

# CITY OF ALBUQUERQUE

*Planning Department*  
David Campbell, Director



*Mayor Timothy M. Keller*

November 13, 2018

Joel Hernandez, P.E.  
Tierra West, LLC  
5571 Midway Park Place NE  
Albuquerque, NM, 87109

**RE: Overture Senior Active Adult  
6410 Palomas Ave NE  
Grading and Drainage Plan  
Engineer's Stamp Date: 10/17/18  
Hydrology File: D18D056A**

Dear Mr. Hernandez:

PO Box 1293

Based upon the information provided in your resubmittal received 10/18/2018, the Grading and Drainage Plan is approved for Building Permit and Grading Permit.

Albuquerque

Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Prior to approval in support of Permanent Release of Occupancy by Hydrology, Engineer Certification per the DPM checklist will be required.

NM 87103

If you have any questions, please contact me at 924-3995 or [rbrissette@cabq.gov](mailto:rbrissette@cabq.gov).

[www.cabq.gov](http://www.cabq.gov)

Sincerely,

Renée C. Brissette, P.E. CFM  
Senior Engineer, Hydrology  
Planning Department

# **DRAINAGE MANAGEMENT PLAN**

**For**

## **Overture Senior Active Adult**

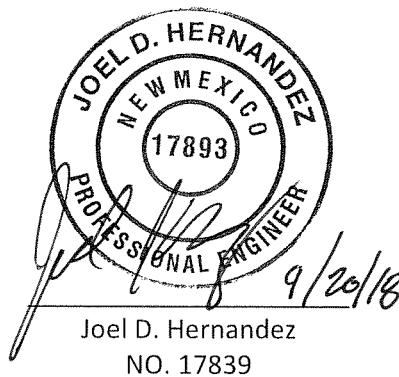
**NEC San Pedro Drive and Palomas Avenue  
Albuquerque, New Mexico**

Prepared by:

Tierra West, LLC  
5571 Midway Park Place NE  
Albuquerque, New Mexico 87109

September 2018

I certify that this report was prepared under my supervision, and I am a registered Professional Engineer in the State of New Mexico in good standing.



Joel D. Hernandez  
NO. 17839

*TW Job No. 2017086*

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### **Appendices**

Hydrologic and Hydraulic Analysis .....	APPENDIX A
Approved Conceptual Master Drainage Plan.....	APPENDIX B

### **Map Pockets**

Grading and Drainage Plan .....	Map Pocket
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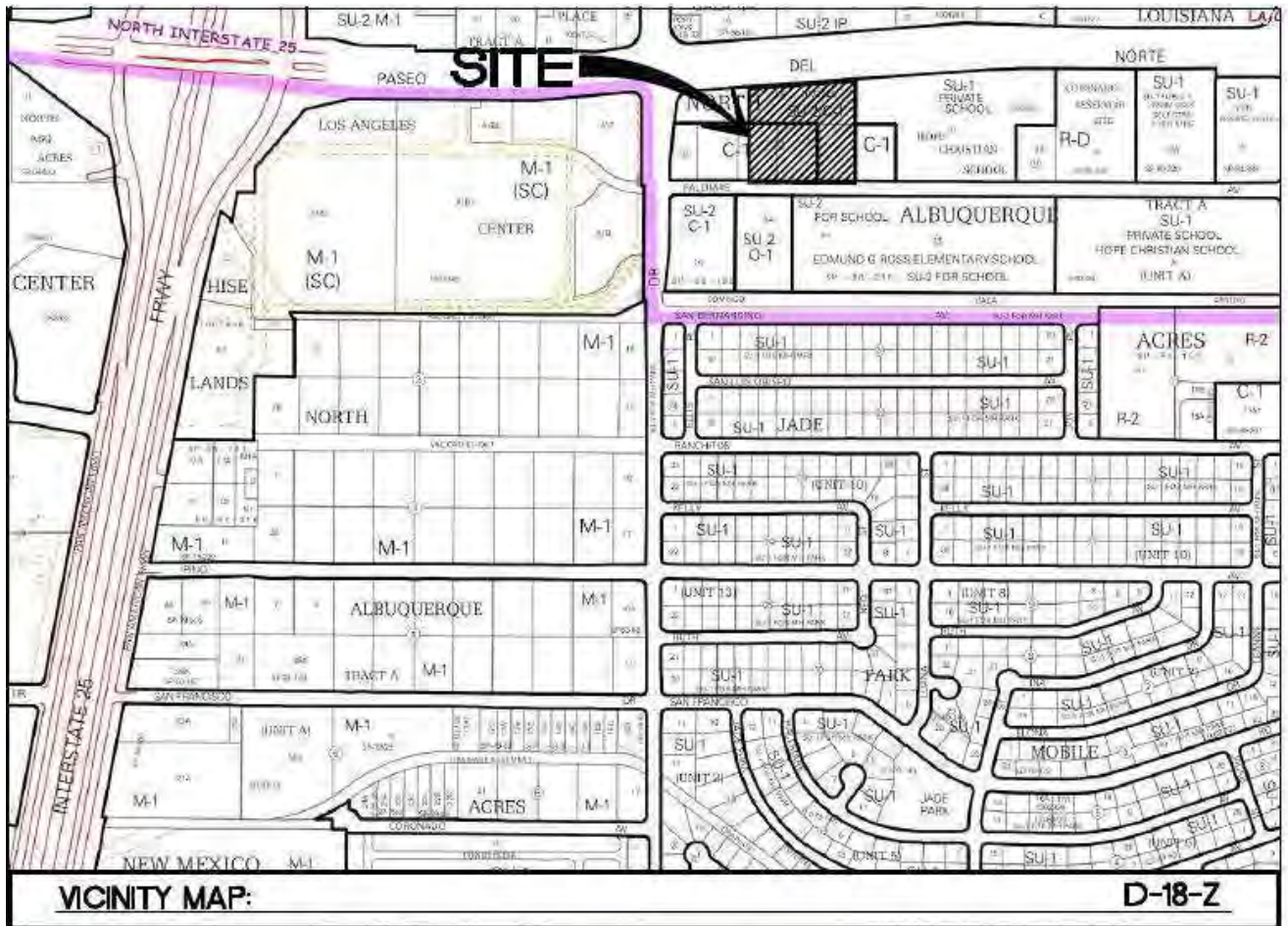
## Introduction

The purpose of this report is to provide a Drainage Management Plan for Development Review Board (DRB) approval of the Site Development Plan for Building Permit. This project consists of a proposed four-story apartment building for age-restricted housing and associated parking, and landscaped areas on a 4.07-acre site. The site is located at the northeast corner of San Pedro Drive and Palomas Ave, NE. The subject property is currently comprised of a portion of Lots 5-A, 28, 29 and 30 for which a Site Development Plan for Subdivision has been approved by the Environmental Planning Commission to reconfigure into five (5) Lots (this proposed subdivision also includes Lots 31 and 32). A concurrent request to DRB to amend the Site Plan for Subdivision to two lots is being processed. This Site Development Plan for Building Permit is for the east lot (proposed Lot 1A) of the project which proposes development of age restricted single family apartments on the easterly lot; commercial use on the westerly lot (proposed Lot 2A) will be addressed by a separate site development plan and drainage study by others.

