

NORTH CAMINO EXISTING COND. 10-YEAR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = nc10b.ext

RUN DATE (MON/DAY/YR) =11/05/1998

USER NO.= RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											TIME= .00
RAINFALL TYPE= 2											RAIN24= 2.660
COMPUTE NM HYD	300.00	-	1	.88400	509.61	34.987	.74209	1.800	.901	PER IMP=	.00
ROUTE MCUNGE	300.80	1	2	.88400	491.66	34.807	.73826	2.150	.869	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.390
COMPUTE NM HYD	301.00	-	1	.85200	374.23	24.387	.53669	1.800	.686	PER IMP=	.00
ADD HYD	301.90	1& 2	11	1.73600	719.02	59.194	.63933	2.050	.647		
*S NORTH EL CAMINO ARROYO AT TRAMWAY ROAD (301.90)											
ROUTE MCUNGE	301.82	11	2	1.73600	710.39	59.155	.63892	2.200	.639	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.230
COMPUTE NM HYD	302.00	-	1	.15760	98.52	3.014	.35859	1.500	.977	PER IMP=	5.00
ADD HYD	302.90	1& 2	5	1.89360	716.62	62.169	.61559	2.200	.591		
ROUTE MCUNGE	302.81	5	1	1.89360	715.07	62.083	.61473	2.207	.590	CCODE =	.2
ROUTE MCUNGE	302.82	1	2	1.89360	714.79	62.328	.61716	2.350	.590	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.150
COMPUTE NM HYD	303.00	-	1	.17060	138.60	5.250	.57705	1.550	1.269	PER IMP=	15.00
ADD HYD	303.90	1& 2	5	2.06420	725.37	67.579	.61385	2.250	.549		
*S NORTH EL CAMINO ARROYO AT HOLBROOK (303.90)											
ROUTE MCUNGE	303.80	5	2	2.06420	718.36	67.496	.61309	2.400	.544	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.090
COMPUTE NM HYD	305.10	-	1	.16910	106.64	4.665	.51729	1.550	.985	PER IMP=	15.00
ADD HYD	305.19	1& 2	5	2.23330	725.99	72.161	.60584	2.400	.508		
*S NORTH EL CAMINO ARROYO AT VENTURA (305.19)											
ROUTE MCUNGE	305.18	5	2	2.23330	719.49	72.114	.60544	2.500	.503	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.050
COMPUTE NM HYD	305.20	-	1	.15290	52.98	2.543	.31188	1.600	.541	PER IMP=	5.00
ADD HYD	305.29	1& 2	5	2.38620	723.74	74.657	.58663	2.500	.474		
*S NORTH EL CAMINO ARROYO AT BARSTOW (305.29)											
ROUTE MCUNGE	305.28	5	2	2.38620	715.20	74.558	.58585	2.650	.468	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.970
COMPUTE NM HYD	312.00	-	1	.06600	24.85	.995	.28279	1.550	.588	PER IMP=	5.00
ADD HYD	312.90	1& 2	5	2.45220	716.31	75.554	.57770	2.650	.456		
*S NORTH EL CAMINO ARROYO AT WYOMING (312.90)											
ROUTE MCUNGE	312.80	5	2	2.45220	715.53	75.502	.57731	2.750	.456	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 1.910
COMPUTE NM HYD	313.00	-	1	.20920	107.68	4.628	.41481	1.550	.804	PER IMP=	15.00
ADD HYD	313.90	1& 2	7	2.66140	719.04	80.130	.56453	2.750	.422		
RAINFALL TYPE= 2											RAIN24= 2.110
COMPUTE NM HYD	304.00	-	1	.08400	63.70	2.306	.51474	1.500	1.185	PER IMP=	15.00
ROUTE MCUNGE	304.80	1	2	.08400	56.24	2.224	.49647	1.800	1.046	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.020
COMPUTE NM HYD	307.00	-	1	.06630	40.21	1.296	.36660	1.500	.948	PER IMP=	8.00
ADD HYD	307.90	1& 2	5	.15030	67.19	3.520	.43918	1.800	.699		
*S NORTH CAMINO ARROYO (TRIB) AT BARSTOW (307.90)											
ROUTE MCUNGE	307.81	5	1	.15030	61.42	3.451	.43053	2.050	.638	CCODE =	.1
ROUTE MCUNGE	307.82	1	2	.15030	58.30	3.431	.42799	2.200	.606	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.970
COMPUTE NM HYD	310.00	-	1	.10330	35.86	1.609	.29205	1.600	.542	PER IMP=	5.00
ADD HYD	310.90	1& 2	6	.25360	63.51	5.040	.37261	2.200	.391		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	306.00	-	1	.10510	93.29	3.072	.54801	1.500	1.387	RAIN24= 2.050 PER IMP= 15.00
ROUTE MCUNGE	306.80	1	2	.10510	79.75	2.971	.52998	1.800	1.186	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	308.00	-	1	.05350	27.40	.856	.29998	1.500	.800	RAIN24= 1.990 PER IMP= 5.00
ADD HYD	308.90	1& 2	5	.15860	87.71	3.827	.45239	1.750	.864	
RAINFALL TYPE= 2										
COMPUTE NM HYD	309.00	-	1	.07960	35.55	1.276	.30060	1.550	.698	RAIN24= 1.990 PER IMP= 5.00
ADD HYD	309.90	1& 5	5	.23820	104.30	5.103	.40166	1.750	.684	
ROUTE MCUNGE	309.80	5	2	.23820	92.92	5.033	.39618	2.050	.610	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	311.00	-	1	.15280	50.41	2.264	.27775	1.600	.515	RAIN24= 1.970 PER IMP= 5.00
ADD HYD	311.90	1& 2	5	.39100	105.56	7.297	.34990	2.050	.422	
ADD HYD	311.99	5& 6	5	.64460	156.12	12.336	.35884	2.050	.378	
*S NORTH CAMINO ARROYO (TRIB) AT CORONADO RUNWAY (311.99)										
ROUTE MCUNGE	311.81	5	1	.64460	153.93	12.295	.35763	2.150	.373	CCODE = .1
ROUTE MCUNGE	311.82	1	2	.64460	153.74	12.137	.35305	2.176	.373	CCODE = .2
*S NORTH EL CAMINO ARROYO AT I-25 (AP 313.99)										
ADD HYD	313.99	2& 7	5	3.30600	767.73	91.988	.52171	2.712	.363	
ROUTE MCUNGE	313.88	5	2	3.30600	767.00	92.027	.52193	2.750	.363	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	314.00	-	1	.09600	44.42	1.482	.28950	1.500	.723	RAIN24= 1.890 PER IMP= 8.00
ADD HYD	314.99	1& 2	5	3.40200	767.81	93.509	.51537	2.750	.353	
RAINFALL TYPE= 2										
COMPUTE NM HYD	315.00	-	1	.04260	16.59	.536	.23596	1.500	.609	RAIN24= 1.890 PER IMP= 5.00
ADD HYD	315.99	1& 5	5	3.44460	768.14	94.045	.51192	2.750	.348	
RECALL HYD	206.39	-	19	.25890	196.30	7.942	.57519	1.500	1.185	
ADD HYD	206.99	5&19	5	3.70350	772.07	101.988	.51634	2.750	.326	
ROUTE MCUNGE	206.88	5	2	3.70350	765.39	101.972	.51626	2.900	.323	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.00	-	1	.05560	63.78	1.623	.54738	1.500	1.792	RAIN24= 1.840 PER IMP= .00
ADD HYD	401.90	1& 2	5	3.75910	765.66	103.595	.51672	2.900	.318	
RAINFALL TYPE= 2										
COMPUTE NM HYD	402.00	-	1	.29180	288.97	10.105	.64929	1.550	1.547	RAIN24= 1.820 PER IMP= 10.00
ADD HYD	402.90	1& 5	6	4.05090	770.06	113.700	.52627	2.900	.297	
*S FLOW TO N. DIVERSION CHANNEL FROM NORTH EL CAMINO ARROYO										
FINISH										

NORTH CAMINO EXISTING COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 INPUT FILE = nc100b.ext

RUN DATE (MON/DAY/YR) = 11/05/1998
 USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 3.990
COMPUTE NM HYD	300.00	-	1	.88400	1001.07	68.141	1.44530	1.800	1.769	PER IMP= .00
ROUTE MCUNGE	300.80	1	2	.88400	976.60	67.873	1.43962	2.050	1.726	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.590
COMPUTE NM HYD	301.00	-	1	.85200	862.46	54.493	1.19922	1.800	1.582	PER IMP= .00
ADD HYD	301.90	1& 2	11	1.73600	1605.38	122.366	1.32164	1.950	1.445	
*S NORTH EL CAMINO ARROYO AT TRAMWAY ROAD (301.90)										
ROUTE MCUNGE	301.82	11	2	1.73600	1587.76	122.145	1.31925	2.100	1.429	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.340
COMPUTE NM HYD	302.00	-	1	.15760	250.67	7.701	.91624	1.500	2.485	PER IMP= 5.00
ADD HYD	302.90	1& 2	5	1.89360	1608.31	129.846	1.28570	2.100	1.327	
ROUTE MCUNGE	302.81	5	1	1.89360	1604.35	129.690	1.28417	2.107	1.324	CCODE = .2
ROUTE MCUNGE	302.82	1	2	1.89360	1583.35	129.725	1.28451	2.250	1.307	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.220
COMPUTE NM HYD	303.00	-	1	.17060	288.51	10.990	1.20788	1.550	2.642	PER IMP= 15.00
ADD HYD	303.90	1& 2	5	2.06420	1604.95	140.715	1.27818	2.250	1.215	
*S NORTH EL CAMINO ARROYO AT HOLBROOK (303.90)										
ROUTE MCUNGE	303.80	5	2	2.06420	1604.39	140.662	1.27770	2.300	1.214	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 3.130
COMPUTE NM HYD	305.10	-	1	.16910	237.45	10.106	1.12056	1.550	2.194	PER IMP= 15.00
ADD HYD	305.19	1& 2	5	2.23330	1625.07	150.768	1.26580	2.300	1.137	
*S NORTH EL CAMINO ARROYO AT VENTURA (305.19)										
ROUTE MCUNGE	305.18	5	2	2.23330	1622.13	150.810	1.26615	2.350	1.135	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 3.070
COMPUTE NM HYD	305.20	-	1	.15290	146.57	6.756	.82850	1.600	1.498	PER IMP= 5.00
ADD HYD	305.29	1& 2	5	2.38620	1638.02	157.566	1.23810	2.300	1.073	
*S NORTH EL CAMINO ARROYO AT BARSTOW (305.29)										
ROUTE MCUNGE	305.28	5	2	2.38620	1624.82	157.418	1.23694	2.450	1.064	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.960
COMPUTE NM HYD	312.00	-	1	.06600	70.11	2.761	.78428	1.550	1.660	PER IMP= 5.00
ADD HYD	312.90	1& 2	5	2.45220	1629.17	160.179	1.22476	2.450	1.038	
*S NORTH EL CAMINO ARROYO AT WYOMING (312.90)										
ROUTE MCUNGE	312.80	5	2	2.45220	1617.14	159.972	1.22317	2.550	1.030	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.870
COMPUTE NM HYD	313.00	-	1	.20920	266.92	10.599	.94995	1.550	1.994	PER IMP= 15.00
ADD HYD	313.90	1& 2	7	2.66140	1629.15	170.571	1.20170	2.550	.956	
RAINFALL TYPE= 2										RAIN24= 3.160
COMPUTE NM HYD	304.00	-	1	.08400	138.67	4.992	1.11436	1.500	2.579	PER IMP= 15.00
ROUTE MCUNGE	304.80	1	2	.08400	127.81	4.955	1.10611	1.700	2.377	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.030
COMPUTE NM HYD	307.00	-	1	.06630	98.65	3.196	.90371	1.500	2.325	PER IMP= 8.00
ADD HYD	307.90	1& 2	5	.15030	174.12	8.151	1.01683	1.700	1.810	
*S NORTH CAMINO ARROYO (TRIB) AT BARSTOW (307.90)										
ROUTE MCUNGE	307.81	5	1	.15030	164.37	8.121	1.01304	1.850	1.709	CCODE = .1
ROUTE MCUNGE	307.82	1	2	.15030	158.65	8.051	1.00441	1.950	1.649	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.950
COMPUTE NM HYD	310.00	-	1	.10330	100.32	4.361	.79158	1.600	1.517	PER IMP= 5.00
ADD HYD	310.90	1& 2	6	.25360	191.03	12.412	.91771	1.950	1.177	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	306.00	-	1	.10510	193.49	6.528	1.16463	1.500	2.877	RAIN24= 3.080 PER IMP= 15.00
ROUTE MCUNGE	306.80	1	2	.10510	175.36	6.480	1.15606	1.700	2.607	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	308.00	-	1	.05350	72.95	2.296	.80462	1.500	2.131	RAIN24= 2.980 PER IMP= 5.00
ADD HYD	308.90	1& 2	5	.15860	209.86	8.776	1.03751	1.700	2.067	
RAINFALL TYPE= 2										
COMPUTE NM HYD	309.00	-	1	.07960	96.16	3.439	.80999	1.550	1.888	RAIN24= 2.990 PER IMP= 5.00
ADD HYD	309.90	1& 5	5	.23820	278.03	12.215	.96148	1.650	1.824	
ROUTE MCUNGE	309.80	5	2	.23820	261.19	12.109	.95318	1.850	1.713	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	311.00	-	1	.15280	147.74	6.278	.77037	1.600	1.511	RAIN24= 2.950 PER IMP= 5.00
ADD HYD	311.90	1& 2	5	.39100	335.66	18.387	.88174	1.800	1.341	
ADD HYD	311.99	5& 6	5	.64460	501.68	30.799	.89589	1.850	1.216	
*S NORTH CAMINO ARROYO (TRIB) AT CORONADO RUNWAY (311.99)										
ROUTE MCUNGE	311.81	5	1	.64460	496.75	30.745	.89430	1.900	1.204	CCODE = .1
ROUTE MCUNGE	311.82	1	2	.64460	494.22	30.142	.87677	1.913	1.198	CCODE = .2
*S NORTH EL CAMINO ARROYO AT I-25 (AP 313.99)										
ADD HYD	313.99	2& 7	5	3.30600	1725.31	199.649	1.13231	2.520	.815	
ROUTE MCUNGE	313.88	5	2	3.30600	1710.28	199.463	1.13126	2.600	.808	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	314.00	-	1	.09600	125.48	3.960	.77351	1.500	2.042	RAIN24= 2.830 PER IMP= 8.00
ADD HYD	314.99	1& 2	5	3.40200	1713.33	203.423	1.12116	2.600	.787	
RAINFALL TYPE= 2										
COMPUTE NM HYD	315.00	-	1	.04260	50.88	1.578	.69449	1.500	1.866	RAIN24= 2.830 PER IMP= 5.00
ADD HYD	315.99	1& 5	5	3.44460	1714.65	205.001	1.11588	2.600	.778	
RECALL HYD	206.39	-	19	.25890	415.02	16.003	1.15898	1.500	2.505	
ADD HYD	206.99	5&19	5	3.70350	1726.61	221.004	1.11890	2.600	.728	
ROUTE MCUNGE	206.88	5	2	3.70350	1725.43	221.017	1.11896	2.650	.728	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.00	-	1	.05560	114.46	3.498	1.17962	1.500	3.217	RAIN24= 2.760 PER IMP= .00
ADD HYD	401.90	1& 2	5	3.75910	1726.99	224.515	1.11986	2.650	.718	
RAINFALL TYPE= 2										
COMPUTE NM HYD	402.00	-	1	.29180	557.26	20.367	1.30869	1.550	2.984	RAIN24= 2.730 PER IMP= 10.00
ADD HYD	402.90	1& 5	6	4.05090	1741.11	244.882	1.13346	2.650	.672	
*S FLOW TO N. DIVERSION CHANNEL FROM NORTH EL CAMINO ARROYO										
FINISH										

NORTH CAMINO FUTURE CONDITION 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 INPUT FILE = ctn10.fut

RUN DATE (MON/DAY/YR) = 11/05/1998
 USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											TIME= .00
RAINFALL TYPE= 2											RAIN24= 2.660
COMPUTE NM HYD	300.00	-	1	.88400	509.61	34.987	.74209	1.800	.901	PER IMP=	.00
ROUTE MCUNGE	300.80	1	2	.88400	491.66	34.807	.73826	2.150	.869	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.390
COMPUTE NM HYD	301.00	-	1	.85200	374.23	24.387	.53669	1.800	.686	PER IMP=	.00
ADD HYD	301.90	1& 2	11	1.73600	719.02	59.194	.63933	2.050	.647		
*S NORTH EL CAMINO ARROYO AT TRAMWAY ROAD (301.90)											
ROUTE MCUNGE	301.82	11	2	1.73600	710.39	59.155	.63892	2.200	.639	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.230
COMPUTE NM HYD	302.00	-	1	.15760	213.57	7.816	.92984	1.500	2.117	PER IMP=	30.00
ADD HYD	302.90	1& 2	5	1.89360	725.20	66.971	.66313	2.200	.598		
ROUTE MCUNGE	302.81	5	1	1.89360	724.01	66.514	.65861	2.207	.597	CCODE =	.1
ROUTE MCUNGE	302.82	1	2	1.89360	722.36	66.661	.66006	2.350	.596	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.150
COMPUTE NM HYD	303.00	-	1	.17060	184.83	6.966	.76565	1.550	1.693	PER IMP=	20.00
ADD HYD	303.90	1& 2	5	2.06420	732.53	73.627	.66878	2.350	.554		
*S NORTH EL CAMINO ARROYO AT HOLBROOK (303.90)											
ROUTE MCUNGE	303.80	5	2	2.06420	728.11	73.609	.66863	2.400	.551	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.090
COMPUTE NM HYD	305.10	-	1	.16910	159.69	6.681	.74082	1.550	1.476	PER IMP=	20.00
ADD HYD	305.19	1& 2	5	2.23330	738.56	80.291	.67409	2.400	.517		
*S NORTH EL CAMINO ARROYO AT VENTURA (305.19)											
ROUTE MCUNGE	305.18	5	2	2.23330	731.91	80.183	.67319	2.500	.512	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.050
COMPUTE NM HYD	305.20	-	1	.15290	125.07	5.912	.72493	1.600	1.278	PER IMP=	20.00
ADD HYD	305.29	1& 2	5	2.38620	740.94	86.095	.67651	2.500	.485		
*S NORTH EL CAMINO ARROYO AT BARSTOW (305.29)											
ROUTE MCUNGE	305.28	5	2	2.38620	732.79	86.015	.67588	2.650	.480	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.970
COMPUTE NM HYD	312.00	-	1	.06600	61.07	2.401	.68219	1.550	1.446	PER IMP=	20.00
ADD HYD	312.90	1& 2	5	2.45220	734.79	88.416	.67605	2.650	.468		
*S NORTH EL CAMINO ARROYO AT WYOMING (312.90)											
ROUTE MCUNGE	312.80	5	2	2.45220	728.86	88.364	.67565	2.750	.464	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.910
COMPUTE NM HYD	313.00	-	1	.20920	250.33	11.376	1.01961	1.550	1.870	PER IMP=	49.00
ADD HYD	313.90	1& 2	7	2.66140	735.95	99.740	.70269	2.750	.432		
RAINFALL TYPE= 2											RAIN24= 2.110
COMPUTE NM HYD	304.00	-	1	.08400	92.34	3.106	.69341	1.500	1.718	PER IMP=	17.00
ROUTE MCUNGE	304.80	1	2	.08400	78.37	3.012	.67222	1.800	1.458	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.020
COMPUTE NM HYD	307.00	-	1	.05820	66.17	2.165	.69744	1.500	1.776	PER IMP=	20.00
ADD HYD	307.90	1& 2	5	.14220	99.20	5.176	.68254	1.750	1.090		
*S NORTH CAMINO ARROYO (TRIB) AT BARSTOW (307.90)											
ROUTE MCUNGE	307.81	5	1	.14220	88.26	5.063	.66764	2.000	.970	CCODE =	.1
ROUTE MCUNGE	307.82	1	2	.14220	85.25	5.016	.66138	2.150	.937	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.970
COMPUTE NM HYD	310.00	-	1	.10330	80.12	3.483	.63215	1.600	1.212	PER IMP=	17.00
ADD HYD	310.90	1& 2	6	.24550	98.33	8.499	.64908	2.150	.626		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										RAIN24= 2.050
COMPUTE NM HYD	306.00	-	1	.10510	117.15	3.751	.66916	1.500	1.742	PER IMP= 17.00
ROUTE MCUNGE	306.80	1	2	.10510	98.22	3.614	.64474	1.750	1.460	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 1.990
COMPUTE NM HYD	308.00	-	1	.05350	58.24	1.845	.64646	1.500	1.701	PER IMP= 17.00
ADD HYD	308.90	1& 2	5	.15860	116.73	5.459	.64532	1.750	1.150	
RAINFALL TYPE= 2										RAIN24= 1.990
COMPUTE NM HYD	309.00	-	1	.07960	75.82	2.731	.64323	1.550	1.488	PER IMP= 17.00
ADD HYD	309.90	1& 5	5	.23820	149.47	8.189	.64462	1.750	.980	
ROUTE MCUNGE	309.80	5	2	.23820	139.37	8.158	.64219	1.950	.914	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 1.970
COMPUTE NM HYD	311.00	-	1	.15280	116.78	5.107	.62669	1.600	1.194	PER IMP= 17.00
ADD HYD	311.90	1& 2	5	.39100	198.87	13.265	.63613	1.750	.795	
ADD HYD	311.99	5& 6	5	.63650	261.16	21.764	.64113	1.700	.641	
*S NORTH CAMINO ARROYO (TRIB) AT CORONADO RUNWAY (311.99)										
ROUTE MCUNGE	311.81	5	1	.63650	255.38	21.718	.63977	2.050	.627	CCODE = .1
ROUTE MCUNGE	311.82	1	2	.63650	253.91	21.418	.63092	2.042	.623	CCODE = .2
*S NORTH EL CAMINO ARROYO AT I-25 (AP 313.99)										
ADD HYD	313.99	2& 7	5	3.29790	796.87	120.575	.68552	2.712	.378	
ROUTE MCUNGE	313.88	5	2	3.29790	795.23	120.610	.68572	2.750	.377	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 1.890
COMPUTE NM HYD	314.00	-	1	.09600	144.47	5.411	1.05679	1.500	2.351	PER IMP= 50.00
ADD HYD	314.99	1& 2	5	3.39390	796.95	126.021	.69622	2.750	.367	
RAINFALL TYPE= 2										RAIN24= 1.890
COMPUTE NM HYD	315.00	-	1	.04260	64.05	2.396	1.05449	1.500	2.349	PER IMP= 50.00
ADD HYD	315.99	1& 5	5	3.43650	797.72	128.417	.70066	2.750	.363	
RECALL HYD	206.39	-	19	.25890	376.50	15.625	1.13156	1.500	2.272	
ADD HYD	206.99	5&19	5	3.69540	847.98	144.041	.73085	1.950	.359	
ROUTE MCUNGE	206.88	5	2	3.69540	840.62	143.957	.73042	2.100	.355	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 1.840
COMPUTE NM HYD	401.00	-	1	.05560	82.44	3.035	1.02340	1.500	2.317	PER IMP= 50.00
ADD HYD	401.90	1& 2	5	3.75100	852.31	146.992	.73476	2.050	.355	
RAINFALL TYPE= 2										RAIN24= 1.820
COMPUTE NM HYD	402.00	-	1	.29180	360.39	15.753	1.01221	1.550	1.930	PER IMP= 50.00
ADD HYD	402.90	1& 5	6	4.04280	1033.27	162.744	.75479	1.650	.399	
*S FLOW TO N. DIVERSION CHANNEL FROM NORTH EL CAMINO ARROYO										
FINISH										

NORTH CAMINO FUTURE CONDITION 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 INPUT FILE = ctn100.fut

RUN DATE (MON/DAY/YR) = 11/05/1998
 USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											TIME= .00
RAINFALL TYPE= 2											RAIN24= 3.990
COMPUTE NM HYD	300.00	-	1	.88400	1001.07	68.141	1.44530	1.800	1.769	PER IMP=	.00
ROUTE MCUNGE	300.80	1	2	.88400	976.60	67.873	1.43962	2.050	1.726	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.590
COMPUTE NM HYD	301.00	-	1	.85200	862.46	54.493	1.19922	1.800	1.582	PER IMP=	.00
ADD HYD	301.90	1& 2	11	1.73600	1605.38	122.366	1.32164	1.950	1.445		
*S NORTH EL CAMINO ARROYO AT TRAMWAY ROAD (301.90)											
ROUTE MCUNGE	301.82	11	2	1.73600	1587.76	122.145	1.31925	2.100	1.429	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.340
COMPUTE NM HYD	302.00	-	1	.15760	377.68	14.313	1.70288	1.500	3.744	PER IMP=	30.00
ADD HYD	302.90	1& 2	5	1.89360	1631.74	136.458	1.35117	2.100	1.346		
ROUTE MCUNGE	302.81	5	1	1.89360	1628.16	135.566	1.34235	2.107	1.343	CCODE =	.2
ROUTE MCUNGE	302.82	1	2	1.89360	1623.95	135.667	1.34335	2.200	1.340	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 3.220
COMPUTE NM HYD	303.00	-	1	.17060	340.12	13.452	1.47844	1.550	3.115	PER IMP=	20.00
ADD HYD	303.90	1& 2	5	2.06420	1655.63	149.119	1.35451	2.150	1.253		
*S NORTH EL CAMINO ARROYO AT HOLBROOK (303.90)											
ROUTE MCUNGE	303.80	5	2	2.06420	1651.58	149.012	1.35354	2.250	1.250	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.130
COMPUTE NM HYD	305.10	-	1	.16910	302.07	13.014	1.44303	1.550	2.791	PER IMP=	20.00
ADD HYD	305.19	1& 2	5	2.23330	1682.34	162.026	1.36032	2.250	1.177		
*S NORTH EL CAMINO ARROYO AT VENTURA (305.19)											
ROUTE MCUNGE	305.18	5	2	2.23330	1680.41	162.092	1.36087	2.300	1.176	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 3.070
COMPUTE NM HYD	305.20	-	1	.15290	236.60	11.517	1.41235	1.600	2.418	PER IMP=	20.00
ADD HYD	305.29	1& 2	5	2.38620	1709.09	173.609	1.36417	2.300	1.119		
*S NORTH EL CAMINO ARROYO AT BARSTOW (305.29)											
ROUTE MCUNGE	305.28	5	2	2.38620	1695.57	173.479	1.36314	2.400	1.110	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.960
COMPUTE NM HYD	312.00	-	1	.06600	114.08	4.765	1.35364	1.550	2.701	PER IMP=	20.00
ADD HYD	312.90	1& 2	5	2.45220	1702.84	178.243	1.36288	2.400	1.085		
*S NORTH EL CAMINO ARROYO AT WYOMING (312.90)											
ROUTE MCUNGE	312.80	5	2	2.45220	1700.61	178.267	1.36306	2.423	1.084	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.870
COMPUTE NM HYD	313.00	-	1	.20920	437.50	19.847	1.77880	1.550	3.268	PER IMP=	49.00
ADD HYD	313.90	1& 2	7	2.66140	1727.38	198.075	1.39547	2.423	1.014		
RAINFALL TYPE= 2											RAIN24= 3.160
COMPUTE NM HYD	304.00	-	1	.08400	171.88	6.183	1.38016	1.500	3.197	PER IMP=	17.00
ROUTE MCUNGE	304.80	1	2	.08400	157.05	6.140	1.37048	1.700	2.921	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.030
COMPUTE NM HYD	307.00	-	1	.05820	121.68	4.267	1.37458	1.500	3.267	PER IMP=	20.00
ADD HYD	307.90	1& 2	5	.14220	213.89	10.406	1.37215	1.700	2.350		
*S NORTH CAMINO ARROYO (TRIB) AT BARSTOW (307.90)											
ROUTE MCUNGE	307.81	5	1	.14220	203.98	10.321	1.36091	1.850	2.241	CCODE =	.1
ROUTE MCUNGE	307.82	1	2	.14220	202.08	10.318	1.36053	1.900	2.220	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.950
COMPUTE NM HYD	310.00	-	1	.10330	154.99	7.037	1.27736	1.600	2.344	PER IMP=	17.00
ADD HYD	310.90	1& 2	6	.24550	271.70	17.356	1.32553	1.850	1.729		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										RAIN24= 3.080
COMPUTE NM HYD	306.00	-	1	.10510	220.14	7.537	1.34456	1.500	3.273	PER IMP= 17.00
ROUTE MCUNGE	306.80	1	2	.10510	197.50	7.491	1.33639	1.700	2.936	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.980
COMPUTE NM HYD	308.00	-	1	.05350	108.45	3.706	1.29894	1.500	3.167	PER IMP= 17.00
ADD HYD	308.90	1& 2	5	.15860	248.41	11.197	1.32375	1.700	2.447	
RAINFALL TYPE= 2										RAIN24= 2.990
COMPUTE NM HYD	309.00	-	1	.07960	143.06	5.522	1.30064	1.550	2.808	PER IMP= 17.00
ADD HYD	309.90	1& 5	5	.23820	353.40	16.719	1.31603	1.650	2.318	
ROUTE MCUNGE	309.80	5	2	.23820	353.09	16.763	1.31953	1.750	2.316	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 2.950
COMPUTE NM HYD	311.00	-	1	.15280	230.86	10.352	1.27025	1.600	2.361	PER IMP= 17.00
ADD HYD	311.90	1& 2	5	.39100	530.42	27.115	1.30027	1.700	2.120	
ADD HYD	311.99	5& 6	5	.63650	765.49	44.471	1.31001	1.750	1.879	
*S NORTH CAMINO ARROYO (TRIB) AT CORONADO RUNWAY (311.99)										
ROUTE MCUNGE	311.81	5	1	.63650	764.71	44.450	1.30941	1.800	1.877	CCODE = .2
ROUTE MCUNGE	311.82	1	2	.63650	762.27	43.031	1.26760	1.772	1.871	CCODE = .2
*S NORTH EL CAMINO ARROYO AT I-25 (AP 313.99)										
ADD HYD	313.99	2& 7	5	3.29790	1909.25	238.385	1.35532	1.793	.905	
ROUTE MCUNGE	313.88	5	2	3.29790	1869.26	238.121	1.35382	1.900	.886	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.830
COMPUTE NM HYD	314.00	-	1	.09600	239.07	9.345	1.82515	1.500	3.891	PER IMP= 50.00
ADD HYD	314.99	1& 2	5	3.39390	1930.42	247.466	1.36716	1.900	.889	
RAINFALL TYPE= 2										RAIN24= 2.830
COMPUTE NM HYD	315.00	-	1	.04260	104.88	4.130	1.81763	1.500	3.847	PER IMP= 50.00
ADD HYD	315.99	1& 5	5	3.43650	1957.19	251.596	1.37274	1.900	.890	
RECALL HYO	206.39	-	19	.25890	619.79	26.255	1.90144	1.500	3.741	
ADD HYD	206.99	5&19	5	3.69540	2138.80	277.851	1.40978	1.900	.904	
ROUTE MCUNGE	206.88	5	2	3.69540	2137.06	277.937	1.41022	1.950	.904	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 2.760
COMPUTE NM HYD	401.00	-	1	.05560	135.75	5.261	1.77417	1.500	3.815	PER IMP= 50.00
ADD HYO	401.90	1& 2	5	3.75100	2167.63	283.198	1.41561	1.950	.903	
RAINFALL TYPE= 2										RAIN24= 2.730
COMPUTE NM HYD	402.00	-	1	.29180	631.89	27.357	1.75788	1.550	3.384	PER IMP= 50.00
ADD HYD	402.90	1& 5	6	4.04280	2351.65	310.555	1.44031	1.900	.909	
*S FLOW TO N. DIVERSION CHANNEL FROM NORTH EL CAMINO ARROYO										
FINISH										

EL CAMINO EXISTING CONDITION 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = ec10b.ext

RUN DATE (MON/DAY/YR) =11/05/1998
USER NO.= RSTECNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											TIME= .00
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	201.00	-	1	.13390	81.06	2.525	.35355	1.500	.946	PER IMP=	5.00
RAINFALL TYPE= 2											RAIN24= 2.330
COMPUTE NM HYD	200.00	-	3	.30300	197.06	5.927	.36680	1.550	1.016	PER IMP=	.00
*S EL CAMINO ARROYO AT TRAMWAY BLVD (200.0)											
ROUTE MCUNGE	200.80	3	4	.30300	174.00	5.666	.35063	1.750	.897	CCODE =	.1
ADD HYD	201.90	4& 1	5	.43690	201.09	8.191	.35152	1.750	.719		
*S EL CAMINO ARROYO (MAIN) AT BROWNING (201.9)											
ROUTE MCUNGE	201.80	5	2	.43690	186.22	8.142	.34941	1.950	.666	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.170
COMPUTE NM HYD	202.10	-	1	.10310	68.80	2.226	.40486	1.500	1.043	PER IMP=	8.00
ADD HYD	202.19	1& 2	5	.54000	196.34	10.368	.36000	1.950	.568		
*S EL CAMINO ARROYO (MAIN) AT EUBANK (202.19)											
ROUTE MCUNGE	202.18	5	2	.54000	194.42	10.406	.36132	2.000	.563	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.130
COMPUTE NM HYD	202.20	-	1	.10990	79.79	2.692	.45922	1.500	1.134	PER IMP=	10.00
ADD HYD	202.29	1& 2	5	.64990	205.88	13.098	.37788	2.000	.495		
*S EL CAMINO ARROYO (MAIN) AT HOLBROOK (202.29)											
ROUTE MCUNGE	202.28	5	2	.64990	193.86	13.046	.37639	2.200	.466	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.050
COMPUTE NM HYD	202.30	-	1	.06840	55.75	1.887	.51719	1.500	1.274	PER IMP=	15.00
ADD HYD	202.39	1& 2	6	.71830	197.66	14.933	.38980	2.200	.430		
*S EL CAMINO ARROYO (MAIN) AT VENTURA (202.39)											
RAINFALL TYPE= 2											RAIN24= 2.090
COMPUTE NM HYD	203.10	-	1	.12580	81.09	2.799	.41718	1.500	1.007	PER IMP=	10.00
*S EL CAMINO ARROYO (TRIBUTARY) AT HOLBROOK (203.1)											
ROUTE MCUNGE	203.18	1	2	.12580	71.96	2.717	.40496	1.750	.894	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.050
COMPUTE NM HYD	203.20	-	1	.04850	31.32	1.043	.40304	1.500	1.009	PER IMP=	10.00
ADD HYD	203.29	1& 2	5	.17430	82.94	3.759	.40442	1.750	.744		
ADD HYD	202.90	5& 6	5	.89260	216.21	18.692	.39265	2.200	.378		
ROUTE MCUNGE	202.80	5	2	.89260	213.98	18.682	.39243	2.250	.375	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.010
COMPUTE NM HYD	203.30	-	1	.05580	34.52	1.152	.38696	1.500	.967	PER IMP=	10.00
ADD HYD	203.39	1& 2	5	.94840	216.00	19.833	.39211	2.250	.356		
*S EL CAMINO ARROYO (MAIN) AT BARSTOW (203.39)											
ROUTE MCUNGE	203.98	5	2	.94840	206.24	19.779	.39104	2.450	.340	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.970
COMPUTE NM HYD	204.00	-	1	.21190	89.98	4.166	.36862	1.600	.664	PER IMP=	10.00
ADD HYD	204.90	1& 2	5	1.16030	230.99	23.945	.38694	2.000	.311		
*S EL CAMINO ARROYO AT WYOMING (204.9)											
ROUTE MCUNGE	204.80	5	2	1.16030	223.54	23.884	.38595	2.200	.301	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.930
COMPUTE NM HYD	204.20	-	1	.13330	71.82	2.508	.35273	1.500	.842	PER IMP=	10.00
ADD HYD	204.29	1& 2	5	1.29360	229.07	26.392	.38253	2.200	.277		
RAINFALL TYPE= 2											RAIN24= 1.940
COMPUTE NM HYD	204.10	-	1	.14840	71.63	2.914	.36813	1.550	.754	PER IMP=	10.00
ROUTE MCUNGE	204.18	1	2	.14840	69.91	2.905	.36706	1.600	.736	CCODE =	.2

EL CAMINO EXISTING CONDITION 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = ec100b.ext

RUN DATE (MON/DAY/YR) =11/05/1998
USER NO.= RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											TIME= .00
RAINFALL TYPE= 2											RAIN24= 3.370
COMPUTE NM HYD	201.00	-	1	.13390	207.70	6.497	.90979	1.500	2.424	PER IMP=	5.00
RAINFALL TYPE= 2											RAIN24= 3.500
COMPUTE NM HYD	200.00	-	3	.30300	487.60	15.384	.95201	1.550	2.514	PER IMP=	.00
*S EL CAMINO ARROYO AT TRAMWAY BLVD (200.0)											
ROUTE MCUNGE	200.80	3	4	.30300	453.59	15.176	.93914	1.700	2.339	CCODE =	.1
ADD HYD	201.90	4& 1	5	.43690	559.37	21.674	.93014	1.650	2.000		
*S EL CAMINO ARROYO (MAIN) AT BROWNING (201.9)											
ROUTE MCUNGE	201.80	5	2	.43690	534.38	21.592	.92664	1.800	1.911	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.250
COMPUTE NM HYD	202.10	-	1	.10310	164.12	5.334	.97014	1.500	2.487	PER IMP=	8.00
ADD HYD	202.19	1& 2	5	.54000	589.48	26.926	.93495	1.750	1.706		
*S EL CAMINO ARROYO (MAIN) AT EUBANK (202.19)											
ROUTE MCUNGE	202.18	5	2	.54000	565.85	26.878	.93327	1.850	1.637	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.190
COMPUTE NM HYD	202.20	-	1	.10990	180.49	6.162	1.05124	1.500	2.566	PER IMP=	10.00
ADD HYD	202.29	1& 2	5	.64990	607.66	33.040	.95322	1.850	1.461		
*S EL CAMINO ARROYO (MAIN) AT HOLBROOK (202.29)											
ROUTE MCUNGE	202.28	5	2	.64990	588.10	33.013	.95244	1.950	1.414	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.070
COMPUTE NM HYD	202.30	-	1	.06840	118.36	4.064	1.11411	1.500	2.704	PER IMP=	15.00
ADD HYD	202.39	1& 2	6	.71830	607.11	37.077	.96783	1.950	1.321		
*S EL CAMINO ARROYO (MAIN) AT VENTURA (202.39)											
RAINFALL TYPE= 2											RAIN24= 3.140
COMPUTE NM HYD	203.10	-	1	.12580	192.92	6.547	.97587	1.500	2.396	PER IMP=	10.00
*S EL CAMINO ARROYO (TRIBUTARY) AT HOLBROOK (203.1)											
ROUTE MCUNGE	203.18	1	2	.12580	176.32	6.466	.96366	1.700	2.190	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 3.070
COMPUTE NM HYD	203.20	-	1	.04850	74.17	2.466	.95343	1.500	2.389	PER IMP=	10.00
ADD HYD	203.29	1& 2	5	.17430	211.27	8.932	.96081	1.700	1.894		
ADD HYD	202.90	5& 6	5	.89260	685.83	46.009	.96646	1.950	1.201		
ROUTE MCUNGE	202.80	5	2	.89260	680.66	45.950	.96522	1.990	1.192	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 3.020
COMPUTE NM HYD	203.30	-	1	.05580	83.29	2.757	.92642	1.500	2.332	PER IMP=	10.00
ADD HYD	203.39	1& 2	5	.94840	691.83	48.707	.96294	1.990	1.140		
*S EL CAMINO ARROYO (MAIN) AT BARSTOW (203.39)											
ROUTE MCUNGE	203.98	5	2	.94840	689.37	48.718	.96315	2.050	1.136	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.960
COMPUTE NM HYD	204.00	-	1	.21190	235.18	10.122	.89562	1.600	1.734	PER IMP=	10.00
ADD HYD	204.90	1& 2	5	1.16030	752.02	58.839	.95082	1.800	1.013		
*S EL CAMINO ARROYO AT WYOMING (204.9)											
ROUTE MCUNGE	204.80	5	2	1.16030	737.66	58.784	.94992	1.900	.993	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.890
COMPUTE NM HYD	204.20	-	1	.13330	185.49	6.169	.86774	1.500	2.174	PER IMP=	10.00
ADD HYD	204.29	1& 2	5	1.29360	770.82	64.953	.94145	1.900	.931		
RAINFALL TYPE= 2											RAIN24= 2.910
COMPUTE NM HYD	204.10	-	1	.14840	181.03	7.086	.89528	1.550	1.906	PER IMP=	10.00
ROUTE MCUNGE	204.18	1	2	.14840	177.98	7.092	.89603	1.599	1.874	CCODE =	.2

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
A00 HYD	204.19	2& 5	5	1.44200	832.88	72.022	.93648	1.899	.902	
*S EL CAMINO ARROYO AT CORONADO AIRPORT (204.29)										
ROUTE MCUNGE	204.28	5	2	1.44200	831.34	70.867	.92147	1.901	.901	CC00E = .2
RAINFALL TYPE= 2										RAIN24= 2.850
COMPUTE NM HYD	205.00	-	1	.04590	123.43	5.191	2.12030	1.500	4.202	PER IMP= 70.00
ADD HYD	205.90	1& 2	5	1.48790	864.92	75.765	.95476	1.901	.908	
*S EL CAMINO ARROYO AT I-25 (205.90)										
RAINFALL TYPE= 2										RAIN24= 2.830
COMPUTE NM HYD	206.10	-	1	.12210	154.08	5.425	.83315	1.550	1.972	PER IMP= 10.00
RAINFALL TYPE= 2										RAIN24= 2.830
COMPUTE NM HYD	206.20	-	2	.05610	76.53	2.493	.83315	1.500	2.132	PER IMP= 10.00
A00 HYD	206.29	1& 2	5	.17820	228.80	7.918	.83315	1.500	2.006	
ROUTE	206.28	5	3	.17820	232.98	7.918	.83315	1.550	2.043	
RAINFALL TYPE= 2										RAIN24= 2.760
COMPUTE NM HYD	206.40	-	1	.03270	59.24	2.164	1.24111	1.500	2.831	PER IMP= 30.00
ROUTE	206.48	1	2	.03270	58.63	2.165	1.24112	1.500	2.802	
ADD HYD	206.49	2& 3	5	.21090	289.80	10.083	.89640	1.550	2.147	
RAINFALL TYPE= 2										RAIN24= 2.760
COMPUTE NM HYD	206.30	-	1	.04800	138.49	5.921	2.31273	1.500	4.508	PER IMP= 86.00
ADD HYD	206.39	1& 5	6	.25890	415.02	16.003	1.15899	1.500	2.505	
FINISH										

EL CAMINO FUTURE CONDITION 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = ctec10.fut

RUN DATE (MON/DAY/YR) =11/05/1998
USER NO.= RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											TIME= .00
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	201.00	-	1	.13390	81.06	2.525	.35355	1.500	.946	PER IMP=	5.00
RAINFALL TYPE= 2											RAIN24= 2.330
COMPUTE NM HYD	200.00	-	3	.30300	197.06	5.927	.36680	1.550	1.016	PER IMP=	.00
*S EL CAMINO ARROYO AT TRAMWAY BLVD (200.0)											
ROUTE MCUNGE	200.80	3	4	.30300	174.00	5.666	.35063	1.750	.897	CCODE =	.1
ADD HYD	201.90	4 & 1	5	.43690	201.09	8.191	.35152	1.750	.719		
*S EL CAMINO ARROYO (MAIN) AT BROWNING (201.9)											
ROUTE MCUNGE	201.80	5	2	.43690	186.22	8.142	.34941	1.950	.666	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.170
COMPUTE NM HYD	202.10	-	1	.10310	121.81	3.961	.72027	1.500	1.846	PER IMP=	17.00
ADD HYD	202.19	1 & 2	5	.54000	204.64	12.102	.42022	1.950	.592		
*S EL CAMINO ARROYO (MAIN) AT EUBANK (202.19)											
ROUTE MCUNGE	202.18	5	2	.54000	202.93	12.151	.42189	2.050	.587	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.130
COMPUTE NM HYD	202.20	-	1	.10990	122.75	4.133	.70507	1.500	1.745	PER IMP=	17.00
ADD HYD	202.29	1 & 2	5	.64990	232.22	16.283	.46978	1.600	.558		
*S EL CAMINO ARROYO (MAIN) AT HOLBROOK (202.29)											
ROUTE MCUNGE	202.28	5	2	.64990	230.99	16.359	.47196	1.700	.555	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 2.050
COMPUTE NM HYD	202.30	-	1	.05186	68.69	2.399	.86748	1.500	2.069	PER IMP=	30.00
ADD HYD	202.39	1 & 2	6	.70176	260.20	18.758	.50119	1.700	.579		
*S EL CAMINO ARROYO (MAIN) AT VENTURA (202.39)											
RAINFALL TYPE= 2											RAIN24= 2.090
COMPUTE NM HYD	203.10	-	1	.12580	136.73	4.591	.68422	1.500	1.698	PER IMP=	17.00
*S EL CAMINO ARROYO (TRIBUTARY) AT HOLBROOK (203.1)											
ROUTE MCUNGE	203.18	1	2	.12580	123.10	4.553	.67854	1.700	1.529	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.050
COMPUTE NM HYD	203.20	-	1	.03946	52.27	1.826	.86748	1.500	2.070	PER IMP=	30.00
ADD HYD	203.29	1 & 2	5	.16526	145.32	6.378	.72365	1.700	1.374		
ADD HYD	202.90	5 & 6	5	.86702	405.52	25.136	.54359	1.700	.731		
ROUTE MCUNGE	202.80	5	2	.86702	387.67	25.102	.54286	1.750	.699	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 2.010
COMPUTE NM HYD	203.30	-	1	.12590	193.61	7.465	1.11170	1.500	2.403	PER IMP=	50.00
ADD HYD	203.39	1 & 2	5	.99292	457.85	32.567	.61498	1.750	.720		
*S EL CAMINO ARROYO (MAIN) AT BARSTOW (203.39)											
ROUTE MCUNGE	203.98	5	2	.99292	456.60	32.559	.61483	1.750	.719	CCODE =	.2
RAINFALL TYPE= 2											RAIN24= 1.970
COMPUTE NM HYD	204.00	-	1	.10230	153.51	5.899	1.08121	1.500	2.345	PER IMP=	50.00
*S EL CAMINO ARROYO AT WYOMING (204.9)											
ADD HYD	204.90	1 & 2	5	1.09522	511.98	38.458	.65839	1.750	.730		
ROUTE MCUNGE	204.80	5	2	1.09522	493.64	38.428	.65788	1.850	.704	CCODE =	.1
COMPUTE NM HYD	204.40	-	1	.02968	30.96	.987	.62352	1.500	1.630	PER IMP=	17.00
ROUTE MCUNGE	204.81	1	6	.02968	25.83	.967	.61064	1.750	1.360	CCODE =	.1
RAINFALL TYPE= 2											RAIN24= 1.940
COMPUTE NM HYD	204.30	-	1	.11120	117.18	3.694	.62281	1.500	1.646	PER IMP=	17.00
ADD HYD	204.91	1 & 6	10	.14088	118.58	4.660	.62024	1.500	1.315		
ADD HYD	204.92	10 & 2	5	1.23610	541.01	43.088	.65359	1.800	.684		
*S (204.92=EL CAMINO ARROYO AT LOUISIANA)											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
ROUTE MCUNGE	204.82	5	3	1.23610	539.85	43.061	.65318	1.850	.682	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	1.930
COMPUTE NM HYD	204.20	-	1	.06875	118.86	4.812	1.31248	1.500	2.701	PER IMP=	70.00
ADD HYD	204.29	1& 3	5	1.30485	575.04	47.873	.68791	1.850	.689		
RAINFALL TYPE= 2										RAIN24=	1.940
COMPUTE NM HYD	204.10	-	1	.12880	131.42	5.455	.79413	1.550	1.594	PER IMP=	30.00
ROUTE MCUNGE	204.18	1	2	.12880	129.96	5.451	.79354	1.600	1.577	CCODE =	.2
ADD HYD	204.19	2& 5	5	1.43365	683.28	53.324	.69740	1.600	.745		
*S EL CAMINO ARROYO AT CORONADO AIRPORT (204.29)											
ROUTE MCUNGE	204.28	5	2	1.43365	681.88	53.303	.69712	1.650	.743	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	1.900
COMPUTE NM HYD	205.00	-	1	.05430	92.31	3.726	1.28671	1.500	2.656	PER IMP=	70.00
*S EL CAMINO ARROYO AT 1-25 (205.90)											
ADD HYD	205.90	1& 2	5	1.48795	731.24	57.029	.71864	1.650	.768		
RAINFALL TYPE= 2										RAIN24=	1.890
COMPUTE NM HYD	206.10	-	1	.12210	166.02	6.844	1.05092	1.500	2.124	PER IMP=	50.00
RAINFALL TYPE= 2										RAIN24=	1.890
COMPUTE NM HYD	206.20	-	2	.05610	83.55	3.144	1.05092	1.500	2.327	PER IMP=	50.00
ADD HYD	206.29	1& 2	5	.17820	249.57	9.988	1.05092	1.500	2.188		
ROUTE	206.28	5	3	.17820	247.14	9.988	1.05092	1.550	2.167		
RAINFALL TYPE= 2										RAIN24=	1.890
COMPUTE NM HYD	206.40	-	1	.03270	48.70	1.833	1.05092	1.500	2.327	PER IMP=	50.00
ROUTE	206.48	1	2	.03270	47.99	1.833	1.05094	1.500	2.293		
ADD HYD	206.49	2& 3	5	.21090	293.29	11.821	1.05092	1.550	2.173		
RAINFALL TYPE= 2										RAIN24=	1.890
COMPUTE NM HYD	206.30	-	1	.04800	90.31	3.804	1.48587	1.500	2.940	PER IMP=	86.00
ADD HYD	206.39	1& 5	6	.25890	376.50	15.625	1.13156	1.500	2.272		
FINISH											

