

June 24, 2008

Scott McGee, P.E.

Isaacson & Arfman, P.A.

128 Monroe Street N.E.

Albuquerque, NM 87108

Re: Albuquerque Retirement Residence, 8301 Palomas NE,

(D-19/D025)

Approval of Permanent Certificate of Occupancy,

Engineer's Stamp Date 4/10/2007

Certification dated: 6/24/08

P.O. Box 1293

Mr. McGee:

Albuquerque

Based upon the information provided in your submittal received 6/24/08, the above referenced certification is approved for release of Permanent Certificate of Occupancy by Hydrology.

If you have any questions, you can contact me at 924-3982.

New Mexico 87103

Sincerely,

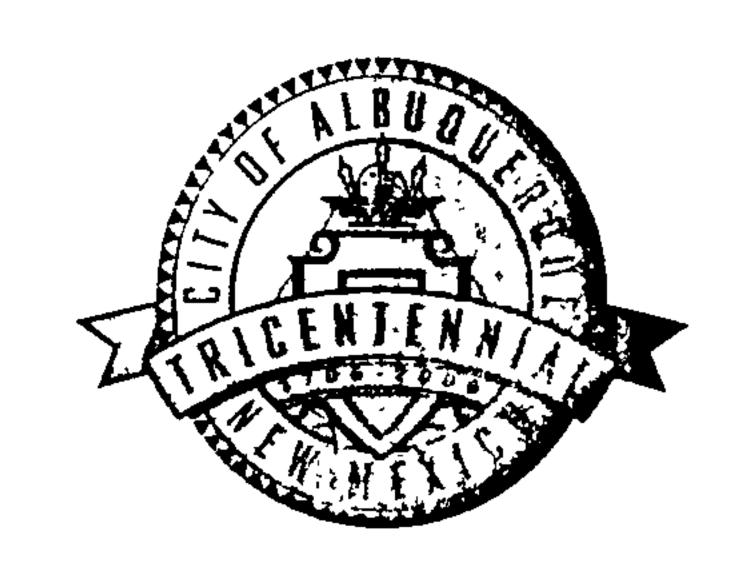
www.cabq.gov

I imothy E. Sims

Plan Checker-Hydrology, Planning Dept Development and Building Services

C: CO Clerk—Katrina Sigala

file



April 27, 2007

Scott M. McGee, P.E. Isaacson & Arfman, P.A. 128 Monroe St. NE Albuquerque, NM 87108

Re: Albuquerque Retirement Residence Grading and Drainage Plan Engineer's Stamp dated 4-10-07 (D19/D25)

Dear Mr. McGee,

Based upon the information provided in your submittal dated 4-12-07, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

P.O. Box 1293

This project requires a National Pollutant Discharge Elimination System (NPDES) permit.

Albuquerque

Prior to Certificate of Occupancy release:

New Mexico 87103

- The slope on the east side of the property that has a slope greater than 3:1 requires an engineer specified means of erosion control. Indicate method used upon certification.
- Engineer Certification per the DPM checklist will be required.

www.cabq.gov

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne, E.I.

Engineering Associate, Planning Dept. Development and Building Services

C: file

December 22, 2006

Scott M. McGee, PE Isaacson & Arfman, PA 128 Monroe St. NE. Albuquerque, NM 87108



Re: Albuquerque Retirement Residence, Lots 19-22 Block 21 Tract A

Grading and Drainage Plan

Engineer's Stamp dated 12-13-06 (D19/D25)

Dear Mr. McGee,

Based upon the information provided in your submittal received 12-13-06, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology. Also, prior to Certificate of Occupancy release, Engineer Certification of the grading plan per the DPM checklist will be required.

P.O. Box 1293

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. If you have any questions regarding this permit please feel free to call the DMD Storm Drainage Design section at 768-3654 (Sertil Kanbar).

Albuquerque

If you have any questions, you can contact me at 924-3977.

New Mexico 87103

www.cabq.gov

Rudy E. Rael, Associate Engineer

Planning Department.

