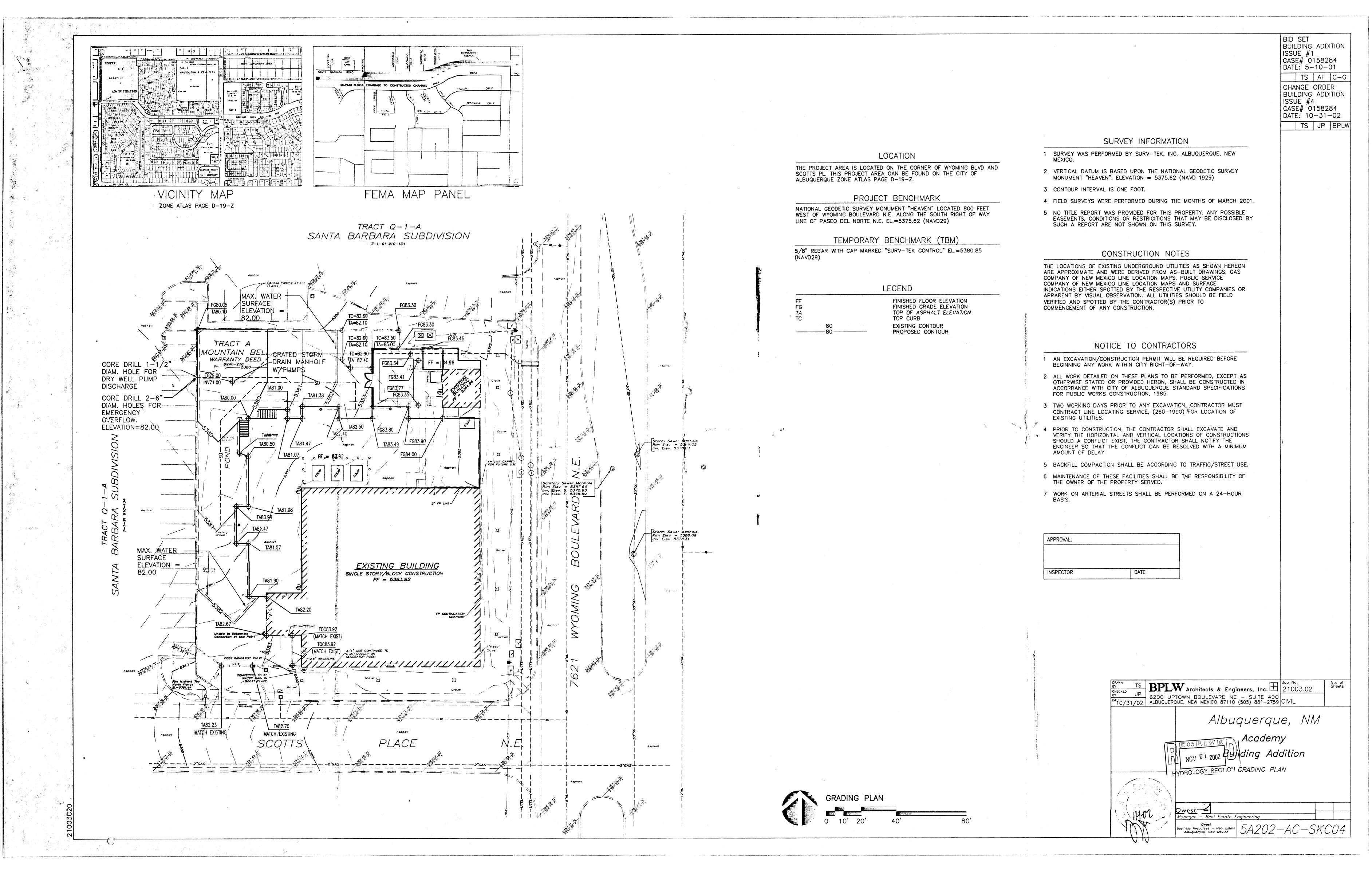