As shown on the vicinity map below, the site is located at the northeast corner of San Pedro Drive and Palomas Ave, NE and bound by Paseo Del Norte on the north, an undeveloped portion of Hope Christian School property to the east, Palomas Avenue and developed office buildings to the south, and undeveloped property to the west proposed for commercial development as described above.

The site lies within Precipitation Zone 3 according to Section 22.2 of the DPM. As shown on FEMA Flood Map 35001C0137H, the site lies outside any flood hazard zone.

## Exhibit A-Vicinity Map





## Exhibit B- Flood Insurance Rate Map

### National Flood Hazard Layer FIRMette



## **Pre-Developed Conditions**

The overall site has been previously graded and mostly undeveloped with the exception of an existing single-story office building on the southeast portion of the property which is slated for demolition.

Surface runoff from the site generally flows from east to west and is conveyed to Palomas Avenue and San Pedro Drive where curb inlets intercept the flow into an existing 84-inch storm drain line. Offsite runoff calculated at 8.3 CFS from the property to the east drains onto the site by surface runoff. Existing runoff from the site is calculated at 13.9 CFS onto proposed Lot 2A (Basin EX-2) and 2.5 CFS onto Palomas Avenue (Basin EX-1) as indicated on the Pre-Development Drainage Basin map included in Appendix A.

## **Post Developed conditions**

The intent of the drainage configuration is to provide a drainage solution consistent with the DPM and the approved Conceptual Master Drainage Master Plan prepared by RESPEC (stamp date 3/22/18, Appendix B) for the overall development. This is achieved by the grading and drainage design which will accept offsite flows in a proposed concrete V-ditch along the easterly property boundary upstream and convey them through the proposed private storm drain system which will discharge into the existing junction box on the 84" storm drain system. The total drainage from the site is calculated at 26.83 CFS which is in line with the 27.9 CFS anticipated by the Conceptual Master Drainage Master Plan referenced above. All drainage is conveyed via the proposed storm drain system with the exception of the 0.66 CFS from Basin B5 which will need to drain onto Palomas Avenue due to elevation constraints. The net flow onto Palomas Avenue will decrease from 2.5 CFS to 0.66 CFS.

The building design includes an interior courtyard which is designed to drain via an underground drain system and connecting to the proposed storm drain along the northerly building side ; roof drains will also interconnect into the private system.

## **First-Flush Water Quality Considerations**

This project has elected to pay the in-lieu fee to address the first flush requirements. The total first flush volume was calculated to be 4,045 cubic feet. Calculations can be found in Appendix A.

## **Conclusion**

This Drainage Management Plan provides for grading and drainage elements which are capable of safely conveying the 100-year, 6-hour storm which meet the City DPM requirements and is in conformance with the approved Conceptual Drainage Management Plan approved for this area. With this submittal, we request Drainage Report Approval for the Site Development Plan for Building Permit.

# **APPENDIX A**

## **HYDROLOGY**



# **AGE RESTRICTED SENIOR LIVING** Existing Conditions Basin Data Table

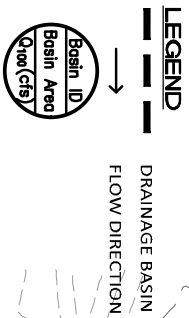
This table is based on the DPM Section 22.2, Zone: 3										
BASIN	Area (SQ. FT.)	Area (AC.)	Land Treatment Percentages				Q(100) (cfs/ac.)	Q(100) (CFS)	V(100) (inches)	V(100) (CF)
			A	B	C	D				
EXISTING CONDITIONS										
EX1	28158	0.65	0.0%	0.0%	69.7%	30.3%	3.93	2.54	1.61	3788
EX2	149478	3.43	0.0%	0.0%	93.6%	16.4%	4.05	13.91	1.59	19862
OS1		3.30	60.0%		40.0%		2.50	8.26		
TOTAL		7.38						16.44	1.61	23649
OFFSITE BASIN (Not included in total)										

Proposed Conditions Basin Data Tables

Parking Lot Basins											
BASIN	Area (SQ. FT.)	Area (AC.)	Land Treatment Percentages				Q(100) (cfs/ac.)	Q(100) (CFS)	V(100) (inches)	V(100) (CF)	1ST FLUSH (CF)
			A	B	C	D					
PROPOSED CONDITIONS											
B1	29626	0.68	0.0%	0.0%	16.4%	83.6%	4.76	3.24	2.18	5393	702
B2	16683	0.38	0.0%	0.0%	17.2%	82.8%	4.75	1.82	2.18	3025	391
B3	7235	0.17	0.0%	0.0%	19.7%	80.3%	4.71	0.78	2.15	1296	165
B4	12183	0.28	0.0%	0.0%	11.6%	88.4%	4.84	1.35	2.24	2270	305
B5	6119	0.14	0.0%	0.0%	21.0%	79.0%	4.69	0.66	2.14	1089	137
B6	7637	0.18	0.0%	0.0%	14.3%	85.7%	4.80	0.84	2.21	1405	185
B7	15789	0.36	0.0%	0.0%	14.3%	85.7%	4.80	1.74	2.21	2904	383
B8	10720	0.25	0.0%	0.0%	13.1%	86.9%	4.81	1.18	2.22	1983	264
TOTAL		2.19						11.62	15.29	17381	2533