Sincerely

Building and Development Services

C: Sertil Kanbar CC: file

DECEMBER 11, 2006

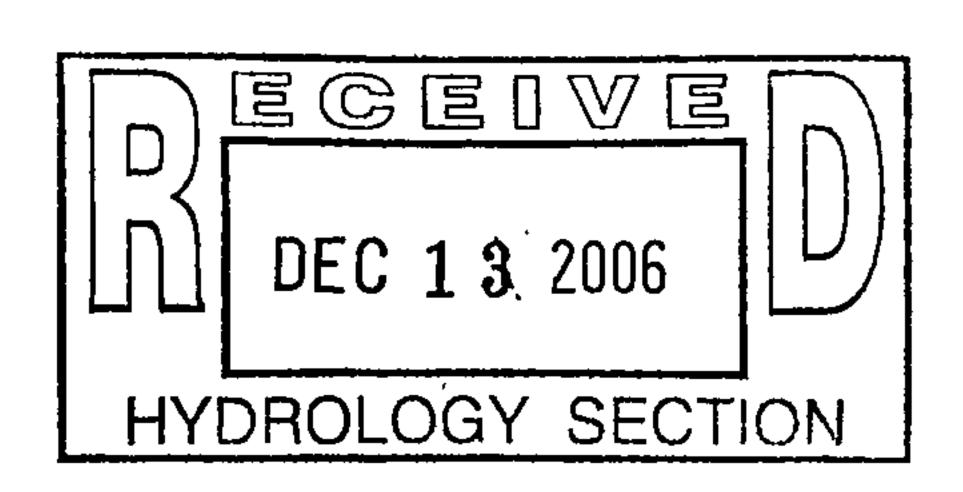
SUPPLEMENTAL INFORMATION

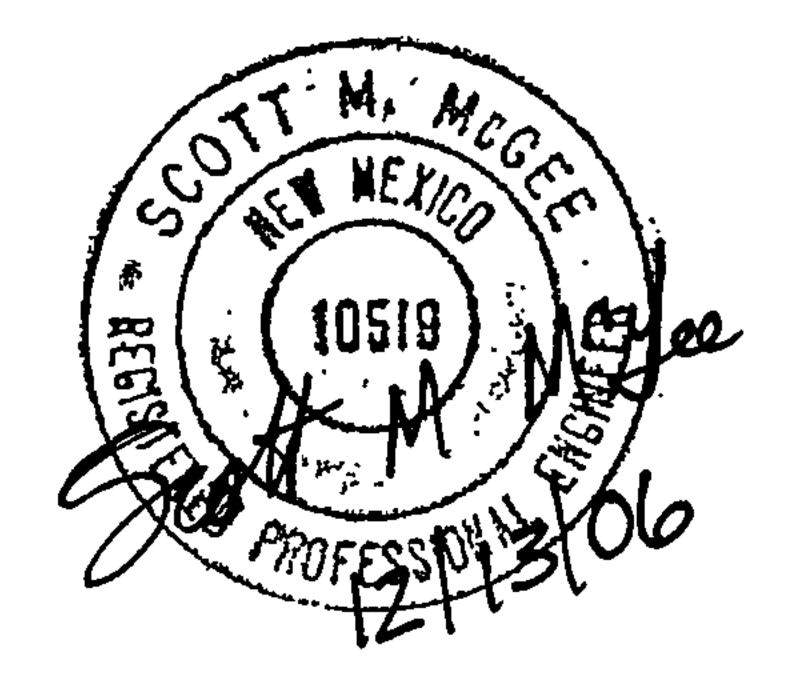
FOR

Albuquerque Retirement Residence

By

ISAACSON AND ARFMAN, PA 128 MONROE ST. N.E. ALBUQUERQUE, NM 87108 Project No. 1499





ALBUQUERQUE RETIREMENT RESIDENCE DRAINAGE SUMMARY

December 6, 2006

I&A Project No.

1499

Architect:

Curry Brandaw Architects

Legal:

₹⇒

Lots 19-22, Block 21, Tract A, Unit A, North Albuquerque Acres

Zone Map:

D-19

Flood Zone:

Zone X (Outside 500 Year Flood Zone)

Existing Conditions:

The Property is an undeveloped commercial property located in Albuquerque's Northeast Heights. Palomas Ave. NE borders the property to the south, undeveloped property and the Paseo Del Norte R.O.W. to the north, and undeveloped commercial properties to the east and west. The undeveloped site is covered with native vegetation and naturally slopes at 5% from east to west to discharge approximately 6.6 cfs (100% Treatment A) to the Paseo Del Norte R.O.W.

Proposed Conditions:

The proposed construction will fully develop the site with a 118 unit retirement residence, paved access / parking and associated landscaping. Per the calculations, the developed runoff of 15.1 cfs will be released to Palomas Avenue NE. A 'V' shaped bar ditch will be constructed within the north half of the Palomas R.O.W. (by C.O.A. Work Order) from the west 825 ft to the commercial development currently being built at the SE corner of Wyoming Blvd. and Paseo Del Norte NE. Runoff will then be accepted and routed through that site to public storm drain facilities.

ALBUQUERQUE RETIREMENT RESIDENCE DRAINAGE SUMMARY

December 6, 2006

REPORT:

Drainage Summary	PAGE 1
Vicinity Map D-19	
Existing Drainage Patterns	
Floodzone FIRMette Map	
SUPPORTING EXHIBITS and CALCULATIONS:	
Historic / Developed Discharge – Overall Calculations	5
Drainage Sub-Basin Map	6
Sub-Basin Calculations	
Sub-Basin Discharge Summary Table	11
PRODUCT INFORMATION:	
ADS 2'x2' Inlet Grate Capacity and Details	
ADS Inline Drain with Domed Grate Capacity and Details	
12" PVC Pipe Capacity	
18" PVC Pipe Capacity	