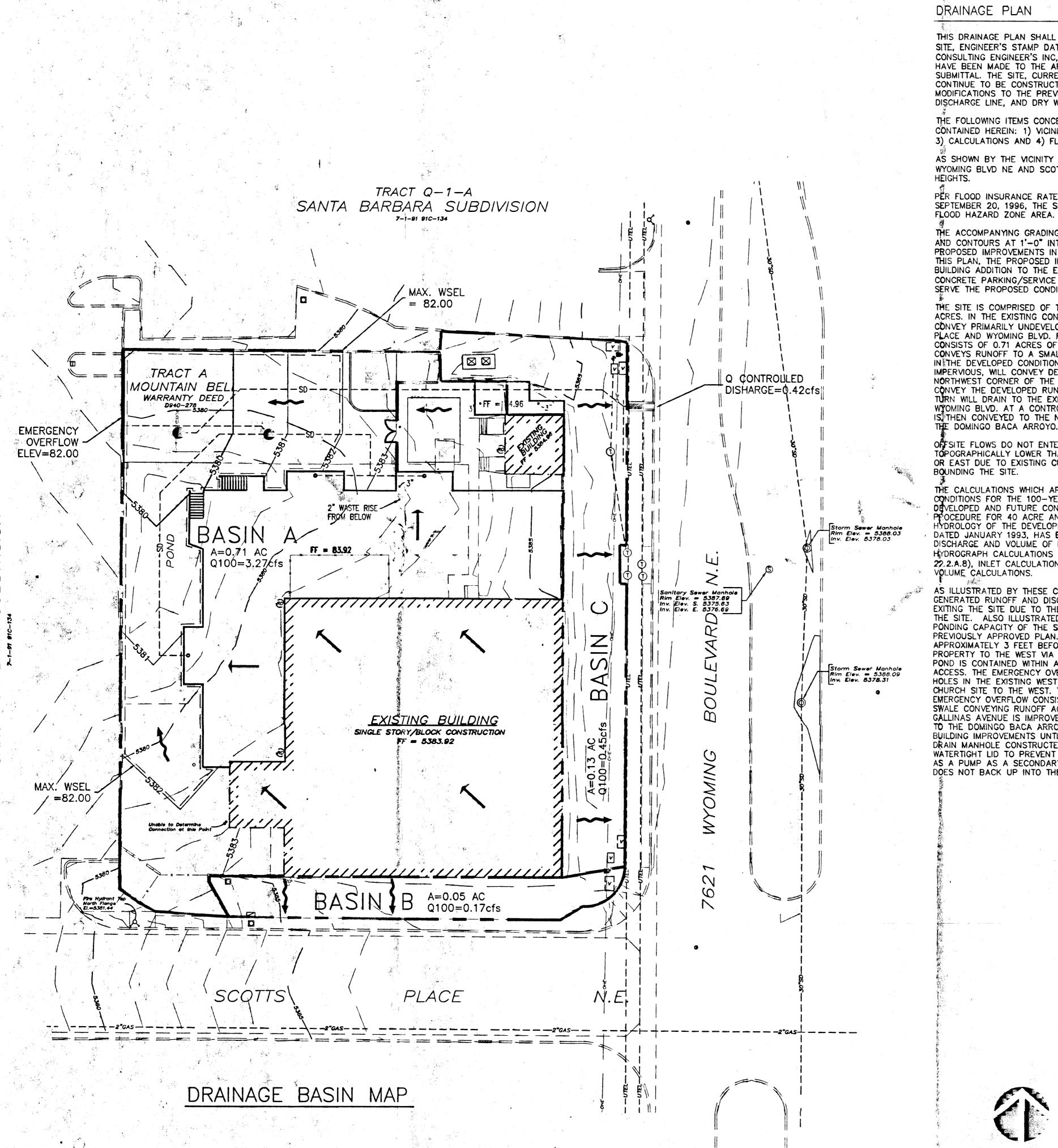