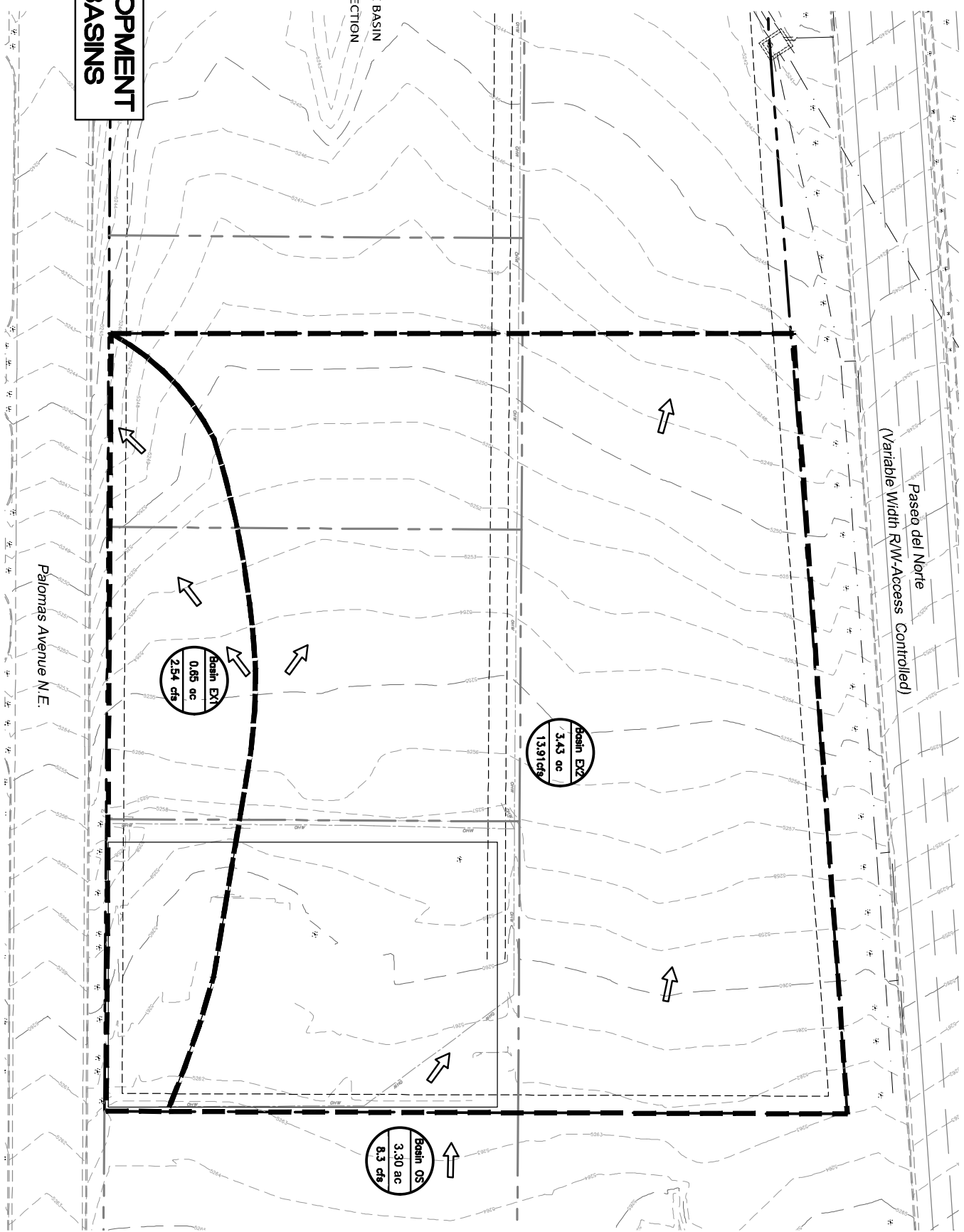
Building Basins											
BASIN	Area (SQ. FT.)	Area (AC.)	Land Treatment Percentages				Q(100) (cfs/ac.)	Q(100) (CFS)	V(100) (inches)	V(100) (CF)	1ST FLUSH (CF)
			A	B	C	D					
PROPOSED CONDITIONS											
RB1	10689	0.25	0.0%	0.0%	0.0%	100.0%	5.02	1.23	2.36	2102	303
RB2	9930	0.23	0.0%	0.0%	0.0%	100.0%	5.02	1.14	2.36	1953	281
RB3	2913	0.07	0.0%	0.0%	0.0%	100.0%	5.02	0.34	2.36	573	83
RB4	7990	0.18	0.0%	0.0%	0.0%	100.0%	5.02	0.92	2.36	1571	226
RB5	3775	0.09	0.0%	0.0%	0.0%	100.0%	5.02	0.44	2.36	742	107
RB6	20577	0.47	0.0%	0.0%	50.0%	50.0%	4.24	2.00	1.83	3129	292
RB7	15544	0.36	0.0%	0.0%	50.0%	50.0%	4.24	1.57	1.83	2364	220
TOTAL		1.64						7.58	15.45	12435	1512