	17 1 - 7 11 11 - 7 11 1	CALC	ULA	ΓΙΟΝ	S: Albuquerque Re	tireme	nt Cent	er : Nov	. 7, 20	06	·
Based on Draina	ge Des				Albuquerque Section 2						
					ON-SIT						
AREA OF SITE:					154420	SF	· • • • • • • • • • • • • • • • • • • •	=	3.54	Ac.	
HISTORIC FLO	ows:				DEVELOPED FLO)WS:				EXCESS PRE	CCIP:
On-Site Histo	oric La	nd Conditio	n		On-Site Devel	loped L	and Cor	idition		Precip. Zone	3
Area a	=	15	4420	SF	Area a	=		0	SF	Ea =	0.66
Area b	=		0	SF	Area b	=		38605	SF	Eb =	0.92
Area c	=		0	SF	Area c	=		15442	SF	Ec =	1.29
Area d	=		0	SF	Area d	=		100373	SF	Ed = 1	2.36
Total Area	=	15	4420	SF	Total Area	=		154420	SF		
Historic E		Weighted	•		ar, 6-Hour Storm) EaAa + EbAb + EcA Aa + Ab + Ac	+ Ad	l <u>Ad</u>			1416 - 14	1
THSONIC E			0.00 1	III.	Developed E		<u> </u>	1.89	in.	4.770	Down
On-Site Volume	of Run	off: V360 =	=		E*A / 12						
Historic V360			8493	CF	Developed V360	=		24360	CF		
On-Site Peak Dis For Precipitation Qpa Qbb			= QpaA	a+Qp	bAb+QpcAc+QpdAd Qpc Qpd	l / 43,5 =	60 3.45 5.02				
Historic Qp	=	2.00	6.6	CES	Developed Qp		J.UZ	15.1	CFS		
TIDIOTIO QP				CLO	Preserved Ab			13.1	CLO		

1499 DPM Calculations - 100 yr 6 hr1.xls

BASIN NO. 1		DESCRIPTION	Drains to I	NLET 1
Area of basin flows =	20889	SF =	0.5 Ac.	
The following calculation	ons are based on Tr	eatment areas as shown in table to	the right	
		ted Excess Precipitation (see form	_	
	Weighted E		TREATI	MENT
	Sub-basin Volum	e of Runoff (see formula above)		0%
	V360	= 3234 C		20%
	Sub-basin Peak I	ischarge Rate: (see formula above	<u>C</u> =	20%
	Qp	= 2.0 cf	D =	60%
BASIN NO. 2	, , , , , , , , , , , , , , , , , , ,	DESCRIPTION	Drains to II	VLET 2
Area of basin flows =	689	SF =	0.0 Ac.	
The following calculation	ns are based on Tr	eatment areas as shown in table to	the right	
		ted Excess Precipitation (see form		
	Weighted E		TREATI	MENT
		e of Runoff (see formula above)		0%
	V360	= 94 C		50%
	Sub-basin Peak D	ischarge Rate: (see formula above		0%
	Qp	= 0.1 cf		50%
BASIN NO. 3		DESCRIPTION	Drains to It	VLET 3
Area of basin flows =	950	SF =		
The following calculation	ns are based on Tr	eatment areas as shown in table to		
		ted Excess Precipitation (see form	_	
	Weighted E	= 1.64 in.	TREATI	MENT
	Sub-basin Volum	e of Runoff (see formula above)		0%
	V360	= 130 Cl	$\mathbf{B} =$	50%
	Sub-basin Peak D	ischarge Rate: (see formula above	C =	0%
	Qp	= 0.1 cf	D =	50%
BASIN NO. 4		DESCRIPTION	Drains to IN	ILET 4
Area of basin flows =	755	SF =	0.0 Ac.	
The following calculatio	ns are based on Tr	eatment areas as shown in table to		
		ed Excess Precipitation (see form	——————————————————————————————————————	
	Weighted E	= 1.64 in.	TREATN	IENT
	Sub-basin Volum	e of Runoff (see formula above)	A =	0%
	V360	= 103 CF		50%
	Sub-basin Peak D	ischarge Rate: (see formula above	C =	0%
	Qp	= 0.1 cfs	D =	50%
BASIN NO. 5		DESCRIPTION	Drains to IN	ILET 5
Area of basin flows =	2638	SF =	0.1 Ac.	
The following calculation	ns are based on Tro	eatment areas as shown in table to	the right	
	Sub-basin Weight	ed Excess Precipitation (see formi	ıla above)	
	Weighted E	= 2.14 in.	TREATM	1ENT
	Sub-basin Volume	e of Runoff (see formula above)	A =	0%
	V360	= 471 CF		15%
	Sub-basin Peak D	ischarge Rate: (see formula above	<u>C</u> =	0%
	Qp	= 0.3 cfs	D =	85%