EL CAMINO FUTURE CONDITION 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = ctec100.fut

RUN DATE (MON/DAY/YR) = 11/05/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											
RAINFALL TYPE= 2										TIME=	.00
COMPUTE NM HYD	201.00	-	1	.13390	207.70	6.497	.90979	1.500	2.424	RAIN24=	3.370
RAINFALL TYPE= 2										PER IMP=	5.00
COMPUTE NM HYD	200.00	-	3	.30300	487.60	15.384	.95201	1.550	2.514	RAIN24=	3.500
*S EL CAMINO ARROYO AT TRAMWAY BLVD (200.0)										PER IMP=	.00
ROUTE MCUNGE	200.80	3	4	.30300	453.59	15.176	.93914	1.700	2.339	CCODE =	.1
ADD HYD	201.90	4& 1	5	.43690	559.37	21.674	.93014	1.650	2.000		
*S EL CAMINO ARROYO (MAIN) AT BROWNING (201.9)											
ROUTE MCUNGE	201.80	5	2	.43690	534.38	21.592	.92664	1.800	1.911	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.250
COMPUTE NM HYD	202.10	-	1	.10310	223.44	7.813	1.42091	1.500	3.386	PER IMP=	17.00
ADD HYD	202.19	1& 2	5	.54000	615.10	29.405	1.02101	1.750	1.780		
*S EL CAMINO ARROYO (MAIN) AT EUBANK (202.19)											
ROUTE MCUNGE	202.18	5	2	.54000	591.52	29.266	1.01619	1.850	1.712	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.190
COMPUTE NM HYD	202.20	-	1	.10990	229.39	8.219	1.40216	1.500	3.261	PER IMP=	17.00
ADD HYD	202.29	1& 2	5	.64990	652.42	37.485	1.08146	1.850	1.569		
*S EL CAMINO ARROYO (MAIN) AT HOLBROOK (202.29)											
ROUTE MCUNGE	202.28	5	2	.64990	647.14	37.484	1.08143	1.950	1.556	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	3.070
COMPUTE NM HYD	202.30	-	1	.05186	118.89	4.423	1.59917	1.500	3.582	PER IMP=	30.00
ADD HYD	202.39	1& 2	6	.70176	673.28	41.907	1.11969	1.900	1.499		
*S EL CAMINO ARROYO (MAIN) AT VENTURA (202.39)											
RAINFALL TYPE= 2										RAIN24=	3.140
COMPUTE NM HYD	203.10	-	1	.12580	257.46	9.143	1.36267	1.500	3.198	PER IMP=	17.00
*S EL CAMINO ARROYO (TRIBUTARY) AT HOLBROOK (203.1)											
ROUTE MCUNGE	203.18	1	2	.12580	237.85	9.047	1.34848	1.650	2.954	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.070
COMPUTE NM HYD	203.20	-	1	.03946	90.47	3.366	1.59917	1.500	3.582	PER IMP=	30.00
ADD HYD	203.29	1& 2	5	.16526	289.97	12.413	1.40833	1.650	2.742		
ADD HYD	202.90	5& 6	5	.86702	797.80	54.320	1.17471	1.900	1.438		
ROUTE MCUNGE	202.80	5	2	.86702	786.15	54.278	1.17380	1.950	1.417	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.020
COMPUTE NM HYD	203.30	-	1	.12590	320.68	12.845	1.91302	1.500	3.980	PER IMP=	50.00
ADD HYD	203.39	1& 2	5	.99292	900.27	67.123	1.26753	1.700	1.417		
*S EL CAMINO ARROYO (MAIN) AT BARSTOW (203.39)											
ROUTE MCUNGE	203.98	5	2	.99292	891.55	67.080	1.26671	1.800	1.403	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.960
COMPUTE NM HYD	204.00	-	1	.10230	255.42	10.191	1.86790	1.500	3.901	PER IMP=	50.00
*S EL CAMINO ARROYO AT WYOMING (204.9)											
ADD HYD	204.90	1& 2	5	1.09522	977.51	77.271	1.32287	1.750	1.395		
ROUTE MCUNGE	204.80	5	2	1.09522	971.65	77.219	1.32198	1.850	1.386	CCODE =	.1
COMPUTE NM HYD	204.40	-	1	.02968	58.84	2.006	1.26739	1.500	3.098	PER IMP=	17.00
ROUTE MCUNGE	204.81	1	6	.02968	57.97	2.037	1.28687	1.550	3.052	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	2.910
COMPUTE NM HYD	204.30	-	1	.11120	224.48	7.507	1.26586	1.500	3.154	PER IMP=	17.00
ADD HYD	204.91	1& 6	10	.14088	267.70	9.544	1.27029	1.500	2.969		
ADD HYD	204.92	10& 2	5	1.23610	1047.47	86.764	1.31609	1.800	1.324		
*S (204.92=EL CAMINO ARROYO AT LOUISIANA)											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
ROUTE MCUNGE	204.82	5	3	1.23610	1046.91	86.713	1.31531	1.850	1.323	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.890
COMPUTE NM HYD	204.20	-	1	.06875	187.81	7.907	2.15643	1.500	4.268	PER IMP=	70.00
ADD HYD	204.29	1& 3	5	1.30485	1104.21	94.619	1.35963	1.850	1.322		
RAINFALL TYPE= 2										RAIN24=	2.910
COMPUTE NM HYD	204.10	-	1	.12880	239.10	10.225	1.48848	1.550	2.901	PER IMP=	30.00
ROUTE MCUNGE	204.18	1	2	.12880	236.02	10.215	1.48706	1.550	2.863	CCODE =	.2
ADD HYD	204.19	2& 5	5	1.43365	1213.28	104.834	1.37108	1.650	1.322		
*S EL CAMINO ARROYO AT CORONADO AIRPORT (204.29)											
ROUTE MCUNGE	204.28	5	2	1.43365	1211.93	104.826	1.37097	1.650	1.321	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	2.850
COMPUTE NM HYD	205.00	-	1	.05430	146.01	6.140	2.12030	1.500	4.202	PER IMP=	70.00
*S EL CAMINO ARROYO AT I-25 (205.90)											
ADD HYD	205.90	1& 2	5	1.48795	1294.36	110.966	1.39831	1.600	1.359		
RAINFALL TYPE= 2										RAIN24=	2.830
COMPUTE NM HYD	206.10	-	1	.12210	279.20	11.808	1.81320	1.500	3.573	PER IMP=	50.00
RAINFALL TYPE= 2										RAIN24=	2.830
COMPUTE NM HYD	206.20	-	2	.05610	137.40	5.425	1.81320	1.500	3.827	PER IMP=	50.00
ADD HYD	206.29	1& 2	5	.17820	416.60	17.233	1.81319	1.500	3.653		
ROUTE	206.28	5	3	.17820	407.60	17.233	1.81320	1.550	3.574		
RAINFALL TYPE= 2										RAIN24=	2.760
COMPUTE NM HYD	206.40	-	1	.03270	80.27	3.102	1.77859	1.500	3.836	PER IMP=	50.00
ROUTE	206.48	1	2	.03270	79.75	3.102	1.77860	1.500	3.811		
ADD HYD	206.49	2& 3	5	.21090	482.92	20.334	1.80783	1.550	3.578		
RAINFALL TYPE= 2										RAIN24=	2.760
COMPUTE NM HYD	206.30	-	1	.04800	138.49	5.921	2.31273	1.500	4.508	PER IMP=	86.00
ADD HYD	206.39	1& 5	6	.25890	619.79	26.255	1.90144	1.500	3.741		
FINISH											

LC DIVERSION EXISTING COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = lcdv10b.ext

RUN DATE (MON/DAY/YR) =11/05/1998
USER NO.= RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											
RAINFALL TYPE= 2										TIME=	.00
COMPUTE NM HYD	103.20	-	1	.06160	68.41	2.326	.70815	1.500	1.735	RAIN24=	2.370
ROUTE MCUNGE	103.80	1	2	.06160	63.03	2.316	.70501	1.600	1.599	PER IMP=	14.00
RAINFALL TYPE= 2										CCODE =	.1
COMPUTE NM HYD	103.40	-	1	.09530	92.00	2.969	.58416	1.500	1.508	RAIN24=	2.330
ADD HYD	103.49	1& 2	11	.15690	137.74	5.285	.63160	1.550	1.372	PER IMP=	10.00
*S SOUTH LACUEVA TRIB @ TRAMWAY BLVD (103.49)											
RAINFALL TYPE= 2										RAIN24=	2.330
COMPUTE NM HYD	103.50	-	1	.06250	94.68	3.605	1.08145	1.500	2.367	PER IMP=	35.00
ADD HYD	103.59	1&11	5	.21940	222.82	8.890	.75975	1.550	1.587		
ROUTE MCUNGE	103.48	5	2	.21940	206.68	8.801	.75213	1.700	1.472	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.300
COMPUTE NM HYD	104.10	-	1	.12000	132.31	5.006	.78225	1.500	1.723	PER IMP=	25.00
ADD HYD	104.19	1& 2	5	.33940	263.37	13.807	.76277	1.700	1.212		
*S SOUTH LACUEVA TRIB @ LOWELL (104.19)											
ROUTE MCUNGE	104.18	5	2	.33940	255.73	13.789	.76176	1.800	1.177	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.230
COMPUTE NM HYD	104.20	-	1	.08040	48.56	1.825	.42552	1.550	.944	PER IMP=	8.00
ADD HYD	104.29	1& 2	6	.41980	273.75	15.613	.69736	1.800	1.019		
*S SOUTH LACUEVA TRIB @ BROWNING (104.29)											
RAINFALL TYPE= 2										RAIN24=	2.500
COMPUTE NM HYD	103.00	-	1	.21200	184.92	6.637	.58699	1.550	1.363	PER IMP=	.00
ROUTE RESERVOIR	103.11	1	2	.21200	151.32	6.637	.58699	1.650	1.115	AC-FT=	.752
ROUTE MCUNGE	103.81	2	3	.21200	150.12	6.566	.58073	1.800	1.106	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.400
COMPUTE NM HYD	103.60	-	1	.06840	72.88	2.959	.81112	1.500	1.665	PER IMP=	26.00
ADD HYD	103.19	1& 3	5	.28040	172.67	9.525	.63693	1.800	.962		
RAINFALL TYPE= 2										RAIN24=	2.400
COMPUTE NM HYD	103.10	-	1	.22990	149.93	6.117	.49885	1.600	1.019	PER IMP=	.00
ROUTE MCUNGE	103.88	1	2	.22990	144.39	6.107	.49806	1.750	.981	CCODE =	.1
ADD HYD	103.99	5& 2	5	.51030	316.60	15.632	.57436	1.750	.969		
*S NORTH LACUEVA TRIB @ TRAMWAY BLVD (103.99)											
ROUTE MCUNGE	103.18	5	2	.51030	315.19	15.675	.57596	1.800	.965	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	2.270
COMPUTE NM HYD	105.10	-	1	.13820	123.03	4.265	.57864	1.500	1.391	PER IMP=	15.00
ADD HYD	105.19	1& 2	5	.64850	348.40	19.940	.57653	1.800	.839		
*S NORTH LACUEVA TRIB @ LOWELL (105.19)											
ROUTE MCUNGE	105.18	5	2	.64850	343.94	19.906	.57554	1.900	.829	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.220
COMPUTE NM HYD	105.20	-	1	.10580	73.17	2.369	.41977	1.500	1.081	PER IMP=	8.00
ADD HYD	105.29	1& 2	5	.75430	356.37	22.275	.55369	1.900	.738		
*S NORTH LACUEVA TRIB @ BROWNING (105.29)											
ADD HYD	105.90	5& 6	17	1.17410	604.74	37.888	.60506	1.850	.805		
*S TOTAL RUNOFF TO LACUEVA DIVERSION CHANNEL (105.9)											
FINISH											

LC DIVERSION EXISTING COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = lcdv100b.ext

RUN DATE (MON/DAY/YR) =11/05/1998
USER NO.= RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											
RAINFALL TYPE= 2										TIME=	.00
COMPUTE NM HYD	103.20	-	1	.06160	127.12	4.677	1.42358	1.500	3.224	RAIN24=	3.550
ROUTE MCUNGE	103.80	1	2	.06160	121.23	4.662	1.41909	1.600	3.075	PER IMP=	14.00
RAINFALL TYPE= 2										CCODE =	.1
COMPUTE NM HYD	103.40	-	1	.09530	184.28	6.348	1.24889	1.500	3.021	RAIN24=	3.500
ADD HYD	103.49	1& 2	11	.15690	289.35	11.010	1.31571	1.550	2.882	PER IMP=	10.00
*S SOUTH LACUEVA TRIB @ TRAMWAY BLVD (103.49)											
RAINFALL TYPE= 2											
COMPUTE NM HYD	103.50	-	1	.06250	157.65	6.408	1.92240	1.500	3.941	RAIN24=	3.500
ADD HYD	103.59	1&11	5	.21940	432.28	17.418	1.48853	1.500	3.079	PER IMP=	35.00
ROUTE MCUNGE	103.48	5	2	.21940	408.77	17.240	1.47338	1.650	2.911	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.450
COMPUTE NM HYD	104.10	-	1	.12000	251.45	9.569	1.49513	1.500	3.274	PER IMP=	25.00
ADD HYD	104.19	1& 2	5	.33940	553.42	26.809	1.48106	1.650	2.548		
*S SOUTH LACUEVA TRIB @ LOWELL (104.19)											
ROUTE MCUNGE	104.18	5	2	.33940	548.56	26.834	1.48244	1.650	2.525	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	3.350
COMPUTE NM HYD	104.20	-	1	.08040	114.02	4.326	1.00893	1.550	2.216	PER IMP=	8.00
ADD HYD	104.29	1& 2	6	.41980	634.68	31.160	1.39175	1.650	2.362		
*S SOUTH LACUEVA TRIB @ BROWNING (104.29)											
RAINFALL TYPE= 2											
COMPUTE NM HYD	103.00	-	1	.21200	365.76	14.423	1.27562	1.550	2.696	RAIN24=	3.750
ROUTE RESERVOIR	103.11	1	2	.21200	341.18	14.423	1.27562	1.600	2.515	PER IMP=	.00
ROUTE MCUNGE	103.81	2	3	.21200	329.31	14.386	1.27236	1.750	2.427	AC-FT=	1.951
RAINFALL TYPE= 2										CCODE =	.1
COMPUTE NM HYD	103.60	-	1	.06840	137.46	5.606	1.53665	1.500	3.140	RAIN24=	3.600
ADD HYD	103.19	1& 3	5	.28040	382.39	19.992	1.33683	1.750	2.131	PER IMP=	26.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	103.10	-	1	.22990	326.84	14.085	1.14876	1.600	2.221	RAIN24=	3.600
ROUTE MCUNGE	103.88	1	2	.22990	316.24	14.054	1.14622	1.700	2.149	PER IMP=	.00
ADD HYD	103.99	5& 2	5	.51030	694.78	34.046	1.25095	1.750	2.127	CCODE =	.1
*S NORTH LACUEVA TRIB @ TRAMWAY BLVD (103.99)											
ROUTE MCUNGE	103.18	5	2	.51030	672.61	33.939	1.24701	1.850	2.059	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.400
COMPUTE NM HYD	105.10	-	1	.13820	260.36	9.014	1.22294	1.500	2.944	PER IMP=	15.00
ADD HYD	105.19	1& 2	5	.64850	728.66	42.952	1.24188	1.850	1.756		
*S NORTH LACUEVA TRIB @ LOWELL (105.19)											
ROUTE MCUNGE	105.18	5	2	.64850	728.46	42.985	1.24281	1.850	1.755	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	3.330
COMPUTE NM HYD	105.20	-	1	.10580	172.94	5.646	1.00054	1.500	2.554	PER IMP=	8.00
ADD HYD	105.29	1& 2	5	.75430	763.62	48.630	1.20882	1.850	1.582		
*S NORTH LACUEVA TRIB @ BROWNING (105.29)											
ADD HYD	105.90	5& 6	17	1.17410	1245.48	79.791	1.27423	1.750	1.657		
*S TOTAL RUNOFF TO LACUEVA DIVERSION CHANNEL (105.9)											
FINISH											

LC DIVERSION FUTURE CWD. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = lcdv10x.fut

RUN DATE (MON/DAY/YR) = 11/05/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	103.20	-	1	.06160	87.21	3.311	1.00774	1.500	2.212		RAIN24= 2.370
ROUTE MCUNGE	103.80	1	2	.06160	86.26	3.315	1.00904	1.550	2.188		PER IMP= 29.00
RAINFALL TYPE= 2											CCODE = .2
COMPUTE NM HYD	103.40	-	1	.09530	142.27	5.497	1.08145	1.500	2.333		RAIN24= 2.330
ADD HYD	103.49	1& 2	11	.15690	217.05	8.812	1.05302	1.500	2.333		PER IMP= 35.00
*S SOUTH LACUEVA TRIB @ TRAMWAY BLVD (103.49)									2.162		
RAINFALL TYPE= 2											
COMPUTE NM HYD	103.50	-	1	.06250	94.68	3.605	1.08145	1.500	2.367		RAIN24= 2.330
ADD HYD	103.59	1&11	5	.21940	311.73	12.416	1.06112	1.500	2.220		PER IMP= 35.00
ROUTE MCUNGE	103.48	5	2	.21940	286.01	12.329	1.05366	1.650	2.037		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.300
COMPUTE NM HYD	104.10	-	1	.13670	203.74	7.422	1.01804	1.500	2.329		PER IMP= 30.00
ADD HYD	104.19	1& 2	5	.35610	396.93	19.751	1.03999	1.650	1.742		
*S SOUTH LACUEVA TRIB @ LOWELL (104.19)											
ROUTE MCUNGE	104.18	5	2	.35610	391.15	19.808	1.04297	1.700	1.716		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 2.230
COMPUTE NM HYD	104.20	-	1	.04710	50.46	1.877	.74734	1.550	1.674		PER IMP= 17.00
ADD HYD	104.29	1& 2	6	.40320	419.22	21.685	1.00844	1.700	1.625		
*S SOUTH LACUEVA TRIB @ BROWNING (104.29)											
RAINFALL TYPE= 2											
COMPUTE NM HYD	103.00	-	1	.21200	184.92	6.637	.58699	1.550	1.363		RAIN24= 2.500
ROUTE RESERVOIR	103.11	1	2	.21200	151.32	6.637	.58699	1.650	1.115		PER IMP= .00
ROUTE MCUNGE	103.81	2	3	.21200	150.12	6.566	.58073	1.800	1.106		AC-FT= .752
RAINFALL TYPE= 2											CCODE = .1
COMPUTE NM HYD	103.60	-	1	.06840	101.97	4.042	1.10801	1.500	2.329		RAIN24= 2.400
ADD HYD	103.19	1& 3	5	.28040	181.24	10.608	.70935	1.750	1.010		PER IMP= 35.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	103.10	-	1	.22990	149.93	6.117	.49885	1.600	1.019		RAIN24= 2.400
ROUTE MCUNGE	103.88	1	2	.22990	144.39	6.107	.49806	1.750	.981		PER IMP= .00
ADD HYD	103.99	5& 2	5	.51030	325.62	16.715	.61416	1.750	.997		CCODE = .1
*S NORTH LACUEVA TRIB @ TRAMWAY BLVD (103.99)											
ROUTE MCUNGE	103.18	5	2	.51030	323.28	16.780	.61656	1.850	.990		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 2.270
COMPUTE NM HYD	105.10	-	1	.13050	173.78	6.150	.88358	1.500	2.081		PER IMP= 25.00
ADD HYD	105.19	1& 2	5	.64080	377.37	22.930	.67094	1.750	.920		
*S NORTH LACUEVA TRIB @ LOWELL (105.19)											
ROUTE MCUNGE	105.18	5	2	.64080	375.00	23.002	.67305	1.850	.914		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 2.220
COMPUTE NM HYD	105.20	-	1	.10450	125.95	4.123	.73985	1.500	1.883		PER IMP= 17.00
ADD HYD	105.29	1& 2	5	.74530	404.61	27.126	.68242	1.800	.848		
*S NORTH LACUEVA TRIB @ BROWNING (105.29)											
ADD HYD	105.90	5& 6	17	1.14850	767.03	48.811	.79687	1.750	1.044		
*S TOTAL RUNOFF TO LACUEVA DIVERSION CHANNEL (105.9)											
FINISH											

LA CUEVA EXISTING COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = lc10bm.ext

RUN DATE (MON/DAY/YR) = 11/05/1998
USER NO. = RSTECNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	101.00	-	1	.60700	498.34	23.352	.72133	1.650	1.283		RAIN24= 2.700
ROUTE MCUNGE	101.80	1	2	.60700	465.97	23.106	.71372	1.950	1.199		PER IMP= .00
RAINFALL TYPE= 2											CCODE = .1
COMPUTE NM HYD	100.00	-	1	1.21400	598.79	45.333	.70016	1.850	.771		RAIN24= 2.800
ROUTE MCUNGE	100.80	1	3	1.21400	582.68	45.134	.69708	2.150	.750		PER IMP= .00
ADD HYD	101.88	3& 2	5	1.82100	963.69	68.239	.70263	2.000	.827		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.430
COMPUTE NM HYD	102.00	-	1	.87500	439.84	23.497	.50350	1.700	.785		PER IMP= .00
ADD HYD	102.90	1& 5	5	2.69600	1187.21	91.736	.63800	1.950	.688		
*S LA CUEVA TRIBUTARY	ARROYO @ TRAMWAY BLVD. (102.9)			(102.9)							
ROUTE MCUNGE	102.80	5	2	2.69600	1173.65	91.522	.63651	2.100	.680		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	107.20	-	1	.17200	95.27	3.958	.43146	1.550	.865		PER IMP= 9.00
ADD HYD	107.29	1& 2	4	2.86800	1188.43	95.480	.62421	2.100	.647		
*S NORTH LA CUEVA AT	GLENDALE AND BROWNING (107.29)			(107.29)							
RAINFALL TYPE= 2											RAIN24= 2.330
COMPUTE NM HYD	102.10	-	1	.09300	59.20	1.592	.32090	1.500	.995		PER IMP= .00
*S LA CUEVA TRIBUTARY	@ TRAMWAY BLVD.										
ROUTE MCUNGE	102.18	.1	2	.09300	50.75	1.482	.29872	1.800	.853		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	107.10	-	1	.18080	105.83	3.422	.35487	1.500	.915		PER IMP= 5.00
ADD HYD	107.19	1& 2	5	.27380	105.83	4.904	.33580	1.500	.604		
*S NORTH LA CUEVA (TRIB)	AT FLORENCE AND BROWNING (107.19)			(107.19)							
ADD HYD	107.90	5& 4	3	3.14180	1213.68	100.383	.59908	2.100	.604		
RAINFALL TYPE= 2											RAIN24= 2.270
COMPUTE NM HYD	106.00	-	1	.04360	35.83	1.337	.57515	1.500	1.284		PER IMP= 17.00
ROUTE MCUNGE	106.80	1	2	.04360	31.28	1.283	.55176	1.750	1.121		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.210
COMPUTE NM HYD	106.10	-	1	.11160	81.48	2.796	.46971	1.500	1.141		PER IMP= 10.00
ADD HYD	106.19	1& 2	6	.15520	82.80	4.079	.49275	1.500	.834		
*S LA CUEVA TRIBUTARY	ARROYO @ BROWNING (106.19)			(106.19)							
ADD HYD	107.99	3& 6	5	3.29700	1232.52	104.462	.59407	2.100	.584		
ROUTE MCUNGE	107.80	5	2	3.29700	1232.04	104.512	.59436	2.150	.584		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 2.150
COMPUTE NM HYD	109.00	-	1	.10060	69.18	2.314	.43128	1.500	1.075		PER IMP= 10.00
ADD HYD	109.90	1& 2	5	3.39760	1237.79	106.826	.58953	2.150	.569		
RAINFALL TYPE= 2											RAIN24= 2.170
COMPUTE NM HYD	108.00	-	7	.20550	127.09	4.801	.43804	1.550	.966		PER IMP= 10.00
ADD HYD	109.99	5& 7	5	3.60310	1251.87	111.627	.58089	2.150	.543		
*S LA CUEVA ARROYO @	EUBANK (MAIN) (109.99)			(109.99)							
ROUTE MCUNGE	109.88	5	2	3.60310	1234.25	111.421	.57982	2.300	.535		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.090
COMPUTE NM HYD	110.00	-	1	.16340	79.91	3.502	.40187	1.550	.764		PER IMP= 10.00
ADD HYD	110.90	1& 2	5	3.76650	1241.62	114.923	.57210	2.300	.515		
*S LA CUEVA ARROYO @	VENTURA (MAIN) (110.90)			(110.90)							
ROUTE MCUNGE	110.80	5	2	3.76650	1240.39	114.910	.57203	2.300	.515		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 2.010
COMPUTE NM HYD	111.10	-	1	.11940	73.86	2.453	.38525	1.500	.967		PER IMP= 10.00
ADD HYD	111.19	1& 2	5	3.88590	1244.16	117.363	.56629	2.300	.500		
COMPUTE NM HYD	111.00	-	1	.06740	31.33	1.044	.29052	1.500	.726		PER IMP= 5.00
ADD HYD	111.99	1& 5	5	3.95330	1245.96	118.408	.56159	2.300	.492		
*S LA CUEVA ARROYO @	BARSTOW (111.99)			(111.99)							
ROUTE MCUNGE	111.80	5	2	3.95330	1244.99	118.391	.56151	2.350	.492		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 1.970
COMPUTE NM HYD	112.10	-	1	.08940	129.98	5.152	1.08050	1.500	2.272		PER IMP= 50.00
COMPUTE NM HYD	112.20	-	3	.08260	110.08	4.342	.98553	1.500	2.082		PER IMP= 45.00
*S COMBINE HYD.'S	112.1 AND 112.2 AS 112.8										
ADD HYD	112.80	1& 3	1	.17200	240.06	9.493	1.03489	1.500	2.181		
ADD HYD	112.90	1& 2	5	4.12530	1256.29	127.885	.58125	2.350	.476		
*S LA CUEVA ARROYO @	WYOMING (112.90)			(112.90)							
ROUTE MCUNGE	112.80	5	2	4.12530	1244.04	127.813	.58093	2.450	.471		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 1.950
COMPUTE NM HYD	113.00	-	1	.11360	58.65	1.793	.29591	1.500	.807		PER IMP= 5.00
ADD HYD	113.90	1& 2	5	4.23890	1246.08	129.606	.57329	2.450	.459		
*S LA CUEVA MAIN ARROYO @	LOUISIANA (113.90)			(113.90)							
ROUTE MCUNGE	113.80	5	4	4.23890	1235.17	129.525	.57293	2.550	.455		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 1.900

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
COMPUTE NM HYD	115.00	-	3	.13370	64.79	1.981	.27786	1.500	.757	PER IMP= 5.00
ADD HYD	115.90	4& 3	5	4.37260	1236.96	131.506	.56391	2.550	.442	
*S LA CUEVA NORTH (MAIN) ARROYO @ I-25 (115.90)										
RECALL HYD	205.90	-	9	1.48790	243.55	32.441	.40881	2.200	.256	
ADD HYD	115.99	5& 9	5	5.86050	1443.56	163.947	.52453	2.550	.385	
ROUTE MCUNGE	115.80	5	2	5.86050	1436.37	163.933	.52449	2.600	.383	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 1.890
COMPUTE NM HYD	118.00	-	1	.06490	107.89	4.414	1.27523	1.500	2.598	PER IMP= 70.00
ADD HYD	118.90	1& 2	5	5.92540	1438.61	168.347	.53271	2.600	.379	
RAINFALL TYPE= 2										RAIN24= 1.890
COMPUTE NM HYD	118.10	-	1	.03060	16.20	.547	.33545	1.500	.827	PER IMP= 10.00
ADD HYD	118.99	1& 5	5	5.95600	1439.01	168.894	.53169	2.600	.378	
*S LA CUEVA NORTH (MAIN) ARROYO @ SAN MATEO										
ROUTE MCUNGE	118.80	5	2	5.95600	1436.19	168.839	.53152	2.600	.377	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 1.840
COMPUTE NM HYD	126.00	-	1	.07370	121.94	4.869	1.23882	1.500	2.585	PER IMP= 70.00
ADD HYD	126.90	1& 2	5	6.02970	1438.60	173.708	.54016	2.600	.373	
ROUTE MCUNGE	126.80	5	2	6.02970	1436.67	173.680	.54008	2.650	.372	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 1.800
COMPUTE NM HYD	127.00	-	1	.06330	75.56	2.120	.62810	1.500	1.865	PER IMP= 10.00
ADD HYD	127.90	1& 2	8	6.09300	1437.47	175.800	.54099	2.650	.369	
*S NORTH LA CUEVA ARROYO AT APPROXIMATELY JEFFERSON ST.										
*S NOW BEGIN THE SOUTH LA CUEVA ARROYO EAST OF I-25										
RAINFALL TYPE= 2										RAIN24= 1.900
COMPUTE NM HYD	116.00	-	5	.13090	77.34	2.783	.39870	1.500	.923	PER IMP= 15.00
*S DIVIDE HYD WITH 50 % TO CONVENIENCE CENTER POND										
DIVIDE HYD	116.10	5	10	.06545	38.67	1.392	.39870	1.500	.923	
	116.20	AND	11	.06545	38.67	1.392	.39870	1.500	.923	
*S SIGNAL HILL POND										
ROUTE RESERVOIR	116.18	10	7	.06545	14.31	1.392	.39870	1.750	.342	AC-FT= .572
*S ROUTE TO SAN PEDRO										
ROUTE	116.19	7	9	.06545	14.27	1.392	.39870	1.750	.341	
COMPUTE NM HYD	117.40	-	1	.07500	37.59	1.292	.32308	1.500	.783	PER IMP= 10.00
ROUTE MCUNGE	117.45	1	2	.07500	34.13	1.257	.31427	1.650	.711	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 1.930
COMPUTE NM HYD	117.31	-	17	.02500	36.12	1.394	1.04565	1.500	2.258	PER IMP= 50.00
*S ROUTE THRU SONORA POND										
ROUTE RESERVOIR	117.35	17	20	.02500	13.23	1.401	1.05042	1.750	.827	AC-FT= .468
COMPUTE NM HYD	117.32	-	18	.00900	13.01	.502	1.04565	1.500	2.259	PER IMP= 50.00
*S SIGNAL HILL POND										
ROUTE RESERVOIR	117.36	18	17	.00900	5.14	.502	1.04561	1.700	.892	AC-FT= .163
*S ADD 2 POND DISCHARGES										
ADD HYD	117.37	20&17	17	.03400	18.37	1.902	1.04914	1.750	.844	
*S ROUTE TO SAN PEDRO 30" PIPE										
ROUTE	117.39	17	20	.03400	18.36	1.902	1.04916	1.750	.844	
COMPUTE NM HYD	117.30	-	3	.08630	58.74	2.044	.44400	1.500	1.063	PER IMP= 15.00
*S TOTAL Q TO SAN PEDRO& SIGNAL SD + STREET (117.33)										
ADD HYD	117.33	20& 3	6	.12030	71.36	3.946	.61503	1.500	.927	
*S ADD IN 117.45 AT ALAMEDA 117.41 TO SD 117.42 TO ALAMEDA INTERSECTION										
DIVIDE HYD	117.41	2	7	.06402	20.00	1.073	.31426	1.600	.488	
	117.42	AND	4	.01098	14.13	.184	.31426	1.650	2.011	
*S ADD IN Q TO SD AT ALAMEDA 117.44										
ADD HYD	117.44	7&20	20	.09802	38.36	2.975	.56916	1.750	.612	
COMPUTE NM HYD	117.50	-	1	.05500	95.92	3.907	1.33209	1.500	2.725	PER IMP= 70.00
*S ADD IN Q TO ALAMEDA FROM 117.42 AS 117.55										
ADD HYD	117.44	1& 4	1	.06598	95.92	4.091	1.16272	1.500	2.272	
*S COMBINE HYDS 117.3 AND 117.55 AS TOTAL ALAMEDA INT Q (117.59)										
ADD HYD	117.59	3& 1	2	.15228	154.66	6.135	.75541	1.500	1.587	
*S BASIN 117.21 TO RETENTION BASIN										
COMPUTE NM HYD	117.21	-	1	.02340	33.81	1.305	1.04565	1.500	2.258	PER IMP= 50.00
COMPUTE NM HYD	117.20	-	1	.13910	69.79	3.624	.48856	1.600	.784	PER IMP= 20.00
DIVIDE HYD	117.92	1	6	.13910	69.79	3.624	.48856	1.600	.784	
	117.27	AND	10	.00000	.00	.000	.00000	-.050	.000	
*S ADD IN Q AT OAKLEND/EAGLE ROCK (117.93)										
ADD HYD	117.93	20& 6	1	.23712	106.85	6.600	.52188	1.600	.704	
*S ADD CONVENIENCE CENTER FLOW (117.94)										
ADD HYD	117.94	1& 9	1	.30257	118.15	7.992	.49523	1.600	.610	
*S TOTAL RUNOFF TO SAN PEDRO STORM DRAIN										
ROUTE	117.98	1	2	.30257	114.40	7.992	.49523	1.650	.591	
*S COMBINE HYDS 117.98 AND 116.2 (116.99)										
ADD HYD	116.99	11& 2	5	.36802	136.51	9.383	.47806	1.650	.580	
*S SOUTH LACUEVA ARROYO AT CONFLUENCE JUST WEST OF I-25 CROSSING										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3	NOTATION
ADD HYD	117.29	10& 5	5	.36802	136.51	9.383	.47806	1.650	.580		
ROUTE MCUNGE	117.28	5	2	.36802	133.53	9.371	.47745	1.700	.567		
RAINFALL TYPE= 2											
COMPUTE NM HYD	119.00	-	1	.05490	91.31	3.734	1.27523	1.500	2.599		CCODE = .1
ADD HYD	119.90	1& 2	5	.42292	174.23	13.105	.58101	1.650	.644		RAIN24= 1.890
RAINFALL TYPE= 2											PER IMP= 70.00
COMPUTE NM HYD	120.00	-	1	.02680	30.60	1.281	.89639	1.500	1.784		RAIN24= 1.880
ADD HYD	120.90	1& 5	5	.44972	190.83	14.386	.59980	1.650	.663		PER IMP= 50.00
ROUTE MCUNGE	120.80	5	2	.44972	190.83	14.395	.60017	1.600	.663		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 1.840
COMPUTE NM HYD	125.00	-	1	.05080	83.53	3.351	1.23688	1.500	2.569		PER IMP= 70.00
ADD HYD	125.90	1& 2	6	.50052	256.83	17.746	.66479	1.550	.802		
*S LA CUEVA ARROYO SOUTH BEFORE WILDFLOWER FLOWS JUST UPSTREAM OF 96" RCP											
COMPUTE NM HYD	121.00	-	1	.04890	21.36	.664	.25476	1.500	.683		PER IMP= 5.00
*S WILDFLOWER ARROYO AT SAN MATEO BLVD.											
DIVIDE HYD	121.83	1	11	.02445	10.68	.332	.25475	1.500	.683		
	121.84	AND	12	.02445	10.68	.332	.25475	1.500	.683		
ROUTE MCUNGE	121.81	11	2	.02445	9.50	.332	.25477	1.750	.607		CCODE = .1
COMPUTE NM HYD	123.00	-	1	.01850	22.92	.836	.84750	1.500	1.936		PER IMP= 40.00
ADD HYD	123.90	1& 2	5	.04295	23.47	1.168	.51006	1.500	.854		
ROUTE MCUNGE	121.82	12	2	.02445	10.38	.319	.24484	1.700	.663		CCODE = .2
COMPUTE NM HYD	122.00	-	1	.02250	31.38	1.184	.98695	1.500	2.179		PER IMP= 50.00
ADD HYD	122.90	1& 2	3	.04695	32.29	1.504	.60046	1.500	1.075		
COMPUTE NM HYD	124.00	-	1	.02510	35.00	1.321	.98695	1.500	2.179		PER IMP= 50.00
ADD HYD	124.91	3& 1	4	.07205	67.29	2.825	.73510	1.500	1.459		
DIVIOE HYD	122.85	4	1	.07205	67.29	2.825	.73510	1.500	1.459		
	124.92	AND	2	.00000	.00	.000	.00000	-.050	.000		
*S TOTAL OUTFLOW TO ALAMEDA FROM JEFFERSON VIA 36" STORM DRAIN.											
ROUTE MCUNGE	124.83	2	3	.04695	.00	.000	.00000	-.050	.000		CCODE = .2
ADD HYD	124.91	5& 3	4	.08990	23.47	1.168	.24368	1.500	.408		
*S SUBTRACT FLOW INTERCEPTED BY 36" PIPE TO WATERCRESS CHANNEL											
DIVIDE HYD	123.81	4	1	.08990	23.47	1.168	.24368	1.500	.408		
	123.99	AND	2	.00000	.00	.000	.00000	-.050	.000		
ADD HYD	125.99	6& 1	3	.59042	278.96	18.914	.60067	1.550	.738		
*S TOTAL FLOW AT ENTRANCE TO 96" PIPE											
ROUTE	125.88	3'	1	.59042	281.57	18.915	.60067	1.550	.745		
ADD HYD	127.91	1& 8	8	6.68342	1467.70	194.715	.54626	2.600	.343		
ADD HYD	127.92	2& 8	5	6.68342	1467.70	194.715	.54626	2.600	.343		
RAINFALL TYPE= 2											
COMPUTE NM HYD	128.00	-	1	.13730	136.97	4.592	.62710	1.550	1.559		RAIN24= 1.790
ADD HYD	128.90	1& 5	5	6.82072	1471.34	199.307	.54789	2.600	.337		PER IMP= 10.00
*S LA CUEVA ARROYO TOTAL @ THE NORTH DIVERSION CHANNEL											
FINISH											

LA CUEVA EXISTING COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = lc100bm.ext

RUN DATE (MON/DAY/YR) = 11/05/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
START										
RAINFALL TYPE= 2										TIME= .00
COMPUTE NM HYD	101.00	-	1	.60700	953.84	45.841	1.41600	1.650	2.455	RAIN24= 4.050
ROUTE MCUNGE	101.80	1	2	.60700	912.90	45.469	1.40451	1.850	2.350	PER IMP= .00
RAINFALL TYPE= 2										CCODE = .1
COMPUTE NM HYD	100.00	-	1	1.21400	1201.70	87.881	1.35730	1.850	1.547	RAIN24= 4.200
ROUTE MCUNGE	100.80	1	3	1.21400	1177.21	87.700	1.35451	2.100	1.515	PER IMP= .00
ADD HYD	101.88	3 & 2	5	1.82100	1882.13	133.168	1.37117	1.950	1.615	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	102.00	-	1	.87500	1033.60	54.313	1.16386	1.700	1.846	RAIN24= 3.650
ADD HYD	102.90	1 & 5	5	2.69600	2554.30	187.482	1.30389	1.900	1.480	PER IMP= .00
*S LA CUEVA TRIBUTARY	ARROYO @ TRAMWAY BLVD. (102.9)									
ROUTE MCUNGE	102.80	5	2	2.69600	2515.07	187.100	1.30123	2.050	1.458	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.370
COMPUTE NM HYD	107.20	-	1	.17200	237.40	9.430	1.02797	1.550	2.157	PER IMP= 9.00
ADD HYD	107.29	1 & 2	4	2.86800	2552.53	196.529	1.28484	2.050	1.391	
*S NORTH LA CUEVA AT	GLENDALE AND BROWNING (107.29)									
RAINFALL TYPE= 2										
COMPUTE NM HYD	102.10	-	1	.09300	147.55	4.405	.88811	1.500	2.479	RAIN24= 3.500
*S LA CUEVA TRIBUTARY	@ TRAMWAY BLVD.									PER IMP= .00
ROUTE MCUNGE	102.18	1	2	.09300	134.74	4.349	.87687	1.700	2.264	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.380
COMPUTE NM HYD	107.10	-	1	.18080	280.74	8.922	.92528	1.500	2.426	PER IMP= 5.00
ADD HYD	107.19	1 & 2	5	.27380	303.70	13.271	.90883	1.600	1.733	
*S NORTH LA CUEVA (TRIB)	AT FLORENCE AND BROWNING (107.19)									
ADD HYD	107.90	5 & 4	3	3.14180	2611.52	209.801	1.25207	2.000	1.299	
RAINFALL TYPE= 2										
COMPUTE NM HYD	106.00	-	1	.04360	77.44	2.829	1.21672	1.500	2.775	RAIN24= 3.400
ROUTE MCUNGE	106.80	1	2	.04360	69.78	2.777	1.19435	1.700	2.501	PER IMP= 17.00
RAINFALL TYPE= 2										CCODE = .1
COMPUTE NM HYD	106.10	-	1	.11160	187.37	6.465	1.08624	1.500	2.623	RAIN24= 3.340
ADD HYD	106.19	1 & 2	6	.15520	196.76	9.243	1.11660	1.600	1.981	PER IMP= 10.00
*S LA CUEVA TRIBUTARY	ARROYO @ BROWNING (106.19)									
ADD HYD	107.99	3 & 6	5	3.29700	2658.92	219.043	1.24570	2.000	1.260	
ROUTE MCUNGE	107.80	5	2	3.29700	2647.18	218.906	1.24491	2.100	1.255	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.230
COMPUTE NM HYD	109.00	-	1	.10060	164.16	5.441	1.01402	1.500	2.550	PER IMP= 10.00
ADD HYD	109.90	1 & 2	5	3.39760	2662.81	224.346	1.23808	2.100	1.225	
RAINFALL TYPE= 2										
COMPUTE NM HYD	108.00	-	7	.20550	311.04	11.246	1.02605	1.550	2.365	RAIN24= 3.250
ADD HYD	109.99	5 & 7	5	3.60310	2698.71	235.592	1.22598	2.100	1.170	PER IMP= 10.00
*S LA CUEVA ARROYO @	EUBANK (MAIN) (109.99)									
ROUTE MCUNGE	109.88	5	2	3.60310	2673.21	235.416	1.22507	2.200	1.159	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.130
COMPUTE NM HYD	110.00	-	1	.16340	202.35	8.385	.96218	1.550	1.935	PER IMP= 10.00
ADD HYD	110.90	1 & 2	5	3.76650	2695.65	243.801	1.21366	2.200	1.118	
*S LA CUEVA ARROYO @	VENTURA (MAIN) (110.90)									
ROUTE MCUNGE	110.80	5	2	3.76650	2692.04	243.873	1.21402	2.200	1.117	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 3.020
COMPUTE NM HYD	111.10	-	1	.11940	184.36	5.962	.93630	1.500	2.413	PER IMP= 10.00
ADD HYD	111.19	1 & 2	5	3.88590	2703.93	249.835	1.20549	2.200	1.087	
RAINFALL TYPE= 2										
COMPUTE NM HYD	111.00	-	1	.06740	88.80	2.940	.81779	1.500	2.059	RAIN24= 3.050
ADD HYD	111.99	1 & 5	5	3.95330	2710.28	252.775	1.19888	2.200	1.071	PER IMP= 5.00
*S LA CUEVA ARROYO @	BARSTOW (111.99)									
ROUTE MCUNGE	111.80	5	2	3.95330	2706.82	251.708	1.19382	2.229	1.070	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 2.960
COMPUTE NM HYD	112.10	-	1	.08940	219.11	8.942	1.87543	1.500	3.829	PER IMP= 50.00
COMPUTE NM HYD	112.20	-	3	.08260	191.10	7.690	1.74561	1.500	3.615	PER IMP= 45.00
*S COMBINE HYD.'S	112.1 AND 112.2 AS 112.8									
ADD HYD	112.80	1 & 3	1	.17200	410.21	16.632	1.81308	1.500	3.726	
ADD HYD	112.90	1 & 2	5	4.12530	2737.52	267.687	1.21667	2.229	1.037	
*S LA CUEVA ARROYO @	WYOMING (112.90)									
ROUTE MCUNGE	112.80	5	2	4.12530	2732.89	267.664	1.21657	2.273	1.035	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 2.930
COMPUTE NM HYD	113.00	-	1	.11360	160.25	4.875	.80462	1.500	2.204	PER IMP= 5.00
ADD HYD	113.90	1 & 2	5	4.23890	2740.90	272.538	1.20552	2.273	1.010	
*S LA CUEVA MAIN ARROYO @	LOUISIANA (113.90)									
ROUTE MCUNGE	113.80	5	4	4.23890	2736.89	272.527	1.20547	2.300	1.009	CCODE = .2