Manager – Real Estate Engineering Qwest
Business Resources - Real Estate
Albuquerque, New Mexico

SA202-AC-SKC03





DRAINAGE PLAN

THIS DRAINAGE PLAN SHALL SUPERCEDE THE APPROVED DRAINAGE PLAN FOR THIS SITE, ENGINEER'S STAMP DATE 06-08-01, PREPARED BY CHAVEZ AND GRIEVES CONSULTING ENGINEER'S INC, DRAINAGE FILE D-19-D4A. MINOR MODIFICATIONS HAVE BEEN MADE TO THE AFOREMENTIONED DRAINAGE PLAN NECESSITATING THIS SUBMITTAL. THE SITE, CURRENTLY UNDER CONSTRUCTION, HAS BEEN AND WILL CONTINUE TO BE CONSTRUCTED PER THE ORIGINAL PLANS WITH THE EXCEPTION OF MODIFICATIONS TO THE PREVIOUSLY PROPOSED STORM DRAIN INLET, PUMP DISCHARGE LINE, AND DRY WELL.

THE FOLLOWING ITEMS CONCERNING THE QWEST ACADEMY DRAINAGE PLAN ARE CONTAINED HEREIN: 1) VICINITY MAP; 2) GRADING PLAN (SHEET CO4 AS ATTACHED); 3) CALCULATIONS AND 4) FLOODPLAIN MAP.

AS SHOWN BY THE VICINITY MAP, THE SITE LIES AT THE NORTHWEST CORNER OF WYOMING BLVD NE AND SCOTT'S PLACE NE IN ALBUQUERQUE'S NORTHEAST

PER FLOOD INSURANCE RATE MAP 141 OF 825 FOR BERNALILLO COUNTY, DATED SEPTEMBER 20, 1996, THE SITE DOES NOT LIE WITHIN NOR ADJACENT TO A

THE ACCOMPANYING GRADING PLAN SHOWS EXISTING AND PROPOSED SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS, AS WELL AS THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS IN RELATION TO EXISTING CONDITIONS. AS SHOWN BY THIS PLAN, THE PROPOSED IMPROVEMENTS CONSIST OF THE CONSTRUCTION OF A BUILDING ADDITION TO THE EXISTING BUILDING SERVING THE SITE, AN ASPHALTIC CONCRETE PARKING/SERVICE AREA, AS WELL AS VARIOUS UTILITY IMPROVEMENTS TO SERVE THE PROPOSED CONDITION OF THE SITE.

THE SITE IS COMPRISED OF THREE SEPARATE DRAINAGE BASINS TOTALING 0.89 ACRES. IN THE EXISTING CONDITION, THE TWO SMALLER BASINS, B AND C, CONVEY PRIMARILY UNDEVELOPED STORM WATER RUNOFF DIRECTLY TO SCOTT'S PLACE AND WYOMING BLVD. RESPECTIVELY. THE LARGEST BASIN, BASIN A, WHICH CONSISTS OF 0.71 ACRES OF PERVIOUS AND IMPERVIOUS AREA, CURRENTLY CONVEYS RUNOFF TO A SMALL RETENTION POND AT THE WEST SIDE OF THE SITE IN THE DEVELOPED CONDITION, BASIN A, WHICH WILL BE 100 PERCENT IMPERVIOUS, WILL CONVEY DEVELOPED RUNOFF TO A STORM INLET AT THE NORTHWEST CORNER OF THE SITE. THIS INLET WILL CONTAIN A PUMP THAT WILL CONVEY THE DEVELOPED RUNOFF TO THE BACK OF A SIDEWALK CULVERT THAT IN TURN WILL DRAIN TO THE EXISTING CURB AND GUTTER ON THE WEST SIDE OF WYOMING BLVD. AT A CONTROLLED RATE OF 0.42 CFS. CURB AND GUTTER RUNOFF THEN CONVEYED TO THE NORTH VIA WYOMING BLVD., A SHORT DISTANCE TO E DOMINGO BACA ARROYO.

OFFSITE FLOWS DO NOT ENTER THE SITE FROM THE NORTH OR WEST, WHICH LIE TOPOGRAPHICALLY LOWER THAN THE SITE. FLOWS DO NOT ENTER FROM THE SOUTH OR EAST DUE TO EXISTING CURB AND GUTTER IN THE DEVELOPED ROADWAYS BOUNDING THE SITE.