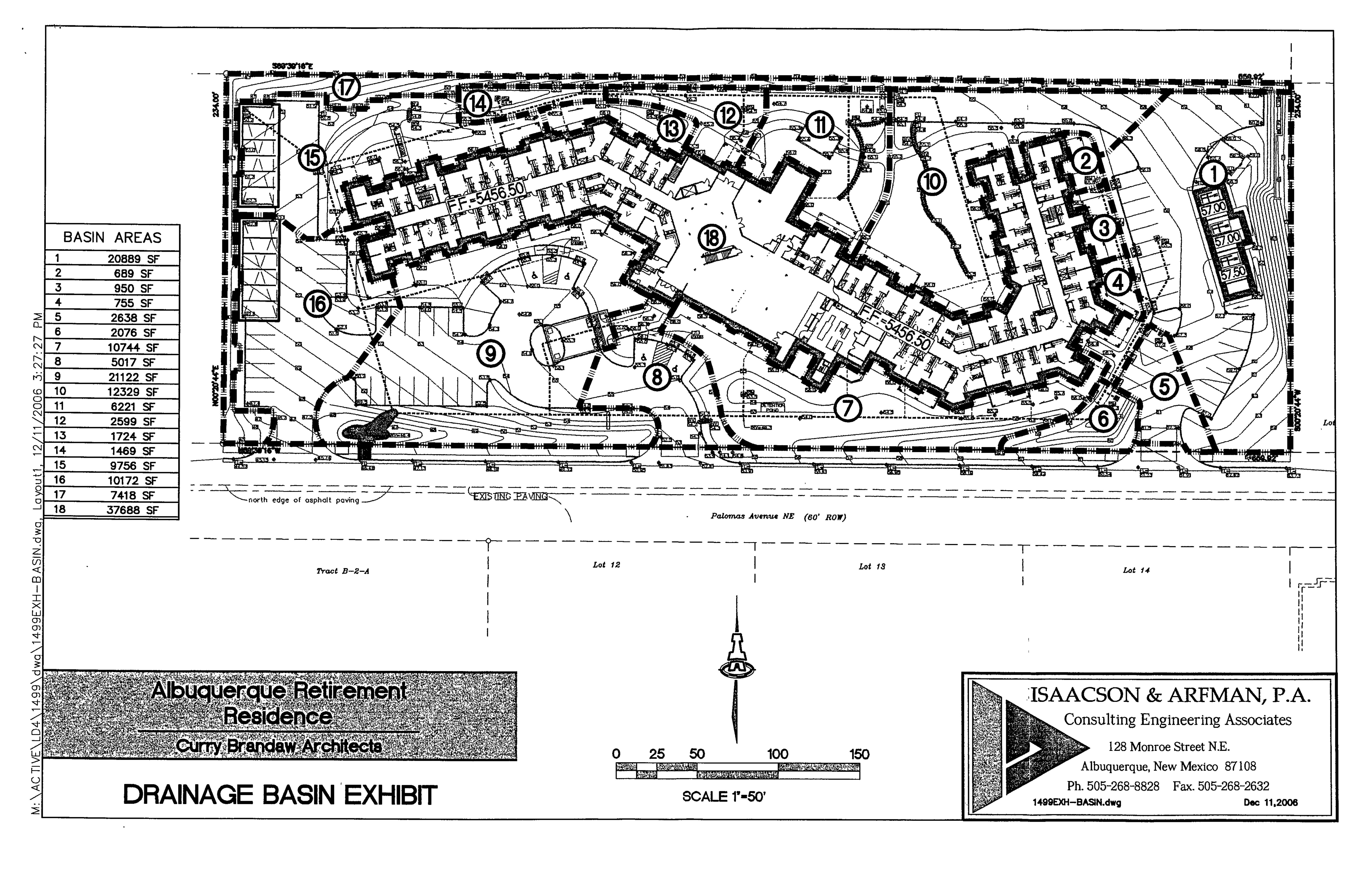
1499 DPM Calculations - 100 yr 6 hr1.xls

BASIN NO. 6	· · · · · · · · · · · · · · · · · · ·	DESCRIPTION		Drains to I	NLET 6	
Area of basin flows =	2076	SF	=	0.0 Ac.		
The following calculation	ns are based on Tr	eatment areas as shown in tab	le to the right			
	Sub-basin Weigh	ted Excess Precipitation (see f	formula above)			
	Weighted E			TREATI	IENT	
	Sub-basin Volum	e of Runoff (see formula above	/e)	A ==	0%	
	V360	= 247	CF	B =	20%	
	Sub-basin Peak I	Discharge Rate: (see formula a			60%	
	Qp	· · · · · · · · · · · · · · · · · · ·		D =	20%	
BASIN NO. 7		DESCRIPTION		Drains to IN		
Area of basin flows =	10744	SF	=	0.2 Ac.		
The following calculation	ns are based on Tr	eatment areas as shown in tab	le to the right			
		ted Excess Precipitation (see f	_			
	Weighted E			TREATN	MENT	
	Sub-basin Volum	e of Runoff (see formula above	· · · · · · · · · · · · · · · · · · ·	A ==	0%	
	V360	= 1067	CF	B =	70%	
	Sub-basin Peak I	ischarge Rate: (see formula al	bove)	C =	15%	
	Qp	= 0.8	cfs	D =	15%	
BASIN NO. 8		DESCRIPTION	Dı	rains to Palomas v	ia Access Drive	
Area of basin flows =	5017	SF	==	0.1 Ac.		
The following calculation	ns are based on Tr	eatment areas as shown in tab	le to the right			
		ted Excess Precipitation (see f	•			
	Weighted E		······································	TREATN	IENT	
	Sub-basin Volum	e of Runoff (see formula above		A =	0%	
	V360	= 896	CF	B =	15%	
	Sub-basin Peak D	ischarge Rate: (see formula al	oove)	C =	0%	
	Qp	= 0.5	cfs	D =	85%	
BASIN NO. 9		DESCRIPTION		Drains to Lan	dscaping	
Area of basin flows =	21122	SF	=	0.5 Ac.		
The following calculation	ns are based on Tr	eatment areas as shown in tab	le to the right			
		ted Excess Precipitation (see f	_			
	Weighted E	= 1.82		TREATM	1ENT	
	Sub-basin Volum	e of Runoff (see formula abov	re)	A =	0%	
	V360	= 3205	CF	B =	30%	
	Sub-basin Peak D	ischarge Rate: (see formula al	ove)	C =	10%	
	Qp	= 2.0	cfs	D =	60%	
BASIN NO. 10		DESCRIPTION		Drains to IN	LET 8	
Area of basin flows =	12329	SF	=	0.3 Ac.		
The following calculation	ns are based on Tr	eatment areas as shown in tabl	e to the right			
	Sub-basin Weight	ed Excess Precipitation (see for	ormula above)	<u></u>		
	Weighted E	= 1.68 i	n.	TREATM	ENT	
	Sub-basin Volum	e of Runoff (see formula abov	e)	A =	0%	
	V360	= 1723	CF	$\mathbf{B} =$	40%	
	Sub-basin Peak D	ischarge Rate: (see formula ab	ove)	C =	10% 50%	