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	115.00	-	3	.13370	183.65	5.503	.77170	1.500	2.146	RAIN24= 2.850
ADD HYD	115.90	4& 3	5	4.37260	2745.20	278.030	1.19221	2.300	.981	PER IMP= 5.00
*S LA CUEVA NORTH (MAIN) ARROYO @ I-25 (115.90)										
RECALL HYD	205.90	-	9	1.48790	864.92	75.764	.95475	1.901	.908	
ADD HYD	115.99	5& 9	5	5.86050	3388.92	353.273	1.13026	2.266	.904	
ROUTE MCUNGE	115.80	5	2	5.86050	3384.43	352.332	1.12725	2.303	.902	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	118.00	-	1	.06490	172.75	7.273	2.10117	1.500	4.159	RAIN24= 2.830
ADD HYD	118.90	1& 2	5	5.92540	3393.77	359.096	1.13630	2.303	.895	PER IMP= 70.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	118.10	-	1	.03060	42.12	1.370	.83948	1.500	2.150	RAIN24= 2.830
ADD HYD	118.99	1& 5	5	5.95600	3395.86	360.432	1.13467	2.303	.891	PER IMP= 10.00
*S LA CUEVA NORTH (MAIN) ARROYO @ SAN MATEO										
ROUTE MCUNGE	118.80	5	2	5.95600	3393.69	359.673	1.13228	2.316	.890	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	126.00	-	1	.07370	195.81	8.059	2.05022	1.500	4.151	RAIN24= 2.760
ADD HYD	126.90	1& 2	5	6.02970	3403.69	367.139	1.14166	2.316	.882	PER IMP= 70.00
ROUTE MCUNGE	126.80	5	2	6.02970	3401.46	367.136	1.14165	2.335	.881	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	127.00	-	1	.06330	134.62	4.342	1.28604	1.500	3.323	RAIN24= 2.700
ADD HYD	127.90	1& 2	8	6.09300	3406.55	371.409	1.14294	2.335	.874	PER IMP= 10.00
*S NORTH LA CUEVA ARROYO AT APPROXIMATELY JEFFERSON ST.										
*S NOW BEGIN THE SOUTH LA CUEVA ARROYO EAST OF I-25										
RAINFALL TYPE= 2										
COMPUTE NM HYD	116.00	-	5	.13090	193.62	6.460	.92539	1.500	2.311	RAIN24= 2.850
*S DIVIDE HYD WITH 50 % TO CONVENIENCE CENTER POND										
DIVIDE HYD	116.10	5	10	.06545	96.81	3.230	.92539	1.500	2.311	PER IMP= 15.00
	116.20	AND	11	.06545	96.81	3.230	.92539	1.500	2.311	
*S SIGNAL HILL POND										
ROUTE RESERVOIR	116.18	10	7	.06545	30.65	3.230	.92539	1.750	.732	AC-FT= 1.508
*S ROUTE TO SAN PEDRO										
ROUTE	116.19	7	9	.06545	30.64	3.230	.92539	1.800	.731	
RAINFALL TYPE= 2										
COMPUTE NM HYD	117.40	-	1	.07500	102.09	3.335	.83378	1.500	2.127	RAIN24= 2.890
ROUTE MCUNGE	117.45	1	2	.07500	94.34	3.293	.82330	1.600	1.965	PER IMP= 10.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	117.31	-	17	.02500	60.81	2.418	1.81357	1.500	3.801	CCODE = .1
*S ROUTE THRU SONORA POND										
ROUTE RESERVOIR	117.35	17	20	.02500	21.16	2.423	1.81746	1.800	1.323	RAIN24= 2.890
COMPUTE NM HYD	117.32	-	18	.00900	21.90	.871	1.81357	1.500	3.803	PER IMP= 50.00
*S SIGNAL HILL POND										
ROUTE RESERVOIR	117.36	18	17	.00900	6.23	.870	1.81354	1.850	1.082	AC-FT= .359
*S ADD 2 POND DISCHARGES										
ADD HYD	117.37	20&17	17	.03400	27.39	3.294	1.81641	1.800	1.259	
*S ROUTE TO SAN PEDRO 30" PIPE										
ROUTE	117.39	17	20	.03400	27.40	3.294	1.81643	1.800	1.259	
COMPUTE NM HYD	117.30	-	3	.08630	136.26	4.588	.99691	1.500	2.467	PER IMP= 15.00
*S TOTAL Q TO SAN PEDRO& SIGNAL SD + STREET (117.33)										
ADD HYD	117.33	20& 3	6	.12030	153.48	7.882	1.22852	1.500	1.993	
*S ADD IN 117.45 AT ALAMEDA 117.41 TO SD 117.42 TO ALAMEDA INTERSECTION										
DIVIDE HYD	117.41	2	7	.03888	20.00	1.707	.82330	1.500	.804	
	117.42	AND	4	.03612	74.34	1.586	.82330	1.600	3.215	
*S ADD IN Q TO SD AT ALAMEDA 117.44										
ADD HYD	117.44	7&20	20	.07288	47.40	5.001	1.28664	1.800	1.016	
COMPUTE NM HYD	117.50	-	1	.05500	151.76	6.417	2.18755	1.500	4.311	PER IMP= 70.00
*S ADD IN Q TO ALAMEDA FROM 117.42 AS 117.55										
ADD HYD	117.44	1& 4	1	.09112	187.75	8.003	1.64671	1.550	3.219	
*S COMBINE HYDS 117.3 AND 117.55 AS TOTAL ALAMEDA INT Q (117.59)										
ADD HYD	117.59	3& 1	2	.17742	314.46	12.591	1.33064	1.550	2.769	
RAINFALL TYPE= 2										
*S BASIN 117.21 TO RETENTION BASIN										
COMPUTE NM HYD	117.21	-	1	.02340	56.92	2.263	1.81357	1.500	3.801	RAIN24= 2.890
COMPUTE NM HYD	117.20	-	1	.13910	160.02	7.778	1.04839	1.600	1.797	PER IMP= 50.00
DIVIDE HYD	117.92	1	6	.10519	70.00	5.881	1.04838	1.450	1.040	PER IMP= 20.00
	117.27	AND	10	.03391	90.02	1.896	1.04838	1.600	4.147	
*S ADD IN Q AT OAKLEND/EAGLE ROCK (117.93)										
ADD HYD	117.93	20& 6	1	.17806	117.40	10.882	1.14589	1.800	1.030	
*S ADD CONVENIENCE CENTER FLOW (117.94)										
ADD HYD	117.94	1& 9	1	.24351	148.04	14.112	1.08662	1.800	.950	
*S TOTAL RUNOFF TO SAN PEDRO STORM DRAIN										
ROUTE	117.98	1	2	.24351	147.87	14.112	1.08663	1.850	.949	
*S COMBINE HYDS 117.98 AND 116.2 (116.99)										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ADD HYD	116.99	118 2	5	.30896	211.66	17.343	1.05247	1.550	1.070	
*S SOUTH LACUEVA ARROYO AT CONFLUENCE JUST WEST OF I-25 CROSSING										
ADD HYD	117.29	108 5	5	.34288	296.16	19.239	1.05206	1.600	1.350	
ROUTE MCUNGE	117.28	5	2	.34288	296.02	18.933	1.03535	1.601	1.349	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 2.830
COMPUTE NM HYD	119.00	-	1	.05490	146.12	6.152	2.10117	1.500	4.159	PER IMP= 70.00
ADD HYD	119.90	18 2	5	.39778	407.62	24.933	1.17526	1.568	1.601	
RAINFALL TYPE= 2										RAIN24= 2.820
COMPUTE NM HYD	120.00	-	1	.02680	55.08	2.259	1.58045	1.500	3.211	PER IMP= 50.00
ADD HYD	120.90	18 5	5	.42458	454.55	27.138	1.19847	1.535	1.673	
ROUTE MCUNGE	120.80	5	2	.42458	449.48	26.684	1.17842	1.582	1.654	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.760
COMPUTE NM HYD	125.00	-	1	.05080	134.25	5.548	2.04765	1.500	4.129	PER IMP= 70.00
ADD HYD	125.90	18 2	6	.47538	551.24	31.991	1.26182	1.582	1.812	
*S LA CUEVA ARROYO SOUTH BEFORE WILDFLOWER FLOWS JUST UPSTREAM OF 96" RCP										
RAINFALL TYPE= 2										RAIN24= 2.810
COMPUTE NM HYD	121.00	-	1	.04890	63.24	1.947	.74674	1.500	2.021	PER IMP= 5.00
*S WILDFLOWER ARROYO AT SAN MATEO BLVD.										
DIVIDE HYD	121.83	1	11	.02445	31.62	.974	.74673	1.500	2.021	
	121.84	AND	12	.02445	31.62	.974	.74673	1.500	2.021	
ROUTE MCUNGE	121.81	11	2	.02445	29.40	.952	.73022	1.650	1.879	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.750
COMPUTE NM HYD	123.00	-	1	.01850	40.84	1.521	1.54168	1.500	3.449	PER IMP= 40.00
ADD HYD	123.90	18 2	5	.04295	59.16	2.473	1.07972	1.550	2.152	
ROUTE MCUNGE	121.82	12	2	.02445	28.91	.957	.73427	1.650	1.847	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.750
COMPUTE NM HYD	122.00	-	1	.02250	53.55	2.068	1.72356	1.500	3.719	PER IMP= 50.00
ADD HYD	122.90	18 2	3	.04695	62.44	3.026	1.20836	1.600	2.078	
RAINFALL TYPE= 2										RAIN24= 2.740
COMPUTE NM HYD	124.00	-	1	.02510	59.74	2.301	1.71856	1.500	3.719	PER IMP= 50.00
ADD HYD	124.91	38 1	4	.07205	115.71	5.326	1.38609	1.500	2.509	
DIVIDE HYD	122.85	4	1	.06779	90.00	5.012	1.38609	1.450	2.074	
	124.92	AND	2	.00426	25.71	.315	1.38609	1.500	9.437	
*S TOTAL OUTFLOW TO ALAMEDA FROM JEFFERSON VIA 36" STORM DRAIN.										
ROUTE MCUNGE	124.83	2	3	.00426	23.96	.294	1.29335	1.600	8.795	CCODE = .1
ADD HYD	124.91	58 3	4	.04721	82.22	2.767	1.09897	1.600	2.722	
*S SUBTRACT FLOW INTERCEPTED BY 36" PIPE TO WATERCRESS CHANNEL										
DIVIDE HYD	123.81	4	1	.04584	70.00	2.687	1.09897	1.550	2.386	
	123.99	AND	2	.00136	12.22	.080	1.09897	1.600	14.016	
ADD HYD	125.99	68 1	3	.52122	621.24	34.619	1.24535	1.582	1.862	
*S TOTAL FLOW AT ENTRANCE TO 96" PIPE										
ROUTE	125.88	3	1	.52122	620.76	34.625	1.24559	1.600	1.861	
ADD HYD	127.91	18 8	8	6.61422	3518.59	405.173	1.14858	2.316	.831	
ADD HYD	127.92	28 8	5	6.61558	3518.59	405.251	1.14857	2.316	.831	
RAINFALL TYPE= 2										RAIN24= 2.690
COMPUTE NM HYD	128.00	-	1	.13730	254.82	9.454	1.29103	1.550	2.900	PER IMP= 10.00
ADD HYD	128.90	18 5	5	6.75288	3535.11	414.559	1.15106	2.316	.818	
*S LA CUEVA ARROYO TOTAL @ THE NORTH DIVERSION CHANNEL FINISH										

LA CUEVA FUTURE CONDITION 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = lc10bm.fut

RUN DATE (MON/DAY/YR) = 11/06/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	101.00	-	1	.60700	498.34	23.352	.72133	1.650	1.283		RAIN24= 2.700
ROUTE MCUNGE	101.80	1	2	.60700	465.97	23.106	.71372	1.950	1.199		PER IMP= .00
RAINFALL TYPE= 2											CCODE = .1
COMPUTE NM HYD	100.00	-	1	1.21400	598.79	45.333	.70016	1.850	.771		RAIN24= 2.800
ROUTE MCUNGE	100.80	1	3	1.21400	582.68	45.134	.69708	2.150	.750		PER IMP= .00
ADD HYD	101.88	3& 2	5	1.82100	963.69	68.239	.70263	2.000	.827		CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	102.00	-	1	.87500	392.78	20.905	.44797	1.700	.701		RAIN24= 2.430
ADD HYD	102.90	1& 5	5	2.69600	1166.38	89.145	.61998	2.000	.676		PER IMP= .00
*S LA CUEVA TRIBUTARY	ARROYO @ TRAMWAY BLVD. (102.9)										
ROUTE MCUNGE	102.80	5	2	2.69600	1151.50	89.066	.61943	2.100	.667		CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	107.20	-	1	.17200	168.43	7.115	.77560	1.550	1.530		RAIN24= 2.450
ADD HYD	107.29	1& 2	4	2.86800	1176.56	96.181	.62880	2.100	.641		PER IMP= 17.00
*S NORTH LA CUEVA AT	GLENDALE AND BROWNING (107.29)										
RAINFALL TYPE= 2											
COMPUTE NM HYD	102.10	-	1	.09300	60.53	1.622	.32704	1.500	1.017		RAIN24= 2.330
*S LA CUEVA TRIBUTARY	@ TRAMWAY BLVD.										PER IMP= .00
ROUTE MCUNGE	102.18	1	2	.09300	51.38	1.502	.30282	1.800	.863		CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	107.10	-	1	.18080	208.11	7.151	.74160	1.500	1.798		RAIN24= 2.250
ADD HYD	107.19	1& 2	5	.27380	208.11	8.653	.59256	1.500	1.188		PER IMP= 17.00
*S NORTH LA CUEVA (TRIB) AT FLORENCE AND BROWNING (107.19)											
ADD HYD	107.90	5& 4	3	3.14180	1213.69	104.834	.62564	2.100	.604		
RAINFALL TYPE= 2											
COMPUTE NM HYD	106.00	-	1	.04360	52.14	1.732	.74501	1.500	1.869		RAIN24= 2.270
ROUTE MCUNGE	106.80	1	2	.04360	45.40	1.684	.72416	1.750	1.627		PER IMP= 17.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	106.10	-	1	.11160	126.34	4.342	.72952	1.500	1.769		RAIN24= 2.230
ADD HYD	106.19	1& 2	6	.15520	127.67	6.026	.72801	1.500	1.285		PER IMP= 17.00
*S LA CUEVA TRIBUTARY	ARROYO @ BROWNING (106.19)										
ADD HYD	107.99	3& 6	5	3.29700	1238.89	110.860	.63046	2.100	.587		
ROUTE MCUNGE	107.80	5	2	3.29700	1236.49	110.915	.63077	2.150	.586		CCODE = .2
RAINFALL TYPE= 2											
COMPUTE NM HYD	109.00	-	1	.10060	114.66	3.750	.69888	1.500	1.781		RAIN24= 2.150
ADD HYD	109.90	1& 2	5	3.39760	1245.19	114.665	.63279	2.150	.573		PER IMP= 17.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	108.00	-	7	.20550	207.96	7.759	.70798	1.550	1.581		RAIN24= 2.170
ADD HYD	109.99	5& 7	5	3.60310	1267.39	122.424	.63708	2.150	.550		PER IMP= 17.00
*S LA CUEVA ARROYO @	EUBANK (MAIN) (109.99)										
ROUTE MCUNGE	109.88	5	2	3.60310	1250.06	122.037	.63506	2.300	.542		CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	110.00	-	1	.16340	138.24	5.774	.66257	1.550	1.322		RAIN24= 2.080
ADD HYD	110.90	1& 2	5	3.76650	1261.77	127.811	.63625	2.300	.523		PER IMP= 17.00
*S LA CUEVA ARROYO @	VENTURA (MAIN) (110.90)- FINAL										
*S ROUTE TO BARSTOW											
ROUTE MCUNGE	110.88	5	2	3.76650	1257.14	127.065	.63254	2.321	.522		CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	111.00	-	1	.05330	57.02	1.823	.64118	1.500	1.672		RAIN24= 2.000
COMPUTE NM HYD	111.40	-	4	.01410	15.09	.482	.64118	1.500	1.672		PER IMP= 17.00
*S COMBINE HYD.'S	111.0	AND	111.4	AS 111.49							
ADD HYD	111.49	1& 4	4	.06740	72.12	2.305	.64117	1.500	1.672		
*S ROUTE TO LA CUEVA	CHANEL @ OAKLAND										
*S PIPE ROUTING											
ROUTE	111.48	4	5	.06740	70.85	2.305	.64118	1.550	1.642		
RAINFALL TYPE= 2											
COMPUTE NM HYD	111.10	-	1	.05000	57.41	2.054	.77030	1.500	1.794		RAIN24= 2.020
COMPUTE NM HYD	111.30	-	6	.04200	64.56	2.498	1.11532	1.500	2.402		PER IMP= 26.00
ADD HYD	111.39	1& 6	1	.09200	121.96	4.552	.92780	1.500	2.071		PER IMP= 50.00
*S COMBINE HYD.'S	111.48	AND	111.39	AS 111.88							
ADD HYD	111.88	1& 5	1	.15940	188.09	6.857	.80661	1.500	1.844		
*S LA CUEVA CHANNEL AT BARSTOW (111.89)-NOT FINAL											
ADD HYD	111.89	1& 2	5	3.92590	1266.37	133.807	.63906	2.321	.504		
*S DIVERSION FROM NDB	ABOVE CARRINGTON										
COMPUTE NM HYD	924.10	-	6	.02500	38.43	1.487	1.11533	1.500	2.402		PER IMP= 50.00
COMPUTE NM HYD	924.20	-	7	.01900	29.21	1.130	1.11532	1.500	2.402		PER IMP= 50.00
*S COMBINE HYD.'S	924.10	AND	924.20	AS 924.22							

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
ADD HYD	924.22	6 & 7	6	.04400	67.64	2.617	1.11531	1.500	2.402		
*S ROUTE TO BARSTOW IN ALAMEDA AS	924.28	6	2	.04400	64.86	2.617	1.11533	1.550	2.303		
ROUTE	111.99	5 & 2	5	3.96990	1270.00	136.367	.64407	2.321	.500		
*S LA CUEVA MAIN AT BARSTOW	111.80	5	2	3.96990	1266.79	136.407	.64426	2.345	.499	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	112.10	-	1	.08940	129.98	5.152	1.08050	1.500	2.272	RAIN24=	1.970
COMPUTE NM HYD	112.20	-	3	.08260	120.22	4.760	1.08050	1.500	2.274	PER IMP=	50.00
*S COMBINE HYD. 'S	112.1	AND	112.2	AS 112.8						PER IMP=	50.00
ADD HYD	112.80	1 & 3	1	.17200	250.20	9.912	1.08050	1.500	2.273		
ADD HYD	112.90	1 & 2	5	4.14190	1278.68	146.154	.66163	2.345	.482		
*S LA CUEVA ARROYO @ WYOMING (112.90)											
ROUTE MCUNGE	112.80	5	2	4.14190	1278.48	146.123	.66148	2.354	.482	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	113.00	-	1	.10000	159.65	6.393	1.19873	1.500	2.495	RAIN24=	1.950
COMPUTE NM HYD	116.10	-	7	.10280	164.05	6.572	1.19873	1.500	2.493	PER IMP=	60.00
ADD HYD	113.80	1 & 7	1	.20280	323.70	12.965	1.19872	1.500	2.494	PER IMP=	60.00
ADD HYD	113.90	1 & 2	5	4.34470	1292.43	158.918	.68583	2.354	.465		
*S LA CUEVA MAIN ARROYO @ LOUISIANA (113.90)											
ROUTE MCUNGE	113.80	5	4	4.34470	1288.56	158.931	.68588	2.400	.463	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	115.00	-	3	.12020	189.15	7.581	1.18257	1.500	2.459	RAIN24=	1.900
ADD HYD	115.90	4 & 3	5	4.46490	1295.73	166.512	.69926	2.400	.453	PER IMP=	62.00
*S LA CUEVA NORTH (MAIN) ARROYO @ I-25 (115.90)											
RECALL HYD	205.90	-	9	1.53490	814.33	59.816	.73069	1.600	.829		
ADD HYD	115.99	5 & 9	5	5.99980	1803.23	226.328	.70730	1.600	.470		
ROUTE MCUNGE	115.80	5	2	5.99980	1797.48	222.462	.69522	1.858	.468	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	118.00	-	1	.06490	115.66	4.475	1.29291	1.500	2.785	RAIN24=	1.890
ADD HYD	118.90	1 & 2	5	6.06470	1833.46	226.708	.70091	1.639	.472	PER IMP=	70.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	118.10	-	1	.03060	45.38	1.714	1.05011	1.500	2.317	RAIN24=	1.890
ADD HYD	118.99	1 & 5	5	6.09530	1860.53	228.351	.70244	1.612	.477	PER IMP=	50.00
*S LA CUEVA NORTH (MAIN) ARROYO @ SAN MATEO											
ROUTE MCUNGE	118.80	5	2	6.09530	1855.00	226.425	.69651	1.658	.476	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	126.00	-	1	.07370	121.94	4.869	1.23882	1.500	2.585	RAIN24=	1.840
ADD HYD	126.90	1 & 2	5	6.16900	1918.44	230.982	.70205	1.658	.486	PER IMP=	70.00
ROUTE MCUNGE	126.80	5	2	6.16900	1907.47	230.980	.70204	1.686	.483	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	127.00	-	1	.06330	102.87	4.070	1.20560	1.500	2.539	RAIN24=	1.800
ADD HYD	127.90	1 & 2	8	6.23230	1955.25	234.830	.70649	1.686	.490	PER IMP=	70.00
*S NORTH LA CUEVA ARROYO AT APPROXIMATELY JEFFERSON ST.											
*S NOW BEGIN THE SOUTH LA CUEVA ARROYO EAST OF I-25											
RAINFALL TYPE= 2											
COMPUTE NM HYD	117.40	-	1	.05120	80.83	3.225	1.18115	1.500	2.467	RAIN24=	1.930
*S ROUTE in alameda to san pedro										PER IMP=	60.00
ROUTE	117.49	1	2	.05120	75.83	3.225	1.18116	1.550	2.314		
COMPUTE NM HYD	117.30	-	1	.11720	167.85	6.536	1.04565	1.500	2.238	PER IMP=	50.00
ADD HYD	117.39	2 & 1	3	.16840	238.31	9.761	1.08685	1.500	2.211		
*S TOTAL Q AT ALAMEDA & SAN PEDRO (117.39) WITHOUT PONDS											
COMPUTE NM HYD	117.50	-	1	.05500	95.92	3.907	1.33209	1.500	2.725	PER IMP=	70.00
*S TOTAL Q AT ALAMEDA INT (117.5)											
*S ROUTE IN SAN PEDRO FROM ALAMEDA TO OAKLAND (117.38)											
ROUTE	117.38	3	2	.16840	234.01	9.761	1.08685	1.550	2.171		
COMPUTE NM HYD	117.20	-	1	.05000	72.23	2.788	1.04565	1.500	2.257	PER IMP=	50.00
*S COMBINE HYDS 117.38 AND 117.2 AT OAKLAND (117.69)											
ADD HYD	117.39	2 & 1	3	.21840	298.51	12.550	1.07741	1.550	2.136		
*S ROUTE FROM OAKLAND TO EAGLE ROCK (117.68)											
ROUTE	117.68	3	2	.21840	300.78	12.550	1.07742	1.550	2.152		
COMPUTE NM HYD	116.20	-	4	.07190	113.32	4.529	1.18116	1.500	2.463	PER IMP=	60.00
*S COMBINE HYDS 117.68 AND 116.2 AT EAGLE ROCK (116.29)											
ADD HYD	116.39	2 & 4	3	.29030	401.44	17.079	1.10311	1.500	2.161		
DIVIDE HYD	117.25	3	6	.21404	150.00	12.592	1.10311	1.400	1.095		
COMPUTE NM HYD	116.49	AND	7	.07626	251.44	4.487	1.10311	1.500	5.152		
ADD HYD	117.22	-	5	.01560	27.22	1.108	1.33209	1.500	2.726	PER IMP=	70.00
*S TOTAL Q TO WILDFLOWER DIVERSION AT I-25 (117.27)											
ADD HYD	117.27	5 & 6	10	.22964	177.22	13.701	1.11866	1.500	1.206		
ROUTE	117.98	7	2	.07626	257.23	4.487	1.10321	1.550	5.270		
COMPUTE NM HYD	116.21	-	4	.03440	45.58	1.682	.91666	1.500	2.070	PER IMP=	40.00
ADD HYD	116.99	4 & 2	5	.11066	298.26	6.169	1.04521	1.550	4.211		
*S TOTAL Q AT I-25 & MODESTO (116.99)											
*S SOUTH LA CUEVA ARROYO AT CONFLUENCE JUST WEST OF I-25 CROSSING											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ADD HYD	117.29	10& 5	5	.34030	472.23	19.869	1.09478	1.550	2.168	
ROUTE MCUNGE	117.28	5	2	.34030	467.67	19.035	1.04878	1.551	2.147	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	119.00	-	1	.05490	146.12	6.152	2.10117	1.500	4.159	RAIN24= 2.830
ADD HYD	119.90	1& 2	5	.39520	595.85	24.933	1.18294	1.551	2.356	PER IMP= 70.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	120.00	-	1	.02680	44.20	1.810	1.26628	1.500	2.577	RAIN24= 1.880
ADD HYD	120.90	1& 5	5	.42200	634.60	26.661	1.18457	1.551	2.350	PER IMP= 70.00
ROUTE MCUNGE	120.80	5	2	.42200	628.86	26.140	1.16144	1.575	2.328	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	125.00	-	1	.05080	83.53	3.351	1.23688	1.500	2.569	RAIN24= 1.840
ADD HYD	125.90	1& 2	6	.47280	694.11	29.285	1.16135	1.575	2.294	PER IMP= 70.00
*S LA CUEVA ARROYO SOUTH BEFORE WILDFLOWER FLOWS JUST UPSTREAM OF 96" RCP										
RAINFALL TYPE= 2										
COMPUTE NM HYD	121.00	-	1	.04890	81.21	3.287	1.26052	1.500	2.595	RAIN24= 1.870
*S WILDFLOWER ARROYO AT SAN MATEO BLVD.										PER IMP= 70.00
DIVIDE HYD	121.83	1	11	.02445	40.61	1.644	1.26051	1.500	2.595	
	121.84	AND	12	.02445	40.61	1.644	1.26051	1.500	2.595	
ROUTE MCUNGE	121.81	11	2	.02445	38.67	1.641	1.25820	1.600	2.471	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	123.00	-	1	.01850	25.99	.971	.98395	1.500	2.195	RAIN24= 1.830
ADD HYD	123.90	1& 2	5	.04295	61.75	2.611	1.14005	1.550	2.246	PER IMP= 50.00
ROUTE MCUNGE	121.82	12	2	.02445	39.57	1.651	1.26615	1.600	2.529	CCODE = .2
COMPUTE NM HYD	122.00	-	1	.02250	31.61	1.181	.98395	1.500	2.195	PER IMP= 50.00
ADD HYD	122.90	1& 2	3	.04695	67.11	2.832	1.13089	1.550	2.234	
COMPUTE NM HYD	124.00	-	1	.02510	35.26	1.317	.98395	1.500	2.195	PER IMP= 50.00
ADD HYD	124.91	3& 1	4	.07205	100.22	4.149	1.07970	1.500	2.173	
DIVIDE HYD	122.85	4	1	.07070	90.00	4.071	1.07970	1.500	1.989	
	124.92	AND	2	.00135	10.22	.078	1.07970	1.500	11.792	
*S TOTAL OUTFLOW TO ALAMEDA FROM JEFFERSON VIA 36" STORM DRAIN.										
ROUTE MCUNGE	124.83	2	3	.00135	6.25	.076	1.05367	1.650	7.206	CCODE = .1
ADD HYD	124.91	5& 3	4	.04430	62.57	2.688	1.13740	1.550	2.207	
*S SUBTRACT FLOW INTERCEPTED BY 36" PIPE TO WATERCRESS CHANNEL										
DIVIDE HYD	123.81	4	1	.04430	62.57	2.688	1.13740	1.550	2.207	
	123.99	AND	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	125.99	6& 1	3	.51710	755.38	31.819	1.15376	1.575	2.282	
*S TOTAL FLOW AT ENTRANCE TO 96" PIPE										
ROUTE	125.88	3	1	.51710	745.11	31.849	1.15483	1.600	2.251	
ADD HYD	127.91	1& 8	8	6.74940	2555.42	266.678	.74084	1.661	.592	
ADD HYD	127.92	2& 8	5	6.74940	2555.42	266.678	.74084	1.661	.592	
RAINFALL TYPE= 2										
COMPUTE NM HYD	128.00	-	1	.13730	193.62	8.777	1.19860	1.550	2.203	RAIN24= 1.790
ADD HYD	128.90	1& 5	5	6.88670	2690.28	274.995	.74871	1.661	.610	PER IMP= 70.00
*S LA CUEVA ARROYO TOTAL @ THE NORTH DIVERSION CHANNEL										
FINISH										

LA CUEVA FUTURE CONDITION 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = lc100bm.fut

RUN DATE (MON/DAY/YR) = 11/06/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	101.00	-	1	.60700	953.84	45.841	1.41600	1.650	2.455		RAIN24= 4.050
ROUTE MCUNGE	101.80	1	2	.60700	912.90	45.469	1.40451	1.850	2.350		PER IMP= .00
RAINFALL TYPE= 2											CCODE = .1
COMPUTE NM HYD	100.00	-	1	1.21400	1201.70	87.881	1.35730	1.850	1.547		RAIN24= 4.200
ROUTE MCUNGE	100.80	1	3	1.21400	1177.21	87.700	1.35451	2.100	1.515		PER IMP= .00
ADD HYD	101.88	3& 2	5	1.82100	1882.13	133.168	1.37117	1.950	1.615		CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	102.00	-	1	.87500	1033.60	54.313	1.16386	1.700	1.846		RAIN24= 3.650
ADD HYD	102.90	1& 5	5	2.69600	2554.30	187.482	1.30389	1.900	1.480		PER IMP= .00
*S LA CUEVA TRIBUTARY	ARROYO @ TRAMWAY BLVD. (102.9)										
ROUTE MCUNGE	102.80	5	2	2.69600	2515.07	187.100	1.30123	2.050	1.458		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.370
COMPUTE NM HYD	107.20	-	1	.17200	325.87	13.591	1.48157	1.550	2.960		PER IMP= 17.00
ADD HYD	107.29	1& 2	4	2.86800	2575.01	200.690	1.31205	2.050	1.403		
*S NORTH LA CUEVA AT	GLENDALE AND BROWNING (107.29)										
RAINFALL TYPE= 2											
COMPUTE NM HYD	102.10	-	1	.09300	149.17	4.457	.89867	1.500	2.506		RAIN24= 3.500
*S LA CUEVA TRIBUTARY	@ TRAMWAY BLVD.										PER IMP= .00
ROUTE MCUNGE	102.18	1	2	.09300	136.20	4.395	.88600	1.700	2.288		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.380
COMPUTE NM HYD	107.10	-	1	.18080	399.38	14.303	1.48327	1.500	3.452		PER IMP= 17.00
ADD HYD	107.19	1& 2	5	.27380	399.38	18.697	1.28040	1.500	2.279		
*S NORTH LA CUEVA (TRIB)	AT FLORENCE AND BROWNING (107.19)										
ADD HYD	107.90	5& 4	3	3.14180	2666.73	219.388	1.30929	2.000	1.326		
RAINFALL TYPE= 2											RAIN24= 3.400
COMPUTE NM HYD	106.00	-	1	.04360	95.85	3.448	1.48281	1.500	3.435		PER IMP= 17.00
ROUTE MCUNGE	106.80	1	2	.04360	86.06	3.396	1.46043	1.700	3.084		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.340
COMPUTE NM HYD	106.10	-	1	.11160	238.47	8.685	1.45925	1.500	3.339		PER IMP= 17.00
ADD HYD	106.19	1& 2	6	.15520	257.25	12.081	1.45958	1.550	2.590		
*S LA CUEVA TRIBUTARY	ARROYO @ BROWNING (106.19)										
ADD HYD	107.99	3& 6	5	3.29700	2733.21	231.469	1.31636	2.000	1.295		
ROUTE MCUNGE	107.80	5	2	3.29700	2713.95	231.319	1.31551	2.100	1.286		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.230
COMPUTE NM HYD	109.00	-	1	.10060	216.30	7.552	1.40751	1.500	3.359		PER IMP= 17.00
ADD HYD	109.90	1& 2	5	3.39760	2736.13	238.871	1.31823	2.100	1.258		
RAINFALL TYPE= 2											RAIN24= 3.250
COMPUTE NM HYD	108.00	-	7	.20550	404.63	15.573	1.42091	1.550	3.077		PER IMP= 17.00
ADD HYD	109.99	5& 7	5	3.60310	2789.17	254.444	1.32409	2.100	1.210		
*S LA CUEVA ARROYO @	EUBANK (MAIN) (109.99)										
ROUTE MCUNGE	109.88	5	2	3.60310	2763.62	254.113	1.32237	2.200	1.198		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.130
COMPUTE NM HYD	110.00	-	1	.16340	275.61	11.738	1.34689	1.550	2.636		PER IMP= 17.00
ADD HYD	110.90	1& 2	5	3.76650	2796.05	265.851	1.32343	2.200	1.160		
*S LA CUEVA ARROYO @	VENTURA (MAIN) (110.90)- FINAL										
*S ROUTE TO BARSTOW											
ROUTE MCUNGE	110.88	5	2	3.76650	2792.08	263.889	1.31366	2.219	1.158		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.020
COMPUTE NM HYD	111.00	-	1	.05330	108.83	3.739	1.31526	1.500	3.190		PER IMP= 17.00
COMPUTE NM HYD	111.40	-	4	.01410	28.80	.989	1.31527	1.500	3.192		PER IMP= 17.00
*S COMBINE HYD.'S	111.0	AND	111.4	AS 111.49							
ADD HYD	111.49	1& 4	4	.06740	137.63	4.728	1.31525	1.500	3.191		
*S ROUTE TO LA CUEVA	CHANNEL @ OAKLAND										
*S PIPE ROUTING											
ROUTE	111.48	4	5	.06740	133.79	4.728	1.31527	1.550	3.102		
RAINFALL TYPE= 2											RAIN24= 3.050
COMPUTE NM HYD	111.10	-	1	.09690	195.97	7.699	1.48968	1.500	3.160		PER IMP= 26.00
RAINFALL TYPE= 2											RAIN24= 3.050
COMPUTE NM HYD	111.30	-	6	.04200	107.90	4.348	1.94094	1.500	4.014		PER IMP= 50.00
ADD HYD	111.39	1& 6	1	.13890	303.87	12.046	1.62612	1.500	3.418		
*S COMBINE HYD.'S	111.48	AND	111.39	AS 111.88							
ADD HYD	111.88	1& 5	1	.20630	436.39	16.774	1.52456	1.500	3.305		
*S LA CUEVA CHANNEL	AT BARSTOW (111.89)-NOT FINAL										
ADD HYD	111.89	1& 2	5	3.97280	2827.78	280.143	1.32216	2.194	1.112		
*S DIVERSION FROM NDB	ABOVE CARRINGTON										
COMPUTE NM HYD	924.10	-	6	.02500	64.23	2.588	1.94094	1.500	4.015		PER IMP= 50.00
COMPUTE NM HYD	924.20	-	7	.01900	48.82	1.967	1.94094	1.500	4.015		PER IMP= 50.00
*S COMBINE HYD.'S	924.10	AND	924.20	AS 924.22							

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
ADD HYD	924.22	6 & 7	6	.04400	113.05	4.555	1.94093	1.500	4.015	
*S ROUTE TO BARSTOW IN ALAMEDA AS	924.28	6	2	.04400	109.83	4.555	1.94094	1.500	3.900	
ROUTE	111.99	5 & 2	5	4.01680	2838.79	284.498	1.32801	2.194	1.104	
ADD HYD	111.99	5 & 2	5	4.01680	2838.79	284.498	1.32801	2.194	1.104	
*S LA CUEVA MAIN AT BARSTOW	111.99	FINAL								
ROUTE MCUNGE	111.80	5	2	4.01680	2835.32	284.453	1.32780	2.229	1.103	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	112.10	-	1	.08940	219.11	8.942	1.87543	1.500	3.829	RAIN24= 2.960
COMPUTE NM HYD	112.20	-	3	.08260	202.31	8.262	1.87543	1.500	3.827	PER IMP= 50.00
*S COMBINE HYD.'S	112.1 AND	112.2 AS	112.8							PER IMP= 50.00
ADD HYD	112.80	1 & 3	1	.17200	421.41	17.204	1.87543	1.500	3.828	
ADD HYD	112.90	1 & 2	5	4.18880	2869.92	300.975	1.34723	2.202	1.071	
*S LA CUEVA ARROYO @ WYOMING (112.90)										
ROUTE MCUNGE	112.80	5	2	4.18880	2864.32	300.965	1.34719	2.257	1.068	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	113.00	-	1	.10000	262.65	10.797	2.02440	1.500	4.104	RAIN24= 2.930
COMPUTE NM HYD	116.10	-	7	.10280	270.05	11.099	2.02440	1.500	4.105	PER IMP= 60.00
ADD HYD	113.80	1 & 7	1	.20280	532.70	21.896	2.02440	1.500	4.104	PER IMP= 60.00
ADD HYD	113.90	1 & 2	5	4.39160	2897.59	322.031	1.37492	2.257	1.031	
*S LA CUEVA MAIN ARROYO @ LOUISIANA (113.90)										
ROUTE MCUNGE	113.80	5	4	4.39160	2893.34	322.114	1.37527	2.284	1.029	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	115.00	-	3	.12020	312.21	12.750	1.98890	1.500	4.058	RAIN24= 2.850
ADD HYD	115.90	4 & 3	5	4.51180	2912.58	334.701	1.39094	2.248	1.009	PER IMP= 62.00
*S LA CUEVA NORTH (MAIN) ARROYO @ I-25 (115.90)										
RECALL HYD	205.90	-	9	1.48800	1294.36	110.966	1.39827	1.600	1.359	
ADD HYD	115.99	5 & 9	5	5.99980	3754.12	444.694	1.38972	2.212	.978	
ROUTE MCUNGE	115.80	5	2	5.99980	3752.12	438.666	1.37088	2.218	.977	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	118.00	-	1	.06490	172.75	7.273	2.10117	1.500	4.159	RAIN24= 2.830
ADD HYD	118.90	1 & 2	5	6.06470	3765.43	445.430	1.37712	2.197	.970	PER IMP= 70.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	118.10	-	1	.03060	75.34	2.966	1.81763	1.500	3.847	RAIN24= 2.830
ADD HYD	118.99	1 & 5	5	6.09530	3771.22	448.225	1.37880	2.197	.967	PER IMP= 50.00
*S LA CUEVA NORTH (MAIN) ARROYO @ SAN MATEO										
ROUTE MCUNGE	118.80	5	2	6.09530	3767.60	446.431	1.37328	2.240	.966	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	126.00	-	1	.07370	195.81	8.059	2.05022	1.500	4.151	RAIN24= 2.760
ADD HYD	126.90	1 & 2	5	6.16900	3793.29	453.897	1.37957	1.803	.961	PER IMP= 70.00
ROUTE MCUNGE	126.80	5	2	6.16900	3791.23	453.916	1.37963	1.810	.960	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	127.00	-	1	.06330	165.35	6.750	1.99932	1.500	4.081	RAIN24= 2.700
ADD HYD	127.90	1 & 2	8	6.23230	3846.45	460.185	1.38447	1.810	.964	PER IMP= 70.00
*S NORTH LA CUEVA ARROYO AT APPROXIMATELY JEFFERSON ST.										
*S NOW BEGIN THE SOUTH LA CUEVA ARROYO EAST OF I-25										
RAINFALL TYPE= 2										
COMPUTE NM HYD	117.40	-	1	.05120	132.07	5.432	1.98937	1.500	4.030	RAIN24= 2.890
*S ROUTE in alameda to san pedro										PER IMP= 60.00
ROUTE	117.49	1	2	.05120	124.38	5.432	1.98938	1.550	3.796	
RAINFALL TYPE= 2										
COMPUTE NM HYD	117.30	-	1	.11720	286.33	11.336	1.81357	1.500	3.817	RAIN24= 2.890
ADD HYD	117.39	2 & 1	3	.16840	405.11	16.768	1.86702	1.500	3.759	PER IMP= 50.00
*S TOTAL Q AT ALAMEDA & SAN PEDRO (117.39) WITHOUT PONDS										
COMPUTE NM HYD	117.50	-	1	.05500	151.76	6.417	2.18755	1.500	4.311	PER IMP= 70.00
*S TOTAL Q AT ALAMEDA INT (117.5)										
*S ROUTE IN SAN PEDRO FROM ALAMEDA TO OAKLAND (117.38)										
ROUTE	117.38	3	2	.16840	392.75	16.768	1.86702	1.550	3.644	
COMPUTE NM HYD	117.20	-	1	.05000	121.61	4.836	1.81357	1.500	3.800	PER IMP= 50.00
*S COMBINE HYDS 117.38 AND 117.2 AT OAKLAND (117.69)										
ADD HYD	117.39	2 & 1	3	.21840	512.70	21.604	1.85478	1.500	3.668	
*S ROUTE FROM OAKLAND TO EAGLE ROCK (117.68)										
ROUTE	117.68	3	2	.21840	512.47	21.605	1.85478	1.550	3.666	
COMPUTE NM HYD	116.20	-	4	.07190	185.54	7.629	1.98937	1.500	4.032	PER IMP= 60.00
*S COMBINE HYDS 117.68 AND 116.2 AT EAGLE ROCK (116.29)										
ADD HYD	116.39	2 & 4	3	.29030	676.91	29.233	1.88811	1.550	3.643	
DIVIDE HYD	117.25	3	6	.23564	325.00	23.729	1.88811	1.400	2.155	
COMPUTE NM HYD	116.49	AND	7	.05466	351.91	5.504	1.88811	1.550	10.060	
ADD HYD	117.22	-	5	.01560	43.06	1.820	2.18755	1.500	4.313	PER IMP= 70.00
*S TOTAL Q TO WILDFLOWER DIVERSION AT I-25 (117.27)										
ADD HYD	117.27	5 & 6	10	.25124	368.06	25.549	1.90670	1.500	2.289	
ROUTE	117.98	7	2	.05466	346.60	5.504	1.88826	1.550	9.908	
COMPUTE NM HYD	116.21	-	4	.03440	79.13	3.024	1.64799	1.500	3.594	PER IMP= 40.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ADD HYD	116.99	4 & 2	5	.08906	417.68	8.528	1.79544	1.550	7.328	
*S TOTAL Q AT I-25 & MODESTO (116.99)										
*S SOUTH LACUEVA ARROYO AT CONFLUENCE JUST WEST OF I-25 CROSSING										
ADD HYD	117.29	10 & 5	5	.34030	780.59	34.077	1.87758	1.550	3.584	
ROUTE MCUNGE	117.28	5	2	.34030	774.07	32.451	1.78799	1.531	3.554	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 2.830
COMPUTE NM HYD	119.00	-	1	.05490	146.12	6.152	2.10117	1.500	4.159	PER IMP= 70.00
ADD HYD	119.90	1 & 2	5	.39520	909.43	38.277	1.81603	1.531	3.596	
RAINFALL TYPE= 2										RAIN24= 2.820
COMPUTE NM HYD	120.00	-	1	.02680	70.98	2.990	2.09159	1.500	4.139	PER IMP= 70.00
ADD HYD	120.90	1 & 5	5	.42200	975.18	41.108	1.82646	1.531	3.611	
ROUTE MCUNGE	120.80	5	2	.42200	965.62	40.506	1.79973	1.561	3.575	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 2.760
COMPUTE NM HYD	125.00	-	1	.05080	134.25	5.548	2.04765	1.500	4.129	PER IMP= 70.00
ADD HYD	125.90	1 & 2	6	.47280	1084.17	45.692	1.81204	1.539	3.583	
*S LA CUEVA ARROYO SOUTH BEFORE WILDFLOWER FLOWS JUST UPSTREAM OF 96" RCP										
RAINFALL TYPE= 2										RAIN24= 2.810
COMPUTE NM HYD	121.00	-	1	.04890	130.08	5.442	2.08650	1.500	4.156	PER IMP= 70.00
*S WILDFLOWER ARROYO AT SAN MATEO BLVD.										
DIVIDE HYD	121.83	1	11	.02445	65.04	2.721	2.08649	1.500	4.156	
	121.84	AND	12	.02445	65.04	2.721	2.08649	1.500	4.156	
ROUTE MCUNGE	121.81	11	2	.02445	60.11	2.697	2.06844	1.600	3.841	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.750
COMPUTE NM HYD	123.00	-	1	.01850	44.03	1.701	1.72356	1.500	3.719	PER IMP= 50.00
ADD HYD	123.90	1 & 2	5	.04295	93.48	4.398	1.91987	1.550	3.401	
ROUTE MCUNGE	121.82	12	2	.02445	60.16	2.690	2.06322	1.600	3.845	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 2.750
COMPUTE NM HYD	122.00	-	1	.02250	53.55	2.068	1.72356	1.500	3.719	PER IMP= 50.00
ADD HYD	122.90	1 & 2	3	.04695	100.95	4.759	1.90043	1.550	3.360	
RAINFALL TYPE= 2										RAIN24= 2.740
COMPUTE NM HYD	124.00	-	1	.02510	59.74	2.301	1.71856	1.500	3.719	PER IMP= 50.00
ADD HYD	124.91	3 & 1	4	.07205	154.29	7.059	1.83707	1.550	3.346	
DIVIDE HYD	122.85	4	1	.06183	90.00	6.058	1.83707	1.450	2.274	
	124.92	AND	2	.01022	64.29	1.001	1.83707	1.550	9.833	
*S TOTAL OUTFLOW TO ALAMEDA FROM JEFFERSON VIA 36" STORM DRAIN.										
ROUTE MCUNGE	124.83	2	3	.01022	58.69	.966	1.77240	1.585	8.976	CCODE = .1
ADD HYD	124.91	5 & 3	4	.05317	150.58	5.359	1.88982	1.585	4.425	
*S SUBTRACT FLOW INTERCEPTED BY 36" PIPE TO WATERCRESS CHANNEL										
DIVIDE HYD	123.81	4	1	.04179	70.00	4.212	1.88982	1.489	2.617	
	123.99	AND	2	.01138	80.58	1.147	1.88982	1.585	11.068	
ADD HYD	125.99	6 & 1	3	.51459	1154.17	49.630	1.80834	1.539	3.505	
*S TOTAL FLOW AT ENTRANCE TO 96" PIPE										
ROUTE	125.88	3	1	.51459	1132.56	49.585	1.80670	1.550	3.439	
ADD HYD	127.91	1 & 8	8	6.74689	4572.38	509.254	1.41525	1.615	1.059	
ADD HYD	127.92	2 & 8	5	6.75827	4645.52	510.373	1.41597	1.615	1.074	
RAINFALL TYPE= 2										RAIN24= 2.690
COMPUTE NM HYD	128.00	-	1	.13730	318.30	14.604	1.99431	1.550	3.622	PER IMP= 70.00
ADD HYD	128.90	1 & 5	5	6.89557	4910.89	523.958	1.42471	1.615	1.113	
*S LA CUEVA ARROYO TOTAL @ THE NORTH DIVERSION CHANNEL										
FINISH										

NORTH DOMINGO BACA EXISTING COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 INPUT FILE = nbac10.ext

RUN DATE (MON/DAY/YR) = 11/05/1998
 USER NO. = RSTECNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 10-YR RETURN PERIOD, 24-HR STORM, EXISTING CONDITIONS											
*S MUSKINGUM-CUNGE CHANNEL ROUTING											
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	900.00	-	1	.44100	328.97	17.063	.72545	1.700	1.166		RAIN24= 2.670
*S ROUTE HYD. 900 THROUGH 902.1 (900.8)											PER IMP= .00
ROUTE MCUNGE	900.80	1	2	.44100	320.94	16.998	.72271	1.850	1.137		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.390
COMPUTE NM HYD	902.10	-	5	.08870	56.10	2.878	.60832	1.600	.988		PER IMP= 10.00
*S (902.91=NORTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.91	2& 5	2	.52970	352.97	19.876	.70356	1.850	1.041		
RAINFALL TYPE= 2											RAIN24= 2.570
COMPUTE NM HYD	901.00	-	1	.11800	165.92	4.570	.72623	1.500	2.197		PER IMP= .00
*S 20% AVULSION FROM SOUTH DOMINGO BACA WATERSHED SUB-BASIN 400.3											
RECALL HYD	400.30	-	15	.25500	348.72	9.922	.72958	1.500	2.137		
DIVIDE HYD	400.61	15	15	.05100	69.74	1.984	.72957	1.500	2.137		
	400.6SDB AND	19		.20400	278.98	7.938	.72957	1.500	2.137		
*S COMBINE 901 AND 20% AVULSION AS 901.1											
ADD HYD	901.10	1&15	1	.16900	235.66	6.555	.72724	1.500	2.179		
*S ROUTE HYD. 901.1 THROUGH 902.2 (901.8)											
ROUTE MCUNGE	901.80	1	3	.16900	203.55	6.482	.71913	1.700	1.882		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.390
COMPUTE NM HYD	902.20	-	5	.12200	77.32	3.958	.60832	1.600	.990		PER IMP= 10.00
*S (902.92=SOUTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.92	3& 5	3	.29100	274.14	10.440	.67267	1.700	1.472		
*S (902.93=SUM OF FLOWS AT ARROYO JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.93	2& 3	1	.82070	579.31	30.316	.69260	1.750	1.103		
ROUTE MCUNGE	902.80	1	2	.82070	575.74	30.305	.69235	1.850	1.096		CCODE = .2
COMPUTE NM HYD	902.30	-	3	.07300	98.65	3.816	.98008	1.500	2.111		PER IMP= 30.00
*S (902.99=TOTAL FLOW @ TRAMWAY FROM 902 SUBBASINS, 901, 900)											
ADD HYD	902.99	2& 3	1	.89370	601.47	34.120	.71585	1.800	1.052		

*S BEGIN: PRIMROSE POINTE DEVELOPMENT											

ROUTE MCUNGE	902.90	1	17	.89370	599.92	34.113	.71569	1.850	1.049		CCODE = .1
*S ROUTE 902.9 TO PRIMROSE POINTE CONCRETE CHANNEL AS 902.99											
ROUTE MCUNGE	902.99	17	16	.89370	597.11	34.100	.71542	1.850	1.044		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.430
COMPUTE NM HYD	906.00	-	12	.02368	34.64	1.391	1.10124	1.500	2.286		PER IMP= 35.00
ADD HYD	906.10	16&12	2	.91738	605.20	35.491	.72538	1.850	1.031		
*S ROUTE THRU THROUGH PRIMROSE POINTE TO LOWELL											
ROUTE MCUNGE	906.19	2	16	.91738	592.23	35.438	.72431	1.950	1.009		CCODE = .1
COMPUTE NM HYD	904.00	-	1	.04130	60.83	2.315	1.05117	1.500	2.301		PER IMP= 30.00
*S ROUTE HYDOGRAPH THRU POND NDB1											
ROUTE RESERVOIR	904.90	1	17	.04130	24.10	2.113	.95949	1.700	.912		AC-FT= .953
COMPUTE NM HYD	905.00	-	10	.00920	13.56	.516	1.05117	1.500	2.303		PER IMP= 30.00
*S ROUTE 905 HYDOGRAPH THRU POND NDB2											
ROUTE RESERVOIR	905.90	10	18	.00920	6.89	.467	.95141	1.650	1.171		AC-FT= .226
*S BASIN 903.0											
COMPUTE NM HYD	903.00	-	2	.06405	94.20	3.591	1.05117	1.500	2.298		PER IMP= 30.00
*S POND 3 DISCHARGE TO THE NORTH DOMINGO BACA ARROYO											
ROUTE RESERVOIR	903.90	2	24	.06405	2.00	2.613	.76483	2.600	.049		AC-FT= 2.731
ADD HYD	906.29	24&18	26	.07325	8.10	3.079	.78826	1.650	.173		
ADD HYD	906.39	17&26	1	.11455	32.19	5.193	.84999	1.700	.439		
*S ***** TOTAL NORTH DOMINGO BACA FLOW AT LOWELL STREET *****											
ADD HYD	906.99	1&16	1	1.03193	617.89	40.631	.73826	1.950	.936		