TOTAL REQUIRED FIRST FLUSH VOLUME4044 CF



Analysis Point  
0100

**PRE-DEVELOPMENT  
DRAINAGE BASINS**





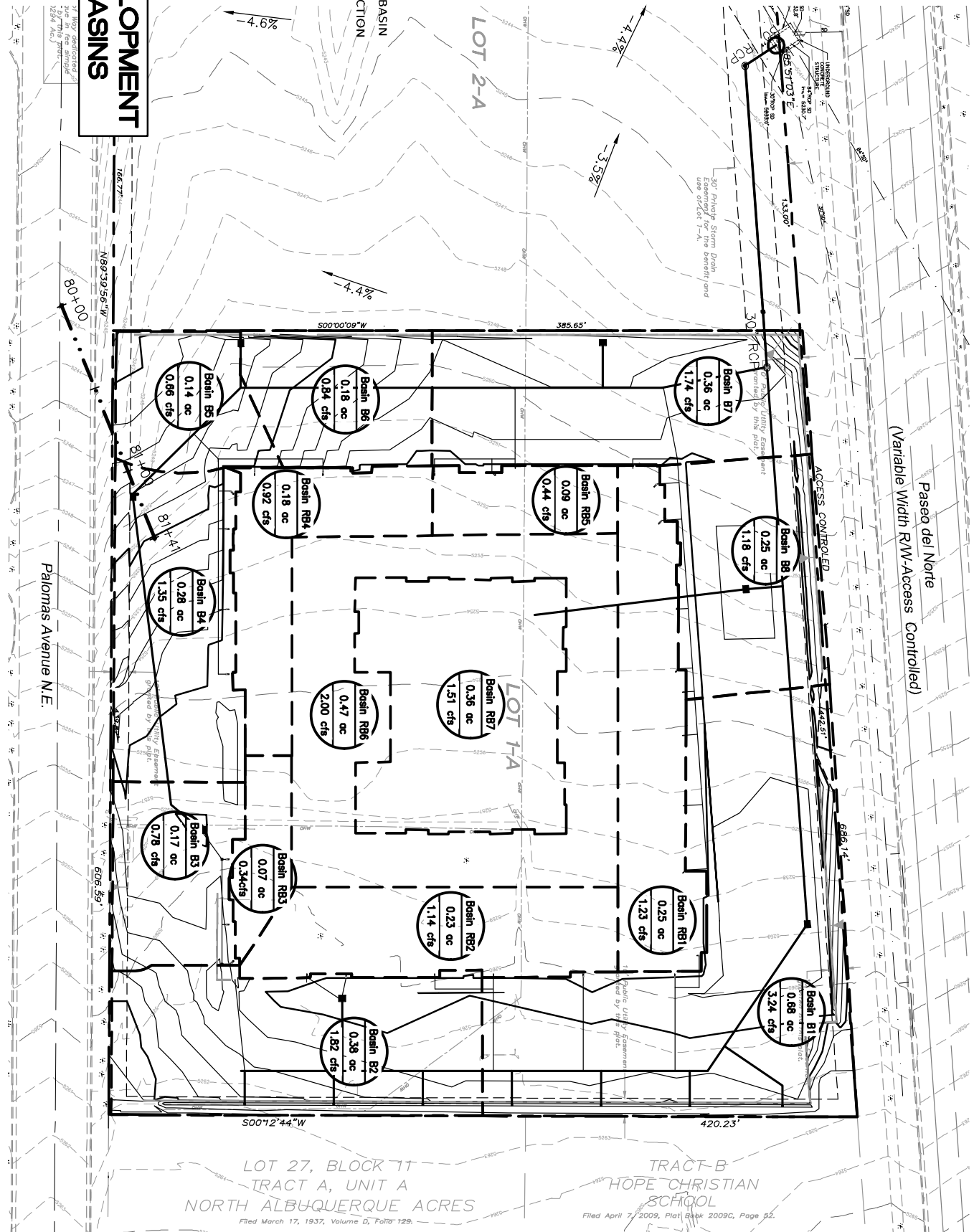
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**Q100**



!!!

FLOW DIRECTION





## Worksheet for Circular Pipe - P100

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.03500	ft/ft
Diameter	12	in
Discharge	1.12	ft <sup>3</sup> /s

### Results

Normal Depth	2.91	in
Flow Area	0.15	ft <sup>2</sup>
Wetted Perimeter	1.03	ft
Hydraulic Radius	1.72	in
Top Width	0.86	ft
Critical Depth	0.45	ft
Percent Full	24.3	%
Critical Slope	0.00349	ft/ft
Velocity	7.60	ft/s
Velocity Head	0.90	ft
Specific Energy	1.14	ft
Froude Number	3.23	
Maximum Discharge	9.32	ft <sup>3</sup> /s
Discharge Full	8.66	ft <sup>3</sup> /s
Slope Full	0.00058	ft/ft
Flow Type	SuperCritical	

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Bentley FlowMaster V8i (SELECTseries 1) [08.11.01.03]

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## Worksheet for Circular Pipe - P101

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01200	ft/ft
Diameter	12	in
Discharge	3.39	ft <sup>3</sup> /s

### Results

Normal Depth	7.18	in
Flow Area	0.49	ft <sup>2</sup>
Wetted Perimeter	1.77	ft

Hydraulic Radius	3.33	in
Top Width	0.98	ft
Critical Depth	0.79	ft
Percent Full	59.8	%
Critical Slope	0.00579	ft/ft
Velocity	6.92	ft/s
Velocity Head	0.74	ft
Specific Energy	1.34	ft
Froude Number	1.73	
Maximum Discharge	5.46	ft <sup>3</sup> /s
Discharge Full	5.07	ft <sup>3</sup> /s
Slope Full	0.00536	ft/ft
Flow Type	SuperCritical	

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## Worksheet for Circular Pipe - P102

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.02300	ft/ft
Diameter	12	in
Discharge	4.43	ft <sup>3</sup> /s

### Results

Normal Depth	6.91	in
Flow Area	0.47	ft <sup>2</sup>
Wetted Perimeter	1.72	ft
Hydraulic Radius	3.26	in
Top Width	0.99	ft
Critical Depth	0.88	ft
Percent Full	57.6	%
Critical Slope	0.00821	ft/ft
Velocity	9.46	ft/s
Velocity Head	1.39	ft
Specific Energy	1.97	ft
Froude Number	2.42	
Maximum Discharge	7.56	ft <sup>3</sup> /s
Discharge Full	7.02	ft <sup>3</sup> /s
Slope Full	0.00915	ft/ft
Flow Type	SuperCritical	

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## Worksheet for Circular Pipe - P103

### Project Description

Friction Method                      Manning Formula  
Solve For                              Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.08700	ft/ft
Diameter	12	in
Discharge	7.64	ft <sup>3</sup> /s

### Results

Normal Depth	6.42	in
Flow Area	0.43	ft <sup>2</sup>
Wetted Perimeter	1.64	ft
Hydraulic Radius	3.13	in
Top Width	1.00	ft
Critical Depth	0.98	ft
Percent Full	53.5	%
Critical Slope	0.02450	ft/ft
Velocity	17.88	ft/s
Velocity Head	4.97	ft
Specific Energy	5.50	ft
Froude Number	4.82	
Maximum Discharge	14.69	ft <sup>3</sup> /s
Discharge Full	13.66	ft <sup>3</sup> /s
Slope Full	0.02721	ft/ft
Flow Type	SuperCritical	

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## Worksheet for Circular Pipe - P104

### Project Description

Friction Method                      Manning Formula  
Solve For                              Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.02650	ft/ft
Diameter	18	in
Discharge	19.19	ft <sup>3</sup> /s