BASIN NO. 11		DESCRIPTION	Drains to	INLET 9
Area of basin flows =	6221	SF =	0.1 Ac.	
The following calculation	ns are based on Tr	eatment areas as shown in table to	the right	
	Sub-basin Weigh	ted Excess Precipitation (see formu	ıla above)	
	Weighted E	= 1.53 in.	TREAT	MENT
	Sub-basin Volum	e of Runoff (see formula above)	A =	0%
	V360	= 795 CF	B =	50%
	Sub-basin Peak I	ischarge Rate: (see formula above	C ==	10%
	Qp	= 0.5 cfs	D =	40%
BASIN NO. 12		DESCRIPTION	Drains to I	NLET 10
Area of basin flows =	2599	SF =	0.1 Ac.	
The following calculation	ns are based on Tr	eatment areas as shown in table to	the right	
	Sub-basin Weigh	ted Excess Precipitation (see formu	ıla above)	
	Weighted E	= 1.68 in.	TREAT	MENT
	Sub-basin Volum	e of Runoff (see formula above)		0%
	V360	= 363 CF		40%
	Sub-basin Peak D	ischarge Rate: (see formula above)	<u>C</u> =	10%
	Qp	= 0.2 cfs	D =	50%
BASIN NO. 13		DESCRIPTION	Drains to I	NLET 11
Area of basin flows =	1724	SF =	0.0 Ac.	
The following calculation	ns are based on Tr	eatment areas as shown in table to	the right	
	Sub-basin Weight	ted Excess Precipitation (see formu	ıla above)	
	Weighted E	= 1.39 in.	TREAT	MENT
	Sub-basin Volum	e of Runoff (see formula above)	A =	0%
	V360	= 200 CF	B =	60%
	Sub-basin Peak D	ischarge Rate: (see formula above)	C =	10%
	Qp	= 0.1 cfs	D =	30%
BASIN NO. 14		DESCRIPTION	Drains to II	NLET 12
Area of basin flows =	1469	SF =	0.0 Ac.	
The following calculation	ns are based on Tr	eatment areas as shown in table to	the right	
-	Sub-basin Weight	ed Excess Precipitation (see formu	la above)	
	Weighted E	= 1.68 in.	TREAT	MENT
	Sub-basin Volum	e of Runoff (see formula above)	A =	0%
	V360	= 205 CF	\Box	40%
-	Sub-basin Peak D	ischarge Rate: (see formula above)		10%
	Qp	= 0.1 cfs	D =	50%
BASIN NO. 15		DESCRIPTION	Drains to IN	VLET 13
Area of basin flows =	9756	SF =	0.2 Ac.	
The following calculation	ns are based on Tro	eatment areas as shown in table to t		
	Sub-basin Weight	ed Excess Precipitation (see formu	la above)	
	Weighted E	= 1.89 in.	TREAT	MENT
	Sub-basin Volume	e of Runoff (see formula above)		0%
	V360	= 1539 CF	B =	25%
	Sub-basin Peak D	ischarge Rate: (see formula above)		10%
	Qp	= 1.0 cfs	<u></u>	65%
•			7	

1499 DPM Calculations - 100 yr 6 hr1.xls

BASIN NO. 16		DESCI	RIPTION		Drains to Palomas	via Access D	rive
Area of basin flows =	10173	SF		=	0.2 Ac.		
The following calculation	ns are based on Tr	eatment areas as s	shown in table	e to the	e right		
	Sub-basin Weigh	ted Excess Precip	itation (see fo	rmula	above)		
	Weighted E	===	2.22 ir	1.	TREAT	MENT	
	Sub-basin Volum	e of Runoff (see f	formula above	*)	A =	0%	
	V360		1879	CF	B =	10%	
	Sub-basin Peak D	ischarge Rate: (se	ee formula abo	ove)	C =	0%	
	Qp	=	1.1	cfs	D =	90%	
BASIN NO. 17		DESCI	RIPTION		Perimeter Land	iscape Basin	
Area of basin flows =	7418			=	0.2 Ac.		
The following calculation	ns are based on Tr	eatment areas as s	shown in table	to the	e right		
	Sub-basin Weight	ted Excess Precipi	itation (see fo	rmula	above)		
	Weighted E	=	1.11 ir	1.	TREAT	MENT	
	Sub-basin Volum	e of Runoff (see f	ormula above	;)	A =	0%	
	V360	=	683	CF	B =	50%	
	Sub-basin Peak D	ischarge Rate: (se	e formula abo	ove)	C =	50%	
	Qp	=	0.5	cfs	D =	0%	
BASIN NO. 18		DESCR	RIPTION	•	Building Roof Drainage	to Private Sto	orm Drain
Area of basin flows =	37688	SF			0.9 Ac.		
The following calculation	ns are based on Tr	eatment areas as s	hown in table	to the	right		
	Sub-basin Weighted Excess Precipitation (see formula above)						
	Weighted E	=	2.36 in	1.	TREAT	MENT	
	Sub-basin Volum	e of Runoff (see f	ormula above)	A =	0%	
	V360	=	7412	CF	B =	0%	
	Sub-basin Peak D	ischarge Rate: (se	e formula abo	ove)	C =	0%	
	Qp	=	4.3	cfs	D =	100%	