*S END: PRIMROSE POINTE DEVELOPMENT											

ROUTE MCUNGE	906.88	1	2	1.03193	617.56	40.635	.73833	2.000	.935		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 2.230
COMPUTE NM HYD	910.21	-	1	.01370	9.05	.279	.38120	1.500	1.033		PER IMP= 5.00
ADD HYD	910.29	2& 1	5	1.04563	618.61	40.913	.73365	2.000	.924		
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	910.31	-	2	.15630	164.33	5.839	.70050	1.500	1.643		PER IMP= 20.00
ADD HYD	910.90	5& 2	1	1.20193	643.76	46.753	.72934	2.000	.837		
ROUTE MCUNGE	910.88	1	2	1.20193	630.94	46.703	.72857	2.100	.820		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.220
COMPUTE NM HYD	912.11	-	3	.05900	39.99	1.374	.43659	1.500	1.059		PER IMP= 10.00
*S N D B ARROYO BETWEEN BROWNING AND EUBANK (AP 912.95) - UNBULKED											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
ADD HYD	912.95	3& 2	1	1.26093	635.18	48.077	.71490	2.100	.787		
COMPUTE NM HYD	912.21	-	2	.19600	141.21	5.094	.48730	1.550	1.126	PER IMP=	10.00
ADD HYD	912.98	1& 2	15	1.45693	652.01	53.171	.68428	2.100	.699		
RAINFALL TYPE= 2											
COMPUTE NM HYD	402.50	-	4	.02800	52.16	2.306	1.54415	1.500	2.911	RAIN24=	2.230
COMPUTE NM HYD	402.60	-	5	.02430	45.27	2.001	1.54415	1.500	2.911	PER IMP=	70.00
*S COMBINE 402.5 AND 402.6 AS 402.9 (AT BROWNING)									2.911	PER IMP=	70.00
ADD HYD	402.90	4& 5	11	.05230	97.43	4.307	1.54414	1.500	2.911		
*S ROUTE 402.9 AS 402.8 IN 42-INCH SD TO END OF 915.2 ROUTE	402.80	11	6	.05230	95.66	4.307	1.54415	1.500	2.858		
RAINFALL TYPE= 2											
COMPUTE NM HYD	915.20	-	2	.03700	23.76	.820	.41567	1.500	1.003	RAIN24=	2.170
COMPUTE NM HYD	915.10	-	29	.03430	22.02	.760	.41567	1.500	1.003	PER IMP=	10.00
ADD HYD	915.90	6& 2	11	.08930	119.41	5.127	1.07657	1.500	2.089	PER IMP=	10.00
*S ROUTE 915.9 AS 915.8 TO NDB DAM IN 54-INCH PIPE											
ROUTE	915.80	11	7	.08930	116.82	5.127	1.07658	1.550	2.044		
*S COMBINE HYD'S 912.98 AND 915.8 AS 912.99 (FLOW TO NDB DAM FROM PDN DRAIN)											
ADD HYD	912.99	15& 7	15	1.54623	673.72	58.298	.70694	2.100	.681		
*S*****											
*S AP 105.9 IS FROM "LA CUEVA, EL CAMINO & NORTH CAMINO ARROYOS - DRAINAGE											
*S MANAGEMENT PLAN" (RESOURCE TECHNOLOGY, INC., 1996).											
*S FILENAME: LCDV100B.EXT											
*S*****											
*S AP 105.9 IS FROM LA CUEVA TRIBUTARY (DIVERTED TO UPPER N. D. B. DAM											
*S THROUGH LA CUEVA DIVERSION CHANNEL)											
RECALL HYD	105.90	-	17	1.17410	604.74	37.888	.60506	1.850	.805		
ROUTE MCUNGE	105.80	17	2	1.17410	603.64	37.853	.60450	1.900	.803	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	911.00	-	3	.05800	37.27	1.620	.52379	1.550	1.004	RAIN24=	2.220
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. FROM LA CUEVA DIVERSION									1.004	PER IMP=	15.00
*S CHANNEL (HYD NO. 911.99) - UNBULKED											
ADD HYD	911.99	2& 3	1	1.23210	614.93	39.473	.60070	1.900	.780		
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. (HYD NO. 914.79)											
*S (BULKED)											
ADD HYD	914.79	15& 1	1	2.77833	1132.30	97.772	.65983	2.050	.637		
*S ROUTING THROUGH UPPER NORTH DOMINGO BACA DAM											
*S											
*S THIS STORAGE OUTFLOW DATA WAS BASED ON TOPOGRAPHY IN JUNE 1994.											
*S OUTFLOW IS TAKEN FROM "DESIGN ANALYSIS REPORT FOR LA CUEVA DIVERSION											
*S CHANNEL" (RESOURCE TECHNOLOGY, INC., 1994). STORAGE INFORMATION WAS											
*S CALCULATED AT RTI. THE ELEVATION OF EMERGENCY SPILLWAY											
*S IS AT 5749.											
ROUTE RESERVOIR	914.99	1	2	2.77833	136.43	97.639	.65893	2.900	.077	AC-FT=	66.369
*S UPPER N.D.B. DAM OUTFLOW HYDROGRAPH AT EUBANK. (HYD NO. 914.99)											
*S AP 914.99 IS THROUGH THE PRINCIPAL SPILLWAY											
*S AP NDB.SPILL IS THROUGH THE EMERGENCY SPILLWAY											
DIVIDE HYD	914.99	2	2	2.77833	136.43	97.639	.65893	2.900	.077		
NDB.SPILL AND		20		.00000	.00	.000	.00000	-.050	.000		
ROUTE MCUNGE	914.68	2	3	2.77833	136.41	97.621	.65881	3.000	.077	CCODE =	.1
ROUTE	SPILL.ROU	20	19	.00000	.00	.000	.00000	-.050	.000		
RAINFALL TYPE= 2											
COMPUTE NM HYD	914.20	-	1	.03500	32.46	1.003	.53739	1.500	1.449	RAIN24=	2.130
*S UNDBD DAM SPILL AT EUBANK (AP 914.29)									1.449	PER IMP=	10.00
ADD HYD	914.29	1&19	1	.03500	32.46	1.003	.53738	1.500	1.449		
ROUTE MCUNGE	914.28	1	2	.03500	29.50	1.003	.53740	1.650	1.317	CCODE =	.1
*S N D B ARROYO BETWEEN EUBANK AND HOLBROOK											
*S (AP 914.78)											
ADD HYD	914.78	2& 3	2	2.81333	136.81	98.624	.65730	2.950	.076		
ROUTE MCUNGE	914.88	2	3	2.81333	136.80	98.618	.65726	3.000	.076	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	919.10	-	1	.07900	65.91	1.888	.44803	1.500	1.304	RAIN24=	2.100
*S N D B ARROYO HOLBROOK (HYD NO. 919.19) - FINAL									1.304	PER IMP=	5.00
ADD HYD	919.19	1& 3	1	2.89233	137.38	100.506	.65154	2.900	.074		
*S BETWEEN HOLBROOK AND VENTURA (AP 919.18) - NOT FINAL											
ROUTE MCUNGE	919.18	1	5	2.89233	137.38	100.497	.65149	3.050	.074	CCODE =	.2
*S ROUTE 915.1 TO HOLBROOK THRU 919.2 AS 915.18											
ROUTE MCUNGE	915.18	29	7	.03430	19.73	.760	.41567	1.700	.899	CCODE =	.1
COMPUTE NM HYD	919.20	-	1	.06100	43.32	1.279	.39312	1.500	1.110	PER IMP=	5.00
ADD HYD	919.28	7& 1	1	.09530	48.28	2.039	.40123	1.550	.792		
*S HOLBROOK (HYD NO. 919.28)-FINAL											
*S ROUTE AP 919.28 TO RP 919.27											
ROUTE MCUNGE	919.27	1	3	.09530	47.35	2.039	.40123	1.600	.776	CCODE =	.1

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ROUTE MCUNGE	919.28	3	2	.09530	46.02	2.039	.40123	1.750	.754	CCODE = .1
*S N D B ARROYO BETWEEN HOLBROOK AND VENTURA. (AP 919.38) - NOT FINAL										
ADD HYD	919.38	2& 5	6	2.98763	141.75	102.536	.64351	1.850	.074	
RAINFALL TYPE= 2										
COMPUTE NM HYD	921.30	-	7	.02840	17.12	.587	.38728	1.500	.942	RAIN24= 2.070 PER IMP= 10.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	913.20	-	1	.01700	11.75	.387	.42635	1.500	1.080	RAIN24= 2.130 PER IMP= 10.00
ROUTE MCUNGE	913.18	1	2	.01700	11.72	.387	.42637	1.700	1.077	CCODE = .2 RAIN24= 2.100 PER IMP= 10.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	918.00	-	1	.11400	45.61	2.545	.41856	1.650	.625	RAIN24= 2.100 PER IMP= 10.00
*S HOLBROOK (AP 918.99) - FINAL										
ADD HYD	918.99	1& 2	1	.13100	54.81	2.931	.41956	1.700	.654	
ROUTE MCUNGE	918.88	1	2	.13100	54.45	2.931	.41957	1.750	.649	CCODE = .2 RAIN24= 2.070 PER IMP= 10.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	921.10	-	1	.02920	17.60	.603	.38728	1.500	.942	PER IMP= 10.00
*S UPSTREAM OF END OF VINYARD ESTATES SUBDIVISION UNIT IV BETWEEN										
*S HOLBROOK & VENTURA										
*S ANAHEIM (AP 921.19) - FINAL										
ADD HYD	921.19	1& 2	1	.16020	60.69	3.534	.41368	1.750	.592	
*S *****										
*S BEGIN: VINYARD ESTATES - UNIT IV										
*S *****										
*S ROUTE 921.19										
*S (48-INCH RCP)										
ROUTE	921.80	1	12	.16020	60.70	3.534	.41368	1.750	.592	
*S (54-INCH RCP)										
ROUTE	921.89	12	16	.16020	60.66	3.534	.41368	1.750	.592	
*S COMBINE 921.89 & 921.3 AS 921.98										
ADD HYD	921.98	16& 7	14	.18860	66.73	4.121	.40970	1.750	.553	
ROUTE RESERVOIR	921.99	14	15	.18860	10.53	1.028	.10223	2.400	.087	AC-FT= 3.190
*S *****										
*S END: VINYARD ESTATES - UNIT IV										
*S *****										
*S N D B ARROYO BETWEEN HOLBROOK & VENTURA (AP 921.78) - FINAL										
ADD HYD	921.78	6&15	1	3.17623	146.51	103.565	.61137	2.450	.072	
ROUTE MCUNGE	921.80	1	2	3.17623	146.36	103.558	.61133	2.550	.072	CCODE = .1 RAIN24= 2.050 PER IMP= 10.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	921.20	-	1	.08740	54.31	1.827	.39203	1.500	.971	PER IMP= 10.00
*S N D B ARROYO AT VENTURA (AP 921.9) - NOT FINAL										
ADD HYD	921.90	1& 2	26	3.26363	149.84	105.386	.60546	1.900	.072	
RAINFALL TYPE= 2										
COMPUTE NM HYD	913.10	-	1	.03600	19.96	.634	.33028	1.500	.866	RAIN24= 2.170 PER IMP= 5.00
ROUTE MCUNGE	913.88	1	2	.03600	17.36	.634	.33028	1.800	.754	CCODE = .1 RAIN24= 2.100 PER IMP= 5.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	917.00	-	1	.04060	21.97	.693	.32022	1.500	.845	PER IMP= 5.00
*S HOLBROOK (AP 917.99) - FINAL										
ADD HYD	917.99	1& 2	1	.07660	24.10	1.327	.32494	1.750	.492	
ROUTE MCUNGE	917.88	1	2	.07660	23.96	1.328	.32495	1.850	.489	CCODE = .2 RAIN24= 2.070 PER IMP= 10.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	920.10	-	1	.02020	13.52	.448	.41586	1.500	1.045	PER IMP= 10.00
*S INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT NORTH END										
*S BETWEEN HOLBROOK AND VENTURA (AP 920.19) - FINAL (UNBULKED)										
ADD HYD	920.19	2& 1	1	.09680	33.38	1.775	.34391	1.600	.539	
COMPUTE NM HYD	920.00	-	5	.04310	23.68	.890	.38728	1.500	.858	PER IMP= 10.00
*S INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT EAST (NORTH END)										
*S BETWEEN HOLBROOK AND VENTURA (AP 920) - FINAL (UNBULKED)										
*S *****										
*S BEGIN: VINYARD ESTATES - UNIT IV										
*S *****										
RAINFALL TYPE= 2										
COMPUTE NM HYD	922.10	-	2	.00700	4.91	.180	.48193	1.500	1.096	RAIN24= 2.010 PER IMP= 17.00
COMPUTE NM HYD	923.00	-	12	.00700	4.92	.158	.42326	1.500	1.098	PER IMP= 10.00
COMPUTE NM HYD	922.20	-	9	.01480	21.04	.781	.98968	1.500	2.221	PER IMP= 40.00
COMPUTE NM HYD	922.30	-	16	.04150	58.97	2.190	.98968	1.500	2.220	PER IMP= 40.00
*S COMBINE 920.1 + 922.1 AS 922.19										
ADD HYD	922.19	1& 2	3	.10380	36.97	1.955	.35321	1.600	.556	
*S ROUTE 920 TO POND AS 920.8										
ROUTE MCUNGE	920.80	5	4	.04310	23.61	.890	.38725	1.550	.856	CCODE = .1
*S ROUTE 922.19 THRU POND AS 920.88										
ROUTE RESERVOIR	920.88	3	7	.10380	18.83	1.955	.35321	2.000	.283	AC-FT= .850
*S ADD 920.88 920.8 AS 920.9 (POND Q + 920)										
ADD HYD	920.90	4& 7	7	.14690	28.11	2.845	.36319	1.600	.299	
*S ROUTE POND DISCHARGE & 920 THRU 30" PIPE AS 920.98										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4	NOTATION
ROUTE	920.98	7	8	.14690	28.53	2.845	.36319	1.600	.304		
*S ADD 922.2 + 920.98 AS 922.28											
ADD HYD	922.28	8 & 9	10	.16170	45.84	3.627	.42053	1.550	.443		
*S Route 922.28 flows to Ventura Interceptor AS 922.29											
ROUTE	922.29	10	11	.16170	45.99	3.627	.42053	1.550	.444		
*S Route 923 flows to 48" Ventura Line AS 923.8											
ROUTE	923.80	12	13	.00700	5.00	.158	.42335	1.500	1.115		
*S ADD 923.8 + 922.29 AS 923.9											
ADD HYD	923.90	11 & 13	14	.16870	50.69	3.785	.42064	1.550	.469		
*S ROUTE 923.9 TO MAIN POND AS 923.91											
ROUTE	923.91	14	15	.16870	50.54	3.785	.42064	1.550	.468		
*S Total flows entering main pond											
*S ADD 923.91 + 923.3 AS 923.92											
ADD HYD	923.92	16 & 15	17	.21020	103.47	5.975	.53298	1.550	.769		
*S ROUT 923.92 THRU POND AS 923.99											
ROUTE RESERVOIR	923.99	17	18	.21020	40.43	5.756	.51343	1.950	.301	AC-FT=	2.154

*S	END: VINYARD ESTATES - UNIT IV										

*S	*****										
*S N D B ARROYO AT VENTURA (AP 921.99) - FINAL											
ADD HYD	921.99	18 & 26	1	3.47383	190.08	111.142	.59989	1.900	.085		
DIVIDE HYD	950.00	1	18	1.14636	62.73	36.677	.59989	1.900	.085		
	951.00	AND	26	2.32747	127.35	74.465	.59989	1.900	.085		
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.10	-	3	.01200	12.54	.408	.63728	1.500	1.633	RAIN24=	2.010
*S LA CUEVA SUBDIVISION (AP 926.1) - FINAL										PER IMP=	17.71
ROUTE RESERVOIR	926.18	3	2	.01200	2.00	.252	.39430	1.950	.260	AC-FT=	.269
*S N D B ARROYO AT -400 FT WEST OF VENTURA. (HYD NO. 926.19) - FINAL											
ADD HYD	926.19	1 & 2	1	3.48583	192.07	111.394	.59918	1.900	.086		
ROUTE MCUNGE	926.17	1	2	3.48583	191.94	111.383	.59912	2.050	.086	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.00	-	1	.03750	36.64	1.326	.66301	1.500	1.527	RAIN24=	2.000
*S N D B ARROYO AT BARSTOW. (AP 926.79) - NOT FINAL										PER IMP=	25.00
ADD HYD	926.79	1 & 2	11	3.52333	197.63	112.709	.59980	2.050	.088		
RAINFALL TYPE= 2											
COMPUTE NM HYD	925.20	-	1	.01400	5.62	.169	.22680	1.500	.628	RAIN24=	2.010
ROUTE MCUNGE	925.27	1	3	.01400	4.77	.165	.22122	1.750	.532	PER IMP=	2.00
COMPUTE NM HYD	925.10	-	1	.06400	96.60	3.766	1.10339	1.500	2.359	CCODE =	.1
*S COMBINE 925.1 & 925.27 AS 925.18 (VINYARD FLOW TO APS SITE)										PER IMP=	50.00
ADD HYD	925.18	1 & 3	1	.07800	96.78	3.931	.94504	1.500	1.939		
*S (925.19=4-FOOT STORM SEWER ROUTING)											
ROUTE	925.19	1	2	.07800	95.34	3.931	.94505	1.500	1.910		
COMPUTE NM HYD	925.40	-	4	.03700	58.52	2.246	1.13797	1.500	2.471	PER IMP=	50.00
*S INFLOW INTO DETENTION POND AT BARSTOW IN VINEYARD ESTATES											
*S (AP 925.48) - FINAL											
ADD HYD	925.48	2 & 4	15	.11500	153.87	6.177	1.00711	1.500	2.091		
COMPUTE NM HYD	922.00	-	1	.02100	16.25	.665	.59360	1.550	1.209	PER IMP=	15.00
ROUTE MCUNGE	922.80	1	2	.02100	15.52	.652	.58174	1.700	1.155	CCODE =	.1
COMPUTE NM HYD	924.10	-	1	.01900	7.75	.233	.22984	1.500	.638	PER IMP=	2.00
*S INTO DETENTION POND JUST UPSTREAM OF CARRINGTON SUBDIVISION											
*S (AP 924.19) - (UNBULKED)											
ADD HYD	924.19	1 & 2	1	.04000	19.95	.884	.41457	1.650	.779		
*S THE FOLLOWING DETENTION POND INFORMATION WAS CALCULATED BY RTI USING THE											
*S DESIGN DATA IN GRADING & DRAINAGE PLAN FOR CARRINGTON SUBDIVISION											
*S (COMMUNITY SCIENCES CORPORATION, 1992)											
ROUTE RESERVOIR	924.18	1	3	.04000	2.29	.884	.41452	2.350	.089	AC-FT=	.603
ROUTE MCUNGE	924.17	3	2	.04000	2.29	.884	.41451	2.550	.089	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	924.20	-	1	.00700	3.99	.136	.36334	1.500	.891	RAIN24=	1.990
ROUTE MCUNGE	924.28	1	4	.00700	3.29	.133	.35641	1.750	.734	PER IMP=	10.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	924.30	-	1	.02700	41.20	1.593	1.10648	1.500	2.384	CCODE =	.1
*S CARRINGTON SUBDIVISION (AP 924.3)										RAIN24=	2.000
COMPUTE NM HYD	924.40	-	7	.01720	10.81	.358	.39054	1.500	.982	PER IMP=	50.00
*S TOTAL CARRINGTON DISCHARGE (AP 924.38) - FINAL											
ADD HYD	924.38	1 & 2	3	.06700	41.21	2.478	.69336	1.500	.961		
*S TOTAL BARSTOW Q NORTH OF CARRINGTON (AP 924.48) - FINAL											
ADD HYD	924.48	7 & 4	4	.02420	10.97	.491	.38063	1.500	.708		
*S TOTAL BARSTOW @ CARRINGTON (AP 924.99) - FINAL											
ADD HYD	924.99	4 & 3	5	.09120	52.18	2.969	.61038	1.500	.894		
*S AP 924.91 INTO STORM SEWER											
*S AP 924.92 INTO BARSTOW											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 5	NOTATION
DIVIDE HYD	924.91	5	3	.07530	20.00	2.451	.61038	1.40D	.415		
	924.92	AND 4		.01590	32.18	.518	.61038	1.500	3.162		
*S (924.81=24-INCH STORM SEWER ROUTING)											
ROUTE	924.81	3	2	.07530	21.26	2.451	.61038	1.850	.441		
COMPUTE NM HYD	925.30	-	1	.01050	15.82	.614	1.09694	1.500	2.354	PER IMP=	50.00
*S COMBINE CARRINGTON FLOW WITH VINYARD FLOW 925.3 + 924.81 AS 925.38											
ADD HYD	925.38	1& 2	5	.08580	34.87	3.066	.66991	1.500	.635		
ROUTE	924.81	5	2	.08580	34.69	3.066	.66992	1.550	.632		
*S BARSTOW SD WITH VINYARD SD FLOW AT POND											
ADD HYD	925.38	15& 2	5	.20080	187.86	9.243	.86303	1.500	1.462		
ADD HYD	925.39	5& 4	5	.21670	220.04	9.760	.84449	1.500	1.587		
*S (DETENTION POND AT BARSTOW AT APS MS)											
ROUTE RESERVOIR	925.85	5	2	.21670	54.00	9.760	.84447	1.950	.389	AC-FT=	4.618
ADD HYD	926.99	2&11	5	3.74003	250.82	122.469	.61398	2.050	.105		
*S N D B ARROYO AT BARTSOW (AP 926.99) - FINAL											
ROUTE MCUNGE	926.70	5	3	3.74003	250.10	122.462	.61394	2.100	.104	CCODE =	.2
ROUTE MCUNGE	926.80	3	5	3.74003	249.18	122.421	.61374	2.150	.104	CCODE =	.1
COMPUTE NM HYD	932.40	-	3	.05740	91.59	3.605	1.17753	1.500	2.493	PER IMP=	55.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	930.00	-	1	.08500	125.30	4.874	1.07504	1.500	2.303	PER IMP=	50.00
*S N D B ARROYO AT WYOMING (AP 930.59) -											
ADD HYD	930.59	1& 3	1	.14240	216.89	8.478	1.11635	1.500	2.380		
*S N D B ARROYO AT WYOMING (AP 930.99) - FINAL											
ADD HYD	930.99	1& 5	1	3.88243	281.65	130.899	.63217	2.050	.113		
ROUTE MCUNGE	930.80	1	12	3.88243	275.92	130.798	.63168	2.250	.111	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	935.00	-	14	.11000	53.54	1.668	.28428	1.500	.761	PER IMP=	5.00
ADD HYD	930.85	14&12	13	3.99243	278.98	132.465	.62211	2.250	.109		

*S BEGIN: WYOMING STORM DRAIN											

COMPUTE NM HYD	929.00	-	4	.02400	41.00	1.658	1.29529	1.500	2.669	PER IMP=	65.00
COMPUTE NM HYD	931.00	-	1	.06050	62.04	3.619	1.12156	1.650	1.602	PER IMP=	55.00
*S NO POND AT LA CUEVA HIGH SCHOOL											
ADD HYD	931.80	4& 1	1	.08450	90.06	5.277	1.17089	1.550	1.665		
COMPUTE NM HYD	932.10	-	4	.00730	11.58	.450	1.15472	1.500	2.479	PER IMP=	55.00
*S CHURCH (AP 932.1)											
*S WYOMING AND WILSHIRE (AP 931.99)											
ADD HYD	931.99	4& 1	1	.09180	100.19	5.726	1.16960	1.550	1.705		
*S (931.98=5-FOOT PIPE ROUTING)											
ROUTE	931.98	1	3	.09180	99.99	5.726	1.16961	1.550	1.702		
COMPUTE NM HYD	932.20	-	1	.00730	11.46	.450	1.15472	1.500	2.452	PER IMP=	55.00
ADD HYD	932.80	3& 1	1	.09910	110.17	6.176	1.16850	1.550	1.737		
*S AT CORONA W/O CORONA Q											
COMPUTE NM HYD	932.30	-	2	.03130	49.09	1.928	1.15472	1.500	2.450	PER IMP=	55.00
*S EXISTING POND AT NORESTE BETWEEN CORONA AND ANAHEIM											
ROUTE RESERVOIR	932.84	2	5	.03130	6.01	1.922	1.15141	2.100	.300	AC-FT=	1.229
*S COMBINED POND AND WYOMING STORM DRAIN FLOW (AP 932.99)											
ADD HYD	932.99	1& 5	10	.13040	113.12	8.098	1.16440	1.550	1.355		

*S END: WYOMING STORM DRAIN											

ROUTE MCUNGE	932.81	10	2	.13040	108.65	8.098	1.16436	1.700	1.302	CCODE =	.1
COMPUTE NM HYD	934.10	-	1	.04680	61.41	2.303	.92278	1.500	2.050	PER IMP=	40.00
ADD HYD	934.99	1& 2	3	.17720	134.80	10.401	1.10056	1.700	1.189		
*S LNDBDD INFLOW HYDROGRAPH (AP 930.84) - NOT FINAL											
ADD HYD	930.84	3&13	11	4.16963	359.91	142.866	.64244	1.700	.135		
COMPUTE NM HYD	937.00	-	1	.03520	20.09	.680	.36246	1.500	.892	PER IMP=	10.00
*S TOTAL DAM INFLOW HYDROGRAPH (AP 930.86)-FINAL											
ADD HYD	930.86	1&11	11	4.20483	368.87	143.547	.64010	1.700	.137		
*S											
*S LOWER NORTH DOMINGO BACA DETENTION DAM											
*S LOW FLOW OUTLET (PRINCIPLE SPILLWAY). THIS PRINCIPLE SPILLWAY JOINS											
*S ONE OF THE FOUR BOX CULVERTS AT THE I-25. IT ALSO PICKS UP FLOWS FROM											
*S SUB-BASINS 940.2, 939.16, 939.14 & 943.1. THE FLOWS FROM SUB-BASINS											
*S 939.14 AND 943.1 ARE INCLUDED BY SAD221 PROJECT WHICH IS ALREADY											
*S IN PLACE. THE FOLLOWING RATING CURVE IS FOR A 40-INCH ORIFICE PLATE.											
*S EMERGENCY SPILLWAY IS NOT USED IN THE FOLLOWING RATING CURVE.											
*S											
ROUTE RESERVOIR	930.87	11	12	4.20483	146.10	143.665	.64063	5.350	.054	AC-FT=	25.496
*S LNDBAD DAM OUTFLOW HYDROGRAPH (AP 930.87)											
RAINFALL TYPE= 2											
										RAIN24=	1.900

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 6	NOTATION
COMPUTE NM HYD	960.00	-	1	.00750	11.02	.418	1.04522	1.500	2.295	PER IMP=	50.00
*S STORM DRAIN OUT OF LND8D DAM AT LOUISIANA BLVD. (AP 930.89)											
ADD HYD	930.89	1&12	12	4.21233	146.14	144.083	.64135	5.350	.054		
COMPUTE NM HYD	940.20	-	1	.01560	22.23	.853	1.02474	1.500	2.226	PER IMP=	50.00
COMPUTE NM HYD	939.16	-	3	.01280	18.24	.700	1.02474	1.500	2.227	PER IMP=	50.00
ADD HYD	940.26	1&3	3	.02840	40.47	1.552	1.02472	1.500	2.226		
*S AT THE END OF LA CUEVA OESTE PHASE 1, IN STORM DRAIN PIPE (AP 939.88)											
*S (UNBULKED)											
ADD HYD	939.88	12&3	12	4.24073	146.30	145.635	.64391	5.450	.054		
*S STORM DRAIN PIPE FROM THE END OF LA CUEVA OESTE PHASE 1 TILL SAN PEDRO											
*S BENEATH ANAHEIM AVE -- TWO REACHES											
*S RCP DIA=4 FT TO 940.0 OUTFALL											
*S (939.78=4-FOOT PIPE ROUTING)											
ROUTE	939.78	12	15	4.24073	146.30	145.626	.64387	5.450	.054		
COMPUTE NM HYD	940.00	-	1	.01410	20.09	.771	1.02474	1.500	2.227	PER IMP=	50.00
ADD HYD	939.68	15&1	15	4.25483	163.49	146.396	.64513	1.500	.060		
*S RCP DIA=4 FT TO SAN PEDRO											
ROUTE	939.58	15	16	4.25483	164.10	146.393	.64512	1.500	.060		
*S STORM DRAIN PIPE FROM SAN PEDRO TO UTE DR. BENEATH ANAHEIM AVE.											
*S (939.48=RCP DIA=4 FT)											
ROUTE	939.48	16	14	4.25483	163.79	146.388	.64510	1.550	.060		
COMPUTE NM HYD	939.14	-	1	.01120	18.32	.714	1.19546	1.500	2.556	PER IMP=	60.00
COMPUTE NM HYD	943.10	-	3	.01200	19.85	.793	1.23850	1.500	2.585	PER IMP=	65.00
*S STORM DRAIN BENEATH ANAHEIM AT UTE (AP 943.18)											
ADD HYD	943.18	1&3	3	.02320	38.17	1.507	1.21770	1.500	2.571		
*S STORM DRAIN FLOW AT I-25 (AP 943.19)											
ADD HYD	943.19	3&14	3	4.27803	199.00	147.895	.64820	1.500	.073		
COMPUTE NM HYD	940.10	-	1	.00700	11.73	.479	1.28349	1.500	2.618	PER IMP=	70.00
ROUTE MCUNGE	940.80	1	6	.00700	11.70	.479	1.28353	1.600	2.611	CCODE =	.2
COMPUTE NM HYD	943.20	-	1	.01300	6.74	.230	.33102	1.500	.810	PER IMP=	10.00
*S OVERLAND FLOW AT I-25 (AP 943.29)											
ADD HYD	943.29	1&6	5	.02000	16.77	.709	.66435	1.600	1.310		
*S TOTAL NDB Q AT I-25 BOX CULVERT (943.98)											
ADD HYD	943.98	5&3	9	4.29803	214.08	148.603	.64828	1.550	.078		
*S CORONA SD SYSTEM											
COMPUTE NM HYD	942.20	-	1	.10310	121.03	5.193	.94442	1.550	1.834	PER IMP=	45.00
*S (ROUTE IN SD DIA=4 FT)											
ROUTE	942.29	1	5	.10310	122.98	5.193	.94443	1.550	1.864		
COMPUTE NM HYD	942.10	-	2	.04690	62.18	2.362	.94442	1.500	2.072	PER IMP=	45.00
*S TOTAL Q AT CORONADO MH PARK (943.99)											
ADD HYD	943.99	2&9	3	4.34493	275.16	150.965	.65147	1.500	.099		
FINISH											

NORTH DOMINGO BACA EXISTING COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = nbac100.ext

RUN DATE (MON/DAY/YR) = 11/05/1998
USER NO. = RSTECNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 100-YR RETURN PERIOD, 24-HR STORM, FUTURE CONDITIONS											
*S MUSKINGUM-CUNGE CHANNEL ROUTING											
START											
RAINFALL TYPE= 2											
COMPUTE NM HYD	900.00	-	1	.44100	640.16	33.763	1.43550	1.700	2.268	TIME= .00	RAIN24= 4.000
*S ROUTE HYD. 900 THROUGH 902.1 (900.8)											
ROUTE MCUNGE	900.80	1	2	.44100	625.07	33.593	1.42827	1.800	2.215	PER IMP= .00	CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	902.10	-	5	.08870	116.14	6.141	1.29815	1.600	2.046	RAIN24= 3.580	PER IMP= 10.00
*S (902.91=NORTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.91	2& 5	2	.52970	709.60	39.734	1.40648	1.800	2.093	RAIN24= 3.860	PER IMP= .00
RAINFALL TYPE= 2											
COMPUTE NM HYD	901.00	-	1	.11800	281.25	9.218	1.46480	1.500	3.724	RAIN24= 3.860	PER IMP= .00
*S 20% AVULSION FROM SOUTH DOMINGO BACA WATERSHED SUB-BASIN 400.3											
RECALL HYD	400.30	-	15	.25500	620.57	19.788	1.45500	1.500	3.802	RAIN24= 3.802	PER IMP= 10.00
DIVIDE HYD	400.61	15	15	.05100	124.11	3.958	1.45500	1.500	3.802	RAIN24= 3.802	PER IMP= 10.00
	400.65DB	AND	19	.20400	496.45	15.830	1.45500	1.500	3.802	RAIN24= 3.802	PER IMP= 10.00
*S COMBINE 901 AND 20% AVULSION AS 901.1											
ADD HYD	901.10	1&15	1	.16900	405.36	13.176	1.46184	1.500	3.748	RAIN24= 3.802	PER IMP= 10.00
*S ROUTE HYD. 901.1 THROUGH 902.2 (901.8)											
ROUTE MCUNGE	901.80	1	3	.16900	364.15	13.055	1.44840	1.700	3.367	CCODE = .1	RAIN24= 3.580
RAINFALL TYPE= 2											
COMPUTE NM HYD	902.20	-	5	.12200	161.86	8.447	1.29815	1.600	2.073	PER IMP= 10.00	CCODE = .1
*S (902.92=SOUTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.92	3& 5	3	.29100	514.87	21.502	1.38541	1.700	2.765	RAIN24= 3.580	PER IMP= 10.00
*S (902.93=SUM OF FLOWS AT ARROYO JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.93	2& 3	1	.82070	1144.97	61.235	1.39901	1.750	2.180	CCODE = .1	RAIN24= 3.580
ROUTE MCUNGE	902.80	1	2	.82070	1141.81	61.208	1.39837	1.750	2.174	PER IMP= 10.00	CCODE = .1
COMPUTE NM HYD	902.30	-	3	.07300	173.70	6.993	1.79608	1.500	3.718	CCODE = .2	PER IMP= 30.00
*S (902.99=TOTAL FLOW @ TRAMWAY FROM 902 SUBBASINS, 901, 900)											
ADD HYD	902.99	2& 3	1	.89370	1207.42	68.200	1.43086	1.750	2.111	RAIN24= 3.580	PER IMP= 10.00
*S*****											
*S BEGIN: PRIMROSE POINTE DEVELOPMENT											
*S*****											
ROUTE MCUNGE	902.90	1	17	.89370	1199.05	68.190	1.43064	1.800	2.096	CCODE = .1	RAIN24= 3.650
*S ROUTE 902.9 TO PRIMROSE POINTE CONCRETE CHANNEL AS 902.99											
ROUTE MCUNGE	902.99	17	16	.89370	1181.91	68.174	1.43030	1.800	2.066	PER IMP= 35.00	CCODE = .1
RAINFALL TYPE= 2											
COMPUTE NM HYD	906.00	-	12	.02368	58.73	2.476	1.96050	1.500	3.875	RAIN24= 3.650	PER IMP= 35.00
ADD HYD	906.10	16&12	2	.91738	1200.71	70.650	1.44398	1.800	2.045	RAIN24= 3.650	PER IMP= 35.00
*S ROUTE THRU THROUGH PRIMROSE POINTE TO LOWELL											
ROUTE MCUNGE	906.19	2	16	.91738	1174.76	70.507	1.44107	1.900	2.001	CCODE = .1	RAIN24= 3.650
COMPUTE NM HYD	904.00	-	1	.04130	102.46	4.183	1.89923	1.500	3.876	PER IMP= 30.00	CCODE = .1
*S ROUTE HYDOGRAPH THRU POND NDB1											
ROUTE RESERVOIR	904.90	1	17	.04130	35.03	3.899	1.77023	1.800	1.325	AC-FT= 1.773	PER IMP= 30.00
COMPUTE NM HYD	905.00	-	10	.00920	22.84	.932	1.89922	1.500	3.879	AC-FT= 1.773	PER IMP= 30.00
*S ROUTE 905 HYDOGRAPH THRU POND NDB2											
ROUTE RESERVOIR	905.90	10	18	.00920	15.30	.839	1.71009	1.600	2.599	AC-FT= .296	PER IMP= 30.00
*S BASIN 903.0											
COMPUTE NM HYD	903.00	-	2	.06405	158.75	6.488	1.89922	1.500	3.873	AC-FT= .296	PER IMP= 30.00
*S POND 3 DISCHARGE TO THE NORTH DOMINGO BACA ARROYO											
ROUTE RESERVOIR	903.90	2	24	.06405	43.76	5.232	1.53165	1.850	1.068	AC-FT= 3.918	PER IMP= 30.00
ADD HYD	906.29	24&18	26	.07325	52.56	6.071	1.55407	1.850	1.121	AC-FT= 3.918	PER IMP= 30.00
ADD HYD	906.39	17&26	1	.11455	87.40	9.970	1.63201	1.850	1.192	AC-FT= 3.918	PER IMP= 30.00
*S ***** TOTAL NORTH DOMINGO BACA FLOW AT LOWELL STREET *****											
ADD HYD	906.99	1&16	1	1.03193	1259.05	80.477	1.46226	1.900	1.906	AC-FT= 3.918	PER IMP= 30.00
*S*****											
*S END: PRIMROSE POINTE DEVELOPMENT											
*S*****											
ROUTE MCUNGE	906.88	1	2	1.03193	1244.63	80.472	1.46216	2.000	1.885	CCODE = .1	RAIN24= 3.350
RAINFALL TYPE= 2											
COMPUTE NM HYD	910.21	-	1	.01370	21.87	.704	.96392	1.500	2.494	PER IMP= 5.00	RAIN24= 3.350
ADD HYD	910.29	2& 1	5	1.04563	1247.20	81.176	1.45563	2.000	1.864	PER IMP= 5.00	RAIN24= 3.350
RAINFALL TYPE= 2											
COMPUTE NM HYD	910.31	-	2	.15630	326.81	11.680	1.40110	1.500	3.267	PER IMP= 20.00	RAIN24= 3.380
ADD HYD	910.90	5& 2	1	1.20193	1294.74	92.855	1.44854	2.000	1.683	PER IMP= 20.00	RAIN24= 3.380
ROUTE MCUNGE	910.88	1	2	1.20193	1289.81	92.988	1.45060	2.000	1.677	CCODE = .2	RAIN24= 3.330
RAINFALL TYPE= 2											
COMPUTE NM HYD	912.11	-	3	.05900	94.77	3.224	1.02468	1.500	2.510	PER IMP= 10.00	RAIN24= 3.330
*S N D B ARROYO BETWEEN BROWNING AND EUBANK (AP 912.95) - UNBULKED											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
ADD HYD	912.95	3& 2	1	1.26093	1302.18	96.212	1.43067	2.000	1.614		
RAINFALL TYPE= 2											
COMPUTE NM HYD	912.21	-	2	.19600	321.78	11.549	1.10477	1.550	2.565	RAIN24=	3.330
ADD HYD	912.98	1& 2	15	1.45693	1348.83	107.761	1.38683	2.000	1.447	PER IMP=	10.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	402.50	-	4	.02800	82.16	3.763	2.51978	1.500	4.585	RAIN24=	3.350
RAINFALL TYPE= 2										PER IMP=	70.00
COMPUTE NM HYD	402.60	-	5	.02430	70.94	3.243	2.50242	1.500	4.561	RAIN24=	3.330
*S COMBINE 402.5 AND 402.6 AS 402.9 (AT BROWNING)										PER IMP=	70.00
ADD HYD	402.90	4& 5	11	.05230	153.10	7.006	2.51170	1.500	4.574		
*S ROUTE 402.9 AS 402.8 IN 42-INCH SD TO END OF 915.2 ROUTE	402.80	11	6	.05230	150.80	7.006	2.51171	1.500	4.505		
RAINFALL TYPE= 2											
COMPUTE NM HYD	915.20	-	2	.03700	57.51	1.949	.98762	1.500	2.429	RAIN24=	3.250
COMPUTE NM HYD	915.10	-	29	.03430	53.31	1.807	.98763	1.500	2.429	PER IMP=	10.00
ADD HYD	915.90	6& 2	11	.08930	208.31	8.955	1.88022	1.500	3.645	PER IMP=	10.00
*S ROUTE 915.9 AS 915.8 TO NDB DAM IN 54-INCH PIPE											
ROUTE	915.80	11	7	.08930	200.74	8.955	1.88023	1.550	3.512		
*S COMBINE HYD'S 912.98 AND 915.8 AS 912.99 (FLOW TO NDB DAM FROM PDN DRAIN)											
ADD HYD	912.99	15& 7	15	1.54623	1394.43	116.716	1.41533	2.000	1.409		
*S*****											
*S AP 105.9 IS FROM "LA CUEVA, EL CAMINO & NORTH CAMINO ARROYOS - DRAINAGE											
*S MANAGEMENT PLAN" (RESOURCE TECHNOLOGY, INC., 1996).											
*S FILENAME: LCDV100B.EXT											
*S*****											
*S AP 105.9 IS FROM LA CUEVA TRIBUTARY (DIVERTED TO UPPER N. D. B. DAM											
*S THROUGH LA CUEVA DIVERSION CHANNEL)											
RECALL HYD	105.90	-	17	1.17410	1245.48	79.791	1.27423	1.750	1.657		
ROUTE MCUNGE	105.80	17	2	1.17410	1244.13	79.796	1.27431	1.750	1.656	CCODE =	.2
RAINFALL TYPE= 2											
COMPUTE NM HYD	911.00	-	3	.05800	83.52	3.530	1.14108	1.550	2.250	RAIN24=	3.330
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. FROM LA CUEVA DIVERSION										PER IMP=	15.00
*S CHANNEL (HYD NO. 911.99) - UNBULKED											
ADD HYD	911.99	2& 3	1	1.23210	1288.13	83.325	1.26804	1.750	1.634		
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. (HYD NO. 914.79)											
*S (BULKED)											
ADD HYD	914.79	15& 1	1	2.77833	2426.66	200.042	1.35001	1.950	1.365		
*S ROUTING THROUGH UPPER NORTH DOMINGO BACA DAM											
*S											
*S THIS STORAGE OUTFLOW DATA WAS BASED ON TOPOGRAPHY IN JUNE 1994.											
*S OUTFLOW IS TAKEN FROM "DESIGN ANALYSIS REPORT FOR LA CUEVA DIVERSION											
*S CHANNEL" (RESOURCE TECHNOLOGY, INC., 1994). STORAGE INFORMATION WAS											
*S CALCULATED AT RTI. THE ELEVATION OF EMERGENCY SPILLWAY											
*S IS AT 5749.											
ROUTE RESERVOIR	914.99	1	2	2.77833	157.07	199.905	1.34909	3.150	.088	AC-FT=	155.992
*S UPPER N.D.B. DAM OUTFLOW HYDROGRAPH AT EUBANK. (HYD NO. 914.99)											
*S AP 914.99 IS THROUGH THE PRINCIPAL SPILLWAY											
*S AP NDB.SPILL IS THROUGH THE EMERGENCY SPILLWAY											
DIVIDE HYD	914.99	2	2	2.77833	157.07	199.905	1.34909	3.150	.088		
NDB.SPILL AND 20				.00000	.00	.000	.00000	-.050	.000		
ROUTE MCUNGE	914.68	2	3	2.77833	157.07	199.896	1.34903	3.200	.088	CCODE =	.2
ROUTE SPILL.ROU 20 19				.00000	.00	.000	.00000	-.050	.000		
RAINFALL TYPE= 2											
COMPUTE NM HYD	914.20	-	1	.03500	66.39	2.212	1.18478	1.500	2.964	RAIN24=	3.200
*S UNDBD DAM SPILL AT EUBANK (AP 914.29)										PER IMP=	10.00
ADD HYD	914.29	1&19	1	.03500	66.39	2.212	1.18477	1.500	2.964		
ROUTE MCUNGE	914.28	1	2	.03500	61.20	2.212	1.18479	1.650	2.732	CCODE =	.1
*S N D B ARROYO BETWEEN EUBANK AND HOLBROOK											
*S (AP 914.78)											
ADD HYD	914.78	2& 3	2	2.81333	172.03	202.107	1.34698	1.700	.096		
ROUTE MCUNGE	914.88	2	3	2.81333	167.64	202.082	1.34681	1.850	.093	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	919.10	-	1	.07900	142.38	4.487	1.06496	1.500	2.816	RAIN24=	3.150
*S N D B ARROYO HOLBROOK (HYD NO. 919.19) - FINAL										PER IMP=	5.00
ADD HYD	919.19	1& 3	1	2.89233	203.64	206.569	1.33912	1.800	.110		
*S BETWEEN HOLBROOK AND VENTURA (AP 919.18) - NOT FINAL											
ROUTE MCUNGE	919.18	1	5	2.89233	202.09	206.543	1.33895	1.850	.109	CCODE =	.2
*S ROUTE 915.1 TO HOLBROOK THRU 919.2 AS 915.18											
ROUTE MCUNGE	915.18	29	7	.03430	47.82	1.807	.98763	1.750	2.178	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	919.20	-	1	.06100	100.60	3.178	.97669	1.500	2.577	RAIN24=	3.150
ADD HYD	919.28	7& 1	1	.09530	105.36	4.984	.98062	1.550	1.727	PER IMP=	5.00
*S HOLBROOK (HYD NO. 919.28)-FINAL											
*S ROUTE AP 919.2 TO RP 919.27											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3	NOTATION
ROUTE MCUNGE	919.27	1	3	.09530	104.88	4.984	.98063	1.600	1.720	CCODE =	.1
ROUTE MCUNGE	919.28	3	2	.09530	102.24	4.984	.98063	1.750	1.676	CCODE =	.1
*S N D B ARROYO BETWEEN HOLBROOK AND VENTURA. (AP 919.38) - NOT FINAL											
ADD HYD	919.38	2 & 5	6	2.98763	297.04	211.527	1.32752	1.800	.155		
RAINFALL TYPE= 2											
COMPUTE NM HYD	921.30	-	7	.02840	42.42	1.418	.93639	1.500	2.334	RAIN24=	3.100
RAINFALL TYPE= 2										PER IMP=	10.00
COMPUTE NM HYD	913.20	-	1	.01700	27.83	.919	1.01321	1.500	2.558	RAIN24=	3.200
ROUTE MCUNGE	913.18	1	2	.01700	23.51	.911	1.00464	1.800	2.160	PER IMP=	10.00
RAINFALL TYPE= 2										CCODE =	.1
COMPUTE NM HYD	918.00	-	1	.11400	113.10	6.050	.99514	1.650	1.550	RAIN24=	3.150
*S HOLBROOK (AP 918.99) - FINAL											
ADD HYD	918.99	1 & 2	1	.13100	122.15	6.961	.99636	1.700	1.457	PER IMP=	10.00
ROUTE MCUNGE	918.88	1	2	.13100	119.88	6.940	.99337	1.800	1.430	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.100
COMPUTE NM HYD	921.10	-	1	.02920	43.61	1.458	.93639	1.500	2.334	PER IMP=	10.00
*S UPSTREAM OF END OF VINYARD ESTATES SUBDIVISION UNIT IV BETWEEN											
*S HOLBROOK & VENTURA											
*S ANAHEIM (AP 921.19) - FINAL											
ADD HYD	921.19	1 & 2	1	.16020	131.96	8.399	.98298	1.800	1.287		

*S BEGIN: VINYARD ESTATES - UNIT IV											

*S ROUTE 921.19											
*S (48-INCH RCP)											
ROUTE	921.80	1	12	.16020	132.16	8.399	.98298	1.800	1.289		
*S (54-INCH RCP)											
ROUTE	921.89	12	16	.16020	132.32	8.399	.98298	1.800	1.291		
*S COMBINE 921.89 & 921.3 AS 921.98											
ADD HYD	921.98	16 & 7	14	.18860	146.39	9.817	.97596	1.750	1.213		
ROUTE RESERVOIR	921.99	14	15	.18860	118.11	6.724	.66844	1.950	.979	AC-FT=	4.011

*S END: VINYARD ESTATES - UNIT IV											

*S N D B ARROYO BETWEEN HOLBROOK & VENTURA (AP 921.78) - FINAL											
ADD HYD	921.78	6 & 15	1	3.17623	402.87	218.250	1.28838	1.900	.198		
ROUTE MCUNGE	921.80	1	2	3.17623	399.35	218.244	1.28834	1.950	.196	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.070
COMPUTE NM HYD	921.20	-	1	.08740	134.09	4.415	.94707	1.500	2.397	PER IMP=	10.00
*S N D B ARROYO AT VENTURA (AP 921.9) - NOT FINAL											
ADD HYD	921.90	1 & 2	26	3.26363	418.92	222.658	1.27920	1.950	.201		
RAINFALL TYPE= 2										RAIN24=	3.250
COMPUTE NM HYD	913.10	-	1	.03600	52.68	1.688	.87916	1.500	2.287	PER IMP=	5.00
ROUTE MCUNGE	913.88	1	2	.03600	46.53	1.688	.87916	1.800	2.020	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.150
COMPUTE NM HYD	917.00	-	1	.04060	58.42	1.863	.86049	1.500	2.248	PER IMP=	5.00
*S HOLBROOK (AP 917.99) - FINAL											
ADD HYD	917.99	1 & 2	1	.07660	66.30	3.551	.86926	1.750	1.352		
ROUTE MCUNGE	917.88	1	2	.07660	65.96	3.551	.86927	1.850	1.345	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	3.100
COMPUTE NM HYD	920.10	-	1	.02020	31.87	1.058	.98186	1.500	2.465	PER IMP=	10.00
*S INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT NORTH END											
*S BETWEEN HOLBROOK AND VENTURA (AP 920.19) - FINAL (UNBULKED)											
ADD HYD	920.19	2 & 1	1	.09680	86.13	4.609	.89275	1.600	1.390		
RAINFALL TYPE= 2										RAIN24=	3.100
COMPUTE NM HYD	920.00	-	5	.04310	58.97	2.152	.93639	1.500	2.138	PER IMP=	10.00
*S INTO VINEYARD ESTATES SUBDIVISION UNIT IV AT EAST (NORTH END)											
*S BETWEEN HOLBROOK AND VENTURA (AP 920) - FINAL (UNBULKED)											