### Results

Normal Depth	12.90	in
Flow Area	1.36	ft <sup>2</sup>
Wetted Perimeter	3.03	ft
Hydraulic Radius	5.37	in
Top Width	1.35	ft

Critical Depth	1.46	ft
Percent Full	71.7	%
Critical Slope	0.01751	ft/ft
Velocity	14.15	ft/s
Velocity Head	3.11	ft
Specific Energy	4.19	ft
Froude Number	2.49	
Maximum Discharge	23.91	ft <sup>3</sup> /s
Discharge Full	22.23	ft <sup>3</sup> /s
Slope Full	0.01975	ft/ft
Flow Type	SuperCritical	

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## Worksheet for Circular Pipe - P105

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.02600	ft/ft
Diameter	18	in
Discharge	14.50	ft <sup>3</sup> /s

### Results

Normal Depth	10.66	in
Flow Area	1.09	ft <sup>2</sup>
Wetted Perimeter	2.63	ft
Hydraulic Radius	4.96	in
Top Width	1.47	ft
Critical Depth	1.39	ft
Percent Full	59.2	%
Critical Slope	0.00976	ft/ft
Velocity	13.30	ft/s
Velocity Head	2.75	ft
Specific Energy	3.64	ft
Froude Number	2.73	
Maximum Discharge	23.68	ft <sup>3</sup> /s
Discharge Full	22.02	ft <sup>3</sup> /s
Slope Full	0.01128	ft/ft
Flow Type	SuperCritical	

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## Worksheet for Circular Pipe - P106

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01200	ft/ft
Diameter	18	in
Discharge	11.26	ft <sup>3</sup> /s

### Results

Normal Depth	11.66	in
Flow Area	1.21	ft <sup>2</sup>
Wetted Perimeter	2.81	ft
Hydraulic Radius	5.18	in
Top Width	1.43	ft
Critical Depth	1.28	ft
Percent Full	64.8	%
Critical Slope	0.00635	ft/ft
Velocity	9.29	ft/s
Velocity Head	1.34	ft
Specific Energy	2.31	ft
Froude Number	1.78	
Maximum Discharge	16.09	ft <sup>3</sup> /s
Discharge Full	14.96	ft <sup>3</sup> /s
Slope Full	0.00680	ft/ft
Flow Type	SuperCritical	

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## Worksheet for 6" PVC Capacity Check (RB2 Laterals)

### Project Description

Friction Method	Manning Formula
Solve For	Full Flow Capacity

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01000	ft/ft
Normal Depth	6.00	in
Diameter	6.00	in
Discharge	0.73	ft <sup>3</sup> /s

### Results

Discharge	0.73	ft <sup>3</sup> /s
Normal Depth	6.00	in
Flow Area	0.20	ft <sup>2</sup>
Wetted Perimeter	1.57	ft

Hydraulic Radius	1.50	in
Top Width	0.00	ft
Critical Depth	0.43	ft
Percent Full	100.0	%
Critical Slope	0.00929	ft/ft
Velocity	3.71	ft/s
Velocity Head	0.21	ft
Specific Energy	0.71	ft
Froude Number	0.00	
Maximum Discharge	0.78	ft <sup>3</sup> /s
Discharge Full	0.73	ft <sup>3</sup> /s
Slope Full	0.01000	ft/ft
Flow Type	SubCritical	

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-Flowmaster results show that each 6" PVC drain can convey 0.73 cfs at full capacity. The 1.14 cfs produced by basin RB2 is divided among 6 pipes --  $1.14\text{cfs}/6 = 0.19\text{ cfs}$ . Therefore 6" PVC will be adequate to convey the flows from RB2 with redundancy.

## Worksheet for Circular Pipe - RP103

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01000	ft/ft
Diameter	12	in
Discharge	2.20	ft <sup>3</sup> /s

### Results

Normal Depth	5.82	in
Flow Area	0.38	ft <sup>2</sup>
Wetted Perimeter	1.54	ft
Hydraulic Radius	2.94	in
Top Width	1.00	ft
Critical Depth	0.63	ft
Percent Full	48.5	%
Critical Slope	0.00423	ft/ft
Velocity	5.82	ft/s
Velocity Head	0.53	ft
Specific Energy	1.01	ft

Froude Number	1.67
Maximum Discharge	4.98 ft <sup>3</sup> /s
Discharge Full	4.63 ft <sup>3</sup> /s
Slope Full	0.00226 ft/ft
Flow Type	SuperCritical

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## Worksheet for Circular Pipe - RP111 –Internal Roof Basin and Courtyard Drain

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010
Channel Slope	0.01000 ft/ft
Diameter	15 in
Discharge	3.51 ft <sup>3</sup> /s

### Results

Normal Depth	6.77 in
Flow Area	0.54 ft <sup>2</sup>
Wetted Perimeter	1.84 ft
Hydraulic Radius	3.50 in
Top Width	1.24 ft
Critical Depth	0.76 ft
Percent Full	45.1 %
Critical Slope	0.00377 ft/ft
Velocity	6.53 ft/s
Velocity Head	0.66 ft
Specific Energy	1.23 ft
Froude Number	1.75
Maximum Discharge	9.03 ft <sup>3</sup> /s
Discharge Full	8.40 ft <sup>3</sup> /s
Slope Full	0.00175 ft/ft
Flow Type	SuperCritical

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Bentley FlowMaster V8i (SELECTseries 1) [08.11.01.03]

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## Worksheet for Circular Pipe - RP109

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01000	ft/ft
Diameter	12	in
Discharge	1.74	ft <sup>3</sup> /s

### Results

Normal Depth	5.10	in
Flow Area	0.32	ft <sup>2</sup>
Wetted Perimeter	1.42	ft
Hydraulic Radius	2.69	in
Top Width	0.99	ft
Critical Depth	0.56	ft
Percent Full	42.5	%
Critical Slope	0.00386	ft/ft
Velocity	5.48	ft/s
Velocity Head	0.47	ft
Specific Energy	0.89	ft
Froude Number	1.70	
Maximum Discharge	4.98	ft <sup>3</sup> /s
Discharge Full	4.63	ft <sup>3</sup> /s
Slope Full	0.00141	ft/ft
Flow Type	SuperCritical	

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Bentley FlowMaster V8i (SELECTseries 1) [08.11.01.03]