THE CALCULATIONS WHICH APPEAR HEREIN ANALYZE THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT AS WELL AS THE DEVELOPED AND FUTURE CONDITIONS FOR THE 100-YEAR, 24-HOUR EVENT. THE POCEDURE FOR 40 ACRE AND SMALLER BASINS SET BY SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. OTHER CALCULATIONS INCLUDE HYDROGRAPH CALCULATIONS FOR A SMALL WATERSHED (COA DPM SECTION 22.2.4.8), INLET CALCULATIONS FOR THE STORM DRAIN INLET AS WELL AS POND VOLUME CALCULATIONS.

AS ILLUSTRATED BY THESE CALCULATIONS, THERE WILL BE A SLIGHT INCREASE IN GENERATED RUNOFF AND DISCHARGE RATE, AND A MODERATE INCREASE IN RUNOFF EXITING THE SITE DUE TO THE REMOVAL OF THE EXISTING RETENTION SITUATION ON THE SITE. ALSO ILLUSTRATED BY THE CALCULATIONS IS THE EXCESS DETENTION PONDING CAPACITY OF THE SITE WHICH IS SHOWN AS A MODIFICATION TO THE PREVIOUSLY APPROVED PLAN. STORM WATER WILL BE ALLOWED TO RISE APPROXIMATELY 3 FEET BEFORE EXITING THE SITE TO THE ADJACENT CHURCH PROPERTY TO THE WEST VIA AN EMERGENCY OVERFLOW. THE SUBJECT DETENTION POND IS CONTAINED WITHIN A FENCED COMPOUND WITH STRICTLY CONTROLLED ACCESS. THE EMERGENCY OVERFLOW WILL CONSIST OF 2-6 INCH CORE DRILLED HOLES IN THE EXISTING WEST SITE WALL SEPARATING THE SUBJECT SITE AND THE CHURCH SITE TO THE WEST. THE PORTION OF THE CHURCH SITE AFFECTED BY THE EMERGENCY OVERFLOW CONSISTS OF AN ASPHALT PARKING LOT WITH A PAVED SWALE CONVEYING RUNOFF ACROSS THE SITE, DISCHARGING TO GALLINAS AVENUE. GALLINAS AVENUE IS IMPROVED WITH STORM INLETS TO CONVEY DEVELOPED RUNOFF TO THE DOMINGO BACA ARROYO. STORM WATER WOULD NOT ENTER THE SUBJECT BUILDING IMPROVEMENTS UNTIL REACHING A DEPTH OF 4.9 FEET. THE STORM DRAIN MANHOLE CONSTRUCTED AS A DRY WELL WILL BE MODIFIED WITH A WATERTIGHT LID TO PREVENT DETAINED STORM WATER FROM ENTERING IT, AS WELL AS A PUMP AS A SECONDARY MEASURE TO INSURE WATER FROM THE DRY WELL DOES NOT BACK UP INTO THE BASEMENT OF THE PROPOSED BUILDING.

CALCULATION - SITE CHARACTERISTICS

1. Precipitation Zone = 3

2. P360 = 2.60 in. 3. Total Area = 0.89 ac.; 38,850 sf 4. Existing Conditions

Basin A 30,930/0.71 100 Land Treatment Area (sf/ac) % 8,280/0.19 27 22,650/0.52 73 Basin (2,180/0.05 100

2,180/0.05 100 5,740/0.13 100 Land Treatment Area (sf/ac) % 5,740/0.13 100

Land Treatment Area (sf/ac) %

5. Developed Conditions

Basin A 30,930/0.71 100 Land Treatment Area (sf/ac) % 30,930/0.71 100

2,180/0.05 100 Land Treatment Area (sf/ac) %

NO CHANGE

Basin C 5,740/0.13 100 Land Treatment Area (sf/ac) %

NO CHANGE

6. Existing Conditions Calculations

a. Basin A

1. Volume EW = [(EA*AA + EB*AB + EC*AC + ED*AD)]/(AT)EW = [(1.29*0.45)+(2.36*0.52)]/(0.71) = 2.07 in.

100 Year - 6 Hour Event V100 = (EW/12)*ATV100 = 2.07/12*0.71 = 0.1227 ac.ft; 5,340 cf

2. Runoff Q100 = QPA*AA+QPB*AB+QPC*AC+QPD*ADQ100 = 3.45*0.19+5.02*0.52 = 3.27 cfs

b. Basin B

EW = [(EA*AA+EB*AB+EC*AC+ED*AD)]/(AT)EW = [(1.29*0.05)]/(0.05) = 1.29 in.