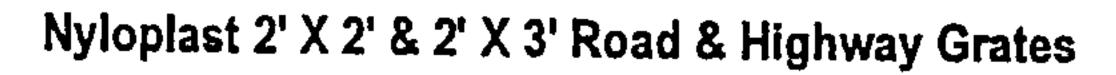
			SUMMA	ARY			
Basin No.	Description		DISCHA	RGE	LOCATION	GRATE	CAPACITY
1	Drains to INLET 1	==	2.0	cfs	Pavement	2'x2' Road Inlet	6.0 cfs
2	Drains to INLET 2	=	0.1	cfs	Landscape	8" Dome	0.8 cfs
3	Drains to INLET 3	=	0.1	cfs	Landscape	8" Dome	0.8 cfs
4	Drains to INLET 4	=	0.1	cfs	Landscape	8" Dome	0.8 cfs
5	Drains to INLET 5	=	0.3	cfs	Pavement	2'x2' Road Inlet	6.0 cfs
6	Drains to INLET 6	=	0.2	cfs	Landscape	12" Dome	1.4 cfs
7	Drains to INLET 7	=	0.8	cfs	Landscape	18" Dome	4.0 cfs
8	Drains to Palomas via Access Drive	=	(0.5)	cfs	-		
9	Drains to Landscaping	=	2.0	cfs			
10	Drains to INLET 8	=	1.1	cfs	Pavement	2'x2' Road Inlet	6.0 cfs
11	Drains to INLET 9	==	0.5	cfs	Landscape	12" Dome	1.4 cfs
12	Drains to INLET 10	=	0.2	cfs	Landscape	8" Dome	0.8 cfs
13	Drains to INLET 11	=	0.1	cfs	Landscape	8" Dome	0.8 cfs
14	Drains to INLET 12	=	0.1	cfs	Landscape	8" Dome	0.8 cfs
15	Drains to INLET 13		1.0	cfs	Pavement	2'x2' Road Inlet	6.0 cfs
16	Drains to Palomas via Access Drive	=		cfs			
17	Perimeter Landscape Basin	=		cfs			
18	Building Roof Drainage to Private Storm I	=		cfs			
	ISCHARGE	=	1.5	cfs		······································	

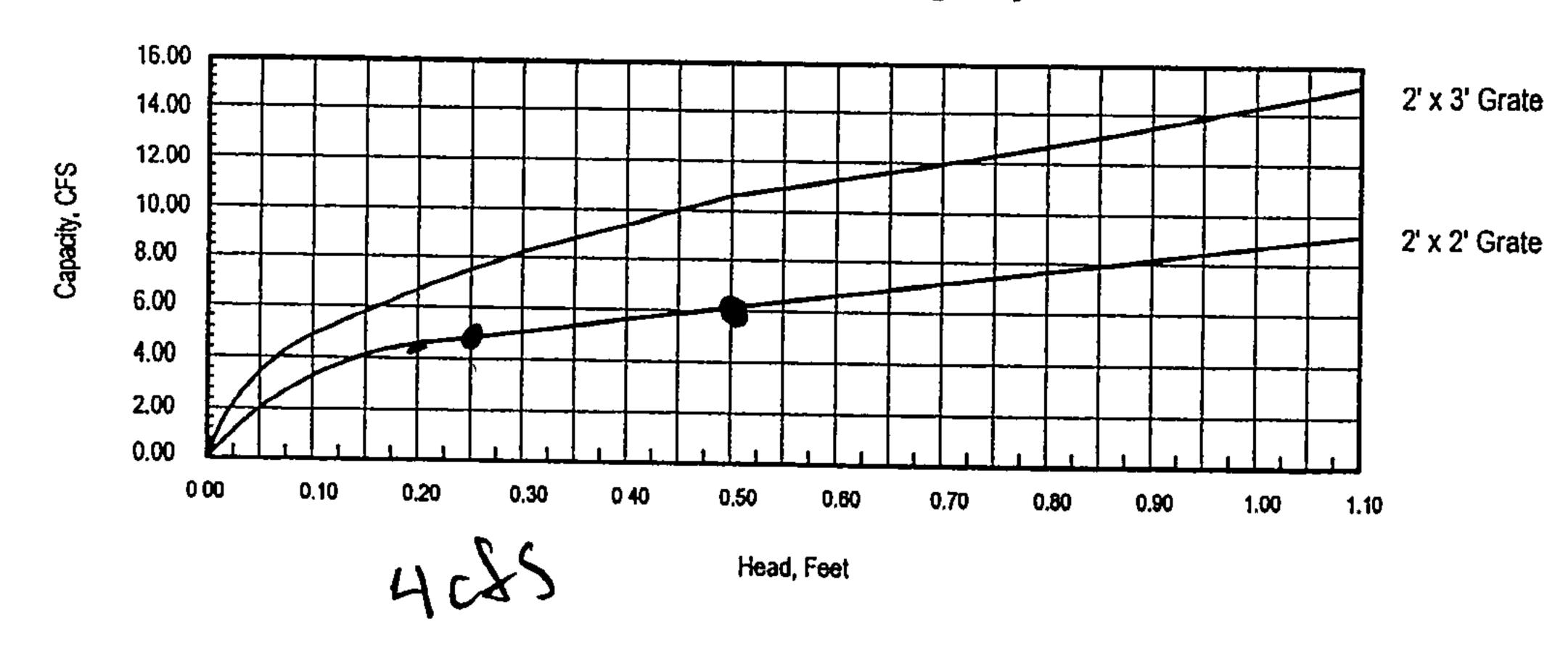
Nyloplast Road & Highway Inlet Capacity Chart