*S BEGIN: VINYARD ESTATES - UNIT IV											

RAINFALL TYPE= 2											
COMPUTE NM HYD	922.10	-	2	.00700	11.18	.394	1.05631	1.500	2.496	RAIN24=	3.030
COMPUTE NM HYD	923.00	-	12	.00700	11.32	.370	.99230	1.500	2.526	PER IMP=	17.00
COMPUTE NM HYD	922.20	-	9	.01480	35.69	1.396	1.76805	1.500	3.767	PER IMP=	10.00
COMPUTE NM HYD	922.30	-	16	.04150	100.03	3.913	1.76805	1.500	3.766	PER IMP=	40.00
*S COMBINE 920.1 + 922.1 AS 922.19											
ADD HYD	922.19	1 & 2	3	.10380	94.57	5.003	.90378	1.600	1.424		
*S ROUTE 920 TO POND AS 920.8											
ROUTE MCUNGE	920.80	5	4	.04310	58.82	2.150	.93541	1.550	2.133	CCODE =	.1
*S ROUTE 922.19 THRU POND AS 920.88											
ROUTE RESERVOIR	920.88	3	7	.10380	42.57	5.003	.90378	2.050	.641	AC-FT=	2.296
*S ADD 920.88 920.8 AS 920.9 (POND Q + 920)											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4	NOTATION
ADD HYD	920.90	4	7	.14690	78.57	7.153	.91305	1.600	.836		
*S ROUTE POND DISCHARGE & 920 THRU 30" PIPE AS 920.98											
ROUTE	920.98	7	8	.14690	77.82	7.153	.91305	1.600	.828		
*S ADD 922.2 + 920.98 AS 922.28											
ADD HYD	922.28	8	9	.16170	103.26	8.549	.99130	1.600	.998		
*S Route 922.28 flows to Ventura Interceptor AS 922.29											
ROUTE	922.29	10	11	.16170	103.02	8.549	.99130	1.600	.995		
*S Route 923 flows to 48" Ventura Line AS 923.8											
ROUTE	923.80	12	13	.00700	11.29	.370	.99238	1.500	2.519		
*S ADD 923.8 + 922.29 AS 923.9											
ADD HYD	923.90	11	13	.16870	111.89	8.919	.99134	1.600	1.036		
*S ROUTE 923.9 TO MAIN POND AS 923.91											
ROUTE	923.91	14	15	.16870	112.43	8.919	.99134	1.600	1.041		
*S Total flows entering main pond											
*S ADD 923.91 + 923.3 AS 923.92											
ADD HYD	923.92	16	15	.21020	200.77	12.833	1.14469	1.550	1.492		
*S ROUT 923.92 THRU POND AS 923.99											
ROUTE RESERVOIR	923.99	17	18	.21020	79.80	12.613	1.12509	2.000	.593	AC-FT=	4.312

*S	END: VINYARD ESTATES - UNIT IV										

*S	N D B ARROYO AT VENTURA (AP 921.99) - FINAL										
ADD HYD	921.99	18	26	3.47383	498.57	235.271	1.26988	1.950	.224		
DIVIDE HYD	950.00	1	18	1.14636	164.53	77.639	1.26988	1.950	.224		
	951.00	AND	26	2.32747	334.04	157.632	1.26988	1.950	.224		
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.10	-	3	.01200	24.21	.836	1.30669	1.500	3.153	RAIN24=	3.030
*S LA CUEVA SUBDIVISION (AP 926.1) - FINAL										PER IMP=	17.71
ROUTE RESERVOIR	926.18	3	2	.01200	6.92	.681	1.06334	1.800	.901	AC-FT=	.473
*S N D B ARROYO AT ~400 FT WEST OF VENTURA. (HYD NO. 926.19) - FINAL											
ADD HYD	926.19	1	2	3.48583	505.07	235.952	1.26917	1.950	.226		
ROUTE MCUNGE	926.17	1	2	3.48583	494.99	235.934	1.26907	2.150	.222	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.00	-	1	.03750	72.25	2.624	1.31188	1.500	3.011	RAIN24=	3.000
*S N D B ARROYO AT BARSTOW. (AP 926.79) - NOT FINAL										PER IMP=	25.00
ADD HYD	926.79	1	2	3.52333	501.43	238.558	1.26953	2.150	.222		
RAINFALL TYPE= 2											
COMPUTE NM HYD	925.20	-	1	.01400	17.65	.536	.71846	1.500	1.970	RAIN24=	3.030
ROUTE MCUNGE	925.27	1	3	.01400	16.37	.531	.71141	1.600	1.827	PER IMP=	2.00
COMPUTE NM HYD	925.10	-	1	.06400	162.00	6.547	1.91802	1.500	3.955	CCODE =	.1
*S COMBINE 925.1 & 925.27 AS 925.18 (VINYARD FLOW TO APS SITE)										PER IMP=	50.00
ADD HYD	925.18	1	3	.07800	168.77	7.078	1.70144	1.500	3.381		
*S (925.19=4-FOOT STORM SEWER ROUTING)											
ROUTE	925.19	1	2	.07800	164.62	7.078	1.70145	1.500	3.298		
COMPUTE NM HYD	925.40	-	4	.03700	96.45	3.887	1.96954	1.500	4.073	PER IMP=	50.00
*S INFLOW INTO DETENTION POND AT BARSTOW IN VINEYARD ESTATES											
*S (AP 925.48) - FINAL											
ADD HYD	925.48	2	4	.11500	261.07	10.965	1.78769	1.500	3.547		
RAINFALL TYPE= 2											
COMPUTE NM HYD	922.00	-	1	.02100	32.59	1.398	1.24859	1.550	2.425	RAIN24=	3.030
ROUTE MCUNGE	922.80	1	2	.02100	31.14	1.391	1.24223	1.650	2.317	PER IMP=	15.00
RAINFALL TYPE= 2										CCODE =	.1
COMPUTE NM HYD	924.10	-	1	.01900	24.11	.733	.72328	1.500	1.983	RAIN24=	3.030
*S INTO DETENTION POND JUST UPSTREAM OF CARRINGTON SUBDIVISION										PER IMP=	2.00
*S (AP 924.19) - (UNBULKED)											
ADD HYD	924.19	1	2	.04000	46.43	2.124	.99571	1.600	1.814		
*S THE FOLLOWING DETENTION POND INFORMATION WAS CALCULATED BY RTI USING THE											
*S DESIGN DATA IN GRADING & DRAINAGE PLAN FOR CARRINGTON SUBDIVISION											
*S (COMMUNITY SCIENCES CORPORATION, 1992)											
ROUTE RESERVOIR	924.18	1	3	.04000	3.73	2.124	.99564	2.450	.146	AC-FT=	1.851
ROUTE MCUNGE	924.17	3	2	.04000	3.72	2.124	.99544	2.700	.145	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	924.20	-	1	.00700	10.16	.335	.89814	1.500	2.267	RAIN24=	2.980
ROUTE MCUNGE	924.28	1	4	.00700	9.33	.330	.88389	1.650	2.082	PER IMP=	10.00
RAINFALL TYPE= 2										CCODE =	.1
COMPUTE NM HYD	924.30	-	1	.02700	68.50	2.753	1.91154	1.500	3.964	RAIN24=	3.000
*S CARRINGTON SUBDIVISION (AP 924.3)										PER IMP=	50.00
COMPUTE NM HYD	924.40	-	7	.01720	26.21	.862	.93974	1.500	2.381	PER IMP=	10.00
*S TOTAL CARRINGTON DISCHARGE (AP 924.38) - FINAL											
ADD HYD	924.38	1	2	.06700	68.50	4.876	1.36461	1.500	1.598		
*S TOTAL BARSTOW @ NORTH OF CARRINGTON (AP 924.48) - FINAL											
ADD HYD	924.48	7	4	.02420	29.41	1.192	.92355	1.550	1.899		
*S TOTAL BARSTOW @ CARRINGTON (AP 924.99) - FINAL											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 5	NOTATION
ADD HYD	924.99	4	3	.09120	95.81	6.068	1.24758	1.500	1.642		
*S	AP 924.91 INTO STORM SEWER										
*S	AP 924.92 INTO BARSTOW										
DIVIDE HYD	924.91	5	3	.06333	20.00	4.214	1.24758	1.350	.493		
	924.92 AND	4		.02787	75.81	1.854	1.24758	1.500	4.251		
*S	(924.81=24-INCH STORM SEWER ROUTING)										
ROUTE	924.81	3	2	.06333	21.11	4.214	1.24757	2.100	.521		
COMPUTE NM HYD	925.30	-	1	.01050	26.43	1.063	1.89760	1.500	3.933	PER IMP=	50.00
*S	COMBINE CARRINGTON FLOW WITH VINYARD FLOW										
ADD HYD	925.38	1	2	.07383	46.00	5.277	1.34001	1.500	.973		
ROUTE	924.81	5	2	.07383	45.19	5.277	1.34001	1.500	.956		
*S	BARSTOW SD WITH VINYARD SD FLOW AT POND										
ADD HYD	925.38	15	2	.18883	306.26	16.241	1.61265	1.500	2.534		
ADD HYD	925.39	5	4	.21670	382.07	18.095	1.56570	1.500	2.755		
*S	(DETENTION POND AT BARSTOW AT APS MS)										
ROUTE RESERVOIR	925.85	5	2	.21670	176.89	18.095	1.56567	1.750	1.275	AC-FT=	6.631
ADD HYD	926.99	2	11	3.74003	601.12	256.653	1.28669	2.100	.251		
*S	N D B ARROYO AT BARTSOW (AP 926.99) - FINAL										
ROUTE MCUNGE	926.70	5	3	3.74003	599.24	256.647	1.28666	2.150	.250	CCODE =	.2
ROUTE MCUNGE	926.80	3	5	3.74003	598.97	256.680	1.28682	2.150	.250	CCODE =	.2
COMPUTE NM HYD	932.40	-	3	.05740	150.03	6.135	2.00401	1.500	4.084	PER IMP=	55.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	930.00	-	1	.08500	211.88	8.468	1.86790	1.500	3.895	PER IMP=	50.00
*S	N D B ARROYO AT WYOMING (AP 930.59) -										
ADD HYD	930.59	1	3	.14240	361.91	14.603	1.92276	1.500	3.971		
*S	N D B ARROYO AT WYOMING (AP 930.99) - FINAL										
ADD HYD	930.99	1	5	3.88243	635.92	271.283	1.31015	2.150	.256		
ROUTE MCUNGE	930.80	1	12	3.88243	631.33	271.094	1.30924	2.200	.254	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	935.00	-	14	.11000	151.68	4.640	.79087	1.500	2.154	PER IMP=	5.00
ADD HYD	930.85	14	12	3.99243	641.74	275.734	1.29495	2.150	.251		
*S	*****										
*S	BEGIN: WYOMING STORM DRAIN										
*S	*****										
COMPUTE NM HYD	929.00	-	4	.02400	65.77	2.752	2.14971	1.500	4.282	PER IMP=	65.00
COMPUTE NM HYD	931.00	-	1	.06050	104.39	6.185	1.91693	1.650	2.696	PER IMP=	55.00
*S	NO POND AT LA CUEVA HIGH SCHOOL										
ADD HYD	931.80	4	1	.08450	149.28	8.937	1.98303	1.550	2.760		
COMPUTE NM HYD	932.10	-	4	.00730	19.08	.766	1.96752	1.500	4.083	PER IMP=	55.00
*S	CHURCH (AP 932.1)										
*S	WYOMING AND WILSHIRE (AP 931.99)										
ADD HYD	931.99	4	1	.09180	165.98	9.703	1.98179	1.550	2.825		
*S	(931.98=5-FOOT PIPE ROUTING)										
ROUTE	931.98	1	3	.09180	165.26	9.703	1.98180	1.550	2.813		
COMPUTE NM HYD	932.20	-	1	.00730	18.89	.766	1.96752	1.500	4.043	PER IMP=	55.00
ADD HYD	932.80	3	1	.09910	182.05	10.469	1.98074	1.550	2.870		
*S	AT CORONA W/O CORONA Q										
COMPUTE NM HYD	932.30	-	2	.03130	80.93	3.284	1.96752	1.500	4.040	PER IMP=	55.00
*S	EXISTING POND AT NORESTE BETWEEN CORONA AND ANAHEIM										
ROUTE RESERVOIR	932.84	2	5	.03130	8.98	3.276	1.96261	2.150	.448	AC-FT=	2.156
*S	COMBINED POND AND WYOMING STORM DRAIN FLOW (AP 932.99)										
ADD HYD	932.99	1	5	.13040	187.75	13.745	1.97639	1.550	2.250		
*S	*****										
*S	END: WYOMING STORM DRAIN										
*S	*****										
ROUTE MCUNGE	932.81	10	2	.13040	181.03	13.745	1.97633	1.700	2.169	CCODE =	.1
COMPUTE NM HYD	934.10	-	1	.04680	107.84	4.148	1.66192	1.500	3.600	PER IMP=	40.00
ADD HYD	934.99	1	2	.17720	230.58	17.893	1.89329	1.700	2.033		
*S	LNDBDD INFLOW HYDROGRAPH (AP 930.84) - NOT FINAL										
ADD HYD	930.84	3	13	4.16963	750.67	293.627	1.32038	2.050	.281		
COMPUTE NM HYD	937.00	-	1	.03520	51.06	1.680	.89462	1.500	2.267	PER IMP=	10.00
*S	TOTAL DAM INFLOW HYDROGRAPH (AP 930.86)-FINAL										
ADD HYD	930.86	1	11	4.20483	756.57	295.307	1.31682	2.050	.281		
*S											
*S	LOWER NORTH DOMINGO BACA DETENTION DAM										
*S	LOW FLOW OUTLET (PRINCIPLE SPILLWAY). THIS PRINCIPLE SPILLWAY JOINS										
*S	ONE OF THE FOUR BOX CULVERTS AT THE I-25. IT ALSO PICKS UP FLOWS FROM										
*S	SUB-BASINS 940.2, 939.16, 939.14 & 943.1. THE FLOWS FROM SUB-BASINS										
*S	939.14 AND 943.1 ARE INCLUDED BY SA0221 PROJECT WHICH IS ALREADY										
*S	IN PLACE. THE FOLLOWING RATING CURVE IS FOR A 40-INCH ORIFICE PLATE.										
*S	EMERGENCY SPILLWAY IS NOT USED IN THE FOLLOWING RATING CURVE.										
*S											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 6 NOTATION
ROUTE RESERVOIR	930.87	11	12	4.20483	181.28	295.307	1.31682	5.400	.067	AC-FT= 66.227
*S LNDBAD DAM OUTFLOW HYDROGRAPH (AP 930.87)										
RAINFALL TYPE= 2										
COMPUTE NM HYD	960.00	-	1	.00750	18.46	.727	1.81681	1.500	3.845	RAIN24= 2.850 PER IMP= 50.00
*S STORM DRAIN OUT OF LNDBD DAM AT LOUISIANA BLVD. (AP 930.89)										
ADD HYD	930.89	1&12	12	4.21233	181.36	296.034	1.31771	5.400	.067	
COMPUTE NM HYD	940.20	-	1	.01560	37.64	1.486	1.78583	1.500	3.770	PER IMP= 50.00
COMPUTE NM HYD	939.16	-	3	.01280	30.89	1.219	1.78582	1.500	3.770	PER IMP= 50.00
ADD HYD	940.26	1&3	3	.02840	68.53	2.705	1.78580	1.500	3.770	
*S AT THE END OF LA CUEVA OESTE PHASE 1, IN STORM DRAIN PIPE (AP 939.88)										
*S (UNBULKED)										
ADD HYD	939.88	12&3	12	4.24073	187.85	298.739	1.32085	1.500	.069	
*S STORM DRAIN PIPE FROM THE END OF LA CUEVA OESTE PHASE 1 TILL SAN PEDRO										
*S BENEATH ANAHEIM AVE -- TWO REACHES										
*S RCP DIA=4 FT TO 940.0 OUTFALL										
*S (939.78=4-FOOT PIPE ROUTING)										
ROUTE	939.78	12	15	4.24073	188.38	298.730	1.32081	1.550	.069	
COMPUTE NM HYD	940.00	-	1	.01410	34.02	1.343	1.78583	1.500	3.770	PER IMP= 50.00
ADD HYD	939.68	15&1	15	4.25483	218.75	300.073	1.32235	1.550	.080	
*S RCP DIA=4 FT TO SAN PEDRO										
ROUTE	939.58	15	16	4.25483	220.40	300.069	1.32233	1.550	.081	
*S STORM DRAIN PIPE FROM SAN PEDRO TO UTE DR. BENEATH ANAHEIM AVE.										
*S (939.48=RCP DIA=4 FT)										
ROUTE	939.48	16	14	4.25483	219.00	300.065	1.32231	1.550	.080	
COMPUTE NM HYD	939.14	-	1	.01120	29.58	1.203	2.01454	1.500	4.127	PER IMP= 60.00
COMPUTE NM HYD	943.10	-	3	.01200	31.97	1.322	2.06603	1.500	4.162	PER IMP= 65.00
*S STORM DRAIN BENEATH ANAHEIM AT UTE (AP 943.18)										
ADD HYD	943.18	1&3	3	.02320	61.55	2.526	2.04114	1.500	4.145	
*S STORM DRAIN FLOW AT I-25 (AP 943.19)										
ADD HYD	943.19	3&14	3	4.27803	273.42	302.590	1.32621	1.550	.100	
COMPUTE NM HYD	940.10	-	1	.00700	18.82	.791	2.11965	1.500	4.202	PER IMP= 70.00
ROUTE MCUNGE	940.80	1	6	.00700	18.75	.791	2.11970	1.600	4.185	CCODE = .2
COMPUTE NM HYD	943.20	-	1	.01300	17.86	.582	.83952	1.500	2.147	PER IMP= 10.00
*S OVERLAND FLOW AT I-25 (AP 943.29)										
ADD HYD	943.29	1&6	5	.02000	33.52	1.373	1.28754	1.550	2.619	
*S TOTAL NOB Q AT I-25 BOX CULVERT (943.98)										
ADD HYD	943.98	5&3	9	4.29803	306.94	303.963	1.32603	1.550	.112	
*S CORONA SD SYSTEM										
COMPUTE NM HYD	942.20	-	1	.10310	212.49	9.227	1.67810	1.550	3.220	PER IMP= 45.00
*S (ROUTE IN SD DIA=4 FT)										
ROUTE	942.29	1	5	.10310	213.70	9.227	1.67810	1.550	3.239	
COMPUTE NM HYD	942.10	-	2	.04690	108.16	4.197	1.67810	1.500	3.603	PER IMP= 45.00
*S TOTAL Q AT CORONADO MH PARK (943.99)										
ADD HYD	943.99	2&9	3	4.34493	407.26	308.161	1.32983	1.500	.146	
FINISH										

NORTH DOMINGO BACA FUTURE COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 INPUT FILE = nbac10.fut

RUN DATE (MON/DAY/YR) = 11/05/1998
 USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 100-YR RETURN PERIOD, 24-HR STORM, FUTURE CONDITIONS											
*S MUSKINGUM-CUNGE CHANNEL ROUTING											
START											
RAINFALL TYPE= 2										TIME=	.00
COMPUTE NM HYD	900.00	-	1	.44100	328.97	17.063	.72545	1.700	1.166	RAIN24=	2.670
*S ROUTE HYD. 900 THROUGH 902.1 (900.8)										PER IMP=	.00
ROUTE MCUNGE	900.80	1	2	.44100	320.94	16.998	.72271	1.850	1.137	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.390
COMPUTE NM HYD	902.10	-	5	.08870	72.14	4.340	.91736	1.600	1.271	PER IMP=	30.00
*S (902.91=NORTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.91	2& 5	2	.52970	362.68	21.338	.75530	1.850	1.070		
RAINFALL TYPE= 2										RAIN24=	2.570
COMPUTE NM HYD	901.00	-	1	.11800	165.92	4.570	.72623	1.500	2.197	PER IMP=	.00
*S 20% AVULSION FROM SOUTH OOMINGO BACA WATERSHED SUB-BASIN 400.3											
RECALL HYD	400.30	-	15	.25500	348.72	9.922	.72958	1.500	2.137		
DIVIDE HYD	400.61	15	15	.05100	69.74	1.984	.72957	1.500	2.137		
	400.6SDB AND		19	.20400	278.98	7.938					
*S COMBINE 901 AND 20% AVULSION AS 901.1											
ADD HYD	901.10	1&15	1	.16900	235.66	6.555	.72724	1.500	2.179		
*S ROUTE HYD. 901.1 THROUGH 902.2 (901.8)											
ROUTE MCUNGE	901.80	1	3	.16900	203.55	6.482	.71913	1.700	1.882	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.390
COMPUTE NM HYD	902.20	-	5	.12200	99.04	5.969	.91736	1.600	1.268	PER IMP=	30.00
*S (902.92=SOUTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.92	3& 5	3	.29100	292.63	12.451	.80223	1.700	1.571		
*S (902.93=SUM OF FLOWS AT ARROYO JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.93	2& 3	1	.82070	607.34	33.788	.77194	1.750	1.156		
ROUTE MCUNGE	902.80	1	2	.82070	606.01	33.755	.77119	1.800	1.154	CCODE =	.2
COMPUTE NM HYD	902.30	-	3	.07300	95.14	3.939	1.01170	1.500	2.036	PER IMP=	35.00
*S (902.99=TOTAL FLOW @ TRAMWAY FROM 902 SUBBASINS, 901, 900)											
ADD HYD	902.99	2& 3	1	.89370	633.76	37.694	.79083	1.800	1.108		

*S BEGIN: PRIMROSE POINTE DEVELOPMENT											

ROUTE MCUNGE	902.90	1	17	.89370	633.05	31.648	.66398	1.806	1.107	CCODE =	.2
*S ROUTE 902.9 TO PRIMROSE POINTE CONCRETE CHANNEL AS 902.99											
ROUTE MCUNGE	902.99	17	16	.89370	632.53	31.635	.66370	1.804	1.106	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	2.430
COMPUTE NM HYD	906.00	-	12	.02368	38.91	1.642	1.30021	1.500	2.567	PER IMP=	45.00
ADD HYD	906.10	16&12	2	.91738	643.63	32.966	.67379	1.804	1.096		
*S ROUTE THRU THROUGH PRIMROSE POINTE TO LOWELL											
ROUTE MCUNGE	906.19	2	16	.91738	630.57	32.874	.67189	1.900	1.074	CCODE =	.1
COMPUTE NM HYD	904.00	-	1	.04130	60.83	2.315	1.05117	1.500	2.301	PER IMP=	30.00
*S ROUTE HYDROGRAPH THRU POND NDB1											
ROUTE RESERVOIR	904.90	1	17	.04130	24.10	2.113	.95949	1.700	.912	AC-FT=	.953
COMPUTE NM HYD	905.00	-	10	.00920	13.56	.516	1.05117	1.500	2.303	PER IMP=	30.00
*S ROUTE 905 HYDROGRAPH THRU POND NDB2											
ROUTE RESERVOIR	905.90	10	18	.00920	6.89	.467	.95141	1.650	1.171	AC-FT=	.226
*S BASIN 903.0											
COMPUTE NM HYD	903.00	-	2	.08968	131.56	5.028	1.05117	1.500	2.292	PER IMP=	30.00
*S POND 3 DISCHARGE TO THE NORTH OOMINGO BACA ARROYO											
ROUTE RESERVOIR	903.90	2	24	.08968	11.58	3.801	.79467	2.150	.202	AC-FT=	3.637
ADD HYD	906.29	24&18	26	.09888	13.28	4.268	.80925	2.150	.210		
ADD HYD	906.39	17&26	1	.14018	32.79	6.381	.85352	1.700	.365		
*S ***** TOTAL NORTH DOMINGO BACA FLOW AT LOWELL STREET *****											
ADD HYD	906.99	1&16	1	1.05756	658.26	39.255	.69596	1.900	.973		

*S END: PRIMROSE POINTE DEVELOPMENT											

ROUTE MCUNGE	906.88	1	2	1.05756	649.88	38.789	.68771	1.963	.960	CCODE =	.2
RAINFALL TYPE= 2										RAIN24=	2.230
COMPUTE NM HYD	910.21	-	1	.01370	16.31	.537	.73520	1.500	1.860	PER IMP=	17.00
ADD HYD	910.29	2& 1	5	1.07126	652.25	39.323	.68827	1.963	.951		
RAINFALL TYPE= 2										RAIN24=	2.250
COMPUTE NM HYD	910.31	-	2	.15630	201.64	7.192	.86272	1.500	2.016	PER IMP=	25.00
ADD HYD	910.90	5& 2	1	1.22756	688.70	46.525	.71064	1.921	.877		
ROUTE MCUNGE	910.88	1	2	1.22756	673.72	46.430	.70919	2.050	.858	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.220
COMPUTE NM HYD	912.11	-	3	.05900	67.97	2.254	.71642	1.500	1.800	PER IMP=	17.00
*S N D B ARROYO BETWEEN BROWNING AND EUBANK (AP 912.95) - UNBULKED											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
ADD HYD	912.95	3& 2	1	1.28656	681.80	48.685	.70952	2.050	.828		
RAINFALL TYPE= 2											
COMPUTE NM HYD	912.21	-	2	.19600	201.16	7.489	.71642	1.550	1.604	RAIN24= 2.220	
ADD HYD	912.98	1& 2	15	1.48256	712.14	56.174	.71043	2.000	.751	PER IMP= 17.00	
RAINFALL TYPE= 2											
COMPUTE NM HYD	402.50	-	4	.02800	52.16	2.306	1.54415	1.500	2.911	RAIN24= 2.230	
COMPUTE NM HYD	402.60	-	5	.02430	45.27	2.001	1.54415	1.500	2.911	PER IMP= 70.00	
*S COMBINE 402.5 AND 402.6 AS 402.9 (AT BROWNING)										PER IMP= 70.00	
ADD HYD	402.90	4& 5	11	.05230	97.43	4.307	1.54414	1.500	2.911		
*S ROUTE 402.9 AS 402.8 IN 42-INCH SD TO END OF 915.2											
ROUTE	402.80	11	6	.05230	95.66	4.307	1.54415	1.500	2.858		
COMPUTE NM HYD	915.20	-	2	.03700	68.99	3.047	1.54415	1.500	2.913	PER IMP= 70.00	
ADD HYD	915.90	6& 2	11	.08930	164.64	7.354	1.54414	1.500	2.881		
*S ROUTE 915.9 AS 915.8 TO NDB DAM IN 54-INCH PIPE											
ROUTE	915.80	11	7	.08930	157.85	7.354	1.54415	1.550	2.762		
*S COMBINE HYD'S 912.98 AND 915.8 AS 912.99 (FLOW TO NDB DAM FROM PDN DRAIN)											
ADD HYD	912.99	15& 7	15	1.57186	751.34	63.528	.75779	2.000	.747		
*S*****											
*S AP 105.9 IS FROM "LA CUEVA, EL CAMINO & NORTH CAMINO ARROYOS - DRAINAGE											
*S MANAGEMENT PLAN" (RESOURCE TECHNOLOGY, INC., 1996).											
*S FILENAME: LCDV100X.FUT											
*S*****											
*S AP 105.9 IS FROM LA CUEVA TRIBUTARY (DIVERTED TO UPPER N. D. B. DAM											
*S THROUGH LA CUEVA DIVERSION CHANNEL)											
*S 10-YR FUTURE CND HYD											
RECALL HYD	105.90	-	17	1.17410	766.40	48.832	.77982	1.750	1.020		
ROUTE MCUNGE	105.80	17	2	1.17410	764.04	48.883	.78065	1.742	1.017	CCODE = .2	
COMPUTE NM HYD	911.00	-	3	.05800	57.52	2.248	.72671	1.550	1.550	PER IMP= 17.00	
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. FROM LA CUEVA DIVERSION											
*S CHANNEL (HYD NO. 911.99) - UNBULKED											
ADD HYD	911.99	2& 3	1	1.23210	792.16	51.129	.77807	1.742	1.005		
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. (HYD NO. 914.79)											
*S (BULKED)											
ADD HYD	914.79	15& 1	1	2.80396	1299.18	114.646	.76664	1.698	.724		
*S ROUTING THROUGH UPPER NORTH DOMINGO BACA DAM											
*S											
*S THIS STORAGE OUTFLOW DATA WAS BASED ON TOPOGRAPHY IN JUNE 1994.											
*S OUTFLOW IS TAKEN FROM "DESIGN ANALYSIS REPORT FOR LA CUEVA DIVERSION											
*S CHANNEL" (RESOURCE TECHNOLOGY, INC., 1994). STORAGE INFORMATION WAS											
*S CALCULATED AT RTI - PROPOSED FUTURE CONDITION EXCAVATION.											
*S THE ELEVATION OF EMERGENCY SPILLWAY IS AT 5749.											
ROUTE RESERVOIR	914.99	1	2	2.80396	129.27	114.554	.76602	2.904	.072	AC-FT= 84.852	
*S UPPER N.O.B. DAM OUTFLOW HYDROGRAPH AT EUBANK. (HYD NO. 914.99)											
*S AP 914.99 IS THROUGH THE PRINCIPAL SPILLWAY											
*S AP NDB.SPILL IS THROUGH THE EMERGENCY SPILLWAY											
DIVIOE HYD	914.99	2	2	2.80396	129.27	114.554	.76602	2.904	.072		
NDB.SPILL AND	20	20		.00000	.00	.000	.00000	-.045	.000		
ROUTE MCUNGE	914.68	2	3	2.80396	129.28	114.552	.76601	2.950	.072	CCODE = .2	
ROUTE	SPILL.ROU	20	19	.00000	.00	.000	.00000	-.050	.000		
RAINFALL TYPE= 2											
COMPUTE NM HYD	914.20	-	1	.03500	44.99	1.631	.87376	1.500	2.009	RAIN24= 2.130	
*S UNDBD DAM SPILL AT EUBANK (AP 914.29)										PER IMP= 30.00	
ADD HYD	914.29	1&19	1	.03500	44.99	1.631	.87375	1.500	2.009		
ROUTE MCUNGE	914.28	1	2	.03500	40.82	1.631	.87377	1.650	1.822	CCODE = .1	
*S N D B ARROYO BETWEEN EUBANK AND HOLBROOK											
*S (AP 914.78)											
ADD HYD	914.78	2& 3	2	2.83896	129.99	116.183	.76733	2.750	.072		
ROUTE MCUNGE	914.88	2	3	2.83896	129.96	116.183	.76733	2.900	.072	CCODE = .1	
RAINFALL TYPE= 2											
COMPUTE NM HYD	919.10	-	1	.07900	88.16	2.863	.67950	1.500	1.744	RAIN24= 2.100	
*S N D B ARROYO HOLBROOK (HYD NO. 919.19) - FINAL										PER IMP= 17.00	
ADD HYD	919.19	1& 3	4	2.91796	136.13	119.046	.76496	1.800	.073		
COMPUTE NM HYD	919.20	-	1	.04270	47.67	1.547	.67950	1.500	1.744	PER IMP= 17.00	
*S HOLBROOK (HYD NO. 919.29) NOT FINAL											
ADD HYD	919.29	1& 4	22	2.96066	153.96	120.593	.76372	1.550	.081		
RAINFALL TYPE= 2											
COMPUTE NM HYD	920.20	-	3	.00850	9.24	.300	.66087	1.500	1.698	RAIN24= 2.070	
RAINFALL TYPE= 2										PER IMP= 17.00	
COMPUTE NM HYD	913.10	-	1	.03600	40.82	1.343	.69964	1.500	1.772	RAIN24= 2.170	
ROUTE MCUNGE	913.88	1	2	.03600	34.88	1.343	.69965	1.800	1.514	PER IMP= 17.00	
RAINFALL TYPE= 2										CCODE = .1	
COMPUTE NM HYD	917.00	-	1	.04060	36.32	1.325	.61202	1.500	1.398	RAIN24= 2.100	
*S HOLBROOK (AP 917.99) - FINAL										PER IMP= 17.00	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ADD HYD	917.99	1& 2	21	.07660	44.45	2.669	.65319	1.750	.907	
*S (AP 920.29)										
ADD HYD	920.29	21& 3	4	.08510	47.83	2.968	.65395	1.500	.878	
*S ROUTE IN HOLBROOK TO CORONA IN 48" SD										
ROUTE	920.28	4	9	.08510	46.73	2.968	.65396	1.550	.858	
RAINFALL TYPE= 2										
COMPUTE NM HYD	913.20	-	1	.01700	22.16	.792	.87376	1.500	2.037	RAIN24= 2.130
ROUTE MCUNGE	913.18	1	2	.01700	21.98	.792	.87378	1.700	2.021	PER IMP= 30.00
RAINFALL TYPE= 2										
COMPUTE NM HYD	918.00	-	1	.11400	79.28	4.131	.67950	1.650	1.087	RAIN24= 2.100
*S HOLBROOK (AP 918.88) - FINAL										PER IMP= 17.00
ADD HYD	918.88	1& 2	1	.13100	96.73	4.924	.70471	1.650	1.154	
*S ADD 920.28 AND 918.89 AND ROUTE TO NDB										
ADD HYD	918.89	1& 9	1	.21610	139.37	7.892	.68472	1.700	1.008	
*S ROUTE IN HOLBROOK TO CARMEL IN 66"SD										
ROUTE	918.99	1	8	.21610	139.05	7.892	.68472	1.700	1.005	
RAINFALL TYPE= 2										
COMPUTE NM HYD	921.40	-	6	.01440	15.64	.508	.66087	1.500	1.697	RAIN24= 2.070
COMPUTE NM HYD	921.30	-	7	.01400	15.21	.493	.66087	1.500	1.697	PER IMP= 17.00
*S COMBINER ALL HOLBROOK AND NDB FLOWS AT HOLBROOK CROSSING										PER IMP= 17.00
*S ADD 918.99 AND 920.4 AS 920.49										
ADD HYD	920.49	8& 6	6	.23050	145.46	8.399	.68323	1.700	.986	
*S ADD 920.49 TO 919.29 AS 919.99										
ADD HYD	919.99	6&22	20	3.19116	293.11	128.992	.75791	1.700	.144	
*S BETWEEN HOLBROOK AND VENTURA (AP 919.18) - NOT FINAL										
ROUTE MCUNGE	919.18	20	5	3.19116	292.55	128.993	.75791	1.750	.143	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	921.10	-	1	.02920	31.71	1.029	.66087	1.500	1.697	RAIN24= 2.070
*S UPSTREAM OF END OF VINYARD ESTATES SUBDIVISION UNIT IV BETWEEN										PER IMP= 17.00
*S HOLBROOK & VENTURA										
*S *****										
*S BEGIN: VINYARD ESTATES - UNIT IV SE CORNER										
*S *****										
*S ROUTE 921.19										
*S (48-INCH RCP)										
ROUTE	921.80	1	12	.02920	31.76	1.029	.66088	1.500	1.700	
*S (54-INCH RCP)										
ROUTE	921.89	12	16	.02920	31.84	1.029	.66088	1.500	1.704	
*S COMBINE 921.89 & 921.3 AS 921.98										
ADD HYD	921.98	16& 7	15	.04320	47.05	1.523	.66085	1.500	1.702	
*S *****										
*S END: VINYARD ESTATES - UNIT IV										
*S *****										
*S N D B ARROYO BETWEEN HOLBROOK & VENTURA (AP 921.78) - FINAL										
ADD HYD	921.78	5&15	1	3.23436	321.26	130.515	.75661	1.600	.155	
ROUTE MCUNGE	921.80	1	2	3.23436	318.91	130.504	.75655	1.600	.154	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	921.20	-	1	.08740	145.56	5.935	1.27323	1.500	2.602	RAIN24= 2.050
*S N D B ARROYO AT VENTURA (AP 921.9) - NOT FINAL										PER IMP= 60.00
ADD HYD	921.90	1& 2	26	3.32176	419.49	136.439	.77014	1.600	.197	
RAINFALL TYPE= 2										
COMPUTE NM HYD	920.10	-	17	.02020	21.94	.712	.66087	1.500	1.697	RAIN24= 2.070
COMPUTE NM HYD	920.00	-	5	.03000	32.58	1.057	.66087	1.500	1.697	PER IMP= 17.00
*S *****										
*S BEGIN: VINYARD ESTATES - UNIT IV										
*S *****										
RAINFALL TYPE= 2										
COMPUTE NM HYD	922.10	-	2	.00700	10.43	.412	1.10436	1.500	2.328	RAIN24= 2.020
COMPUTE NM HYD	923.00	-	12	.00700	12.15	.515	1.37845	1.500	2.711	PER IMP= 50.00
COMPUTE NM HYD	922.20	-	9	.01480	21.04	.784	.99368	1.500	2.221	PER IMP= 70.00
COMPUTE NM HYD	922.30	-	16	.04150	58.97	2.199	.99368	1.500	2.220	PER IMP= 40.00
*S COMBINE 920 + 922.1 AS 922.19										
ADD HYD	922.19	5& 2	4	.03700	43.00	1.470	.74475	1.500	1.816	
*S ROUTE 922.19 THRU 30" PIPE AS 920.98										
ROUTE	920.98	4	8	.03700	42.40	1.470	.74477	1.500	1.791	
*S ADD 922.2 + 920.98 AS 922.28										
ADD HYD	922.28	8& 9	10	.05180	63.43	2.254	.81587	1.500	1.913	
*S Route 922.28 flows to Ventura Interceptor AS 922.29										
ROUTE	922.29	10	11	.05180	63.13	2.254	.81589	1.500	1.904	
RAINFALL TYPE= 2										
COMPUTE NM HYD	922.00	-	1	.02100	17.26	.713	.63633	1.550	1.284	RAIN24= 2.020
*S COMBINE 922 AND 923 AS 923.99										PER IMP= 17.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4	NOTATION
ADD HYD	920.98	1&12	1	.02800	27.97	1.227	.82183	1.550	1.561		
*S ROUTE 920.1 DOWN WILSHIRE TO VENTURA											
ROUTE	920.18	17	12	.02020	21.34	.712	.66089	1.550	1.651		
*S COMBINE 920.18 AND 920.98 AS 920.99											
ADD HYD	920.99	1&12	1	.04820	49.31	1.939	.75437	1.550	1.599		
*S Route 923.99 FLOW FROM NO OF VINYARD IV TO 48" Ventura Line AS 923.8											
ROUTE	923.80	1	13	.04820	49.65	1.939	.75439	1.550	1.609		
*S ADD 923.8 + 922.29 AS 923.9											
ADD HYD	923.90	11&13	14	.10000	110.91	4.193	.78623	1.550	1.733		
*S ROUTE 923.9 TO NDB (NOT FINAL)											
ROUTE	923.91	14	15	.10000	113.48	4.193	.78624	1.550	1.773		
*S ADD 923.91 + 923.3 AS 923.92											
ADD HYD	923.92	16&15	18	.14150	166.41	6.393	.84707	1.550	1.838		
*S NO POND AT VENTURA											
*S*****											
*S											
END: VINYARD ESTATES - UNIT IV											
*S*****											
*S N O B ARROYO AT VENTURA (AP 921.99) - FINAL											
ADD HYD	921.99	18&26	1	3.46326	581.05	142.831	.77328	1.550	.262		
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.10	-	2	.01200	18.12	.709	1.10839	1.500	2.360	RAIN24= 2.020	PER IMP= 50.00
*S LA CUEVA SUBDIVISION (AP 926.1) - NO POND FINAL											
*S N O B ARROYO AT -400 FT WEST OF VENTURA. (HYD NO. 926.19) - FINAL											
ADD HYD	926.19	1&2	1	3.47526	597.21	143.541	.77444	1.550	.269		
ROUTE MCUNGE	926.17	1	2	3.47526	574.30	143.524	.77435	1.650	.258	CCODE = .1	
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.00	-	1	.05780	91.71	3.630	1.17740	1.500	2.479	RAIN24= 2.000	PER IMP= 55.56
*S N D B ARROYO AT BARSTOW. (AP 926.79) - NOT FINAL											
ADD HYD	926.79	1&2	11	3.53306	628.88	147.154	.78095	1.600	.278		
RAINFALL TYPE= 2											
COMPUTE NM HYD	925.20	-	1	.00940	14.20	.556	1.10839	1.500	2.361	RAIN24= 2.020	PER IMP= 50.00
ROUTE MCUNGE	925.27	1	3	.00940	13.07	.551	1.09910	1.600	2.172	CCODE = .1	
COMPUTE NM HYD	925.10	-	1	.06400	96.60	3.783	1.10839	1.500	2.359	PER IMP= 50.00	
*S COMBINE 925.1 & 925.27 AS 925.18 (VINYARD FLOW TO APS SITE)											
ADD HYD	925.18	1&3	1	.07340	103.87	4.334	1.10719	1.500	2.211		
*S (925.19=4.5-FOOT STORM SEWER ROUTING)											
ROUTE	925.19	1	2	.07340	99.09	4.334	1.10720	1.550	2.109		
COMPUTE NM HYD	925.40	-	4	.03700	58.52	2.255	1.14297	1.500	2.471	PER IMP= 50.00	
*S INFLOW NDB FROM VINYARD SD AND APS MID SCHOOL 925.48											
*S (AP 925.48) - FINAL											
ADD HYD	925.48	2&4	15	.11040	151.18	6.590	1.11918	1.550	2.140		
RAINFALL TYPE= 2											
COMPUTE NM HYD	924.30	-	1	.02700	41.20	1.593	1.10648	1.500	2.384	RAIN24= 2.000	PER IMP= 50.00
*S CARRINGTON SUBDIVISION (AP 924.3)											
COMPUTE NM HYD	924.40	-	7	.01720	25.91	1.006	1.09694	1.500	2.354	PER IMP= 50.00	
*S TOTAL BARSTOW @ CARRINGTON (AP 924.99) - FINAL											
ADD HYD	924.99	1&7	5	.04420	67.11	2.600	1.10275	1.500	2.372		
COMPUTE NM HYD	925.30	-	1	.01050	15.82	.614	1.09694	1.500	2.354	PER IMP= 50.00	
*S COMBINE CARRINGTON FLOW WITH VINYARD FLOW 925.3 + 924.99 AS 925.38											
ADD HYD	925.38	1&5	5	.05470	82.93	3.214	1.10163	1.500	2.369		
*S (STORM SEWER FROM GREEN ARBOR TO NDB)											
ROUTE	924.81	5	2	.05470	79.28	3.214	1.10165	1.550	2.265		
*S BARSTOW SD WITH VINYARD SD FLOW AT NDB											
ADD HYD	925.38	15&2	6	.16510	230.46	9.804	1.11337	1.550	2.181		
*S BARSTOW DIVERSION											
COMPUTE NM HYD	926.20	-	1	.04700	65.15	2.418	.96448	1.500	2.166	PER IMP= 40.00	
ROUTE	926.28	1	2	.04700	62.70	2.418	.96448	1.500	2.084		
*S N D B ARROYO AT BARTSOW (AP 926.99) - FINAL											
ADD HYD	926.89	11&2	2	3.58006	679.27	149.572	.78336	1.600	.296		
ADD HYD	926.99	2&6	5	3.74516	875.93	159.375	.79790	1.600	.365		
ROUTE MCUNGE	926.70	5	3	3.74516	856.27	159.323	.79765	1.650	.357	CCODE = .1	
ROUTE MCUNGE	926.80	3	5	3.74516	845.31	159.217	.79712	1.700	.353	CCODE = .1	
COMPUTE NM HYD	932.40	-	3	.05740	91.59	3.605	1.17753	1.500	2.493	PER IMP= 55.00	
RAINFALL TYPE= 2											
COMPUTE NM HYD	930.00	-	1	.08500	136.59	5.499	1.21296	1.500	2.511	RAIN24= 1.970	PER IMP= 60.00
*S N D B ARROYO AT WYOMING (AP 930.59) -											
ADD HYD	930.59	1&3	1	.14240	228.18	9.104	1.19867	1.500	2.504		
*S N O B ARROYO AT WYOMING (AP 930.99) - FINAL											
ADD HYD	930.99	1&5	1	3.88756	944.36	168.321	.81183	1.700	.380		
ROUTE MCUNGE	930.80	1	12	3.88756	935.63	168.408	.81224	1.750	.376	CCODE = .2	
COMPUTE NM HYD	935.00	-	14	.11000	143.07	5.401	.92069	1.500	2.032	PER IMP= 40.00	
ADD HYD	930.85	14&12	13	3.99756	987.12	173.809	.81523	1.700	.386		
*S*****											
*S											
BEGIN: WYOMING STORM DRAIN											
*S*****											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 5	NOTATION
COMPUTE NM HYD	929.00	-	4	.02400	41.00	1.658	1.29529	1.500	2.669	PER IMP=	65.00
COMPUTE NM HYD	931.00	-	1	.06050	66.31	3.878	1.20180	1.650	1.713	PER IMP=	60.00
*S NO POND AT LA CUEVA HIGH SCHOOL FOR FUTURE CONDITION											
ADD HYD	931.80	4& 1	1	.08450	93.78	5.536	1.22834	1.550	1.734		
COMPUTE NM HYD	932.10	-	4	.00730	11.58	.450	1.15472	1.500	2.479	PER IMP=	55.00
*S CHURCH (AP 932.1)											
*S WYOMING AND WILSHIRE (AP 931.99)											
ADD HYD	931.99	4& 1	1	.09180	103.90	5.985	1.22248	1.550	1.768		
*S (931.98=5-FOOT PIPE ROUTING)											
ROUTE	931.98	1	3	.09180	103.49	5.985	1.22249	1.550	1.762		
COMPUTE NM HYD	932.20	-	1	.00730	11.46	.450	1.15472	1.500	2.452	PER IMP=	55.00
ADD HYD	932.80	3& 1	1	.09910	113.67	6.435	1.21749	1.550	1.792		
COMPUTE NM HYD	932.30	-	2	.03130	49.09	1.928	1.15472	1.500	2.450	PER IMP=	55.00
ADD HYD	932.00	2& 1	10	.13040	157.29	8.362	1.20242	1.550	1.885		
*S NO POND AT NORESTE BETWEEN CORONA AND ANAHEIM											