100 Year - 6 Hour Event V100 = (EW/12)*AT

 $V100 = 1.29/12*0.05 \neq 0.0054$ ac.ft; 230 cf 2. Runoff

Q100 = QPA*AA+QPB*AB+QPC*AC+QPD*AD Q100 = 3.45*0.05 = 0.17 cfs

1. Volume

c. Basin C

EW = [(EA*AA+EB*AB+EC*AC+ED*AD)]/(AT)EW = [(1.29*0.13]/(0.13) = 1.29 in.

100 Year - 6 Hour Event V100 = (EW/12)*ATV100 = 1.29/12*0.13 = 0.0140 ac.ft; 610 cf

2. Runoff Q100 = QPA*AA+QPB*AB+QPC*AC+QPD*ADQ100 = 3.45*0.13 = 0.45 cfs

7. Developed Conditions Calculations

a. Basin A

1. Volume EW = [(EA*AA+EB*AB+EC*AC+ED*AD)]/(AT)EW = [(2.36*0.71/(0.71) = 2.36 in.

100 Year - 6 Hour Event V100 = (EW/12)*ATV100 = 2.36/12*0.71 = 0.1396 ac.ft; 6,080 cf BID SET

ISSUE #1

ISSUE #4

BUILDING ADDITION

CASE# 0158284 DATE: 5-10-01

CHANGE ORDER

CASE# 0158284

DATE: 10-31-02

BUILDING ADDITION

TS AF C-G

TS JP BPLW

2. Runoff Q100 = QPA*AA+QPB*AB+QPC*AC+QPD*ADQ100 = 5.02*0.71 = 3.56 cfs

3. Inlet Capacity

 $Q=CA(2gh)^1/2$

A = 1.5 SF $g = 32.2 \text{ Ft/S}^2$ C = 0.6h = 3.0Q = 12.5 cfs

Therefore: Pump Discharge Governs (0.42 cfs)

4. Hydrograph Calculations (Using Hydrograph for small watershed COA DPM Section 22.2.A.8)

Q RELEASE = 0.42 cfs (Pump Discharge) = 2.36 in.

QP = 3.56 cfs $\overrightarrow{AT} = 0.71$ ac. AD = 0.71 ac. tC = 0.2 hr.

tP = 0.19 hr.tb = 0.74171 hr.tpk = 0.25 hr.

Pond Volume Required = 5280 CF

Time to Drain Pond = 5280cf/0.42cfs= 12,571s = 3.5hr.

5. Pond Volume

Elev. Area(SF) Volume(CF) Sum Volume (CF)

____5590

9915 cf >>> 5280 cf a. Basin B

1. Volume NO CHANGE

2. Runoff NO CHANGE

b. Basin C

1. Volume NO CHANGE

2. Runoff NO CHANGE

8. Comparison

a. Existing ~vs~ Developed (Overall Site)

1. Change in Volume for 100 year 6 hr. storm DV = 6,920 - 6,180 = 740 cf (increase)

2. Change in Runoff Rate DQ = 4.18 - 3.89 = 0.29 cfs (increase)

3. Change in Runoff Exiting the Site

DQ = (Dvlpd. 'A', 'B' & 'C') - (Exist. 'B' & 'C')

= 4.18 - 0.62 = 3.56 cfs (increase)

6200 UPTOWN BOULEVARD NE - SUITE 400 ALBUQUERQUE, NEW MEXICO 87110 (505) 881-2759 CIVIL Albuquerque, NM Academy UBuilding Addition NOV 01 2002 DRAINAGE PLAN AND CALCULATIONS Manager - Real Estate Engineering Qwest
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DRAINAGE PLAN AND CALCULATIONS