This chart is based on equations from the FAA Airport Drainage AC 150/5320-5B, 1970, Page 35. Certain assumptions have been made and no two installations will necessarily perform the same way. Safety factors should change with site conditions such that a safety factor 1.25 should be used for an inlet in pavement, and a safety factor of 2.0 should be used in turf areas.

Structure Outlet Pipe Size	Flow Rate CFS *
4"	0.229
6"	0.662
8-	1.441
10*	2.612
12"	4.152
15"	7.126
18"	12.163
24*	25.821

^{*} Maximum flow capacity before road & highway grate begins to backfill. Calculation based on an average pipe slope of 1%.



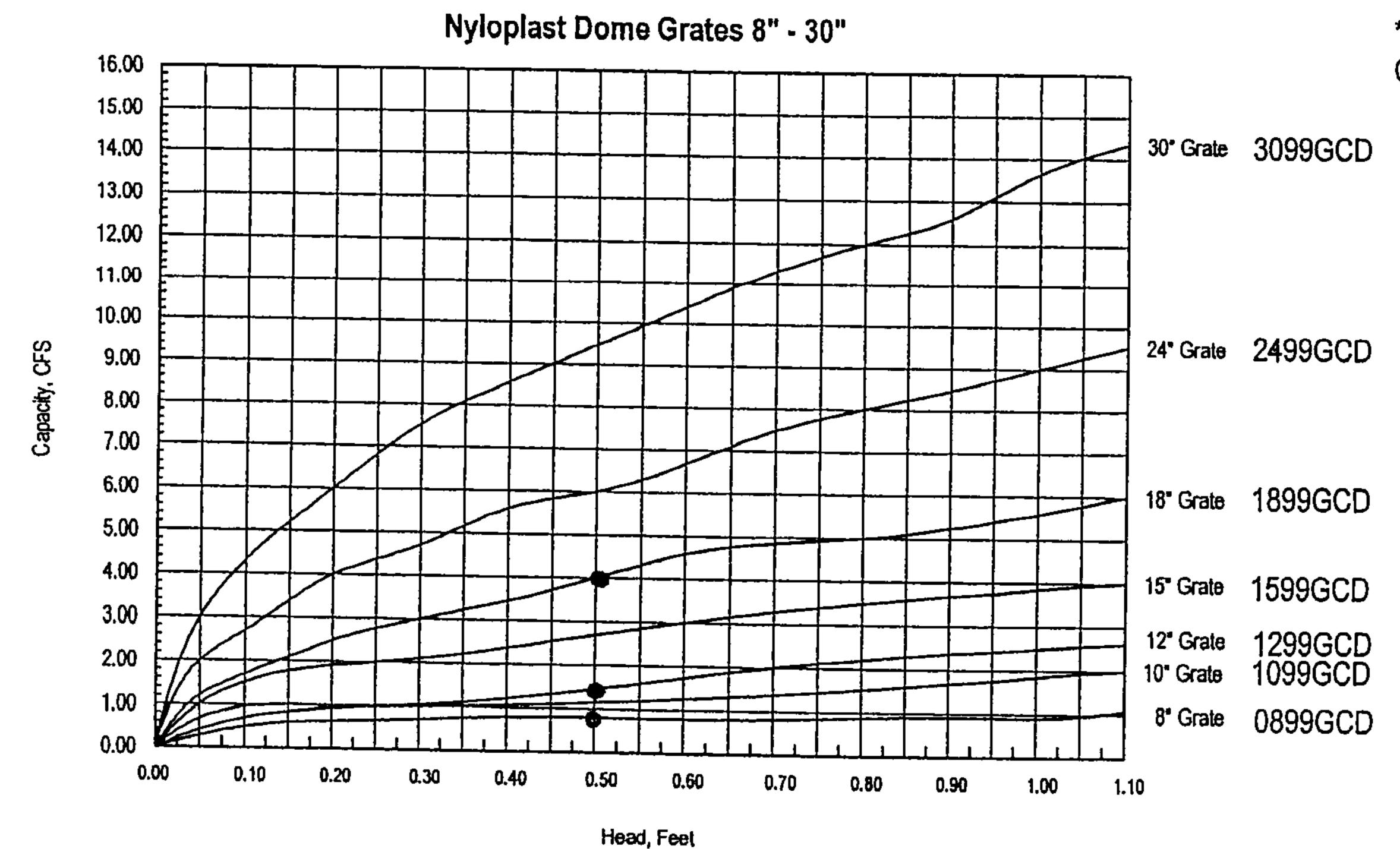


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PERMISSION FROM NYLOPLAST.	DWG SIZE A	SCALE 1:2 SHEET 1 OF 1	DWG NO. 7001-110-084 REV A