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## Worksheet for Circular Pipe - RP110

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01000	ft/ft
Diameter	12	in
Discharge	0.84	ft <sup>3</sup> /s

### Results

Normal Depth	3.46	in
Flow Area	0.19	ft <sup>2</sup>
Wetted Perimeter	1.13	ft
Hydraulic Radius	1.99	in



Top Width	0.91	ft
Critical Depth	0.38	ft
Percent Full	28.9	%
Critical Slope	0.00338	ft/ft
Velocity	4.48	ft/s
Velocity Head	0.31	ft
Specific Energy	0.60	ft
Froude Number	1.73	
Maximum Discharge	4.98	ft <sup>3</sup> /s
Discharge Full	4.63	ft <sup>3</sup> /s
Slope Full	0.00033	ft/ft
Flow Type	SuperCritical	

Bentley Systems, Inc. Haestad Methods Solution Center

Bentley FlowMaster V8i (SELECTseries 1) [08.11.01.03]

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## Worksheet for 8" PVC Capacity Check (RP-105,106,107,108)

### Project Description

Friction Method	Manning Formula
Solve For	Full Flow Capacity

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01000	ft/ft
Normal Depth	8.00	in
Diameter	8.00	in
Discharge	1.57	ft <sup>3</sup> /s

### Results

Discharge	1.57	ft <sup>3</sup> /s
Normal Depth	8.00	in
Flow Area	0.35	ft <sup>2</sup>
Wetted Perimeter	2.09	ft
Hydraulic Radius	2.00	in
Top Width	0.00	ft
Critical Depth	0.58	ft
Percent Full	100.0	%
Critical Slope	0.00907	ft/ft
Velocity	4.50	ft/s
Velocity Head	0.31	ft
Specific Energy	0.98	ft
Froude Number	0.00	
Maximum Discharge	1.69	ft <sup>3</sup> /s
Discharge Full	1.57	ft <sup>3</sup> /s
Slope Full	0.01000	ft/ft
Flow Type	SubCritical	

-The remaining roof drop pipes will be set at 8". The largest flow to be conveyed by one of these pipes (RB1) is 1.23 cfs at the minimum slope of 1%. Therefore 8" PVC will be adequate for the remainder of the roof drains.

## Worksheet for Circular Pipe - P200- Outfall Pipe

### Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

### Input Data

Roughness Coefficient	0.010	
Channel Slope	0.01200	ft/ft
Diameter	30	in
Discharge	26.83	ft³/s

### Results

Normal Depth	14.27	in
Flow Area	2.30	ft²
Wetted Perimeter	3.81	ft
Hydraulic Radius	7.26	in
Top Width	2.50	ft
Critical Depth	1.77	ft
Percent Full	47.6	%
Critical Slope	0.00353	ft/ft
Velocity	11.65	ft/s
Velocity Head	2.11	ft
Specific Energy	3.30	ft
Froude Number	2.14	
Maximum Discharge	62.83	ft³/s
Discharge Full	58.41	ft³/s
Slope Full	0.00253	ft/ft
Flow Type	SuperCritical	

## Capacity of Inlets Along Eastern Ditch

### Orifice Equation

$$Q = CA \sqrt{2gH}$$

$$C = 0.6$$

$$\text{Diameter (in)} = 8$$

$$\text{Area (ft}^2\text{)} = 0.349$$

$$g = 32.2$$

$$H \text{ (Ft)} = 1 \text{ Max depth of water above center of orifice}$$

$$Q \text{ (CFS)} = 1.6807439 \text{ Capacity per inlet}$$

$$Q_{\text{total}} \text{ (CFS)} = 11.765207 \text{ Total Available Capacity of 7 inlets}$$

$$Q_{\text{required}} \text{ (cfs)} = 8.3 \text{ Total offsite runoff}$$

Total Available Capacity > Required Capacity  
therefore **OK**



# City of Albuquerque

Planning Department  
Development & Building Services Division

## DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

**Project Title:** Overture Senior Active Adult Building Permit #: \_\_\_\_\_ Hydrology File #: D18D056  
DRB#: \_\_\_\_\_ EPC#: \_\_\_\_\_ Work Order#: \_\_\_\_\_  
Legal Description: LOT 5-A, 28, 29, AND 30 OF BLOCK II NORTH ALBUQUERQUE ACRES  
City Address: \_\_\_\_\_

**Applicant:** tierra West, LLC Contact: Joel Hernandez  
Address: 5571 Midaway Park Place NE Albuquerque, NM 87109  
Phone#: 505-858-3100 Fax#: 505-858-1118 E-mail: jdhernandez@tierrawestllc.com

**Other Contact:** \_\_\_\_\_ Contact: \_\_\_\_\_  
Address: \_\_\_\_\_  
Phone#: \_\_\_\_\_ Fax#: \_\_\_\_\_ E-mail: \_\_\_\_\_

**TYPE OF DEVELOPMENT:** \_\_\_\_\_ PLAT (# of lots) \_\_\_\_\_ RESIDENCE ☒ DRB SITE \_\_\_\_\_ ADMIN SITE

IS THIS A RESUBMITTAL? ☒ Yes \_\_\_\_\_ No

**DEPARTMENT** \_\_\_\_\_ TRANSPORTATION ☒ HYDROLOGY/DRAINAGE

Check all that Apply:

**TYPE OF SUBMITTAL:**

- ☐ ENGINEER/ARCHITECT CERTIFICATION
- ☐ PAD CERTIFICATION
- ☐ CONCEPTUAL G & D PLAN
- ☒ GRADING PLAN
- ☐ DRAINAGE REPORT
- ☐ DRAINAGE MASTER PLAN
- ☐ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
- ☐ ELEVATION CERTIFICATE
- ☐ CLOMR/LOMR
- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
- ☐ TRAFFIC IMPACT STUDY (TIS)
- ☐ STREET LIGHT LAYOUT
- ☐ OTHER (SPECIFY) \_\_\_\_\_
- ☐ PRE-DESIGN MEETING?