*S END: WYOMING STORM DRAIN											

ROUTE MCUNGE	932.81	10	2	.13040	150.60	8.362	1.20242	1.700	1.805	CCODE =	.1
COMPUTE NM HYD	934.10	-	1	.10310	165.19	6.670	1.21296	1.500	2.503	PER IMP=	60.00
ADD HYD	934.99	1& 2	3	.23350	232.14	15.032	1.20707	1.650	1.553		
*S LNDBDD INFLOW HYDROGRAPH (AP 930.84) - NOT FINAL											
ADD HYD	930.84	3&13	11	4.23106	1209.93	188.841	.83685	1.700	.447		
COMPUTE NM HYD	937.00	-	1	.04520	72.91	2.924	1.21296	1.500	2.520	PER IMP=	60.00
*S TOTAL DAM INFLOW HYDROGRAPH (930.86)-FINAL											
ADD HYD	930.86	1&11	11	4.27626	1241.53	191.765	.84083	1.700	.454		
*S LOWER NORTH DOMINGO BACA DETENTION DAM											
*S LOW FLOW OUTLET (PRINCIPLE SPILLWAY). THIS PRINCIPLE SPILLWAY JOINS											
*S ONE OF THE FOUR BOX CULVERTS AT THE I-25. IT ALSO PICKS UP FLOWS FROM											
*S SUB-BASINS 940.2, 939.16, 939.14 & 943.1. THE FLOWS FROM SUB-BASINS											
*S 939.14 AND 943.1 ARE INCLUDED BY SAD221 PROJECT WHICH IS ALREADY											
*S IN PLACE. THE FOLLOWING RATING CURVE IS FOR A 40-INCH ORIFICE PLATE.											
*S EMERGENCY SPILLWAY IS NOT USED IN THE FOLLOWING RATING CURVE.											
*S											
ROUTE RESERVOIR	930.87	11	12	4.27626	171.36	191.870	.84129	2.900	.063	AC-FT=	51.290
*S LNDBAD DAM OUTFLOW HYDROGRAPH (AP 930.87)											
RAINFALL TYPE= 2											
COMPUTE NM HYD	960.00	-	1	.00750	11.02	.418	1.04522	1.500	2.295	PER IMP=	50.00
*S STORM DRAIN OUT OF LNDBD DAM AT LOUISIANA BLVD. (AP 930.89)											
ADD HYD	930.89	1&12	12	4.28376	171.46	192.288	.84164	2.850	.063		
COMPUTE NM HYD	940.20	-	1	.01560	22.23	.853	1.02474	1.500	2.226	PER IMP=	50.00
COMPUTE NM HYD	939.16	-	3	.01280	18.24	.700	1.02474	1.500	2.227	PER IMP=	50.00
ADD HYD	940.26	1& 3	3	.02840	40.47	1.552	1.02472	1.500	2.226		
*S AT THE END OF LA CUEVA OESTE PHASE 1, IN STORM DRAIN PIPE (AP 939.88)											
*S (UNBULKED)											
ADD HYD	939.88	12& 3	12	4.31216	172.58	193.840	.84285	2.050	.063		
*S STORM DRAIN PIPE FROM THE END OF LA CUEVA OESTE PHASE 1 TILL SAN PEDRO											
*S BENEATH ANAHEIM AVE -- TWO REACHES											
*S RCP DIA=4 FT TO 940.0 OUTFALL											
*S (939.78=4-FOOT PIPE ROUTING)											
ROUTE	939.78	12	15	4.31216	172.54	193.840	.84285	2.050	.063		
COMPUTE NM HYD	940.00	-	1	.01410	20.09	.771	1.02474	1.500	2.227	PER IMP=	50.00
ADD HYD	939.68	15& 1	15	4.32626	176.38	194.611	.84344	1.550	.064		
*S RCP DIA=4 FT TO SAN PEDRO											
ROUTE	939.58	15	16	4.32626	177.23	194.611	.84344	1.550	.064		
*S STORM DRAIN PIPE FROM SAN PEDRO TO UTE DR. BENEATH ANAHEIM AVE.											
*S (939.48=RCP DIA=4 FT)											
ROUTE	939.48	16	14	4.32626	176.28	194.611	.84344	1.550	.064		
COMPUTE NM HYD	939.14	-	1	.01120	18.32	.714	1.19546	1.500	2.556	PER IMP=	60.00
COMPUTE NM HYD	943.10	-	3	.01200	19.85	.793	1.23850	1.500	2.585	PER IMP=	65.00
*S STORM DRAIN BENEATH ANAHEIM AT UTE (AP 943.18)											
ADD HYD	943.18	1& 3	3	.02320	38.17	1.507	1.21770	1.500	2.571		
*S STORM DRAIN FLOW AT I-25 (AP 943.19)											
ADD HYD	943.19	3&14	3	4.34946	210.06	196.118	.84544	1.550	.075		
COMPUTE NM HYD	940.10	-	1	.00700	11.73	.479	1.28349	1.500	2.618	PER IMP=	70.00
ROUTE MCUNGE	940.80	1	6	.00700	11.70	.479	1.28353	1.600	2.611	CCODE =	.2
COMPUTE NM HYD	943.20	-	1	.03750	62.80	2.567	1.28348	1.500	2.617	PER IMP=	70.00
*S OVERLAND FLOW AT I-25 (AP 943.29)											
ADD HYD	943.29	1& 6	5	.04450	70.05	3.046	1.28347	1.500	2.459		
*S TOTAL NOB Q AT I-25 BOX CULVERT (943.98)											
ADD HYD	943.98	5& 3	9	4.39396	275.64	199.164	.84988	1.550	.098		
*S CORONA SD SYSTEM											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 6 NOTATION
COMPUTE NM HYD	942.20	-	1	.10310	136.08	5.996	1.09048	1.550	2.062	PER IMP= 55.00
*S (ROUTE IN SO DIA=4 FT)										
ROUTE	942.29	1	5	.10310	138.34	5.996	1.09048	1.550	2.097	
COMPUTE NM HYD	942.10	-	2	.04690	80.87	3.270	1.30728	1.500	2.694	PER IMP= 70.00
*S CORONA SD AT I-25										
ADD HYD	942.59	2& 5	3	.15000	209.60	9.266	1.15826	1.550	2.183	
*S TOTAL Q AT CORONADO MH PARK (943.99)										
ADD HYD	943.99	3& 9	3	4.54396	485.23	208.430	.86005	1.550	.167	
COMPUTE NM HYD	443.20	-	2	.07030	121.03	4.901	1.30728	1.500	2.690	PER IMP= 70.00
FINISH										

NORTH DOMINGO BACA FUTURE CONDITION 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 INPUT FILE = nbac100.fut

RUN DATE (MON/DAY/YR) = 11/05/1998
 USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 100-YR RETURN PERIOD, 24-HR STORM, FUTURE CONDITIONS											
*S MUSKINGUM-CUNGE CHANNEL ROUTING											
START											
RAINFALL TYPE= 2										TIME=	.00
COMPUTE NM HYD	900.00	-	1	.44100	640.16	33.763	1.43550	1.700	2.268	RAIN24=	4.000
*S ROUTE HYD. 900 THROUGH 902.1 (900.8)										PER IMP=	.00
ROUTE MCUNGE	900.80	1	2	.44100	625.07	33.593	1.42827	1.800	2.215	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.580
COMPUTE NM HYD	902.10	-	5	.08870	134.65	8.032	1.69784	1.600	2.372	PER IMP=	30.00
*S (902.91=NORTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.91	2& 5	2	.52970	719.50	41.625	1.47341	1.800	2.122		
RAINFALL TYPE= 2										RAIN24=	3.860
COMPUTE NM HYD	901.00	-	1	.11800	281.25	9.218	1.46480	1.500	3.724	PER IMP=	.00
*S 20% AVULSION FROM SOUTH DOMINGO BACA WATERSHED SUB-BASIN 400.3											
RECALL HYD	400.30	-	15	.25500	620.57	19.788	1.45500	1.500	3.802		
DIVIDE HYD	400.61	15	15	.05100	124.11	3.958	1.45500	1.500	3.802		
	400.6SDB	AND	19	.20400	496.45	15.830	1.45500	1.500	3.802		
*S COMBINE 901 AND 20% AVULSION AS 901.1											
ADD HYD	901.10	1&15	1	.16900	405.36	13.176	1.46184	1.500	3.748		
*S ROUTE HYD. 901.1 THROUGH 902.2 (901.8)											
ROUTE MCUNGE	901.80	1	3	.16900	364.15	13.055	1.44840	1.700	3.367	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.580
COMPUTE NM HYD	902.20	-	5	.12200	186.80	11.047	1.69784	1.600	2.392	PER IMP=	30.00
*S (902.92=SOUTH BRANCH OF ARROYO AT JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.92	3& 5	3	.29100	536.71	24.102	1.55297	1.650	2.882		
*S (902.93=SUM OF FLOWS AT ARROYO JUNCTION EAST OF TRAMWAY LANE)											
ADD HYD	902.93	2& 3	1	.82070	1172.79	65.727	1.50162	1.750	2.233		
ROUTE MCUNGE	902.80	1	2	.82070	1158.45	65.601	1.49874	1.800	2.206	CCODE =	.1
COMPUTE NM HYD	902.30	-	3	.07300	169.92	7.105	1.82479	1.500	3.637	PER IMP=	35.00
*S (902.99=TOTAL FLOW @ TRAMWAY FROM 902 SUBBASINS, 901, 900)											
ADD HYD	902.99	2& 3	1	.89370	1211.61	72.705	1.52537	1.800	2.118		

*S BEGIN: PRIMROSE POINTE DEVELOPMENT											

ROUTE MCUNGE	902.90	1	17	.89370	1208.11	72.707	1.52542	1.800	2.112	CCODE =	.2
*S ROUTE 902.9 TO PRIMROSE POINTE CONCRETE CHANNEL AS 902.99											
ROUTE MCUNGE	902.99	17	16	.89370	1195.81	72.696	1.52518	1.850	2.091	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.650
COMPUTE NM HYD	906.00	-	12	.02368	63.36	2.816	2.22941	1.500	4.181	PER IMP=	45.00
ADD HYD	906.10	16&12	2	.91738	1214.08	75.512	1.54336	1.850	2.068		
*S ROUTE THRU THROUGH PRIMROSE POINTE TO LOWELL											
ROUTE MCUNGE	906.19	2	16	.91738	1189.60	75.371	1.54048	1.950	2.026	CCODE =	.1
COMPUTE NM HYD	904.00	-	1	.04130	102.46	4.183	1.89923	1.500	3.876	PER IMP=	30.00
*S ROUTE HYDROGRAPH THRU POND NOB1											
ROUTE RESERVOIR	904.90	1	17	.04130	35.03	3.899	1.77023	1.800	1.325	AC-FT=	1.773
COMPUTE NM HYD	905.00	-	10	.00920	22.84	.932	1.89922	1.500	3.879	PER IMP=	30.00
*S ROUTE 905 HYDROGRAPH THRU POND NOB2											
ROUTE RESERVOIR	905.90	10	18	.00920	15.30	.839	1.71009	1.600	2.599	AC-FT=	.296
*S BASIN 903.0											
COMPUTE NM HYD	903.00	-	2	.08968	223.26	9.084	1.89923	1.500	3.890	PER IMP=	30.00
*S POND 3 DISCHARGE TO THE NORTH DOMINGO BACA ARROYO											
ROUTE RESERVOIR	903.90	2	24	.08968	107.77	7.719	1.61393	1.700	1.878	AC-FT=	4.303
ADD HYD	906.29	24&18	26	.09888	121.85	8.558	1.62288	1.700	1.926		
ADD HYD	906.39	17&26	1	.14018	156.53	12.458	1.66629	1.700	1.745		
*S ***** TOTAL NORTH DOMINGO BACA FLOW AT LOWELL STREET *****											
ADD HYD	906.99	1&16	1	1.05756	1291.88	87.829	1.55716	1.900	1.909		

*S END: PRIMROSE POINTE DEVELOPMENT											

ROUTE MCUNGE	906.88	1	2	1.05756	1281.79	87.795	1.55657	2.000	1.894	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.350
COMPUTE NM HYD	910.21	-	1	.01370	30.01	1.073	1.46801	1.500	3.423	PER IMP=	17.00
ADD HYD	910.29	2& 1	5	1.07126	1286.34	88.868	1.55543	2.000	1.876		
RAINFALL TYPE= 2										RAIN24=	3.380
COMPUTE NM HYD	910.31	-	2	.15630	368.32	13.622	1.63413	1.500	3.682	PER IMP=	25.00
ADD HYD	910.90	5& 2	1	1.22756	1344.84	102.490	1.56545	2.000	1.712		
ROUTE MCUNGE	910.88	1	2	1.22756	1334.29	102.279	1.56224	2.050	1.698	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.330
COMPUTE NM HYD	912.11	-	3	.05900	126.55	4.520	1.43652	1.500	3.351	PER IMP=	17.00
*S N D B ARROYO BETWEEN BROWNING AND EUBANK (AP 912.95) - UNBULKED											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
ADD HYD	912.95	3& 2	1	1.28656	1350.52	106.800	1.55647	2.050	1.640		
RAINFALL TYPE= 2											
COMPUTE NM HYD	912.21	-	2	.19600	391.52	15.016	1.43652	1.500	3.121	RAIN24=	3.330
ADD HYD	912.98	1& 2	15	1.48256	1408.79	121.816	1.54061	2.050	1.485	PER IMP=	17.00
RAINFALL TYPE= 2											
COMPUTE NM HYD	402.50	-	4	.02800	82.16	3.763	2.51978	1.500	4.585	RAIN24=	3.350
RAINFALL TYPE= 2										PER IMP=	70.00
COMPUTE NM HYD	402.60	-	5	.02430	70.94	3.243	2.50242	1.500	4.561	RAIN24=	3.330
*S COMBINE 402.5 AND 402.6 AS 402.9 (AT BROWNING)										PER IMP=	70.00
ADD HYD	402.90	4& 5	11	.05230	153.10	7.006	2.51170	1.500	4.574		
*S ROUTE 402.9 AS 402.8 IN 42-INCH SD TO END OF 915.2 ROUTE	402.80	11	6	.05230	150.80	7.006	2.51171	1.500	4.505		
RAINFALL TYPE= 2											
COMPUTE NM HYD	915.20	-	2	.03700	107.40	4.819	2.44196	1.500	4.535	RAIN24=	3.250
ADD HYD	915.90	6& 2	11	.08930	258.20	11.825	2.48280	1.500	4.518	PER IMP=	70.00
*S ROUTE 915.9 AS 915.8 TO NDB DAM IN 54-INCH PIPE											
ROUTE	915.80	11	7	.08930	247.33	11.825	2.48281	1.550	4.328		
*S COMBINE HYD'S 912.98 AND 915.8 AS 912.99 (FLOW TO NDB DAM FROM PDN DRAIN)											
ADD HYD	912.99	15& 7	15	1.57186	1465.10	133.641	1.59414	2.050	1.456		
*S*****											
*S AP 105.9 IS FROM "LA CUEVA, EL CAMINO & NORTH CAMINO ARROYOS - DRAINAGE											
*S MANAGEMENT PLAN" (RESOURCE TECHNOLOGY, INC., 1996).											
*S FILENAME: LCOV100X.FUT											
*S*****											
*S AP 105.9 IS FROM LA CUEVA TRIBUTARY (DIVERTED TO UPPER N. O. B. DAM											
*S THROUGH LA CUEVA DIVERSION CHANNEL)											
RECALL HYD	105.90	-	17	1.14850	1487.55	94.222	1.53823	1.650	2.024		
ROUTE MCUNGE	105.80	17	2	1.14850	1480.99	94.104	1.53631	1.650	2.015	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	911.00	-	3	.05800	106.44	4.444	1.43652	1.550	2.868	RAIN24=	3.330
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. FROM LA CUEVA DIVERSION										PER IMP=	17.00
*S CHANNEL (HYD NO. 911.99) - UNBULKED											
ADD HYD	911.99	2& 3	1	1.20650	1563.93	98.548	1.53151	1.650	2.025		
*S TOTAL INFLOW HYDROGRAPH INTO UPPER N.D.B. DAM. (HYD NO. 914.79)											
*S (BULKED)											
ADD HYD	914.79	15& 1	1	2.77836	2563.31	232.188	1.56694	1.650	1.442		
*S ROUTING THROUGH UPPER NORTH DOMINGO BACA DAM											
*S											
*S THIS STORAGE OUTFLOW DATA WAS BASED ON TOPOGRAPHY IN JUNE 1994.											
*S OUTFLOW IS TAKEN FROM "DESIGN ANALYSIS REPORT FOR LA CUEVA DIVERSION											
*S CHANNEL" (RESOURCE TECHNOLOGY, INC., 1994). STORAGE INFORMATION WAS											
*S CALCULATED AT RTI - PROPOSED FUTURE CONDITION EXCAVATION.											
*S THE ELEVATION OF EMERGENCY SPILLWAY IS AT 5749.											
ROUTE RESERVOIR	914.99	1	2	2.77836	150.76	232.067	1.56612	3.200	.085	AC-FT=	181.374
*S UPPER N.D.B. DAM OUTFLOW HYDROGRAPH AT EUBANK. (HYD NO. 914.99)											
*S AP 914.99 IS THROUGH THE PRINCIPAL SPILLWAY											
*S AP NDB.SPILL IS THROUGH THE EMERGENCY SPILLWAY											
DIVIDE HYD	914.99	2	2	2.77836	150.76	232.067	1.56612	3.200	.085		
NDB.SPILL AND		20		.00000	.00	.000	.00000	-.050	.000		
ROUTE MCUNGE	914.68	2	3	2.77836	150.76	232.061	1.56608	3.250	.085	CCODE =	.2
ROUTE	SPILL.ROU	20	19	.00000	.00	.000	.00000	-.050	.000		
RAINFALL TYPE= 2											
COMPUTE NM HYD	914.20	-	1	.03500	80.30	3.045	1.63137	1.500	3.585	RAIN24=	3.200
*S UNOBD DAM SPILL AT EUBANK (AP 914.29)										PER IMP=	30.00
ADD HYD	914.29	1&19	1	.03500	80.30	3.045	1.63137	1.500	3.585		
ROUTE MCUNGE	914.28	1	2	.03500	79.85	3.045	1.63139	1.600	3.565	CCODE =	.2
*S N O B ARROYO BETWEEN EUBANK AND HOLBROOK											
*S (AP 914.78)											
ADD HYD	914.78	2& 3	2	2.81336	160.68	235.106	1.56690	1.700	.089		
ROUTE MCUNGE	914.88	2	3	2.81336	157.48	235.094	1.56681	1.750	.087	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	919.10	-	1	.07900	167.03	5.808	1.37846	1.500	3.304	RAIN24=	3.150
*S N D B ARROYO HOLBROOK (HYD NO. 919.19) - FINAL										PER IMP=	17.00
ADD HYD	919.19	1& 3	4	2.89236	230.72	240.902	1.56167	1.650	.125		
RAINFALL TYPE= 2											
COMPUTE NM HYD	919.20	-	1	.04270	89.89	3.139	1.37846	1.500	3.289	RAIN24=	3.150
*S HOLBROOK (HYD NO. 919.29) NOT FINAL										PER IMP=	17.00
ADD HYD	919.29	1& 4	22	2.93506	305.75	244.041	1.55900	1.550	.163		
RAINFALL TYPE= 2											
COMPUTE NM HYD	920.20	-	3	.00850	17.50	.607	1.33902	1.500	3.216	RAIN24=	3.100
RAINFALL TYPE= 2										PER IMP=	17.00
COMPUTE NM HYD	913.10	-	1	.03600	76.37	2.701	1.40683	1.500	3.314	RAIN24=	3.250
										PER IMP=	17.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ROUTE MCUNGE	913.88	1	2	.03600	75.95	2.701	1.40683	1.700	3.297	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 3.150
COMPUTE NM HYD	917.00	-	1	.04060	85.47	2.985	1.37846	1.500	3.289	PER IMP= 17.00
*S HOLBROOK (AP 917.99) - FINAL										
ADD HYD	917.99	1& 2	21	.07660	123.74	5.686	1.39178	1.650	2.524	
*S (AP 920.29)										
ADD HYD	920.29	21& 3	4	.08510	134.05	6.293	1.38651	1.650	2.461	
*S ROUTE IN HOLBROOK TO CORONA IN 48" SD										
ROUTE	920.28	4	9	.08510	132.55	6.293	1.38652	1.650	2.434	
RAINFALL TYPE= 2										RAIN24= 3.200
COMPUTE NM HYD	913.20	-	1	.01700	39.50	1.479	1.63137	1.500	3.630	PER IMP= 30.00
ROUTE MCUNGE	913.18	1	2	.01700	32.68	1.456	1.60540	1.800	3.004	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.150
COMPUTE NM HYD	918.00	-	1	.11400	156.75	8.381	1.37846	1.650	2.148	PER IMP= 17.00
*S HOLBROOK (AP 918.88) - FINAL										
ADD HYD	918.88	1& 2	1	.13100	173.88	9.837	1.40790	1.700	2.074	
*S ADD 920.28 AND 918.89 AND ROUTE TO NDB										
ADD HYD	918.89	1& 9	1	.21610	304.68	16.129	1.39948	1.650	2.203	
*S ROUTE IN HOLBROOK TO CARMEL IN 66"SD										
ROUTE	918.99	1	8	.21610	304.20	16.129	1.39948	1.700	2.200	
RAINFALL TYPE= 2										RAIN24= 3.100
COMPUTE NM HYD	921.40	-	6	.01440	29.63	1.028	1.33902	1.500	3.215	PER IMP= 17.00
COMPUTE NM HYD	921.30	-	7	.01400	28.81	1.000	1.33902	1.500	3.215	PER IMP= 17.00
*S COMBINER ALL HOLBROOK AND NDB FLOWS AT HOLBROOK CROSSING										
*S ADD 918.99 AND 920.4 AS 920.49										
ADD HYD	920.49	8& 6	6	.23050	318.07	17.158	1.39570	1.700	2.156	
*S ADD 920.49 TO 919.29 AS 919.99										
ADD HYD	919.99	6&22	20	3.16556	599.30	261.198	1.54711	1.650	.296	
*S BETWEEN HOLBROOK AND VENTURA (AP 919.18) - NOT FINAL										
ROUTE MCUNGE	919.18	20	5	3.16556	596.68	261.191	1.54706	1.650	.295	CCODE = .2
RAINFALL TYPE= 2										RAIN24= 3.100
COMPUTE NM HYD	921.10	-	1	.02920	60.06	2.085	1.33902	1.500	3.214	PER IMP= 17.00
*S UPSTREAM OF END OF VINYARD ESTATES SUBDIVISION UNIT IV BETWEEN										
*S HOLBROOK & VENTURA										
*S *****										
*S BEGIN: VINYARD ESTATES - UNIT IV SE CORNER										
*S *****										
*S ROUTE 921.19										
*S (48-INCH RCP)										
ROUTE	921.80	1	12	.02920	60.11	2.085	1.33904	1.500	3.217	
*S (54-INCH RCP)										
ROUTE	921.89	12	16	.02920	60.14	2.085	1.33904	1.500	3.218	
*S COMBINE 921.89 & 921.3 AS 921.98										
ADD HYD	921.98	16& 7	15	.04320	88.95	3.085	1.33901	1.500	3.217	
*S *****										
*S END: VINYARD ESTATES - UNIT IV										
*S *****										
*S N D B ARROYO BETWEEN HOLBROOK & VENTURA (AP 921.78) - FINAL										
ADD HYD	921.78	5&15	1	3.20876	654.91	264.276	1.54426	1.600	.319	
ROUTE MCUNGE	921.80	1	2	3.20876	652.95	264.238	1.54404	1.600	.318	CCODE = .1
RAINFALL TYPE= 2										RAIN24= 3.070
COMPUTE NM HYD	921.20	-	1	.08740	237.09	9.939	2.13220	1.500	4.239	PER IMP= 60.00
*S N D B ARROYO AT VENTURA (AP 921.9) - NOT FINAL										
ADD HYD	921.90	1& 2	26	3.29616	818.24	274.177	1.55964	1.600	.388	
RAINFALL TYPE= 2										RAIN24= 3.100
COMPUTE NM HYD	920.10	-	17	.02020	41.55	1.443	1.33902	1.500	3.214	PER IMP= 17.00
RAINFALL TYPE= 2										RAIN24= 3.100
COMPUTE NM HYD	920.00	-	5	.03000	61.70	2.142	1.33902	1.500	3.214	PER IMP= 17.00
*S *****										
*S BEGIN: VINYARD ESTATES - UNIT IV										
*S *****										
RAINFALL TYPE= 2										RAIN24= 3.030
COMPUTE NM HYD	922.10	-	2	.00700	17.43	.711	1.90576	1.500	3.890	PER IMP= 50.00
COMPUTE NM HYD	923.00	-	12	.00700	19.32	.844	2.25972	1.500	4.312	PER IMP= 70.00
COMPUTE NM HYD	922.20	-	9	.01480	35.69	1.396	1.76805	1.500	3.767	PER IMP= 40.00
COMPUTE NM HYD	922.30	-	16	.04150	100.03	3.913	1.76805	1.500	3.766	PER IMP= 40.00
*S COMBINE 920 + 922.1 AS 922.19										
ADD HYD	922.19	5& 2	4	.03700	79.13	2.854	1.44623	1.500	3.342	
*S ROUTE 922.19 THRU 30" PIPE AS 920.98										
ROUTE	920.98	4	8	.03700	76.37	2.854	1.44625	1.500	3.225	
*S ADD 922.2 + 920.98 AS 922.28										
ADD HYD	922.28	8& 9	10	.05180	112.05	4.249	1.53817	1.500	3.380	
*S ROUTE 922.28 flows to Ventura Interceptor AS 922.29										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4	NOTATION
ROUTE	922.29	10	11	.05180	110.00	4.249	1.53818	1.550	3.318		
RAINFALL TYPE= 2											
COMPUTE NM HYD	922.00	-	1	.02100	33.77	1.462	1.30533	1.550	2.512	RAIN24= 3.030	
*S COMBINE 922 AND 923 AS 923.99										PER IMP= 17.00	
ADD HYD	920.98	1&12	1	.02800	50.81	2.306	1.54390	1.550	2.835		
*S ROUTE 920.1 DOWN WILSHIRE TO VENTURA											
ROUTE	920.18	17	12	.02020	40.28	1.443	1.33905	1.550	3.116		
*S COMBINE 920.18 AND 920.98 AS 920.99											
ADD HYD	920.99	1&12	1	.04820	91.09	3.748	1.45803	1.550	2.953		
*S Route 923.99 FLOW FROM NO OF VINYARD IV TO 48" Ventura Line AS 923.8											
ROUTE	923.80	1	13	.04820	91.54	3.748	1.45805	1.550	2.967		
*S ADD 923.8 + 922.29 AS 923.9											
ADD HYD	923.90	11&13	14	.10000	201.54	7.998	1.49955	1.550	3.149		
*S ROUTE 923.9 TO NDB (NOT FINAL)											
ROUTE	923.91	14	15	.10000	203.51	7.998	1.49956	1.550	3.180		
*S ADD 923.91 + 923.3 AS 923.92											
ADD HYD	923.92	16&15	18	.14150	293.19	11.911	1.57830	1.550	3.238		
*S NO POND AT VENTURA											
*S*****											
*S	END: VINYARD ESTATES - UNIT IV										
*S*****											
*S	N D B ARROYO AT VENTURA (AP 921.99) - FINAL										
ADD HYD	921.99	18&26	1	3.43766	1105.56	286.088	1.56041	1.550	.503		
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.10	-	2	.01200	30.38	1.228	1.91802	1.500	3.956	RAIN24= 3.030	
*S LA CUEVA SUBDIVISION (AP 926.1) - NO POND FINAL										PER IMP= 50.00	
*S N D B ARROYO AT -400 FT WEST OF VENTURA. (HYD NO. 926.19) - FINAL											
ADD HYD	926.19	1&2	1	3.44966	1132.67	287.316	1.56165	1.550	.513		
ROUTE MCUNGE	926.17	1	2	3.44966	1096.41	287.169	1.56086	1.650	.497	CCODE = .1	
RAINFALL TYPE= 2											
COMPUTE NM HYD	926.00	-	1	.05780	154.00	6.406	2.07818	1.500	4.163	RAIN24= 3.000	
*S N D B ARROYO AT BARSTOW. (AP 926.79) - NOT FINAL										PER IMP= 60.00	
ADD HYD	926.79	1&2	11	3.50746	1192.76	293.576	1.56938	1.600	.531		
RAINFALL TYPE= 2											
COMPUTE NM HYD	925.20	-	1	.00940	23.81	.962	1.91802	1.500	3.957	RAIN24= 3.030	
ROUTE MCUNGE	925.27	1	3	.00940	22.10	.958	1.91061	1.550	3.674	PER IMP= 50.00	
COMPUTE NM HYD	925.10	-	1	.06400	162.00	6.547	1.91802	1.500	3.955	CCODE = .1	
*S COMBINE 925.1 & 925.27 AS 925.18 (VINYARD FLOW TO APS SITE)										PER IMP= 50.00	
ADD HYD	925.18	1&3	1	.07340	179.25	7.505	1.91706	1.500	3.816		
*S (925.19=4.5-FOOT STORM SEWER ROUTING)											
ROUTE	925.19	1	2	.07340	170.72	7.505	1.91707	1.550	3.634		
COMPUTE NM HYD	925.40	-	4	.03700	96.45	3.887	1.96954	1.500	4.073	PER IMP= 50.00	
*S INFLOW NDB FROM VINYARD SD AND APS MID SCHOOL 925.48											
*S (AP 925.48) - FINAL											
ADD HYD	925.48	2&4	15	.11040	256.58	11.391	1.93465	1.550	3.631		
RAINFALL TYPE= 2											
COMPUTE NM HYD	924.30	-	1	.02700	68.50	2.753	1.91154	1.500	3.964	RAIN24= 3.000	
*S CARRINGTON SUBDIVISION (AP 924.3)										PER IMP= 50.00	
COMPUTE NM HYD	924.40	-	7	.01720	43.28	1.741	1.89760	1.500	3.932	PER IMP= 50.00	
*S TOTAL BARSTOW @ CARRINGTON (AP 924.99) - FINAL											
ADD HYD	924.99	1&7	5	.04420	111.78	4.493	1.90611	1.500	3.951		
COMPUTE NM HYD	925.30	-	1	.01050	26.43	1.063	1.89760	1.500	3.933	PER IMP= 50.00	
*S COMBINE CARRINGTON FLOW WITH VINYARD FLOW 925.3 + 924.99 AS 925.38											
ADD HYD	925.38	1&5	5	.05470	138.20	5.556	1.90447	1.500	3.948		
*S (STORM SEWER FROM GREEN ARBOR TO NDB)											
ROUTE	924.81	5	2	.05470	133.00	5.556	1.90448	1.550	3.799		
*S BARSTOW SD WITH VINYARD SD FLOW AT NDB											
ADD HYD	925.38	15&2	6	.16510	389.58	16.947	1.92465	1.550	3.687		
*S BARSTOW DIVERSION											
COMPUTE NM HYD	926.20	-	1	.04700	111.97	4.330	1.72735	1.500	3.722	PER IMP= 40.00	
ROUTE	926.28	1	2	.04700	106.30	4.330	1.72736	1.550	3.534		
*S N D B ARROYO AT BARTSOW (AP 926.99) - FINAL											
ADD HYD	926.89	11&2	2	3.55446	1284.98	297.906	1.57147	1.600	.565		
ADD HYD	926.99	2&6	5	3.71956	1618.82	314.853	1.58715	1.600	.680		
ROUTE MCUNGE	926.70	5	3	3.71956	1585.94	314.748	1.58662	1.600	.666	CCODE = .1	
ROUTE MCUNGE	926.80	3	5	3.71956	1562.13	314.538	1.58556	1.700	.656	CCODE = .1	
COMPUTE NM HYD	932.40	-	3	.05740	150.03	6.135	2.00401	1.500	4.084	PER IMP= 55.00	
RAINFALL TYPE= 2											
COMPUTE NM HYD	930.00	-	1	.08500	224.35	9.278	2.04656	1.500	4.124	RAIN24= 2.960	
*S N D B ARROYO AT WYOMING (AP 930.59) -										PER IMP= 60.00	
ADD HYD	930.59	1&3	1	.14240	374.39	15.413	2.02940	1.500	4.108		
*S N D B ARROYO AT WYOMING (AP 930.99) - FINAL											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 5	NOTATION
ADD HYD	930.99	1& 5	1	3.86196	1732.40	329.950	1.60192	1.700	.701		
ROUTE MCUNGE	930.80	1	12	3.86196	1692.74	329.639	1.60041	1.800	.685	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	2.950
COMPUTE NM HYD	935.00	-	14	.11000	254.57	9.731	1.65876	1.500	3.616	PER IMP=	40.00
ADD HYD	930.85	14&12	13	3.97196	1773.42	339.371	1.60203	1.800	.698		

*S BEGIN: WYOMING STORM DRAIN											

COMPUTE NM HYD	929.00	-	4	.02400	65.77	2.752	2.14971	1.500	4.282	PER IMP=	65.00
COMPUTE NM HYD	931.00	-	1	.06050	109.47	6.529	2.02346	1.650	2.827	PER IMP=	60.00
*S NO POND AT LA CUEVA HIGH SCHOOL FOR FUTURE CONDITION											
ADD HYD	931.80	4& 1	1	.08450	153.96	9.281	2.05931	1.600	2.847		
COMPUTE NM HYD	932.10	-	4	.00730	19.08	.766	1.96752	1.500	4.083	PER IMP=	55.00
*S CHURCH (AP 932.1)											
*S WYOMING AND WILSHIRE (AP 931.99)											
ADD HYD	931.99	4& 1	1	.09180	170.54	10.047	2.05201	1.550	2.903		
*S (931.98=5-FOOT PIPE ROUTING)											
ROUTE	931.98	1	3	.09180	169.55	10.047	2.05201	1.550	2.886		
COMPUTE NM HYD	932.20	-	1	.00730	18.89	.766	1.96752	1.500	4.043	PER IMP=	55.00
ADD HYD	932.80	3& 1	1	.09910	186.34	10.813	2.04578	1.550	2.938		
COMPUTE NM HYD	932.30	-	2	.03130	80.93	3.284	1.96752	1.500	4.040	PER IMP=	55.00
ADD HYD	932.00	2& 1	10	.13040	258.27	14.097	2.02699	1.550	3.095		
*S NO POND AT NORESTE BETWEEN CORONA AND ANAHEIM											

*S END: WYOMING STORM DRAIN											

ROUTE MCUNGE	932.81	10	2	.13040	248.40	14.097	2.02700	1.700	2.976	CCODE =	.1
COMPUTE NM HYD	934.10	-	1	.10310	272.43	11.220	2.04056	1.500	4.129	PER IMP=	60.00
ADD HYD	934.99	1& 2	3	.23350	388.07	25.317	2.03298	1.650	2.597		
*S LNDBDD INFLOW HYDROGRAPH (AP 930.84) - NOT FINAL											
ADD HYD	930.84	3&13	11	4.20546	2107.61	364.688	1.62596	1.750	.783		
COMPUTE NM HYD	937.00	-	1	.04520	119.15	4.919	2.04056	1.500	4.119	PER IMP=	60.00
*S TOTAL DAM INFLOW HYDROGRAPH (930.86)-FINAL											
ADD HYD	930.86	1&11	11	4.25066	2153.83	369.607	1.63037	1.750	.792		
*S											
*S LOWER NORTH DOMINGO BACA DETENTION DAM											
*S LOW FLOW OUTLET (PRINCIPLE SPILLWAY). THIS PRINCIPLE SPILLWAY JOINS											
*S ONE OF THE FOUR BOX CULVERTS AT THE I-25. IT ALSO PICKS UP FLOWS FROM											
*S SUB-BASINS 940.2, 939.16, 939.14 & 943.1. THE FLOWS FROM SUB-BASINS											
*S 939.14 AND 943.1 ARE INCLUDED BY SAD221 PROJECT WHICH IS ALREADY											
*S IN PLACE. THE FOLLOWING RATING CURVE IS FOR A 40-INCH ORIFICE PLATE.											
*S EMERGENCY SPILLWAY IS NOT USED IN THE FOLLOWING RATING CURVE.											
*S											
ROUTE RESERVOIR	930.87	11	12	4.25066	200.70	369.646	1.63054	3.250	.074	AC-FT=	102.949
*S LNDBAD DAM OUTFLOW HYDROGRAPH (AP 930.87)											
RAINFALL TYPE= 2											
COMPUTE NM HYD	960.00	-	1	.00750	18.46	.727	1.81681	1.500	3.845	RAIN24=	2.850
*S STORM DRAIN OUT OF LNDBD DAM AT LOUISIANA BLVD. (AP 930.89)											
ADD HYD	930.89	1&12	12	4.25816	200.83	370.372	1.63086	3.200	.074		
COMPUTE NM HYD	940.20	-	1	.01560	37.64	1.486	1.78583	1.500	3.770	PER IMP=	50.00
COMPUTE NM HYD	939.16	-	3	.01280	30.89	1.219	1.78582	1.500	3.770	PER IMP=	50.00
ADD HYD	940.26	1& 3	3	.02840	68.53	2.705	1.78580	1.500	3.770		
*S AT THE END OF LA CUEVA OESTE PHASE 1, IN STORM DRAIN PIPE (AP 939.88)											
*S (UNBULKED)											
ADD HYD	939.88	12& 3	12	4.28656	203.16	373.077	1.63189	2.050	.074		
*S STORM DRAIN PIPE FROM THE END OF LA CUEVA OESTE PHASE 1 TILL SAN PEDRO											
*S BENEATH ANAHEIM AVE -- TWO REACHES											
*S RCP DIA=4 FT TO 940.0 OUTFALL											
*S (939.78=4-FOOT PIPE ROUTING)											
ROUTE	939.78	12	15	4.28656	202.85	373.068	1.63185	2.050	.074		
COMPUTE NM HYD	940.00	-	1	.01410	34.02	1.343	1.78583	1.500	3.770	PER IMP=	50.00
ADD HYD	939.68	15& 1	15	4.30066	232.55	374.411	1.63235	1.550	.084		
*S RCP DIA=4 FT TO SAN PEDRO											
ROUTE	939.58	15	16	4.30066	234.18	374.407	1.63234	1.550	.085		
*S STORM DRAIN PIPE FROM SAN PEDRO TO UTE DR. BENEATH ANAHEIM AVE.											
*S (939.48=RCP DIA=4 FT)											
ROUTE	939.48	16	14	4.30066	234.08	374.402	1.63232	1.550	.085		
COMPUTE NM HYD	939.14	-	1	.01120	29.58	1.203	2.01454	1.500	4.127	PER IMP=	60.00
COMPUTE NM HYD	943.10	-	3	.01200	31.97	1.322	2.06603	1.500	4.162	PER IMP=	65.00
*S STORM DRAIN BENEATH ANAHEIM AT UTE (AP 943.18)											
ADD HYD	943.18	1& 3	3	.02320	61.55	2.526	2.04114	1.500	4.145		
*S STORM DRAIN FLOW AT I-25 (AP 943.19)											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 6 NOTATION
ADD HYD	943.19	3&14	3	4.32386	288.50	376.928	1.63451	1.550	.104	
COMPUTE NM HYD	940.10	-	1	.00700	18.82	.791	2.11965	1.500	4.202	PER IMP= 70.00
ROUTE MCUNGE	940.80	1	6	.00700	18.75	.791	2.11970	1.600	4.185	CCODE = .2
COMPUTE NM HYD	943.20	-	1	.03750	100.77	4.239	2.11965	1.500	4.199	PER IMP= 70.00
*S OVERLAND FLOW AT I-25 (AP 943.29)										
ADD HYD	943.29	1& 6	5	.04450	112.79	5.031	2.11964	1.500	3.960	
*S TOTAL NDB Q AT I-25 BOX CULVERT (943.89)										
ADD HYD	943.98	5& 3	9	4.36836	394.04	381.959	1.63945	1.550	.141	
*S CORONA SD SYSTEM										
COMPUTE NM HYD	942.20	-	1	.10310	229.08	10.288	1.87097	1.550	3.472	PER IMP= 55.00
*S (ROUTE IN SD DIA=4 FT)										
ROUTE	942.29	1	5	.10310	231.01	10.288	1.87097	1.550	3.501	
COMPUTE NM HYD	942.10	-	2	.04690	128.50	5.391	2.15532	1.500	4.281	PER IMP= 70.00
*S CORONA SD AT I-25										
ADD HYD	942.59	2& 5	3	.15000	344.19	15.679	1.95987	1.550	3.585	
*S TOTAL Q AT CORONADO MH PARK (AP 943.99)										
ADD HYD	943.99	3& 9	3	4.51836	738.23	397.637	1.65009	1.550	.255	
COMPUTE NM HYD	443.20	-	2	.07030	192.62	8.081	2.15532	1.500	4.281	PER IMP= 70.00
FINISH										

SOUTH DOMINGO BACA EXISTING COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
 INPUT FILE = sdb10.ext

RUN DATE (MON/DAY/YR) = 1D/16/1998
 USER NO. = RSTECHNM.8TE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 10-YR, 24-HOUR STORM EXISTING CONDITIONS											
*S SOUTH DOMINGO BACA ARROYO SYSTEM TO EUBANK BLVD.											
*S BEGIN SOUTH DOMINGO BACA BASIN ANALYSIS IN THE SANDIA MOUNTAINS											
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	400.00	-	1	2.06900	854.14	73.559	.66661	1.900	.645		RAIN24= 2.79D
ROUTE MCUNGE	400.80	1	2	2.06900	835.54	73.243	.66376	2.25D	.631		PER IMP= .00
RAINFALL TYPE= 2											CCODE = .1
COMPUTE NM HYD	400.53	-	1	.98300	245.71	24.788	.47281	1.950	.391		RAIN24= 2.370
*S COMBINE HYD 400.53 AND 400.80 AS 400.32											PER IMP= 4.0D
ADD HYD	400.32	1& 2	11	3.05200	1039.56	98.031	.60226	2.200	.532		
*SUB-BASIN 400.52											
COMPUTE NM HYD	400.52	-	1	.07340	77.40	3.245	.82897	1.500	1.648		PER IMP= 25.00
*S COMBINE HYD 400.52 AND 400.32 AS 400.33											
ADD HYD	400.33	1&11	2	3.12540	1046.45	101.276	.60758	2.200	.523		
*SUB-BASIN 400.51											
COMPUTE NM HYD	400.51	-	1	.16880	195.83	7.463	.82897	1.500	1.813		PER IMP= 25.00
*S COMBINE HYD 400.51 AND 400.33 AS 400.59											
ADD HYD	400.59	1& 2	1	3.29420	1060.35	108.739	.61892	2.200	.503		
*S SUM OF SOUTH DOMINGO BACA ARROYO FLOWS @ TRAMWAY (AP 400.59)											
*S ROUTE SUM OF FLOWS IN SDB CONCRETE CHANNEL AT TRAMWAY (400.58)											
ROUTE MCUNGE	400.58	1	3	3.29420	1056.21	108.682	.61860	2.250	.501		CCODE = .1
*S START SOUTH DOMINGO BACA TRIBUTARY BASIN IN THE SANDIA MOUNTAINS											
RAINFALL TYPE= 2											RAIN24= 2.640
COMPUTE NM HYD	400.30	-	1	.25500	348.72	9.922	.72958	1.500	2.137		PER IMP= .00
ROUTE MCUNGE	400.38	1	2	.25500	292.55	9.476	.69676	1.900	1.793		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.370
COMPUTE NM HYD	400.65	-	1	.59190	193.67	16.022	.50753	1.850	.511		PER IMP= 5.00
*S COMBINE HYDS. 400.65 AND 400.38 AS 400.21											
ADD HYD	400.21	1& 2	11	.84690	483.17	25.498	.56451	1.900	.891		
*S (400.61=FLOW E. OF TRAMWAY BETWEEN LIVE OAK ROAD AND TRAMWAY LANE CT.)											
COMPUTE NM HYD	400.61	-	1	.09300	107.60	4.112	.82897	1.500	1.808		PER IMP= 25.00
COMPUTE NM HYD	400.62	-	2	.09800	113.41	4.333	.82897	1.500	1.808		PER IMP= 25.00
*S (400.22=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.22	1& 2	12	.19100	221.01	8.444	.82896	1.500	1.808		
*S (400.23=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.23	11&12	11	1.03790	527.29	33.942	.61317	1.900	.794		
COMPUTE NM HYD	400.66	-	1	.09400	104.95	4.156	.82897	1.500	1.744		PER IMP= 25.00
*S (400.79=TOTAL FLOW AT N. DIV. CHANNEL TO SDB DIV.(E. OF TRAMWAY))											
ADD HYD	400.79	1&11	1	1.13190	552.57	38.098	.63109	1.850	.763		
*S ROUTE HYD. 400.79 AS 400.77 IN CONCRETE CHANNEL (DOM. BACA TRIB.)											
ROUTE MCUNGE	400.77	1	2	1.13190	543.97	38.015	.62973	1.950	.751		CCODE = .1
*S TOTAL FLOW OF S D B ARROYO AND S D B ARROYO TRIBUTARY AT THEIR											
*S CONFLUENCE JUST WEST OF TRAMWAY (AP 400.89)											
ADD HYD	400.89	2& 3	1	4.42610	1366.56	146.698	.62145	2.100	.482		
ROUTE MCUNGE	400.87	1	3	4.42610	1365.84	146.782	.62180	2.150	.482		CCODE = .2
ROUTE MCUNGE	400.88	3	2	4.42610	1363.37	146.757	.62170	2.150	.481		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	400.10	-	1	.24820	209.84	6.636	.50132	1.500	1.321		PER IMP= 10.00
*S COMBINE 400.88 AND 400.1 AS 402.91											
ADD HYD	400.91	1& 2	2	4.67430	1379.12	153.393	.61531	2.150	.461		
RAINFALL TYPE= 2											RAIN24= 2.300
*S AP 400.13 IS SANDIA COLONY											
COMPUTE NM HYD	400.13	-	14	.01390	22.93	.955	1.28770	1.500	2.578		PER IMP= 50.00
ROUTE MCUNGE	400.57	14	6	.01390	18.45	.933	1.25875	1.900	2.074		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	402.14	-	13	.08210	64.91	2.836	.64768	1.550	1.235		PER IMP= 20.00
*S (402.92=INTERMEDIATE COMBINATION STEP AT END OF 402.14)											
ADD HYD	402.92	6&13	6	.09600	65.31	3.769	.73615	1.550	1.063		
*S (400.99=TOTAL INFLOW TO SOUTH DOMINGO BACA DAM)											
ADD HYD	400.99	6& 2	1	4.77030	1396.76	157.162	.61774	2.150	.458		
*S S D B DAM OUTFLOW HYDROGRAPH (400.81)											
ROUTE RESERVOIR	400.81	1	2	4.77030	149.75	157.162	.61774	3.650	.049		AC-FT= 108.015
*S 400.81 IS THROUGH PRINCIPAL SPILLWAY											
*S SDB.SPILL IS THROUGH EMERGENCY SPILLWAY											
DIVIDE HYD	400.81	2	2	4.77030	149.75	157.162	.61774	3.650	.049		
SDB.SPILL AND		20		.00000	.00	.000	.00000	-.050	.000		
ROUTE MCUNGE	400.83	2	1	4.77030	149.75	157.160	.61773	3.700	.049		CCODE = .1
ROUTE MCUNGE	400.82	1	2	4.77030	149.75	157.160	.61773	3.750	.049		CCODE = .2

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.10	-	1	.07950	49.19	1.530	.36073	1.500	.967	RAIN24= 2.220 PER IMP= 5.00
*S S D B ARROYO AT BROWNING ST. (AP 403.99)	403.99	1& 2	1	4.84980	149.93	158.690	.61352	3.700	.048	
ROUTE MCUNGE	403.88	1	2	4.84980	149.93	158.690	.61352	3.850	.048	CCODE = .1
ROUTE MCUNGE	403.82	2	3	4.84980	149.93	158.686	.61350	3.900	.048	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.20	-	1	.11030	62.84	2.446	.41573	1.550	.890	RAIN24= 2.170 PER IMP= 10.00
*S S D B ARROYO AT EUBANK (AP 403.79)- NOT FINAL	403.79	1& 3	3	4.96010	150.28	161.132	.60911	3.800	.047	
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.30	-	1	.04630	26.79	.851	.34459	1.500	.904	RAIN24= 2.220 PER IMP= 5.00
ROUTE MCUNGE	404.38	1	2	.04630	23.91	.851	.34460	1.750	.807	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.20	-	1	.05250	29.10	.925	.33028	1.500	.866	RAIN24= 2.170 PER IMP= 5.00
ADD HYD	404.48	1& 2	5	.09880	34.37	1.776	.33698	1.700	.544	
*S S D B ARROYO @ EUBANK (AP 404.99)	404.99	5& 3	9	5.05890	176.64	162.908	.60379	1.750	.055	
ADD HYD										
*S										
*S END OF STUDY FOR S D B MAIN ARROYO (EUBANK)										
*S										
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.40	-	1	.04320	32.04	1.077	.46745	1.500	1.159	RAIN24= 2.250 PER IMP= 10.00
ROUTE MCUNGE	401.48	1	2	.04320	27.45	1.077	.46746	1.800	.993	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.32	-	1	.05200	36.62	1.242	.44794	1.500	1.100	RAIN24= 2.220 PER IMP= 10.00
ADD HYD	401.39	1& 2	1	.09520	39.39	2.319	.45679	1.750	.646	
ROUTE MCUNGE	401.38	1	6	.09520	39.03	2.319	.45680	1.800	.641	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.50	-	1	.04500	31.62	1.083	.45105	1.500	1.098	RAIN24= 2.250 PER IMP= 10.00
ROUTE MCUNGE	401.58	1	2	.04500	27.69	1.083	.45106	1.750	.962	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.33	-	1	.04300	24.88	.790	.34459	1.500	.904	RAIN24= 2.220 PER IMP= 5.00
ADD HYD	401.34	1& 2	1	.08800	37.34	1.873	.39903	1.700	.663	
ROUTE MCUNGE	401.07	1	3	.08800	37.30	1.873	.39903	1.750	.662	CCODE = .2
ROUTE MCUNGE	401.08	3	2	.08800	37.22	1.873	.39903	1.800	.661	CCODE = .2
ADD HYD	401.18	2& 6	6	.18320	76.25	4.192	.42904	1.800	.650	
ROUTE	SPILL.ROU	20	19	.00000	.00	.000	.00000	-.050	.000	
COMPUTE NM HYD	401.31	-	1	.05380	40.02	1.332	.46439	1.500	1.162	PER IMP= 10.00
ADD HYD	401.35	1& 19	1	.05380	40.02	1.332	.46438	1.500	1.162	
ROUTE MCUNGE	401.28	1	3	.05380	39.66	1.333	.46444	1.600	1.152	CCODE = .2
ADD HYD	401.78	3& 6	1	.23700	108.22	5.525	.43708	1.600	.713	
ROUTE MCUNGE	401.77	1	3	.23700	108.12	5.525	.43708	1.650	.713	CCODE = .2
ROUTE MCUNGE	401.88	3	2	.23700	105.23	5.525	.43708	1.700	.694	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.20	-	1	.08500	47.39	1.497	.33028	1.500	.871	RAIN24= 2.170 PER IMP= 5.00
ADD HYD	401.29	1& 2	1	.32200	126.08	7.022	.40888	1.700	.612	
ADD HYD	404.95	1& 9	2	5.38090	293.04	169.930	.59213	1.750	.085	
*S SUM OF SDB FLOW AT EUBANK (404.95)										
*S SUM OF STUDY AT EUBANK										
*S *****										
FINISH										

