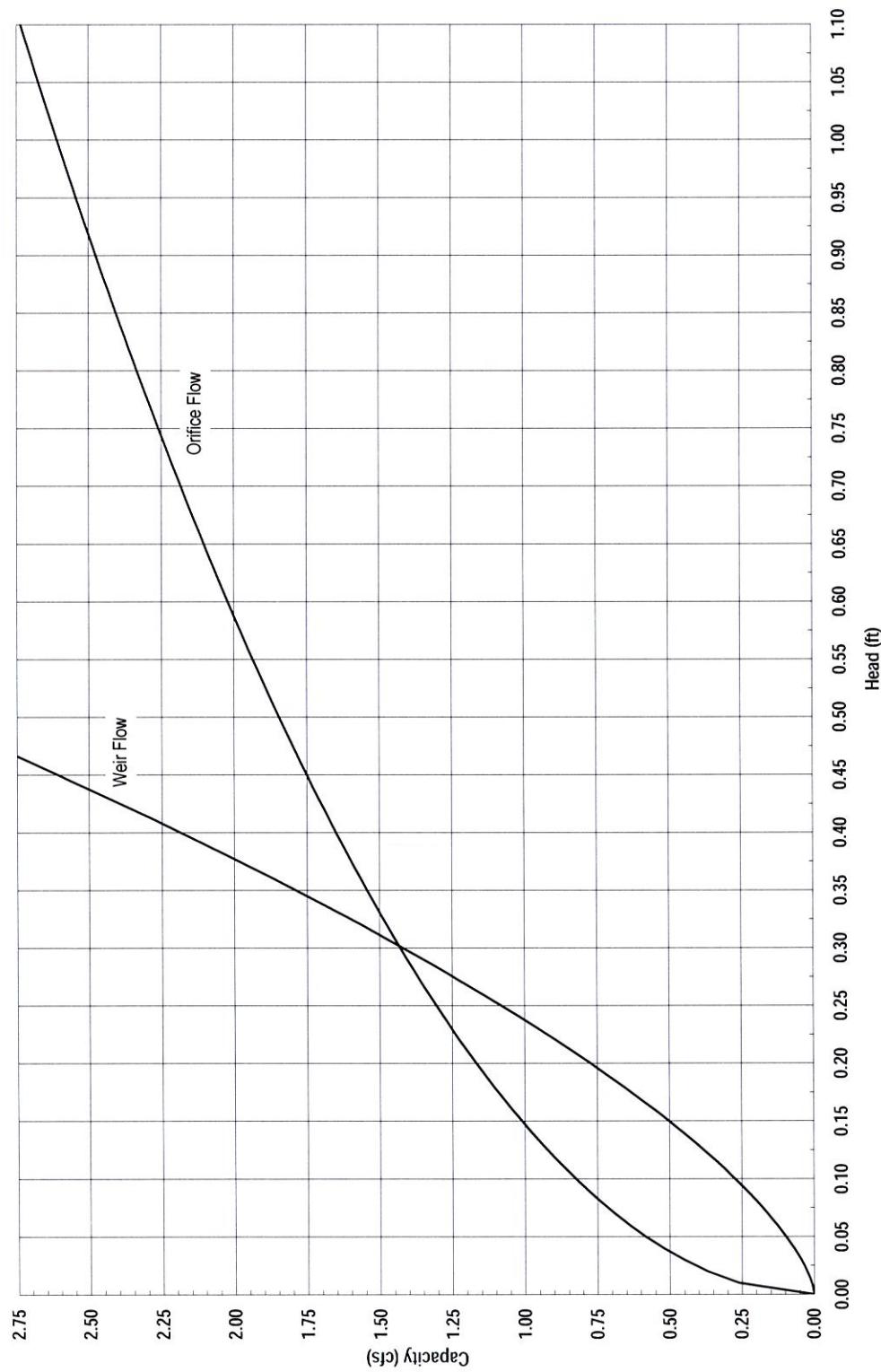
### Nyloplast 2' x 3' Road & Highway Grate Inlet Capacity Chart



**Nyloplast**<sup>®</sup>

3130 Verona Avenue • Buford, Georgia 30518 • (866) 888-8479 / (770) 932-2443 • Fax: (770) 932-2490  
Nyloplast Grate Inlet Capacity Charts - March 2010