**TYPE OF APPROVAL/ACCEPTANCE SOUGHT:**

- ☒ BUILDING PERMIT APPROVAL
- ☐ CERTIFICATE OF OCCUPANCY
- ☐ PRELIMINARY PLAT APPROVAL
- ☐ SITE PLAN FOR SUB'D APPROVAL
- ☐ SITE PLAN FOR BLDG. PERMIT APPROVAL
- ☐ FINAL PLAT APPROVAL
- ☐ SIA/ RELEASE OF FINANCIAL GUARANTEE
- ☐ FOUNDATION PERMIT APPROVAL
- ☐ GRADING PERMIT APPROVAL
- ☐ SO-19 APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ GRADING/ PAD CERTIFICATION
- ☐ WORK ORDER APPROVAL
- ☐ CLOMR/LOMR
- ☐ FLOODPLAIN DEVELOPMENT PERMIT
- ☐ OTHER (SPECIFY) \_\_\_\_\_

DATE SUBMITTED: 10/18/2018 By: Joel Hernandez

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: \_\_\_\_\_

FEE PAID: \_\_\_\_\_



# TIERRA WEST, LLC

October 17, 2018

Renée C. Brissette, P.E. CFM  
Planning Department- Hydrology  
City of Albuquerque  
P.O. Box 1293  
Albuquerque, NM 87103

**RE: OVERTURE SENIOR ACTIVE ADULT  
GRADING AND DRAINAGE (FILE: D18D056A)**

Please find the following responses addressing staff comments from correspondence dated October 1, 2018 listed below:

1. The site currently shows more than 1 acre of disturbance is being proposed. An Erosion and Sediment Control Plan is required and has to be submitted to the storm water quality engineer (Curtis Cherne, PE, [ccherne@cabq.gov](mailto:ccherne@cabq.gov)). Hydrology's approval for Grading or Building Permit will not be given until the submittal of the ESC Plan.

**RESPONSE: Comment noted. An Erosion and Sediment Control Plan will be submitted by separate cover to the Storm Water Quality Engineer.**

2. Please provide the FIRM Map's effective date.

**RESPONSE: The FIRM Map's effective date is 08/16/2012. We added this notation to Sheet C2.**

3. Please provide the benchmark information for the survey contour information provided.

**RESPONSE: We added the benchmark information to Sheet C2 (located in Paseo Del Norte sidewalk, near northeasterly property corner).**

4. Please provide a section of the proposed retaining wall along Paseo del Norte near the Northwest corner of the site and a section of the proposed retaining wall along the western property line.

**RESPONSE: The section views requested are now included on an added Sheet C2.1 for clarity.**

5. Please add a note requesting a waiver for the first flush volume requirement. Also add a note stating, "The required first flush volume for the 142,764.70 SF of impervious cover is 4,045 CF."

**RESPONSE: We have added a note indicating the first flush requirement and formally requesting a waiver to allow in-lieu fee payment onto Sheet C2.**

If you have any questions or need additional information regarding this matter, please do not hesitate to contact me.

Sincerely,

Joel Hernandez, PE

JN: 2017086

RRB/jh/jg

5571 Midway Park Pl. NE  
Albuquerque, NM 87109  
(505) 858-3100 fax (505) 858-1118 1-800-245-3102  
tierrawestllc.com

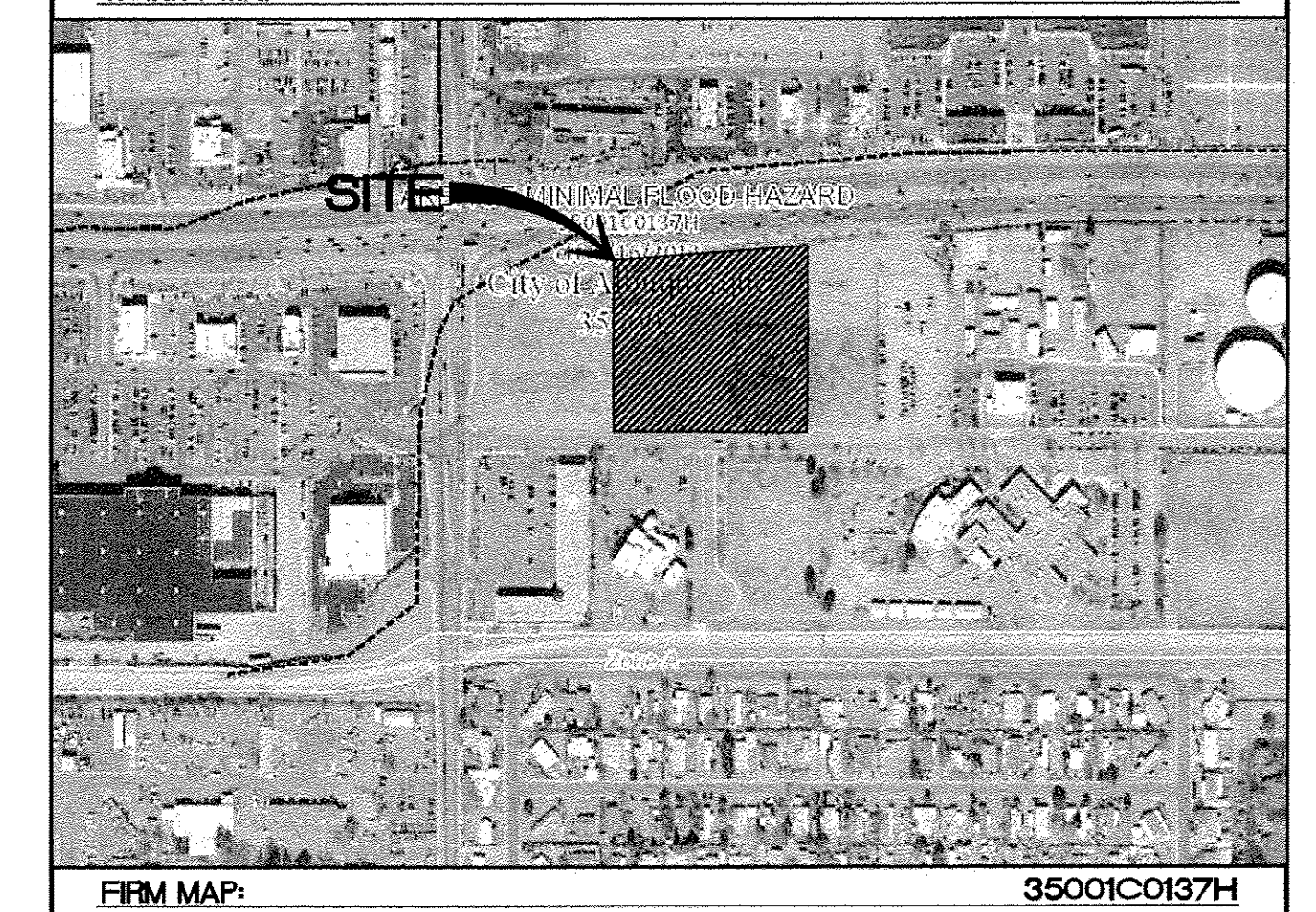
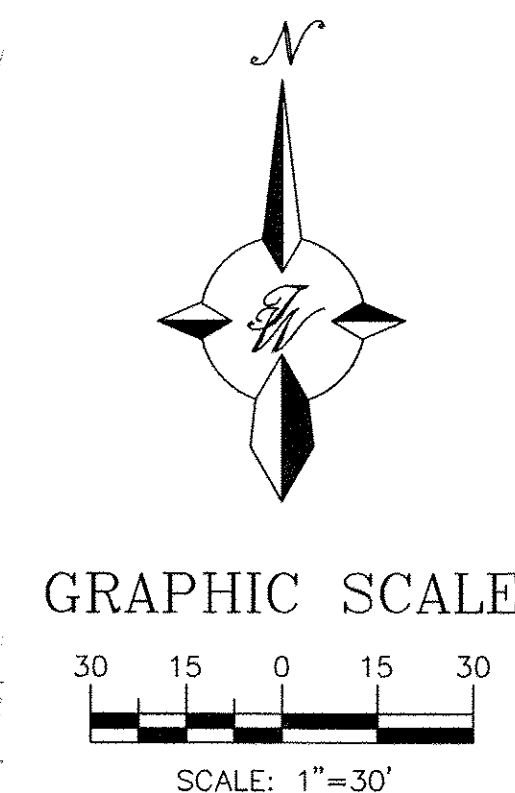