SOUTH DOMINGO BACA EXISTING COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = sdb100.ext

RUN DATE (MON/DAY/YR) = 10/16/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 100-YR, 24-HOUR STORM EXISTING CONDITIONS											
*S SOUTH DOMINGO BACA ARROYO SYSTEM TO EUBANK BLVD.											
*S BEGIN SOUTH DOMINGO BACA BASIN ANALYSIS IN THE SANDIA MOUNTAINS											
START											
RAINFALL TYPE= 2										TIME=	.00
COMPUTE NM HYD	400.00	-	1	2.06900	1792.15	145.723	1.32059	1.950	1.353	RAIN24=	4.180
ROUTE MCUNGE	400.80	1	2	2.06900	1755.40	145.277	1.31656	2.200	1.326	PER IMP=	.00
RAINFALL TYPE= 2										CCODE =	.1
COMPUTE NM HYD	400.53	-	1	.98300	644.20	58.278	1.11161	1.950	1.024	RAIN24=	3.550
*S COMBINE HYD 400.53 AND 400.80 AS 400.32										PER IMP=	4.00
ADD HYD	400.32	1 & 2	11	3.05200	2285.82	203.555	1.25054	2.200	1.170		
*SUB-BASIN 400.52											
COMPUTE NM HYD	400.52	-	1	.07340	146.12	6.190	1.58112	1.500	3.111	PER IMP=	25.00
*S COMBINE HYD 400.52 AND 400.32 AS 400.33											
ADD HYD	400.33	1 & 11	2	3.12540	2300.68	209.745	1.25831	2.150	1.150		
*SUB-BASIN 400.51											
COMPUTE NM HYD	400.51	-	1	.16880	375.02	14.234	1.58112	1.500	3.471	PER IMP=	25.00
*S COMBINE HYD 400.51 AND 400.33 AS 400.59											
ADD HYD	400.59	1 & 2	1	3.29420	2332.65	223.979	1.27485	2.150	1.106		
*S SUM OF SOUTH DOMINGO BACA ARROYO FLOWS @ TRAMWAY (AP 400.59)											
*S ROUTE SUM OF FLOWS IN SDB CONCRETE CHANNEL AT TRAMWAY (400.58)											
ROUTE MCUNGE	400.58	1	3	3.29420	2329.63	223.777	1.27370	2.200	1.105	CCODE =	.1
*S START SOUTH DOMINGO BACA TRIBUTARY BASIN IN THE SANDIA MOUNTAINS											
RAINFALL TYPE= 2										RAIN24=	3.960
COMPUTE NM HYD	400.30	-	1	.25500	620.56	19.788	1.45500	1.500	3.802	PER IMP=	.00
ROUTE MCUNGE	400.38	1	2	.25500	514.18	19.058	1.40134	1.850	3.151	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.550
COMPUTE NM HYD	400.65	-	1	.59190	488.03	36.701	1.16260	1.850	1.288	PER IMP=	5.00
*S COMBINE HYDS. 400.65 AND 400.38 AS 400.21											
ADD HYD	400.21	1 & 2	11	.84690	1002.21	55.759	1.23449	1.850	1.849		
*S (400.61=FLOW E. OF TRAMWAY BETWEEN LIVE OAK ROAD AND TRAMWAY LANE CT.)											
COMPUTE NM HYD	400.61	-	1	.09300	202.27	7.842	1.58112	1.500	3.398	PER IMP=	25.00
COMPUTE NM HYD	400.62	-	2	.09800	213.43	8.264	1.58112	1.500	3.403	PER IMP=	25.00
*S (400.22=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.22	1 & 2	12	.19100	415.71	16.106	1.58112	1.500	3.401		
*S (400.23=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.23	11 & 12	11	1.03790	1104.10	71.866	1.29827	1.850	1.662		
COMPUTE NM HYD	400.66	-	1	.09400	198.10	7.927	1.58112	1.500	3.293	PER IMP=	25.00
*S (400.79=TOTAL FLOW AT N. DIV. CHANNEL TO SDB DIV.(E. OF TRAMWAY))											
ADD HYD	400.79	1 & 11	1	1.13190	1156.42	79.792	1.32176	1.850	1.596		
*S ROUTE HYD. 400.79 AS 400.77 IN CONCRETE CHANNEL (DOM. BACA TRIB.)											
ROUTE MCUNGE	400.77	1	2	1.13190	1144.76	79.736	1.32084	1.850	1.580	CCODE =	.1
*S TOTAL FLOW OF S D B ARROYO AND S D B ARROYO TRIBUTARY AT THEIR											
*S CONFLUENCE JUST WEST OF TRAMWAY (AP 400.89)											
ADD HYD	400.89	2 & 3	1	4.42610	2975.36	303.513	1.28575	2.100	1.050		
ROUTE MCUNGE	400.87	1	3	4.42610	2973.62	303.415	1.28534	2.150	1.050	CCODE =	.2
ROUTE MCUNGE	400.88	3	2	4.42610	2963.81	303.404	1.28529	2.150	1.046	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.380
COMPUTE NM HYD	400.10	-	1	.24820	477.22	14.947	1.12916	1.500	3.004	PER IMP=	10.00
*S COMBINE 400.88 AND 400.1 AS 402.91											
ADD HYD	400.91	1 & 2	2	4.67430	2995.86	318.351	1.27700	2.150	1.001		
RAINFALL TYPE= 2										RAIN24=	3.450
*S AP 400.13 IS SANDIA COLONY											
COMPUTE NM HYD	400.13	-	14	.01390	37.55	1.627	2.19512	1.500	4.221	PER IMP=	50.00
ROUTE MCUNGE	400.57	14	6	.01390	32.26	1.617	2.18090	1.800	3.626	CCODE =	.1
RAINFALL TYPE= 2										RAIN24=	3.380
COMPUTE NM HYD	402.14	-	13	.08210	134.07	5.772	1.31818	1.550	2.552	PER IMP=	20.00
*S (402.92=INTERMEDIATE COMBINATION STEP AT END OF 402.14)											
ADD HYD	402.92	6 & 13	6	.09600	134.96	7.389	1.44309	1.550	2.197		
*S (400.99=TOTAL INFLOW TO SOUTH DOMINGO BACA DAM)											
ADD HYD	400.99	6 & 2	1	4.77030	3025.21	325.739	1.28034	2.150	.991		
*S S D B DAM OUTFLOW HYDROGRAPH (400.81)											
ROUTE RESERVOIR	400.81	1	2	4.77030	173.02	325.749	1.28038	4.250	.057	AC-FT=	258.942
*S 400.81 IS THROUGH PRINCIPAL SPILLWAY											
*S SDB.SPILL IS THROUGH EMERGENCY SPILLWAY											
DIVIDE HYD	400.81	2	2	4.77030	173.02	325.749	1.28038	4.250	.057		
	SDB.SPILL	AND	20	.00000	.00	.000	.00000	-.050	.000		
ROUTE MCUNGE	400.83	2	1	4.77030	173.02	325.747	1.28037	4.300	.057	CCODE =	.1
ROUTE MCUNGE	400.82	1	2	4.77030	173.02	325.746	1.28037	4.350	.057	CCODE =	.1

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.10	-	1	.07950	123.44	3.928	.92653	1.500	2.426	RAIN24= 3.330 PER IMP= 5.00
*S S D B ARROYO AT BROWNING ST. (AP 403.99)										
ADD HYD	403.99	1& 2	1	4.84980	190.10	329.675	1.27457	1.550	.061	
ROUTE MCUNGE	403.88	1	2	4.84980	185.85	329.675	1.27457	1.700	.060	CCODE = .1
ROUTE MCUNGE	403.82	2	3	4.84980	180.89	329.656	1.27450	1.850	.058	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.20	-	1	.11030	154.79	5.802	.98627	1.550	2.193	RAIN24= 3.250 PER IMP= 10.00
*S S D B ARROYO AT EUBANK (AP 403.79)- NOT FINAL										
ADD HYD	403.79	1& 3	3	4.96010	232.99	335.458	1.26809	1.800	.073	
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.30	-	1	.04630	69.12	2.224	.90077	1.500	2.332	RAIN24= 3.330 PER IMP= 5.00
ROUTE MCUNGE	404.38	1	2	.04630	62.61	2.224	.90078	1.750	2.113	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.20	-	1	.05250	76.30	2.443	.87249	1.500	2.271	RAIN24= 3.250 PER IMP= 5.00
ADD HYD	404.48	1& 2	5	.09880	94.09	4.667	.88574	1.700	1.488	
*S S D B ARROYO @ EUBANK (AP 404.99)										
ADD HYD	404.99	5& 3	9	5.05890	320.17	340.126	1.26062	1.750	.099	
*S										
*S END OF STUDY FOR S D B MAIN ARROYO (EUBANK)										
*S										
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.40	-	1	.04320	72.94	2.481	1.07692	1.500	2.638	RAIN24= 3.380 PER IMP= 10.00
ROUTE MCUNGE	401.48	1	2	.04320	62.99	2.442	1.05977	1.750	2.278	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.32	-	1	.05200	85.27	2.893	1.04315	1.500	2.562	RAIN24= 3.330 PER IMP= 10.00
ADD HYD	401.39	1& 2	1	.09520	96.64	5.335	1.05069	1.700	1.586	
ROUTE MCUNGE	401.38	1	6	.09520	95.86	5.335	1.05069	1.800	1.573	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.50	-	1	.04500	73.79	2.519	1.04945	1.500	2.562	RAIN24= 3.380 PER IMP= 10.00
ROUTE MCUNGE	401.58	1	2	.04500	65.67	2.519	1.04945	1.750	2.280	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.33	-	1	.04300	64.19	2.066	.90077	1.500	2.333	RAIN24= 3.330 PER IMP= 5.00
ADD HYD	401.34	1& 2	1	.08800	93.91	4.584	.97679	1.700	1.667	
ROUTE MCUNGE	401.07	1	3	.08800	93.62	4.584	.97680	1.750	1.662	CCODE = .2
ROUTE MCUNGE	401.08	3	2	.08800	93.15	4.584	.97680	1.800	1.654	CCODE = .2
ADD HYD	401.18	2& 6	6	.18320	189.01	9.919	1.01519	1.800	1.612	
ROUTE	SPILL.ROU	20	19	.00000	.00	.000	.00000	-.050	.000	
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.31	-	1	.05380	90.87	3.072	1.07065	1.500	2.639	RAIN24= 3.330 PER IMP= 10.00
ADD HYD	401.35	1& 19	1	.05380	90.87	3.072	1.07065	1.500	2.639	
ROUTE MCUNGE	401.28	1	3	.05380	84.69	3.051	1.06318	1.600	2.460	CCODE = .1
ADD HYD	401.78	3& 6	1	.23700	250.48	12.970	1.02608	1.600	1.651	
ROUTE MCUNGE	401.77	1	3	.23700	247.17	12.970	1.02609	1.700	1.630	CCODE = .1
ROUTE MCUNGE	401.88	3	2	.23700	244.09	12.970	1.02609	1.800	1.609	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.20	-	1	.08500	124.87	3.955	.87249	1.500	2.295	RAIN24= 3.250 PER IMP= 5.00
ADD HYD	401.29	1& 2	1	.32200	285.66	16.925	.98554	1.750	1.386	
ADD HYD	404.95	1& 9	2	5.38090	605.83	357.051	1.24416	1.750	.176	
*S SUM OF SDB FLOW AT EUBANK (404.95)										
*S END OF STUDY AT EUBANK										
*S *****										
FINISH										

SOUTH DOMINGO BACA FUTURE COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = sdb10.fut

RUN DATE (MON/DAY/YR) = 10/16/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 10-YR, 24-HOUR STORM FUTURE CONDITIONS											
*S SOUTH DOMINGO BACA ARROYO SYSTEM TO EUBANK BLVD.											
*S BEGIN SOUTH DOMINGO BACA BASIN ANALYSIS IN THE SANDIA MOUNTAINS											
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	400.00	-	1	2.06900	854.14	73.559	.66661	1.900	.645		RAIN24= 2.790
*S (400=BASIN STARTS AT SANDIA CREST)											
ROUTE MCUNGE	400.80	1	2	2.06900	835.54	73.243	.66376	2.250	.631		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.370
COMPUTE NM HYD	400.53	-	1	.98300	245.71	24.788	.47281	1.950	.391		PER IMP= 4.00
*S											
*S COMBINE HYD 400.53 AND 400.80 AS 400.32											
ADD HYD	400.32	1 & 2	11	3.05200	1039.56	98.031	.60226	2.200	.532		
COMPUTE NM HYD	400.52	-	1	.07340	88.50	3.939	1.00619	1.500	1.884		PER IMP= 35.00
*S COMBINE HYD 400.52 AND 400.32 AS 400.33											
ADD HYD	400.33	1 & 11	2	3.12540	1047.88	101.970	.61174	2.200	.524		
*SUB-BASIN 400.51											
COMPUTE NM HYD	400.51	-	1	.16880	220.94	9.058	1.00619	1.500	2.045		PER IMP= 35.00
*S COMBINE HYD 400.51 AND 400.33 AS 400.34											
ADD HYD	400.34	1 & 2	1	3.29420	1064.66	111.028	.63195	2.200	.505		
*S SUM OF SOUTH DOMINGO BACA ARROYO FLOWS @ TRAMWAY (AP 400.59)											
*S ROUTE SUM OF FLOWS IN SDB CONCRETE CHANNEL AT TRAMWAY (400.58)											
ROUTE MCUNGE	400.58	1	3	3.29420	1060.22	111.017	.63189	2.250	.503		CCODE = .1
*S START SOUTH DOMINGO BACA TRIBUTARY BASIN IN THE SANDIA MOUNTAINS											
RAINFALL TYPE= 2											RAIN24= 2.640
COMPUTE NM HYD	400.30	-	1	.25500	348.72	9.922	.72958	1.500	2.137		PER IMP= .00
ROUTE MCUNGE	400.38	1	2	.25500	292.55	9.476	.69676	1.900	1.793		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 2.370
COMPUTE NM HYD	400.65	-	1	.59190	193.67	16.022	.50753	1.850	.511		PER IMP= 5.00
*S COMBINE HYDS. 400.65 AND 400.38 AS 400.21											
ADD HYD	400.21	1 & 2	11	.84690	483.17	25.498	.56451	1.900	.891		
*S (400.61=FLOW E. OF TRAMWAY BETWEEN LIVE OAK ROAD AND TRAMWAY LANE CT.)											
COMPUTE NM HYD	400.61	-	1	.09300	120.24	4.991	1.00619	1.500	2.020		PER IMP= 35.00
COMPUTE NM HYD	400.62	-	2	.09800	128.39	5.259	1.00619	1.500	2.047		PER IMP= 35.00
*S (400.22=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.22	1 & 2	12	.19100	248.63	10.250	1.00619	1.500	2.034		
*S (400.23=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.23	11 & 12	11	1.03790	538.75	35.747	.64579	1.850	.811		
COMPUTE NM HYD	400.66	-	1	.09400	119.29	5.044	1.00619	1.500	1.983		PER IMP= 35.00
*S (400.79=TOTAL FLOW AT N. DIV. CHANNEL TO SDB DIV.(E. OF TRAMWAY))											
ADD HYD	400.79	1 & 11	1	1.13190	570.68	40.792	.67572	1.850	.788		
ROUTE MCUNGE	400.77	1	2	1.13190	561.11	40.723	.67459	1.950	.775		CCODE = .1
*S (400.89=TOTAL FLOW OF SDB ARROYO AND SDB ARROYO TRIBUTARY AT THEIR											
*S CONFLUENCE JUST WEST OF TRAMWAY)											
ADD HYD	400.89	2 & 3	1	4.42610	1389.97	151.740	.64281	2.100	.491		
ROUTE MCUNGE	400.87	1	3	4.42610	1384.10	151.553	.64202	2.150	.489		CCODE = .1
ROUTE MCUNGE	400.88	3	2	4.42610	1383.08	151.528	.64191	2.150	.488		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 2.250
COMPUTE NM HYD	400.10	-	1	.24200	329.21	12.248	.94897	1.500	2.126		PER IMP= 30.00
*S COMBINE 400.88 AND 400.1 AS 400.91											
ADD HYD	400.91	1 & 2	2	4.66810	1412.51	163.776	.65783	2.150	.473		
RAINFALL TYPE= 2											
*S AP 400.13 IS SANDIA COLONY											
COMPUTE NM HYD	400.13	-	14	.01390	22.93	.955	1.28770	1.500	2.578		PER IMP= 50.00
ROUTE MCUNGE	400.57	14	6	.01390	18.45	.933	1.25875	1.900	2.074		CCODE = .1
COMPUTE NM HYD	402.14	-	1	.08840	90.65	4.139	.87789	1.550	1.602		PER IMP= 30.00
*S (402.92=INTERMEDIATE COMBINATION STEP AT END OF 402.14)											
ADD HYD	402.92	6 & 1	6	.10230	91.04	5.072	.92963	1.550	1.391		
*S (400.93=PARTIAL INFLOW TO SDB DAM - TOTAL=401.99)											
ADD HYD	400.93	6 & 2	1	4.77040	1434.28	168.848	.66365	2.150	.470		
*S 401.5 AND 401.4 DIVERTED TO SDB DAM IN PROPOSED STORM DRAIN ON LOWELL											
COMPUTE NM HYD	401.50	-	18	.04500	54.12	1.809	.75374	1.500	1.879		PER IMP= 17.00
*S ROUTE 401.5 IN 54-INCH STORM DRAIN TO PINO AVE.											
ROUTE	401.80	18	6	.04500	48.82	1.809	.75375	1.550	1.695		
COMPUTE NM HYD	401.40	-	19	.04520	54.36	1.817	.75374	1.500	1.879		PER IMP= 17.00
ADD HYD	401.93	6 & 19	11	.09020	99.11	3.626	.75374	1.500	1.717		
*S ROUTE 401.93 IN 60-INCH STORM DRAIN TO SDB DAM											
ROUTE	401.82	11	7	.09020	100.42	3.626	.75375	1.550	1.739		
*S COMBINE 401.82 AND 400.92 AS 401.99											
*S (401.99=TOTAL INFLOW TO SOUTH DOMINGO BACA DAM)											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
ADD HYD	401.99	7 & 1	1	4.86060	1444.67	172.474	.66533	2.150	.464	
*S	S D B DAM OUTFLOW HYDROGRAPH (400.81)									
ROUTE RESERVOIR	400.81	1	2	4.86060	152.07	172.474	.66533	3.650	.049	AC-FT= 118.924
*S	AP 400.81 IS THROUGH PRINCIPAL SPILLWAY									
*S	AP SDB.SPILL IS THROUGH EMERGENCY SPILLWAY									
DIVIDE HYD	400.81	2	2	4.86060	152.07	172.474	.66533	3.650	.049	
	SDB.SPILL AND	20		.00000	.00	.000	.00000	-.050	.000	
*S	ROUTE 400.81 IN 48-INCH RCP PALOMAS STORM DRAIN TO BROWNING (400.83)									
ROUTE	400.83	2	3	4.86060	152.07	172.474	.66533	3.650	.049	
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.10	-	1	.06730	77.53	2.571	.71642	1.500	1.800	RAIN24= 2.220 PER IMP= 17.00
*S	PALOMAS STORM DRAIN AT BROWNING ST. (403.99)									
ADD HYD	403.99	1 & 3	1	4.92790	170.02	175.045	.66602	1.500	.054	
*S	ROUTE 403.99 IN 54-INCH RCP PALOMAS STORM DRAIN TO END OF 403.3 (403.88)									
ROUTE	403.88	1	2	4.92790	172.38	175.045	.66602	1.550	.055	
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.30	-	1	.01550	17.49	.576	.69689	1.500	1.763	RAIN24= 2.170 PER IMP= 17.00
*S	PALOMAS STORM DRAIN AT END OF 403.3 (403.89)									
ADD HYD	403.89	1 & 2	1	4.94340	188.44	175.621	.66612	1.550	.060	
*S	ROUTE 403.89 IN 54-INCH RCP PALOMAS STORM DRAIN TO EUBANK (403.84)									
ROUTE	403.84	1	2	4.94340	189.34	175.621	.66612	1.550	.060	
COMPUTE NM HYD	408.10	-	1	.00980	11.06	.364	.69689	1.500	1.764	PER IMP= 17.00
*S	PALOMAS STORM DRAIN AT EUBANK (408.9)									
ADD HYD	408.90	1 & 2	1	4.95320	199.49	175.986	.66618	1.550	.063	
*S	ROUTE 408.9 IN 78-INCH RCP NORTH REACH OF EUBANK STORM DRAIN TO									
*S	COMBINATION AT 403.79 (403.84)									
ROUTE	408.80	1	3	4.95320	201.51	175.986	.66618	1.550	.064	
COMPUTE NM HYD	403.20	-	1	.07960	79.67	2.959	.69689	1.550	1.564	PER IMP= 17.00
*S	COMBINE 403.2 AND NORTH REACH OF EUBANK STORM DRAIN FLOW (NORTH SIDE									
*S	OF RANCHITOS (403.79))									
ADD HYD	403.79	1 & 3	8	5.03280	281.19	178.944	.66667	1.550	.087	
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.30	-	1	.04630	53.34	1.769	.71642	1.500	1.800	RAIN24= 2.220 PER IMP= 17.00
ROUTE MCUNGE	404.38	1	2	.04630	47.12	1.769	.71642	1.750	1.590	CCODE = .1 RAIN24= 2.170
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.20	-	1	.05250	59.21	1.951	.69689	1.500	1.762	PER IMP= 17.00
ADD HYD	404.48	1 & 2	5	.09880	68.22	3.720	.70603	1.700	1.079	
*S										
*S	END OF STUDY FOR S D B MAIN ARROYO (EUBANK)									
*S										
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.32	-	1	.05200	59.91	1.987	.71642	1.500	1.800	RAIN24= 2.220 PER IMP= 17.00
ROUTE MCUNGE	401.38	1	6	.05200	59.09	1.987	.71643	1.550	1.776	CCODE = .2
COMPUTE NM HYD	401.33	-	1	.04300	49.54	1.643	.71642	1.500	1.800	PER IMP= 17.00
ROUTE MCUNGE	401.07	1	3	.04300	46.70	1.643	.71643	1.600	1.697	CCODE = .1
ROUTE MCUNGE	401.08	3	2	.04300	46.48	1.643	.71643	1.650	1.689	CCODE = .2
ADD HYD	401.18	2 & 6	6	.09500	103.54	3.630	.71641	1.600	1.703	
ROUTE	SPILL.ROU	20	19	.00000	.00	.000	.00000	-.050	.000	
COMPUTE NM HYD	401.31	-	1	.05180	59.68	1.979	.71642	1.500	1.800	PER IMP= 17.00
ADD HYD	401.35	1 & 19	1	.05180	59.68	1.979	.71641	1.500	1.800	
ROUTE MCUNGE	401.28	1	3	.05180	58.82	1.979	.71643	1.550	1.774	CCODE = .2
ADD HYD	401.78	3 & 6	1	.14680	161.29	5.609	.71641	1.600	1.717	
ROUTE MCUNGE	401.77	1	3	.14680	158.60	5.609	.71643	1.600	1.688	CCODE = .2
ROUTE MCUNGE	401.88	3	2	.14680	155.40	5.609	.71643	1.700	1.654	CCODE = .1 RAIN24= 2.170
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.20	-	1	.08500	96.33	3.172	.69964	1.500	1.771	PER IMP= 17.00
ADD HYD	401.29	1 & 2	1	.23180	194.64	8.781	.71027	1.700	1.312	
RAINFALL TYPE= 2										
COMPUTE NM HYD	507.00	-	4	.00890	9.82	.322	.67932	1.500	1.725	RAIN24= 2.130 PER IMP= 17.00
*S	SOUTH REACH OF EUBANK STORM DRAIN FLOW AT PINO AVE. (401.9)									
ADD HYD	401.90	1 & 4	11	.24070	198.70	9.103	.70912	1.650	1.290	
*S	ROUTE 401.9 IN 84-INCH RCP SOUTH REACH EUBANK STORM DRAIN									
*S	TO COMBINATION WITH 404.48 401.83)									
ROUTE	401.83	11	6	.24070	200.12	9.103	.70912	1.700	1.299	
*S	SOUTH REACH EUBANK STORM DRAIN FLOW AT SOUTH SIDE OF RANCHITOS									
ADD HYD	404.99	5 & 6	11	.33950	268.35	12.824	.70822	1.700	1.235	
*S	*****									
*S	COMBINE HYDS. 404.99 AND 403.79 AS 404.95									
*S	404.95=SUM OF SOUTH DOMINGO SD FLOWS AT EUBANK AND RANCHITOS									
ADD HYD	404.95	8 & 11	2	5.37230	498.45	191.768	.66929	1.550	.145	
*S	END OF STUDY AT EUBANK									
*S	*****									
FINISH										

SOUTH DOMINGO BACA FUTURE COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = sdb100.fut

RUN DATE (MON/DAY/YR) = 1D/16/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION
*S 100-YR, 24-HOUR STORM FUTURE CONDITIONS											
*S SOUTH DOMINGO BACA ARROYO SYSTEM TO EUBANK BLVD.											
*S BEGIN SOUTH DOMINGO BACA BASIN ANALYSIS IN THE SANDIA MOUNTAINS											
START											
RAINFALL TYPE= 2											TIME= .00
COMPUTE NM HYD	400.00	-	1	2.06900	1792.15	145.723	1.32059	1.950	1.353		RAIN24= 4.18D
*S (400=BASIN STARTS AT SANDIA CREST)											PER IMP= .00
ROUTE MCUNGE	400.80	1	2	2.06900	1755.40	145.277	1.31656	2.200	1.326		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.550
COMPUTE NM HYD	400.53	-	1	.98300	644.20	58.278	1.11161	1.950	1.024		PER IMP= 4.00
*S											
*S COMBINE HYD 400.53 AND 400.80 AS 400.32											
ADD HYD	400.32	1& 2	11	3.05200	2285.82	203.555	1.25054	2.200	1.170		
COMPUTE NM HYD	400.52	-	1	.07340	158.70	7.112	1.81680	1.500	3.378		PER IMP= 35.00
*S COMBINE HYD 400.52 AND 400.32 AS 400.33											
ADD HYD	400.33	1&11	2	3.12540	2303.41	210.667	1.26384	2.150	1.152		
*SUB-BASIN 400.51											
COMPUTE NM HYD	400.51	-	1	.16880	402.59	16.356	1.81680	1.500	3.727		PER IMP= 35.00
*S COMBINE HYD 400.51 AND 400.33 AS 400.34											
ADD HYD	400.59	1& 2	1	3.29420	2340.71	227.023	1.29218	2.150	1.110		
*S SUM OF SOUTH DOMINGO BACA ARROYO FLOWS @ TRAMWAY (AP 400.59)											
*S ROUTE SUM OF FLOWS IN SOB CONCRETE CHANNEL AT TRAMWAY (400.58)											
ROUTE MCUNGE	400.58	1	3	3.29420	2339.75	227.128	1.29277	2.200	1.110		CCODE = .2
*S											
START SOUTH DOMINGO BACA TRIBUTARY BASIN IN THE SANDIA MOUNTAINS											
RAINFALL TYPE= 2											RAIN24= 3.960
COMPUTE NM HYD	400.30	-	1	.25500	620.56	19.788	1.45500	1.500	3.802		PER IMP= .00
ROUTE MCUNGE	400.38	1	2	.25500	514.18	19.058	1.40134	1.850	3.151		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.550
COMPUTE NM HYD	400.65	-	1	.59190	488.03	36.701	1.16260	1.850	1.288		PER IMP= 5.00
*S COMBINE HYDS. 400.65 AND 400.38 AS 400.21											
ADD HYD	400.21	1& 2	11	.84690	1002.21	55.759	1.23449	1.850	1.849		
*S (400.61=FLOW E. OF TRAMWAY BETWEEN LIVE OAK ROAD AND TRAMWAY LANE CT.)											
COMPUTE NM HYD	400.61	-	1	.09300	215.52	9.011	1.81680	1.500	3.621		PER IMP= 35.00
COMPUTE NM HYD	400.62	-	2	.09800	230.10	9.496	1.81680	1.500	3.669		PER IMP= 35.00
*S (400.22=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.22	1& 2	12	.19100	445.61	18.507	1.81679	1.500	3.645		
*S (400.23=INTERMEDIATE COMBINATION STEP E. OF TRAMWAY @ N. DIV. CHANNEL INTAKE)											
ADD HYD	400.23	11&12	11	1.03790	1120.12	74.266	1.34164	1.850	1.686		
COMPUTE NM HYD	400.66	-	1	.09400	214.19	9.108	1.81680	1.500	3.560		PER IMP= 35.00
*S (400.79=TOTAL FLOW AT N. DIV. CHANNEL TO SOB DIV.(E. OF TRAMWAY))											
ADD HYD	400.79	1&11	1	1.13190	1179.82	83.375	1.38110	1.850	1.629		
ROUTE MCUNGE	400.77	1	2	1.13190	1168.23	83.295	1.37979	1.850	1.613		CCODE = .1
*S (400.89=TOTAL FLOW OF SDB ARROYO AND SDB ARROYO TRIBUTARY AT THEIR											
*S CONFLUENCE JUST WEST OF TRAMWAY)											
ADD HYD	400.89	2& 3	1	4.42610	3064.28	310.423	1.31502	2.100	1.082		
ROUTE MCUNGE	400.87	1	3	4.42610	3052.66	310.530	1.31548	2.100	1.078		CCODE = .2
ROUTE MCUNGE	400.88	3	2	4.42610	3048.31	310.409	1.31497	2.150	1.076		CCODE = .2
RAINFALL TYPE= 2											RAIN24= 3.380
COMPUTE NM HYD	400.10	-	1	.24200	601.75	22.579	1.74941	1.500	3.885		PER IMP= 30.00
*S COMBINE 400.88 AND 400.1 AS 400.91											
ADD HYD	400.91	1& 2	2	4.66810	3111.66	332.988	1.33749	2.100	1.042		
RAINFALL TYPE= 2											RAIN24= 3.450
*S AP 400.13 IS SANDIA COLONY											
COMPUTE NM HYD	400.13	-	14	.01390	37.55	1.627	2.19512	1.500	4.221		PER IMP= 50.00
ROUTE MCUNGE	400.57	14	6	.01390	32.26	1.617	2.18090	1.800	3.626		CCODE = .1
RAINFALL TYPE= 2											RAIN24= 3.380
COMPUTE NM HYD	402.14	-	1	.08840	168.01	7.582	1.60823	1.550	2.970		PER IMP= 30.00
*S (402.92=INTERMEDIATE COMBINATION STEP AT END OF 402.14)											
ADD HYD	402.92	6& 1	6	.10230	168.90	9.199	1.68603	1.550	2.580		
*S (400.93=PARTIAL INFLOW TO SDB DAM - TOTAL=401.99)											
ADD HYD	400.93	6& 2	1	4.77040	3153.87	342.187	1.34496	2.100	1.033		
*S 401.5 AND 401.4 DIVERTED TO SOB DAM IN PROPOSED STORM DRAIN ON LOWELL											
RAINFALL TYPE= 2											RAIN24= 3.380
COMPUTE NM HYD	401.50	-	18	.04500	97.92	3.517	1.46537	1.500	3.400		PER IMP= 17.00
*S ROUTE 401.5 IN 54-INCH STORM DRAIN TO PINO AVE.											
ROUTE	401.80	18	6	.04500	88.90	3.517	1.46538	1.550	3.087		
RAINFALL TYPE= 2											RAIN24= 3.380
COMPUTE NM HYD	401.40	-	19	.04520	98.36	3.533	1.46537	1.500	3.400		PER IMP= 17.00
ADD HYD	401.93	6&19	11	.09020	184.96	7.049	1.46537	1.500	3.204		
*S ROUTE 401.93 IN 60-INCH STORM DRAIN TO SOB DAM											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
ROUTE	401.82	11	7	.0902D	181.73	7.049	1.46537	1.550	3.148	
*S COMBINE 401.82 AND 400.92 AS 401.99										
*S (401.99=TOTAL INFLOW TO SOUTH OOMINGO BACA DAM)										
ADD HYD	401.99	7& 1	1	4.86060	3178.37	349.237	1.34720	2.100	1.022	
*S S D B DAM OUTFLOW HYDROGRAPH (400.81)										
ROUTE RESERVOIR	400.81	1	2	4.86060	174.81	349.244	1.34722	4.250	.056	AC-FT= 276.472
*S AP 400.81 IS THROUGH PRINCIPAL SPILLWAY										
*S AP SDB.SPILL IS THROUGH EMERGENCY SPILLWAY										
DIVIDE HYD	400.81	2	2	4.86060	174.81	349.244	1.34722	4.250	.056	
	SDB.SPILL AND	20		.00000	.00	.000	.00000	-.050	.000	
*S ROUTE 400.81 IN 48-INCH RCP PALOMAS STORM DRAIN TO BROWNING (400.83)										
ROUTE	400.83	2	3	4.86060	174.81	349.244	1.34722	4.250	.056	
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.10	-	1	.06730	144.54	5.156	1.43652	1.500	3.356	RAIN24= 3.330 PER IMP= 17.00
*S PALOMAS STORM DRAIN AT BROWNING ST. (403.99)										
ADD HYD	403.99	1& 3	1	4.92790	249.59	354.40D	1.34844	1.500	.079	
*S ROUTE 403.99 IN 54-INCH RCP PALOMAS STORM DRAIN TO END OF 403.3 (403.88)										
ROUTE	403.88	1	2	4.92790	248.73	354.399	1.34844	1.550	.079	
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.30	-	1	.01550	32.72	1.157	1.39974	1.500	3.298	RAIN24= 3.250 PER IMP= 17.00
*S PALOMAS STORM DRAIN AT END OF 403.3 (403.89)										
ADD HYD	403.89	1& 2	1	4.94340	279.12	355.557	1.34860	1.500	.088	
*S ROUTE 403.89 IN 54-INCH RCP PALOMAS STORM DRAIN TO EUBANK (403.84)										
ROUTE	403.84	1	2	4.94340	279.70	355.557	1.34860	1.550	.088	
COMPUTE NM HYD	408.10	-	1	.00980	20.69	.732	1.39974	1.500	3.299	PER IMP= 17.00
*S PALOMAS STORM DRAIN AT EUBANK (408.9)										
ADD HYD	408.90	1& 2	1	4.95320	298.97	356.288	1.34870	1.500	.094	
*S ROUTE 408.9 IN 78-INCH RCP NORTH REACH OF EUBANK STORM DRAIN TO										
*S COMBINATION AT 403.79 (403.84)										
ROUTE	408.80	1	3	4.95320	300.31	356.288	1.34870	1.550	.095	
RAINFALL TYPE= 2										
COMPUTE NM HYD	403.20	-	1	.07960	150.24	5.942	1.39974	1.550	2.949	RAIN24= 3.250 PER IMP= 17.00
*S COMBINE 403.2 AND NORTH REACH OF EUBANK STORM DRAIN FLOW (NORTH SIDE										
*S OF RANCHITOS (403.79))										
ADD HYD	403.79	1& 3	8	5.03280	450.55	362.231	1.34951	1.550	.140	
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.30	-	1	.04630	99.31	3.547	1.43652	1.500	3.352	RAIN24= 3.330 PER IMP= 17.00
ROUTE MCUNGE	404.38	1	2	.04630	88.90	3.547	1.43652	1.750	3.000	CCODE = .1 RAIN24= 3.250
RAINFALL TYPE= 2										
COMPUTE NM HYD	404.20	-	1	.05250	110.77	3.919	1.39974	1.500	3.297	PER IMP= 17.00
ADD HYD	404.48	1& 2	5	.09880	137.07	7.466	1.41697	1.700	2.168	
*S										
*S END OF STUDY FOR S D B MAIN ARROYO (EUBANK)										
*S										
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.32	-	1	.05200	111.54	3.984	1.43652	1.500	3.352	RAIN24= 3.330 PER IMP= 17.00
ROUTE MCUNGE	401.38	1	6	.05200	110.74	3.984	1.43652	1.550	3.328	CCODE = .2 RAIN24= 3.330
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.33	-	1	.04300	92.24	3.294	1.43652	1.500	3.352	PER IMP= 17.00
ROUTE MCUNGE	401.07	1	3	.04300	87.01	3.294	1.43653	1.600	3.162	CCODE = .1
ROUTE MCUNGE	401.08	3	2	.04300	86.60	3.294	1.43653	1.600	3.147	CCODE = .2
ADD HYD	401.18	2& 6	6	.09500	193.60	7.278	1.43651	1.600	3.184	
ROUTE	SPILL.ROU	20	19	.00000	.00	.000	.00000	-.050	.000	
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.31	-	1	.05180	111.11	3.969	1.43652	1.500	3.352	RAIN24= 3.330 PER IMP= 17.00
ADD HYD	401.35	1&19	1	.05180	111.11	3.969	1.43651	1.500	3.352	
ROUTE MCUNGE	401.28	1	3	.05180	110.25	3.969	1.43652	1.550	3.326	CCODE = .2
ADD HYD	401.78	3& 6	1	.14680	300.21	11.247	1.43651	1.600	3.195	
ROUTE MCUNGE	401.77	1	3	.14680	299.63	11.247	1.43652	1.600	3.189	CCODE = .2
ROUTE MCUNGE	401.88	3	2	.14680	291.56	11.247	1.43652	1.700	3.103	CCODE = .1 RAIN24= 3.250
RAINFALL TYPE= 2										
COMPUTE NM HYD	401.20	-	1	.08500	180.40	6.345	1.39974	1.500	3.316	PER IMP= 17.00
ADD HYD	401.29	1& 2	1	.23180	378.97	17.592	1.42303	1.650	2.555	
RAINFALL TYPE= 2										
COMPUTE NM HYD	507.00	-	4	.00890	18.50	.650	1.37021	1.500	3.247	RAIN24= 3.200 PER IMP= 17.00
*S SOUTH REACH OF EUBANK STORM DRAIN FLOW AT PINO AVE. (401.9)										
ADD HYD	401.90	1& 4	11	.24070	389.82	18.243	1.42107	1.650	2.530	
*S ROUTE 401.9 IN 84-INCH RCP SOUTH REACH EUBANK STORM DRAIN										
*S TO COMBINATION WITH 404.48 401.83)										
ROUTE	401.83	11	6	.24070	386.59	18.243	1.42108	1.650	2.510	
*S SOUTH REACH EUBANK STORM DRAIN FLOW AT SOUTH SIDE OF RANCHITOS										
ADD HYD	404.99	5& 6	11	.33950	522.95	25.709	1.41988	1.700	2.407	
*S*****										
*S COMBINE HYDS. 404.99 AND 403.79 AS 404.95										
*S 404.95=SUM OF SOUTH OOMINGO SO FLOWS AT EUBANK AND RANCHITOS										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ADD HYD	404.95	8&11	2	5.37230	887.89	387.940	1.35396	1.600	.258	
*S END OF STUDY AT EUBANK										
*S *****										
FINISH										

PINO SYS. EXISTING COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = pino10.ext

RUN DATE (MON/DAY/YR) = 10/16/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION	

*S	NORTH PINO ARROYO (N. OF SAN ANTONIO DR.) AND MAIN PINO ARROYO											
*S	EXISTING CONDITIONS, 10-YR RETURN PERIOD, 24-HR STORM											

START												
*S	BEGIN ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY										TIME=	.00
*S	SUBBASIN 500.0=(FLOW E. OF TRAMWAY N.OF POND N.OF SAN RAFAEL)											
RAINFALL TYPE= 2												
COMPUTE NM HYD	500.00	-	1	.06350	71.54	2.735	.80763	1.500	1.760 PER IMP=	25.00	RAIN24= 2.330	
COMPUTE NM HYD	500.10	-	2	.05490	57.39	2.365	.80763	1.500	1.633 PER IMP=	25.00		
COMPUTE NM HYD	500.20	-	3	.05080	57.23	2.188	.80763	1.500	1.760 PER IMP=	25.00		
*S	ROUTE 500.1 THROUGH 500.2 - (NATURAL CHANNEL WEST OF QUAIL RUN LOOP)											
ROUTE MCUNGE	500.82	2	7	.05490	56.95	2.364	.80736	1.500	1.621 CCODE =	.2		
*S	COMBINE ROUTED 500.1 WITH 500.2 AT AP 500.91											
*S	(FLOW E. OF TRAMWAY COLLECTED AT POND NORTH OF SAN RAFAEL)											
ADD HYD	500.91	3 & 7	11	.10570	114.18	4.552	.80748	1.500	1.688			
*S	DEVELOP HYDROGRAPH 500.3											
*S	(FLOW E. OF TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)											
COMPUTE NM HYD	500.30	-	2	.05320	52.71	2.292	.80763	1.550	1.548 PER IMP=	25.00		
*S	DEVELOP HYDROGRAPH 500.4											
*S	(FLOW E. OF TRAMWAY NEAR QUAIL RUN COURT)											
COMPUTE NM HYD	500.40	-	3	.02090	23.55	.900	.80763	1.500	1.761 PER IMP=	25.00		
*S	COMBINE FLOWS TO ESTIMATE CONTRIBUTION STORM DRAIN AND PINO DAM											
*S	(500.94=COMBINED FLOW AT POND N. OF SAN RAFAEL)											
ADD HYD	500.94	11 & 1	12	.16920	185.72	7.287	.80754	1.500	1.715			
*S	(500.95=COMBINED FLOW AT TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)											
ADD HYD	500.95	12 & 2	13	.22240	237.50	9.579	.80756	1.500	1.669			
*S	(500.96 = ESTIMATED FLOW TO DAM)											
ADD HYD	500.96	13 & 3	19	.24330	261.05	10.479	.80756	1.500	1.677			
*S	END OF ANALYSIS OF WATERSHED EAST OF TRAMWAY											

*S	BEGIN ANALYSIS OF WATERSHED WEST OF TRAMWAY											
RAINFALL TYPE= 2												
COMPUTE NM HYD	501.00	-	1	.01392	11.52	.371	.50006	1.500	1.293 PER IMP=	10.00	RAIN24= 2.200	
*S	(SUBBASIN 501.0=FLOW AT LOWELL STREET BETWEEN CORONADO AND SAN RAFAEL)											
*S	ROUTE 501.0 THROUGH 501.1											
ROUTE MCUNGE	501.80	1	6	.01392	9.72	.371	.50007	1.800	1.091 CCODE =	.1		
COMPUTE NM HYD	501.10	-	2	.03920	32.43	1.045	.50006	1.500	1.293 PER IMP=	10.00		
*S	COMBINE 501.80 WITH 501.1 AS 501.91											
*S	(501.91=FLOW AT BROWNING BETWEEN CORONADO AND SAN RAFAEL)											
ADD HYD	501.91	6 & 2	11	.05312	32.78	1.417	.50005	1.500	.964			
*S	ROUTE 501.91 THROUGH 501.2											
ROUTE MCUNGE	501.81	11	6	.05312	29.94	1.401	.49437	1.650	.881 CCODE =	.1		
RAINFALL TYPE= 2												
COMPUTE NM HYD	501.20	-	2	.05010	38.59	1.244	.46542	1.500	1.204 PER IMP=	10.00	RAIN24= 2.130	
*S	COMBINE 501.81 WITH 501.2 AS 501.99											
*S	(501.99=FLOW AT EUBANK BETWEEN SAN RAFAEL AND DEL REY)											
ADD HYD	501.99	6 & 2	11	.10322	52.55	2.644	.48031	1.600	.795			

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	508.00	-	1	.02590	41.79	1.696	1.22766	1.500	2.521	RAIN24= 2.200 PER IMP= 50.00
*S (508.0=INFLOW FROM SANDIA HEIGHTS SOUTH UNITS 23 AND 24 TO SUBBASIN 502)										
*S ROUTE 508.0 THROUGH 502.0										
ROUTE MCUNGE	508.80	1	6	.02590	41.60	1.692	1.22485	1.550	2.510	CCODE = .2
COMPUTE NM HYD	502.00	-	1	.06295	52.07	1.679	.50006	1.500	1.293	PER IMP= 10.00
*S COMBINE RP 508.0 WITH 502.0 AS 502.91										
*S (502.91=FLOW AT LOWELL BETWEEN SAN RAFAEL AND DEL REY)										
ADD HYD	502.91	1& 6	12	.08885	89.90	3.371	.71133	1.550	1.581	
COMPUTE NM HYD	502.10	-	1	.07500	62.15	2.000	.50006	1.500	1.295	PER IMP= 10.00
*S (502.1=FLOW AT LOWELL IN NORTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)										
*S COMBINE 502.91 WITH 502.1 AS 502.92										
*S (502.92=COMBINED FLOW IN ARROYO JUNCTION W.OF LOWELL BETWEEN										
*S DEL REY AND SAN RAFAEL)										
ADD HYD	502.92	12& 1	13	.16385	147.55	5.371	.61462	1.550	1.407	
*S ROUTE 502.92 THROUGH 502.3										
ROUTE MCUNGE	502.80	13	6	.16385	141.38	5.387	.61647	1.600	1.348	CCODE = .2
COMPUTE NM HYD	502.20	-	1	.03390	24.01	.904	.50006	1.550	1.107	PER IMP= 10.00
*S (502.2=FLOW AT LOWELL IN SOUTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)										
*S ROUTE 502.2 THROUGH 502.3										
ROUTE MCUNGE	502.81	1	7	.03390	22.64	.896	.49571	1.650	1.044	CCODE = .1
*S COMBINE 502.81 WITH 502.80 AS 502.93										
ADD HYD	502.93	6& 7	12	.19775	162.87	6.283	.59576	1.600	1.287	
COMPUTE NM HYD	502.30	-	1	.03950	32.68	1.053	.50006	1.500	1.293	PER IMP= 10.00
*S COMBINE 502.3 WITH 502.93 AS 502.99										
*S (502.99=FLOW TO PINO DIVERSION DROP STRUCTURE)										
ADD HYD	502.99	12& 1	13	.23725	186.87	7.337	.57983	1.600	1.231	
*S *****										
COMPUTE NM HYD	503.00	-	1	.05610	45.82	1.496	.50006	1.500	1.276	PER IMP= 10.00
*S (503.0=FLOW AT BROWNING BETWEEN DEL REY AND SANTA MONICA)										
COMPUTE NM HYD	503.10	-	2	.02200	17.16	.587	.50006	1.500	1.218	PER IMP= 10.00
*S (503.1=FLOW AT BROWNING AT BETWEEN SAN RAFAEL AND DEL REY)										
*S COMBIINE 503.0 AND 503.1 AS 503.91										
ADD HYD	503.91	1& 2	12	.07810	62.97	2.083	.50006	1.500	1.260	
*S (503.91=FLOW AT ARROYO JUNCTION W. OF BROWNING BETWEEN DEL REY										
*S AND SANTA MONICA)										
*S ROUTE 503.91 THROUGH 503.2										
ROUTE MCUNGE	503.81	12	7	.07810	56.10	2.040	.48968	1.700	1.122	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	503.20	-	1	.04940	38.05	1.226	.46542	1.500	1.204	RAIN24= 2.130 PER IMP= 10.00
*S COMBIINE 503.2 AND 503.91 AS 503.99										
*S (503.99=FLOW AT EUBANK BETWEEN DEL REY AND SANTA MONICA)										
ADO HYD	503.99	7& 1	20	.12750	72.59	3.266	.48027	1.650	.890	
*S *****										
*S MISCELLANEOUS BASINS THAT FLOW TO EAST SIDE OF EUBANK										
COMPUTE NM HYD	504.00	-	1	.03720	28.66	.923	.46542	1.500	1.204	PER IMP= 10.00
*S (504.0=FLOW AT EUBANK NORTH OF CORONADO BELOW SAN FRANCISCO)										
COMPUTE NM HYD	505.00	-	2	.02870	22.11	.712	.46542	1.500	1.204	PER IMP= 10.00
*S (505.0=FLOW AT EUBANK FROM SMALL SUBBASIN THAT STRADOLES SAN RAFAEL)										
COMPUTE NM HYD	506.00	-	3	.04840	25.65	1.201	.46542	1.600	.828	PER IMP= 10.00
*S (506.0=FLOW AT EUBANK NORTH OF SAN ANTONIO SOUTH OF SANTA MONICA)										
COMPUTE NM HYD	507.00	-	4	.00890	6.86	.221	.46542	1.500	1.205	PER IMP= 10.00
*S (507.0=FLOW AT EUBANK NORTH OF SAN FRANCISCO BELOW S. DOMINGO										
*S BACA WATERSHED DIVIDE)										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
COMPUTE NM HYD	509.00	-	5	.02590	40.32	1.626	1.17709	1.500	2.432	PER IMP= 50.00
*S (509.0=SANDIA HEIGHTS SOUTH UNITS 21 & 23 SD OUTFALL TO PINO DAM)										
*S *****										
*S HYDROGRAPHS AT QUINTESSENCE STORM DRAINS (EUBANK)										
*S COMBINE 507.0 WITH 504.00 AT CORONADO AND EUBANK										
ADD HYD	511.90	1& 4	12	.04610	35.52	1.144	.46541	1.500	1.204	
*S ROUTE 511.90 THROUGH CHANNEL ON EAST SIDE OF EUBANK										
*S (510.80=FLOW TO MH#4 AT CORONADO AND EUBANK)										
ROUTE MCUNGE	511.80	12	7	.04610	35.49	1.144	.46539	1.550	1.203	CCODE = .2
*S										
*S (511.91= FLOW TO MH#3 AT SAN RAFAEL)										
ADD HYD	511.91	2& 7	13	.07480	56.07	1.857	.46539	1.550	1.171	
*S (511.92=FLOW TO MH#2 AT SANTA MONICA)										
ADD HYD	510.92	13&11	21	.17802	98.35	4.501	.47404	1.550	.863	
*S (511.99=FLOW AT OUTFALL WEST OF EUBANK AT SANTA MONICA)										
ADD HYD	511.99	21&20	22	.30552	161.14	7.767	.47664	1.600	.824	
*S END OF ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY										
*S*****										
*S MAIN PINO ARROYO										
*S EXISTING CONDITIONS, 10-YR RETURN PERIOD, 24-HR STORM										
*S*****										
*S BEGIN MAIN PINO ARROYO BASINS										
*S*****										
RAINFALL TYPE= 2										
COMPUTE NM HYD	510.00	-	1	3.17800	1561.97	112.465	.66354	1.833	.768	RAIN24= 2.730 PER IMP= .00
*S ROUTE 510.0 THRU 510.2 (PA-1 THRU BASIN PA-16)										
ROUTE MCUNGE	510.80	1	6	3.17800	1553.85	112.342	.66281	1.900	.764	CCODE = .1
COMPUTE NM HYD	510.10	-	1	.46400	447.72	16.420	.66354	1.567	1.508	PER IMP= .00
*S ROUTE 510.1 THRU 510.2 (PA-2 THROUGH BASIN PA-15)										
ROUTE MCUNGE	510.81	1	5	.46400	445.82	16.434	.66408	1.633	1.501	CCODE = .2
*S ADD 510.81 AND 510.80										
ADD HYD	510.90	6& 5	11	3.64200	1734.23	128.776	.66297	1.833	.744	
*S ROUTE 510.90 THRU 510.2										
ROUTE MCUNGE	510.82	11	7	3.64200	1729.21	128.746	.66282	1.900	.742	CCODE = .1
COMPUTE NM HYD	510.20	-	1	.27000	170.53	4.494	.31211	1.500	.987	PER IMP= .00
*S ADD 510.9 AND 510.2 (ROUTED PA-1 TO PA-16)										
ADD HYD	510.91	1& 7	12	3.91200	1748.06	133.240	.63861	1.900	.698	
*S ROUTE SUM OF 510.0 TO 510.2 THRU 510.3(PA-1 & PA-16 THROUGH BASIN PA-17)										
ROUTE MCUNGE	510.83	12	7	3.91200	1735.44	132.731	.63617	2.033	.693	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	510.30	-	1	.27800	209.94	6.322	.42637	1.500	1.180	RAIN24= 2.420 PER IMP= 6.00
*S SEPARATE 510.3 (PA-17) INTO NORTH AND SOUTH DRAINAGE										
DIVIDE HYD	510.3N	1	11	.16124	121.77	3.667	.42637	1.500	1.180	
	510.3S	AND	12	.11676	88.18	2.655	.42637	1.500	1.180	
*S SEPARATE 510.3S INTO A (FLOW TO S. EAGLE RIDGE DR.) AND B (FLOW TO CV-023-47)										
DIVIDE HYD	510.3SA	12	13	.06422	48.50	1.460	.42637	1.500	1.180	
	510.3SB	AND	14	.05254	39.68	1.195	.42637	1.500	1.180	
*S ADD FLOW TO CV-D23-47 BACK TO P17N (DS FROM N. EAGLE RIDGE DR.)										
ADD HYD	510.3N	14&11	11	.21378	161.44	4.861	.42636	1.500	1.180	
*S TOTAL DRAINAGE AT END OF BASIN 510.3 (PA-17) IN MAIN ARROYO										
ADD HYD	510.92	11& 7	11	4.12578	1752.12	137.606	.62536	2.033	.664	
*S SEPARATE 510.3SA(P17A) INTO FLOW TO 510.3SC (CV-023-49) AND 510.3SD (DS-023-1										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4 NOTATION
DIVIDE HYD	510.3SC	13	14	.04559	34.43	1.037	.42636	1.500	1.180	
	510.3SD	AND	15	.01862	14.06	.423	.42636	1.500	1.180	
*S ADD FLOW DS FROM EAGLE ROCK PLACE										
ADD HYD	510.93	14&15	14	.06422	48.50	1.460	.42637	1.500	1.180	
*S ROUTE 510.93(P17C&D) THROUGH NATURAL CHANNEL TO CONF. W/MAIN PINO										
ROUTE MCUNGE	510.84	14	7	.06422	41.91	1.433	.41834	1.767	1.020	CCODE = .1
*S ROUTE SUMS 510.92(PA-1 THRU PA-17) THROUGH BASIN 510.4(PA-19)										
ROUTE MCUNGE	510.85	11	8	4.12578	1742.34	137.278	.62387	2.133	.660	CCODE = .1
COMPUTE NM HYD	510.40	-	1	.22100	209.90	7.391	.62702	1.500	1.484	PER IMP= 16.00
*S ADD ROUTED FLOWS TO 510.4										
ADD HYD	510.94	1& 8	19	4.34678	1761.87	144.668	.62403	2.133	.633	
*S 510.99=TOTAL MAIN PINO FLOW AT TRAMWAY										
ADD HYD	510.99	19& 7	20	4.41100	1770.22	146.101	.62104	2.133	.627	
FINISH										