Nyloplast Dome Grate Inlet Capacity Chart

This chart is based on equations from the FAA Airport Drainage AC 150/5320-5B, 1970, Page 35. Certain assumptions have been made and no two installations will necessarily perform the same way. Safety factors should change with site conditions such that a safety factor 1.25 should be used for an inlet in pavement, and a safety factor of 2.0 should be used in turf areas.



Basin Outlet Pipe Size	Flow Rate CFS *
4"	0.229
6⁴	0.662
8*	1.441
10*	2.612
12"	4.152
15*	7.126
18"	12.163
24"	25.821
30"	52,173

* Maximum flow capacity before drain basin begins to backfill. Calculation based on an average pipe slope of 1%.

8" Done 12" Done 12" Done Inlets #6, 9

3130 VERONA AVE

BUFORD, GA 30518

PHN (770) 932-2443

8" - 30" DOME INLET CAPACITY

7001-110-000

FAX (770) 932-2490 www.nylopiast-us.com

REV

18" Dome

1~1ex# 7

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tmp#1.txt

12 ° 0.5% slæ

Manning Pipe Calculator

Given Input Data: Shape Solving for Diameter Depth Slope Manning's n	Flowrate 12.0000 in 12.0000 in
Computed Results: Flowrate	0.7854 ft2 0.7854 ft2 37.6991 in 37.6991 in 4.6333 fps 3.0000 in 100.0000 % 3.6390 cfs

tmp#1.txt

Manning Pipe Calculator

Given Input Data: Shape Solving for Diameter Depth Slope Manning's n	Flowrate 12.0000 in 12.0000 in 0.0100 ft/ft	12"4	1% Slope
Computed Results: Flowrate	0.7854 ft2 0.7854 ft2 37.6991 in 37.6991 in 6.5524 fps 3.0000 in 100.0000 % 5.1463 cfs		

tmp#1.txt

Manning Pipe Calculator

Given Input Data: Shape Solving for Diameter Depth Slope Manning's n	15.0000 in 15.0000 in 0.0100 ft/ft	15"00 1%
Computed Results: Flowrate Area Wetted Area Wetted Perimeter Perimeter Velocity Hydraulic Radius Percent Full Full flow Flowrate Full flow velocity	1.2272 ft2 1.2272 ft2 47.1239 in 47.1239 in 7.6034 fps 3.7500 in 100.0000 % 9.3308 cfs	



Planning Department Transportation Development Services Section

July 8, 2008

Scott M. McGee, P.E. 128 Monroe St. NE Albuquerque, NM 87108

Re: Certification Submittal for Final Building Certificate of Occupancy for

Albuquerque Retirement Residence, [D-19 / D025]

1500 Roma NW

Engineer's Stamp Dated 06/26/08

Dear Mr. McGee:

PO Box 1293

The TCL / Letter of Certification submitted on July 7, 2008 is sufficient for acceptance by this office for final Certificate of Occupancy (C.O.). Notification has been made to the Building and Safety Section.

Albuquerque

Sincerely

NM 87103

www.cabq.gov

Nilo E. Salgado-Fernandez, P.E.

Senior Traffic Engineer

Development and Building Services

Planning Department

c: Engineer

Hydrology file CO Clerk

ISAACSON & ARFMAN, P.A.

Consulting Engineering Associates

Thomas O. Isaacson, PE & LS • Fred C. Arfman, PE Scott M. McGee, PE

June 26, 2008

Mr. Nilo Salgado-Fernandez, PE
Senior Engineer
Transportation Development Coordination
Planning Department
City of Albuquerque
P.O. Box 1293
Albuquerque, NM 87103

RE: Albuquerque Retirement Residence

(DRB No. 1004564 - 8301 Palomas Ave. NE)

Site Plan Certification

Dear Mr. Salgado-Fernandez:

I, Scott M. McGee, NMPE No. 10519, of the firm Isaacson & Arfman, P.A., hereby certify that this project has been constructed and will function in substantial compliance with and in accordance with the design intent of the approved Site Plan (Administrative Amendment Approval dated 04/04/07). Minor parking space variances have been noted on the attached Site Plan. The design firm (previously Curry Brandaw Architects) has provided a letter of authorization for Isaacson & Arfman, P.A. to prepare this certification as they are located out of state in Oregon.

10519

Sincerely,

ISAACSON & ARFMAN, P.A.

Scott M. McGee, PE

SMM/rtl

pc: Colson & Colson (via FAX)

Curry Architects (via FAX)

RECEIVED

JUN 272008

HYDROLOGY SECTION