1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL (CITY) ACCEPTANCE OF ANY PROJECT.

- ① NYLOPLAST CATCH BASIN W/ 18" DROP IN GRATE
- ② NYLOPLAST CATCH BASIN W/ 24" DROP IN GRATE & BIO SNOT (SOUTH OUTLET)
- ③ NYLOPLAST CATCH BASIN W/ 2'X2' CURB INLET, STANDARD GRATE
- ④ NYLOPLAST CATCH BASIN W/ 2'X3' CURB INLET, DIAGONAL GRATE
- ⑤ NYLOPLAST CATCH BASIN W/ 2'X2' CURB INLET, STANDARD GRATE & BIO SNOT
- ⑥ NYLOPLAST CATCH BASIN W/ 2'X2' CURB INLET, STANDARD GRATE & BIO SNOT (NORTH OUTLET)
- ⑦ SEE DWG 532662 FOR STORM DRAIN PLAN AND PROFILE
- DS ROOF DOWNSPOUT. CONNECT TO PVT AREA DRAIN. SEE ARCHITECTURAL PLAN FOR DETAIL.

THE REQUIRED FIRST FLUSH VOLUME FOR THE 142,764.70 SF OF IMPERVIOUS COVER IS 4,045 CF. THIS PROJECT REQUESTS A WAIVER OF THE FIRST-FLUSH VOLUME ONSITE RETENTION REQUIREMENT TO ALLOW FOR THE DENSE URBAN DEVELOPMENT TYPE WHERE OFFSITE MITIGATION IS RECOMMENDED DUE TO THE SIZE OF THE LOT AND PHYSICAL CONDITIONS OF THE SITE WHICH DO NOT LEAVE SUFFICIENT AREA FOR ONSITE COMPLIANCE. PAYMENT-IN-LIEU, AS DETERMINED BY THE CITY ENGINEER, IS REQUIRED.

	CURB & GUTTER
	BOUNDARY LINE
	EASEMENT
	BUILDING
	SIDEWALK
	RETAINING WALL
	CONTOUR MAJOR
	CONTOUR MINOR
	SPOT ELEVATION
	FLOW ARROW
	EXISTING CURB & GUTTER
	EXISTING BOUNDARY LINE
	EXISTING CONTOUR MAJOR
	EXISTING CONTOUR MINOR
	EXISTING SPOT ELEVATION



## NOTICE TO CONTRACTORS

1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY.
2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE INTERIM STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, 1985.
3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES.
4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONNECTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE.
6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED. 7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.

APPROVAL	NAME	DATE
INSPECTOR		

ALL EXISTING UTILITIES SHOWN WERE OBTAINED FROM RESEARCH, AS-BUILTS, SURVEYS OR INFORMATION PROVIDED BY OTHERS. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO AND INCLUDING ANY EXCAVATION, TO DETERMINE THE ACTUAL LOCATION OF UTILITIES AND OTHER IMPROVEMENTS, PRIOR TO STARTING THE WORK. ANY CHANGES FROM THIS PLAN SHALL BE COORDINATED WITH AND APPROVED BY THE ENGINEER.

<p>ENGINEER'S SEAL</p>	<p><b>OVERTURE SENIOR ACTIVE ADULT</b></p>	<p>DRAWN BY DY</p>
	<p><b>GRADING AND DRAINAGE PLAN</b></p>	<p>DATE 10/09/18</p>
	<p> <i><b>TERRA WEST, LLC</b></i> 5571 MIDWAY PARK PLACE NE ALBUQUERQUE, NM 87109 (505) 858-3100 www.terrawestllc.com</p>	<p>2017086-GRB</p>
		<p>SHEET # <b>C2</b></p>
<p>RONALD R. BOHANNAN P.E. #78660</p>		<p>JOB # 2017086</p>



<p>ENGINEER'S SEAL</p>	<p><b>OVERTURE SENIOR ACTIVE ADULT</b></p>	<p>DRAWN BY DY</p>
	<p><b>GRADING AND DRAINAGE DETAILS</b></p>	<p>DATE 10/09/18</p>
<p>RONALD R. BOHANNAN P.E. #7868</p>	<p> <b>TERRA WEST, LLC</b> 5571 MIDWAY PARK PLACE NE ALBUQUERQUE, NM 87109 (505) 858-3100 www.tierrawestllc.com</p>	<p>SHEET # <b>C21</b></p>
<p>RONALD R. BOHANNAN P.E. #7868</p>		<p>JOB # 2017086</p>