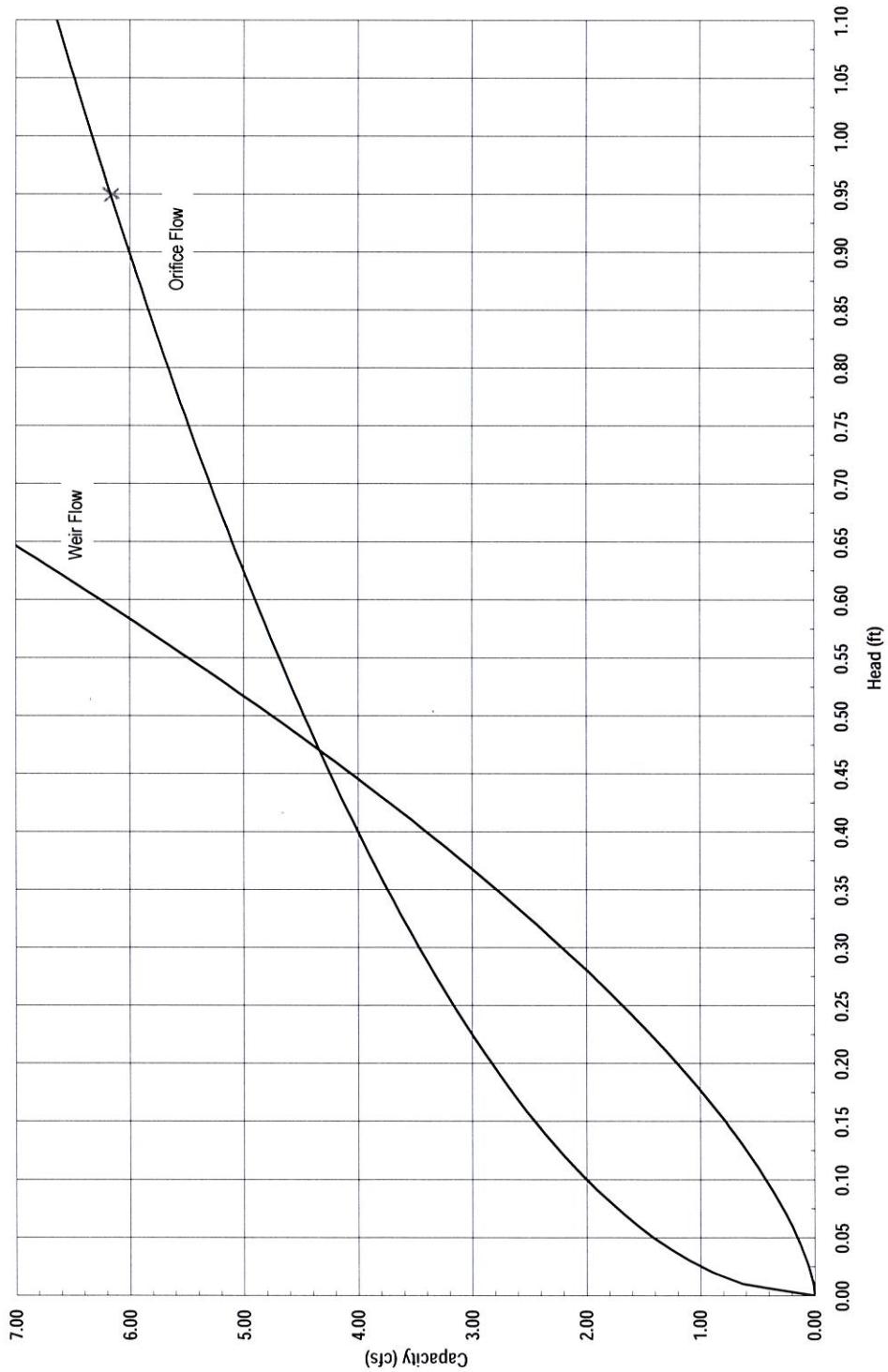
### Nyloplast 12" Dome Grate Inlet Capacity Chart



**Nyloplast**<sup>®</sup>

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### Nyloplast 18" Dome Grate Inlet Capacity Chart



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## **APPENDIX D**

### **SIDEWALK CULVERT CALCULATIONS**

## **BROADSTONE SANTA MONICA**

### **SIDEWALK CULVERT CAPACITY CALCULATION**

Weir equation:  $Q=CLH^{3/2}$

Constant  $C = 3.33$

Curb height  $H = 0.5$  feet

Opening Length  $L = 1.00$  feet

$$Q = 1.2 \text{ cfs}$$

Opening Length  $L = 1.50$  feet

$$Q = 1.8 \text{ cfs}$$

Opening Length  $L = 2.00$  feet

$$Q = 2.4 \text{ cfs}$$

## **APPENDIX E**

### **DRIVING AISLE CAPACITY CALCULATION**

# Channel Report

## **SECTION A-A -- DRIVE AT WEST END OF BUILDING 16**

User-defined		Highlighted	
Invert Elev (ft)	= 71.60	Depth (ft)	= 0.30
Slope (%)	= 1.40	Q (cfs)	= 17.00
N-Value	= 0.017	Area (sqft)	= 5.56
		Velocity (ft/s)	= 3.06
<b>Calculations</b>		Wetted Perim (ft)	= 30.84
Compute by:	Known Q	Crit Depth, Yc (ft)	= 0.34
Known Q (cfs)	= 17.00	Top Width (ft)	= 30.83
		EGL (ft)	= 0.45
<b>(Sta, El, 15)(4, 10, 72, 30, 0.017)-(3, 00, 72, 20, 0.017)-(6, 00, 72, 00, 0.017)-(15, 50, 71, 70, 0.017)-(28, 00, 71, 60, 0.017)-(44, 00, 72, 00, 0.017)</b>			
<b>(Sta, El, 15)(4, 10, 72, 30, 0.017)-(3, 00, 72, 20, 0.017)-(6, 00, 72, 00, 0.017)-(15, 50, 71, 70, 0.017)-(28, 00, 71, 60, 0.017)-(44, 00, 72, 00, 0.017)</b>			