PINO SYS. EXISTING COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = pino100.ext

RUN DATE (MON/DAY/YR) =10/16/1998
USER NO.= RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM IO NO.	TO IO NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION

*S	NORTH PINO ARROYO (N. OF SAN ANTONIO DR.) AND MAIN PINO ARROYO										
*S	EXISTING CONDITIONS, 100-YR RETURN PERIOD, 24-HR STORM										

START										TIME=	.00
*S BEGIN ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY (N. OF MAIN PINO)											
*S SUBBASIN 500.0=(FLOW E. OF TRAMWAY N.OF POND N.OF SAN RAFAEL)											
RAINFALL TYPE= 2											
										RAIN24=	3.500
COMPUTE NM HYD	500.00	-	1	.06350	134.41	5.234	1.54547	1.500	3.307 PER IMP=	25.00	
COMPUTE NM HYD	500.10	-	2	.05490	108.56	4.525	1.54547	1.500	3.090 PER IMP=	25.00	
COMPUTE NM HYD	500.20	-	3	.05080	107.50	4.187	1.54547	1.500	3.307 PER IMP=	25.00	
*S ROUTE 500.1 THROUGH 500.2 - (NATURAL CHANNEL WEST OF QUAIL RUN LOOP)											
ROUTE MCUNGE	500.82	2	7	.05490	108.12	4.525	1.54530	1.500	3.077 CCODE =	.2	
*S COMBINE ROUTED 500.1 WITH 500.2 AT AP 500.91											
*S (FLOW E. OF TRAMWAY COLLECTED AT POND NORTH OF SAN RAFAEL)											
ADD HYD	500.91	3& 7	11	.10570	215.63	8.712	1.54537	1.500	3.187		
*S DEVELOP HYDROGRAPH 500.3											
*S (FLOW E. OF TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)											
COMPUTE NM HYD	500.30	-	2	.05320	99.55	4.385	1.54547	1.550	2.924 PER IMP=	25.00	
*S DEVELOP HYDROGRAPH 500.4											
*S (FLOW E. OF TRAMWAY NEAR QUAIL RUN COURT)											
COMPUTE NM HYD	500.40	-	3	.02090	44.24	1.723	1.54547	1.500	3.307 PER IMP=	25.00	
*S COMBINE FLOWS TO ESTIMATE CONTRIBUTION STORM DRAIN AND PINO DAM											
*S (500.94=COMBINED FLOW AT POND N. OF SAN RAFAEL)											
ADD HYD	500.94	11& 1	12	.16920	350.04	13.946	1.54541	1.500	3.232		
*S (500.95=COMBINED FLOW AT TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)											
ADD HYD	500.95	12& 2	13	.22240	448.51	18.331	1.54542	1.500	3.151		
*S (500.96 = ESTIMATED FLOW TO DAM)											
ADD HYD	500.96	13& 3	19	.24330	492.75	20.053	1.54542	1.500	3.165		
*S END OF ANALYSIS OF WATERSHED EAST OF TRAMWAY											

*S BEGIN ANALYSIS OF WATERSHED WEST OF TRAMWAY											
RAINFALL TYPE= 2											
										RAIN24=	3.300
COMPUTE NM HYD	501.00	-	1	.01392	24.75	.833	1.12163	1.500	2.778 PER IMP=	10.00	
*S (SUBBASIN 501.0=FLOW AT LOWELL STREET BETWEEN CORONAADO AND SAN RAFAEL)											
*S ROUTE 501.0 THROUGH 501.1											
ROUTE MCUNGE	501.80	1	6	.01392	21.42	.815	1.09713	1.750	2.404 CCODE =	.1	
COMPUTE NM HYD	501.10	-	2	.03920	69.66	2.345	1.12163	1.500	2.777 PER IMP=	10.00	
*S COMBINE 501.80 WITH 501.1 AS 501.91											
*S (501.91=FLOW AT BROWNING BETWEEN CORONAADO AND SAN RAFAEL)											
ADD HYD	501.91	6& 2	11	.05312	70.70	3.159	1.11519	1.500	2.080		
*S ROUTE 501.91 THROUGH 501.2											
ROUTE MCUNGE	501.81	11	6	.05312	66.21	3.114	1.09906	1.650	1.947 CCODE =	.1	
RAINFALL TYPE= 2											
										RAIN24=	3.200
COMPUTE NM HYD	501.20	-	2	.05010	85.46	2.840	1.06288	1.500	2.665 PER IMP=	10.00	
*S COMBINE 501.81 WITH 501.2 AS 501.99											
*S (501.99=FLOW AT EUBANK BETWEEN SAN RAFAEL AND DEL REY)											
ADD HYD	501.99	6& 2	11	.10322	122.91	5.954	1.08149	1.600	1.861		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2	NOTATION
RAINFALL TYPE= 2											
COMPUTE NM HYD	508.00	-	1	.02590	68.75	2.903	2.10174	1.500	4.147	RAIN24= 3.300	PER IMP= 50.00
*S (508.0=INFLOW FROM SANDIA HEIGHTS SOUTH UNITS 23 AND 24 TO SUBBASIN 502)											
*S ROUTE 508.0 THROUGH 502.0											
ROUTE MCUNGE	508.80	1	6	.02590	62.86	2.875	2.08108	1.650	3.792	CCODE = .1	
COMPUTE NM HYD	502.00	-	1	.06295	111.88	3.766	1.12163	1.500	2.777	PER IMP= 10.00	
*S COMBINE RP 508.0 WITH 502.0 AS 502.91											
*S (502.91=FLOW AT LOWELL BETWEEN SAN RAFAEL AND DEL REY)											
ADD HYD	502.91	1 & 6	12	.08885	145.37	6.640	1.40130	1.550	2.556		
COMPUTE NM HYD	502.10	-	1	.07500	133.93	4.487	1.12163	1.500	2.790	PER IMP= 10.00	
*S (502.1=FLOW AT LOWELL IN NORTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)											
*S COMBINE 502.91 WITH 502.1 AS 502.92											
*S (502.92=COMBINED FLOW IN ARROYO JUNCTION W.OF LOWELL BETWEEN											
*S DEL REY AND SAN RAFAEL)											
ADD HYD	502.92	12 & 1	13	.16385	270.43	11.127	1.27328	1.500	2.579		
*S ROUTE 502.92 THROUGH 502.3											
ROUTE MCUNGE	502.80	13	6	.16385	270.12	11.102	1.27040	1.550	2.576	CCODE = .1	
COMPUTE NM HYD	502.20	-	1	.03390	52.21	2.028	1.12163	1.550	2.407	PER IMP= 10.00	
*S (502.2=FLOW AT LOWELL IN SOUTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)											
*S ROUTE 502.2 THROUGH 502.3											
ROUTE MCUNGE	502.81	1	7	.03390	50.68	2.010	1.11157	1.600	2.336	CCODE = .1	
*S COMBINE 502.81 WITH 502.80 AS 502.93											
ADD HYD	502.93	6 & 7	12	.19775	315.10	13.111	1.24316	1.550	2.490		
COMPUTE NM HYD	502.30	-	1	.03950	70.20	2.363	1.12163	1.500	2.777	PER IMP= 10.00	
*S COMBINE 502.3 WITH 502.93 AS 502.99											
*S (502.99=FLOW TO PINO DIVERSION DROP STRUCTURE)											
ADD HYD	502.99	12 & 1	13	.23725	380.20	15.474	1.22293	1.550	2.504		
*S *****											
COMPUTE NM HYD	503.00	-	1	.05610	98.61	3.356	1.12163	1.500	2.746	PER IMP= 10.00	
*S (503.0=FLOW AT BROWNING BETWEEN DEL REY AND SANTA MONICA)											
COMPUTE NM HYD	503.10	-	2	.02200	37.19	1.316	1.12163	1.500	2.641	PER IMP= 10.00	
*S (503.1=FLOW AT BROWNING AT BETWEEN SAN RAFAEL AND DEL REY)											
*S COMBINE 503.0 AND 503.1 AS 503.91											
ADD HYD	503.91	1 & 2	12	.07810	135.79	4.672	1.12162	1.500	2.717		
*S (503.91=FLOW AT ARROYO JUNCTION W. OF BROWNING BETWEEN DEL REY											
*S AND SANTA MONICA)											
*S ROUTE 503.91 THROUGH 503.2											
ROUTE MCUNGE	503.81	12	7	.07810	125.63	4.616	1.10819	1.650	2.513	CCODE = .1	
RAINFALL TYPE= 2											
COMPUTE NM HYD	503.20	-	1	.04940	84.27	2.800	1.06288	1.500	2.665	PER IMP= 10.00	
*S COMBINE 503.2 AND 503.91 AS 503.99											
*S (503.99=FLOW AT EUBANK BETWEEN DEL REY AND SANTA MONICA)											
ADD HYD	503.99	7 & 1	20	.12750	176.74	7.416	1.09063	1.600	2.166		
*S *****											
*S MISCELLANEOUS BASINS THAT FLOW TO EAST SIDE OF EUBANK											
COMPUTE NM HYD	504.00	-	1	.03720	63.46	2.109	1.06288	1.500	2.666	PER IMP= 10.00	
*S (504.0=FLOW AT EUBANK NORTH OF CORONADO BELOW SAN FRANCISCO)											
COMPUTE NM HYD	505.00	-	2	.02870	48.96	1.627	1.06288	1.500	2.666	PER IMP= 10.00	
*S (505.0=FLOW AT EUBANK FROM SMALL SUBBASIN THAT STRADDLES SAN RAFAEL)											
COMPUTE NM HYD	506.00	-	3	.04840	58.52	2.744	1.06288	1.600	1.889	PER IMP= 10.00	
*S (506.0=FLOW AT EUBANK NORTH OF SAN ANTONIO SOUTH OF SANTA MONICA)											
COMPUTE NM HYD	507.00	-	4	.00890	15.20	.505	1.06288	1.500	2.668	PER IMP= 10.00	
*S (507.0=FLOW AT EUBANK NORTH OF SAN FRANCISCO BELOW S. DOMINGO											
*S BACA WATERSHED DIVIDE)											

FROM	TO	PEAK	RUNOFF	TIME TO	CFS	PAGE = 3
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COMMAND	HYDROGRAPH IDENTIFICATION	ID NO.	ID NO.	AREA (SQ MI)	DISCHARGE (CFS)	VOLUME (AC-FT)	RUNOFF (INCHES)	PEAK (HOURS)	PER ACRE	NOTATION
COMPUTE NM HYD	509.DD	-	5	.02590	66.74	2.793	2.02208	1.500	4.026	PER IMP= 50.00
*S (509.0=SANDIA HEIGHTS SOUTH UNITS 21 & 23 SD OUTFALL TO PINO DAM)										
*S *****										
*S HYDROGRAPHS AT QUINTESSENCE STORM DRAINS (EUBANK)										
*S COMBINE 507.0 WITH 504.00 AT CORONADO AND EUBANK										
ADD HYD	511.90	1& 4	12	.04610	78.66	2.613	1.06287	1.500	2.666	
*S ROUTE 511.90 THROUGH CHANNEL ON EAST SIDE OF EUBANK										
*S (511.80=FLOW TO MH#4 AT CORONADO AND EUBANK)										
ROUTE MCUNGE	511.80	12	7	.04610	76.64	2.623	1.06691	1.550	2.598	CCODE = .2
*S										
*S (511.91= FLOW TO MH#3 AT SAN RAFAEL)										
ADD HYD	511.91	2& 7	13	.07480	124.96	4.250	1.06535	1.500	2.610	
*S (511.92=FLOW TO MH#2 AT SANTA MONICA)										
ADD HYD	511.92	13&11	21	.17802	239.31	10.204	1.07471	1.550	2.100	
*S (511.99=FLOW AT OUTFALL WEST OF EUBANK AT SANTA MONICA)										
ADD HYD	511.99	21&20	22	.30552	401.48	17.620	1.08135	1.600	2.053	
*S END OF ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY										
*S*****										
*S*****										
*S MAIN PINO ARROYO										
*S EXISTING CONDITIONS, 100-YR RETURN PERIOD, 24-HR STORM										
*S*****										
*S BEGIN MAIN PINO ARROYO BASINS										
*S*****										
RAINFALL TYPE= 2										
COMPUTE NM HYD	510.00	-	1	3.17800	3292.83	228.752	1.34962	1.833	1.619	RAIN24= 4.100 PER IMP= .00
*S ROUTE 510.0 THRU 510.2 (PA-1 THRU BASIN PA-16)										
ROUTE MCUNGE	510.80	1	6	3.17800	3278.95	228.382	1.34744	1.900	1.612	CCODE = .1
COMPUTE NM HYD	510.10	-	1	.46400	869.97	33.399	1.34962	1.567	2.930	PER IMP= .00
*S ROUTE 510.1 THRU 510.2 (PA-2 THROUGH BASIN PA-15)										
ROUTE MCUNGE	510.81	1	5	.46400	852.72	33.322	1.34652	1.633	2.871	CCODE = .1
*S ADD 510.81 AND 510.80										
ADD HYD	510.90	6& 5	11	3.64200	3701.81	261.704	1.34732	1.866	1.588	
*S ROUTE 510.90 THRU 510.2										
ROUTE MCUNGE	510.82	11	7	3.64200	3696.06	261.540	1.34648	1.900	1.586	CCODE = .1
COMPUTE NM HYD	510.20	-	1	.27000	448.49	12.184	.84608	1.500	2.595	PER IMP= .00
*S ADD 510.9 AND 510.2 (ROUTED PA-1 TO PA-16)										
ADD HYD	510.91	1& 7	12	3.91200	3745.17	273.724	1.31194	1.900	1.496	
*S ROUTE SUM OF 510.0 TO 510.2 THRU 510.3(PA-1 & PA-16 THROUGH BASIN PA-17)										
ROUTE MCUNGE	510.83	12	7	3.91200	3723.21	273.118	1.30904	2.000	1.487	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	510.30	-	1	.27800	513.58	15.171	1.02326	1.500	2.887	RAIN24= 3.650 PER IMP= 6.00
*S SEPARATE 510.3 (PA-17) INTO NORTH AND SOUTH DRAINAGE										
DIVIDE HYD	510.3N	1	11	.16124	297.88	8.799	1.02326	1.500	2.887	
	510.3S	AND	12	.11676	215.70	6.372	1.02326	1.500	2.887	
*S SEPARATE 510.3S INTO A (FLOW TO S. EAGLE RIDGE DR.) AND B (FLOW TO CV-023-47)										
DIVIDE HYD	510.3SA	12	13	.06422	118.64	3.505	1.02326	1.500	2.887	
	510.3SB	AND	14	.05254	97.07	2.867	1.02326	1.500	2.887	
*S ADD FLOW TO CV-D23-47 BACK TO P17N (OS FROM N. EAGLE RIDGE DR.)										
ADD HYD	510.3N	14&11	11	.21378	394.94	11.667	1.02326	1.500	2.887	
*S TOTAL DRAINAGE AT END OF BASIN 510.3 (PA-17) IN MAIN ARROYO										

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4 NOTATION
ADD HYD	510.92	11	7 11	4.12578	3762.44	284.812	1.29435	2.000	1.425	
*S SEPARATE 510.3SA(P17A) INTO FLOW TO 510.3SC (CV-D23-49) AND 510.3SD (DS-D23-1										
DIVIDE HYD	510.3SC	13	14	.04559	84.23	2.488	1.02326	1.500	2.887	
	510.3SD	AND	15	.01862	34.40	1.016	1.02326	1.500	2.887	
*S ADD FLOW DS FROM EAGLE ROCK PLACE										
ADD HYD	510.93	14	15 14	.06422	118.64	3.505	1.02326	1.500	2.887	
*S ROUTE 510.93(P17C&D) THROUGH NATURAL CHANNEL TO CONF. W/MAIN PINO										
ROUTE MCUNGE	510.84	14	7	.06422	111.75	3.448	1.00673	1.667	2.719	CCODE = .1
*S ROUTE SUMS 510.92(PA-1 THRU PA-17) THROUGH BASIN 510.4(PA-19)										
ROUTE MCUNGE	510.85	11	8	4.12578	3741.54	284.503	1.29295	2.100	1.417	CCODE = .1
COMPUTE NM HYD	510.40	-	1	.22100	447.07	15.323	1.30000	1.500	3.161	PER IMP= 16.00
*S ADD ROUTED FLOWS TO 510.4										
ADD HYD	510.94	1	8 19	4.34678	3784.61	299.826	1.29331	2.100	1.360	
*S 510.99=TOTAL MAIN PINO FLOW AT TRAMWAY LANE										
ADD HYD	510.99	19	7 20	4.41100	3800.09	303.274	1.28914	2.100	1.346	
FINISH										

PINO SYS. FUTURE COND. 10-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = pino10.fut

RUN DATE (MON/DAY/YR) = 10/19/1998
USER NO. = RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION

*S	NORTH PINO ARROYO (N. OF SAN ANTONIO DR.) AND MAIN PINO ARROYO										
*S	FUTURE CONDITIONS, 10-YR RETURN PERIOD, 24-HR STORM										

START											
*S	BEGIN ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY										
*S	SUBBASIN 500.0=(FLOW E. OF TRAMWAY N.OF POND N.OF SAN RAFAEL)										
RAINFALL TYPE= 2											
COMPUTE NM HYD	500.00	-	1	.06350	81.38	3.327	.98243	1.500	2.003 PER IMP=	35.00	RAIN24= 2.330
COMPUTE NM HYD	500.10	-	2	.05490	65.77	2.877	.98243	1.500	1.872 PER IMP=	35.00	
COMPUTE NM HYD	500.20	-	3	.05080	65.11	2.662	.98243	1.500	2.003 PER IMP=	35.00	
*S	ROUTE 500.1 THROUGH 500.2 - (NATURAL CHANNEL WEST OF QUAIL RUN LOOP)										
ROUTE MCUNGE	500.82	2	7	.05490	65.38	2.876	.98225	1.500	1.861 CCODE =	.2	
*S	COMBINE ROUTED 500.1 WITH 500.2 AT AP 500.91										
*S	(FLOW E. OF TRAMWAY COLLECTED AT POND NORTH OF SAN RAFAEL)										
ADD HYD	500.91	3& 7	11	.10570	130.49	5.538	.98233	1.500	1.929		
*S	DEVELOP HYDROGRAPH 500.3										
*S	(FLOW E. OF TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)										
COMPUTE NM HYD	500.30	-	2	.05320	60.29	2.787	.98243	1.550	1.771 PER IMP=	35.00	
*S	DEVELOP HYDROGRAPH 500.4										
*S	(FLOW E. OF TRAMWAY NEAR QUAIL RUN COURT)										
COMPUTE NM HYD	500.40	-	3	.02090	26.79	1.095	.98243	1.500	2.003 PER IMP=	35.00	
*S	COMBINE FLOWS TO ESTIMATE CONTRIBUTION STORM DRAIN AND PINO DAM										
*S	(500.94=COMBINED FLOW AT POND N. OF SAN RAFAEL)										
ADD HYD	500.94	11& 1	12	.16920	211.87	8.865	.98236	1.500	1.957		
*S	(500.95=COMBINED FLOW AT TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)										
ADD HYD	500.95	12& 2	13	.22240	271.54	11.652	.98238	1.500	1.908		
*S	(500.96 = ESTIMATED FLOW TO DAM)										
ADD HYD	500.96	13& 3	19	.24330	298.34	12.747	.98238	1.500	1.916		
*S	END OF ANALYSIS OF WATERSHED EAST OF TRAMWAY										

*S	BEGIN ANALYSIS OF WATERSHED WEST OF TRAMWAY										
RAINFALL TYPE= 2											
COMPUTE NM HYD	501.00	-	1	.01392	16.22	.534	.71903	1.500	1.821 PER IMP=	17.00	RAIN24= 2.200
*S	(SUBBASIN 501.0=FLOW AT LOWELL STREET BETWEEN CORONADO AND SAN RAFAEL)										
*S	ROUTE 501.0 THROUGH 501.1										
ROUTE MCUNGE	501.80	1	6	.01392	15.75	.525	.70690	1.700	1.768 CCODE =	.2	
COMPUTE NM HYD	501.10	-	2	.03920	45.66	1.503	.71903	1.500	1.820 PER IMP=	17.00	
*S	COMBINE 501.80 WITH 501.1 AS 501.91										
*S	(501.91=FLOW AT BROWNING BETWEEN CORONADO AND SAN RAFAEL)										
ADD HYD	501.91	6& 2	11	.05312	47.89	2.028	.71583	1.500	1.409		
*S	ROUTE 501.91 THROUGH 501.2										
ROUTE MCUNGE	501.81	11	6	.05312	47.24	2.030	.71669	1.650	1.390 CCODE =	.2	
RAINFALL TYPE= 2											
COMPUTE NM HYD	501.20	-	2	.05010	55.26	1.815	.67932	1.500	1.723 PER IMP=	17.00	RAIN24= 2.130
*S	COMBINE 501.81 WITH 501.2 AS 501.99										
*S	(501.99=FLOW AT EUBANK BETWEEN SAN RAFAEL AND DEL REY)										
ADD HYD	501.99	6& 2	11	.10322	97.15	3.846	.69854	1.550	1.471		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	508.00	-	1	.02590	41.79	1.696	1.22766	1.500	2.521	RAIN24= 2.200 PER IMP= 50.00
*S (508.0=INFLOW FROM SANDIA HEIGHTS SOUTH UNITS 23 AND 24 TO SUBBASIN 502)										
*S ROUTE 508.0 THROUGH 502.0										
ROUTE MCUNGE	508.80	1	6	.02590	41.60	1.692	1.22485	1.550	2.510	CCODE = .2
COMPUTE NM HYD	502.00	-	1	.06295	73.32	2.414	.71903	1.500	1.820	PER IMP= 17.00
*S COMBINE RP 508.0 WITH 502.0 AS 502.91										
*S (502.91=FLOW AT LOWELL BETWEEN SAN RAFAEL AND DEL REY)										
ADD HYD	502.91	1& 6	12	.08885	108.57	4.106	.86647	1.550	1.909	
COMPUTE NM HYD	502.10	-	1	.07500	87.34	2.876	.71903	1.500	1.820	PER IMP= 17.00
*S (502.1=FLOW AT LOWELL IN NORTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)										
*S COMBINE 502.91 WITH 502.1 AS 502.92										
*S (502.92=COMBINED FLOW IN ARROYO JUNCTION W.OF LOWELL BETWEEN										
*S DEL REY AND SAN RAFAEL)										
ADD HYD	502.92	12& 1	13	.16385	188.37	6.982	.79898	1.550	1.796	
*S ROUTE 502.92 THROUGH 502.3										
ROUTE MCUNGE	502.80	13	6	.16385	180.16	6.957	.79614	1.550	1.718	CCODE = .2
COMPUTE NM HYD	502.20	-	1	.03390	34.32	1.300	.71903	1.550	1.582	PER IMP= 17.00
*S (502.2=FLOW AT LOWELL IN SOUTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)										
*S ROUTE 502.2 THROUGH 502.3										
ROUTE MCUNGE	502.81	1	7	.03390	33.21	1.291	.71400	1.600	1.531	CCODE = .1
*S COMBINE 502.81 WITH 502.80 AS 502.93										
ADD HYD	502.93	6& 7	12	.19775	211.56	8.248	.78206	1.600	1.672	
COMPUTE NM HYD	502.30	-	1	.03950	46.01	1.515	.71903	1.500	1.820	PER IMP= 17.00
*S COMBINE 502.3 WITH 502.93 AS 502.99										
*S (502.99=FLOW TO PINO DIVERSION DROP STRUCTURE)										
ADD HYD	502.99	12& 1	13	.23725	251.11	9.763	.77156	1.550	1.654	
*S *****										
COMPUTE NM HYD	503.00	-	1	.05610	64.65	2.151	.71903	1.500	1.801	PER IMP= 17.00
*S (503.0=FLOW AT BROWNING BETWEEN DEL REY AND SANTA MONICA)										
COMPUTE NM HYD	503.10	-	2	.02200	24.37	.844	.71903	1.500	1.731	PER IMP= 17.00
*S (503.1=FLOW AT BROWNING AT BETWEEN SAN RAFAEL AND DEL REY)										
*S COMBINE 503.0 AND 503.1 AS 503.91										
ADD HYD	503.91	1& 2	12	.07810	89.02	2.995	.71902	1.500	1.781	
*S (503.91=FLOW AT ARROYO JUNCTION W. OF BROWNING BETWEEN DEL REY										
*S AND SANTA MONICA)										
*S ROUTE 503.91 THROUGH 503.2										
ROUTE MCUNGE	503.81	12	7	.07810	78.79	2.959	.71035	1.700	1.576	CCODE = .1
RAINFALL TYPE= 2										
COMPUTE NM HYD	503.20	-	1	.04940	54.49	1.790	.67932	1.500	1.723	RAIN24= 2.130 PER IMP= 17.00
*S COMBINE 503.2 AND 503.91 AS 503.99										
*S (503.99=FLOW AT EUBANK BETWEEN DEL REY AND SANTA MONICA)										
ADD HYD	503.99	7& 1	20	.12750	104.94	4.749	.69832	1.650	1.286	
*S *****										
*S MISCELLANEOUS BASINS THAT FLOW TO EAST SIDE OF EUBANK										
COMPUTE NM HYD	504.00	-	1	.03720	41.03	1.348	.67932	1.500	1.724	PER IMP= 17.00
*S (504.0=FLOW AT EUBANK NORTH OF CORONADO BELOW SAN FRANCISCO)										
COMPUTE NM HYD	505.00	-	2	.02870	31.66	1.040	.67932	1.500	1.724	PER IMP= 17.00
*S (505.0=FLOW AT EUBANK FROM SMALL SUBBASIN THAT STRADDLES SAN RAFAEL)										
COMPUTE NM HYD	506.00	-	3	.04840	38.27	1.754	.67932	1.600	1.235	PER IMP= 17.00
*S (506.0=FLOW AT EUBANK NORTH OF SAN ANTONIO SOUTH OF SANTA MONICA)										
COMPUTE NM HYD	509.00	-	5	.02590	40.32	1.626	1.17709	1.500	2.432	PER IMP= 50.00
*S (509.0=SANDIA HEIGHTS SOUTH UNITS 21 & 23 SD OUTFALL TO PINO DAM)										
*S *****										
*S HYDROGRAPHS AT QUINTESSENCE STORM DRAINS										

*S ROUTE 504.0 THROUGH CHANNEL ON EAST SIDE OF EUBANK

*S (511.80=FLOW TO MH#4 AT CORONADO AND EUBANK)

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3	NOTATION
ROUTE MCUNGE	511.80	1	7	.03720	40.32	1.352	.68146	1.550	1.693	CCODE =	.2
*S											
*S (510.91= FLOW TO MH#3 AT SAN RAFAEL)											
ADD HYD	511.91	2 & 7	13	.06590	70.39	2.392	.68051	1.500	1.669		
*S (511.92=FLOW TO MH#2 AT SANTA MONICA)											
ADD HYD	511.92	13&11	21	.16912	166.53	6.237	.69152	1.550	1.539		
*S (511.99=FLOW AT OUTFALL WEST OF EUBANK AT SANTA MONICA)											
ADD HYD	511.99	21&20	22	.29662	247.35	10.986	.69444	1.550	1.303		
*S END OF ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY											
*S*****											
*S MAIN PINO ARROYO											
*S FUTURE CONDITIONS, 10-YR RETURN PERIOD, 24-HR STORM											
*S BEGIN MAIN PINO ARROYO BASINS											
*S*****											
RAINFALL TYPE= 2											
COMPUTE NM HYD	510.00	-	1	3.17800	1561.97	112.465	.66354	1.833	.768	RAIN24= 2.730 PER IMP=	.00
*S ROUTE 510.0 THRU 510.2 (PA-1 THRU BASIN PA-16)											
ROUTE MCUNGE	510.80	1	6	3.17800	1553.85	112.342	.66281	1.900	.764	CCODE =	.1
COMPUTE NM HYD	510.10	-	1	.46400	447.72	16.420	.66354	1.567	1.508	PER IMP=	.00
*S ROUTE 510.1 THRU 510.2 (PA-2 THROUGH BASIN PA-15)											
ROUTE MCUNGE	510.81	1	5	.46400	445.82	16.434	.66408	1.633	1.501	CCODE =	.2
*S ADD 510.81 AND 510.80											
ADD HYD	510.90	6 & 5	11	3.64200	1734.23	128.776	.66297	1.833	.744		
*S ROUTE 510.90 THRU 510.2											
ROUTE MCUNGE	510.82	11	7	3.64200	1729.21	128.746	.66282	1.900	.742	CCODE =	.1
COMPUTE NM HYD	510.20	-	1	.27000	170.53	4.494	.31211	1.500	.987	PER IMP=	.00
*S ADD 510.9 AND 510.2 (ROUTED PA-1 TO PA-16)											
ADD HYD	510.91	1 & 7	12	3.91200	1748.06	133.240	.63861	1.900	.698		
*S ROUTE SUM OF 510.0 TO 510.2 THRU 510.3(PA-1 & PA-16 THROUGH BASIN PA-17)											
ROUTE MCUNGE	510.83	12	7	3.91200	1735.44	132.731	.63617	2.033	.693	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	510.30	-	1	.27800	209.94	6.322	.42637	1.500	1.180	RAIN24= 2.420 PER IMP=	6.00
*S SEPARATE 510.3 (PA-17) INTO NORTH AND SOUTH DRAINAGE											
DIVIDE HYD	510.3N	1	11	.16124	121.77	3.667	.42637	1.500	1.180		
	510.3S	AND	12	.11676	88.18	2.655	.42637	1.500	1.180		
*S SEPARATE 510.3S INTO A (FLOW TO S. EAGLE RIDGE DR.) AND B (FLOW TO CV-D23-47)											
DIVIDE HYD	510.3SA	12	13	.06422	48.50	1.460	.42637	1.500	1.180		
	510.3SB	AND	14	.05254	39.68	1.195	.42637	1.500	1.180		
*S ADD FLOW TO CV-D23-47 BACK TO P17N (DS FROM N. EAGLE RIDGE DR.)											
ADD HYD	510.3N	14&11	11	.21378	161.44	4.861	.42636	1.500	1.180		
*S TOTAL DRAINAGE AT END OF BASIN 510.3 (PA-17) IN MAIN ARROYO											
ADD HYD	510.92	11 & 7	11	4.12578	1752.12	137.606	.62536	2.033	.664		
*S SEPARATE 510.3SA(P17A) INTO FLOW TO 510.3SC (CV-D23-49) AND 510.3SD (DS-D23-1											
DIVIDE HYD	510.3SC	13	14	.04559	34.43	1.037	.42636	1.500	1.180		
	510.3SD	AND	15	.01862	14.06	.423	.42636	1.500	1.180		
*S ADD FLOW DS FROM EAGLE ROCK PLACE											
ADD HYD	510.93	14&15	14	.06422	48.50	1.460	.42637	1.500	1.180		
*S ROUTE 510.93(P17C&D) THROUGH NATURAL CHANNEL TO CONF. W/MAIN PINO											
ROUTE MCUNGE	510.84	14	7	.06422	41.91	1.433	.41834	1.767	1.020	CCODE =	.1
*S ROUTE SUMS 510.92(PA-1 THRU PA-17) THROUGH BASIN 510.4(PA-19)											
ROUTE MCUNGE	510.85	11	8	4.12578	1742.34	137.278	.62387	2.133	.660	CCODE =	.1
COMPUTE NM HYD	510.40	-	1	.22100	283.91	11.632	.98685	1.500	2.007	PER IMP=	35.00
*S ADD ROUTED FLOWS TO 510.4											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4 NOTATION
ADD HYD	510.94	1& 8	19	4.34678	1774.33	148.910	.64233	2.133	.638	
*S 510.99=TOTAL MAIN PINO FLOW AT TRAMWAY LANE										
ADD HYD	510.99	19& 7	20	4.41100	1782.68	150.342	.63907	2.133	.631	
FINISH										

PINO SYS. FUTURE COND. 100-YR

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = pino100.fut

RUN DATE (MON/DAY/YR) =10/19/1998
USER NO.= RSTECHNM.STE

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1	NOTATION

*S	NORTH PINO ARROYO (N. OF SAN ANTONIO DR. AND MAIN PINO ARROYO										
*S	FUTURE CONDITIONS, 100-YR RETURN PERIOD, 24-HR STORM										

START											TIME= .00
*S BEGIN ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY (N. OF MAIN PINO)											
*S SUBBASIN 500.0=(FLOW E. OF TRAMWAY N.OF POND N.OF SAN RAFAEL)											
RAINFALL TYPE= 2											
COMPUTE NM HYD	500.00	-	1	.06350	145.38	6.026	1.77931	1.500	3.577	RAIN24= 3.500	PER IMP= 35.00
COMPUTE NM HYD	500.10	-	2	.05490	118.02	5.210	1.77931	1.500	3.359	PER IMP= 35.00	
COMPUTE NM HYD	500.20	-	3	.05080	116.28	4.821	1.77931	1.500	3.577	PER IMP= 35.00	
*S ROUTE 500.1 THROUGH 500.2 - (NATURAL CHANNEL WEST OF QUAIL RUN LOOP)											
ROUTE MCUNGE	500.82	2	7	.05490	117.38	5.209	1.77898	1.500	3.341	CCODE = .2	
*S COMBINE ROUTED 500.1 WITH 500.2 AT AP 500.91											
*S (FLOW E. OF TRAMWAY COLLECTED AT POND NORTH OF SAN RAFAEL)											
ADD HYD	500.91	3& 7	11	.10570	233.66	10.030	1.77913	1.500	3.454		
*S DEVELOP HYDROGRAPH 500.3											
*S (FLOW E. OF TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)											
COMPUTE NM HYD	500.30	-	2	.05320	108.06	5.048	1.77931	1.550	3.174	PER IMP= 35.00	
*S DEVELOP HYDROGRAPH 500.4											
*S (FLOW E. OF TRAMWAY NEAR QUAIL RUN COURT)											
COMPUTE NM HYD	500.40	-	3	.02090	47.85	1.983	1.77931	1.500	3.577	PER IMP= 35.00	
*S COMBINE FLOWS TO ESTIMATE CONTRIBUTION STORM DRAIN AND PINO DAM											
*S (500.94=COMBINED FLOW AT POND N. OF SAN RAFAEL)											
ADD HYD	500.94	11& 1	12	.16920	379.04	16.055	1.77920	1.500	3.500		
*S (500.95=COMBINED FLOW AT TRAMWAY BETWEEN SAN RAFAEL AND QUAIL RUN COURT)											
ADD HYD	500.95	12& 2	13	.22240	486.50	21.104	1.77922	1.500	3.418		
*S (500.96 = ESTIMATED FLOW TO DAM)											
ADD HYD	500.96	13& 3	19	.24330	534.36	23.087	1.77923	1.500	3.432		
*S END OF ANALYSIS OF WATERSHED EAST OF TRAMWAY											

*S BEGIN ANALYSIS OF WATERSHED WEST OF TRAMWAY											
RAINFALL TYPE= 2											
COMPUTE NM HYD	501.00	-	1	.01392	29.99	1.067	1.43773	1.500	3.367	RAIN24= 3.300	PER IMP= 17.00
*S (SUBBASIN 501.0=FLOW AT LOWELL STREET BETWEEN CORONADO AND SAN RAFAEL)											
*S ROUTE 501.0 THROUGH 501.1											
ROUTE MCUNGE	501.80	1	6	.01392	27.74	1.045	1.40707	1.650	3.113	CCODE = .1	
COMPUTE NM HYD	501.10	-	2	.03920	84.43	3.006	1.43773	1.500	3.365	PER IMP= 17.00	
*S COMBINE 501.80 WITH 501.1 AS 501.91											
*S (501.91=FLOW AT BROWNING BETWEEN CORONADO AND SAN RAFAEL)											
ADD HYD	501.91	6& 2	11	.05312	88.83	4.050	1.42968	1.500	2.613		
*S ROUTE 501.91 THROUGH 501.2											
ROUTE MCUNGE	501.81	11	6	.05312	86.72	4.012	1.41607	1.700	2.551	CCODE = .1	
RAINFALL TYPE= 2											
COMPUTE NM HYD	501.20	-	2	.05010	104.03	3.661	1.37021	1.500	3.245	RAIN24= 3.200	PER IMP= 17.00
*S COMBINE 501.81 WITH 501.2 AS 501.99											
*S (501.99=FLOW AT EUBANK BETWEEN SAN RAFAEL AND DEL REY)											
ADD HYD	501.99	6& 2	11	.10322	151.70	7.673	1.39380	1.600	2.296		
*S *****											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
RAINFALL TYPE= 2										
COMPUTE NM HYD	508.00	-	1	.02590	68.75	2.903	2.10174	1.500	4.147	RAIN24= 3.300 PER IMP= 50.00
*S (508.0=INFLOW FROM SANDIA HEIGHTS SOUTH UNITS 23 AND 24 TO SUBBASIN 502)										
*S ROUTE 508.0 THROUGH 502.0										
ROUTE MCUNGE	508.80	1	6	.02590	62.86	2.875	2.08108	1.650	3.792	CCODE = .1
COMPUTE NM HYD	502.00	-	1	.06295	135.59	4.827	1.43773	1.500	3.366	PER IMP= 17.00
*S COMBINE RP 508.0 WITH 502.0 AS 502.91										
*S (502.91=FLOW AT LOWELL BETWEEN SAN RAFAEL AND DEL REY)										
ADD HYD	502.91	1& 6	12	.08885	165.36	7.702	1.62526	1.550	2.908	
COMPUTE NM HYD	502.10	-	1	.07500	162.06	5.751	1.43773	1.500	3.376	PER IMP= 17.00
*S (502.1=FLOW AT LOWELL IN NORTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)										
*S COMBINE 502.91 WITH 502.1 AS 502.92										
*S (502.92=COMBINED FLOW IN ARROYO JUNCTION W.OF LOWELL BETWEEN										
*S DEL REY AND SAN RAFAEL)										
ADD HYD	502.92	12& 1	13	.16385	322.27	13.452	1.53942	1.500	3.073	
*S ROUTE 502.92 THROUGH 502.3										
ROUTE MCUNGE	502.80	13	6	.16385	319.44	13.432	1.53710	1.550	3.046	CCODE = .1
COMPUTE NM HYD	502.20	-	1	.03390	63.93	2.599	1.43773	1.550	2.947	PER IMP= 17.00
*S (502.2=FLOW AT LOWELL IN SOUTH 1/2 OF BLOCK BETWEEN DEL REY AND SANTA MONICA)										
*S ROUTE 502.2 THROUGH 502.3										
ROUTE MCUNGE	502.81	1	7	.03390	62.31	2.596	1.43576	1.600	2.872	CCODE = .1
*S COMBINE 502.81 WITH 502.80 AS 502.93										
ADD HYD	502.93	6& 7	12	.19775	376.26	16.028	1.51973	1.550	2.973	
COMPUTE NM HYD	502.30	-	1	.03950	85.08	3.029	1.43773	1.500	3.365	PER IMP= 17.00
*S COMBINE 502.3 WITH 502.93 AS 502.99										
*S (502.99=FLOW TO PINO DIVERSION DROP STRUCTURE)										
ADD HYD	502.99	12& 1	13	.23725	453.90	19.057	1.50607	1.550	2.989	
*S *****										
COMPUTE NM HYD	503.00	-	1	.05610	120.82	4.302	1.43773	1.500	3.365	PER IMP= 17.00
*S (503.0=FLOW AT BROWNING BETWEEN DEL REY AND SANTA MONICA)										
COMPUTE NM HYD	503.10	-	2	.02200	45.41	1.687	1.43773	1.500	3.225	PER IMP= 17.00
*S (503.1=FLOW AT BROWNING AT BETWEEN SAN RAFAEL AND DEL REY)										
*S COMBINE 503.0 AND 503.1 AS 503.91										
ADD HYD	503.91	1& 2	12	.07810	166.23	5.989	1.43772	1.500	3.326	
*S (503.91=FLOW AT ARROYO JUNCTION W. OF BROWNING BETWEEN DEL REY										
*S AND SANTA MONICA)										
*S ROUTE 503.91 THROUGH 503.2										
ROUTE MCUNGE	503.81	12	7	.07810	163.66	5.970	1.43321	1.600	3.274	CCODE = .2
RAINFALL TYPE= 2										
COMPUTE NM HYD	503.20	-	1	.04940	102.58	3.610	1.37021	1.500	3.245	RAIN24= 3.200 PER IMP= 17.00
*S COMBINE 503.2 AND 503.91 AS 503.99										
*S (503.99=FLOW AT EUBANK BETWEEN DEL REY AND SANTA MONICA)										
ADD HYD	503.99	7& 1	20	.12750	254.92	9.580	1.40879	1.550	3.124	
*S *****										
*S MISCELLANEOUS BASINS THAT FLOW TO EAST SIDE OF EUBANK										
COMPUTE NM HYD	504.00	-	1	.03720	77.25	2.718	1.37021	1.500	3.245	PER IMP= 17.00
*S (504.0=FLOW AT EUBANK NORTH OF CORONADO BELOW SAN FRANCISCO)										
COMPUTE NM HYD	505.00	-	2	.02870	59.60	2.097	1.37021	1.500	3.245	PER IMP= 17.00
*S (505.0=FLOW AT EUBANK FROM SMALL SUBBASIN THAT STRADDLES SAN RAFAEL)										
COMPUTE NM HYD	506.00	-	3	.04840	73.61	3.537	1.37021	1.600	2.376	PER IMP= 17.00
*S (506.0=FLOW AT EUBANK NORTH OF SAN ANTONIO SOUTH OF SANTA MONICA)										
COMPUTE NM HYD	509.00	-	5	.02590	66.74	2.793	2.02208	1.500	4.026	PER IMP= 50.00
*S (509.0=SANDIA HEIGHTS SOUTH UNITS 21 & 23 SD OUTFALL TO PINO DAM)										
*S *****										
*S HYDROGRAPHS AT QUINTESSENCE STORM DRAINS										

*S ROUTE 504.0 THROUGH CHANNEL ON EAST SIDE OF EUBANK

*S (511.80=FLOW TO MH#4 AT CORONADO AND EUBANK)

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3	NOTATION
ROUTE MCUNGE	511.80	1	7	.03720	74.98	2.722	1.37175	1.500	3.149	CCODE =	.2
*S											
*S (511.91= FLOW TO MH#3 AT SAN RAFAEL)											
ADD HYD	511.91	2 & 7	13	.06590	134.58	4.819	1.37107	1.500	3.191		
*S (511.92=FLOW TO MH#2 AT SANTA MONICA)											
ADD HYD	511.92	13&11	21	.16912	275.04	12.492	1.38494	1.550	2.541		
*S (511.99=FLOW AT OUTFALL WEST OF EUBANK AT SANTA MONICA)											
ADD HYD	511.99	21&20	22	.29662	529.96	22.072	1.39519	1.550	2.792		
*S END OF ANALYSIS OF N. PINO WATERSHED EAST OF TRAMWAY											
*S*****											
*S MAIN PINO ARROYO											
*S FUTURE CONDITIONS, 100-YR RETURN PERIOD, 24-HR STORM											
*S*****											
*S BEGIN MAIN PINO ARROYO BASINS											
RAINFALL TYPE= 2											
COMPUTE NM HYD	510.00	-	1	3.17800	3292.83	228.752	1.34962	1.833	1.619	RAIN24= 4.100 PER IMP=	.00
*S ROUTE 510.0 THRU 510.2 (PA-1 THRU BASIN PA-16)											
ROUTE MCUNGE	510.80	1	6	3.17800	3278.95	228.382	1.34744	1.900	1.612	CCODE =	.1
COMPUTE NM HYD	510.10	-	1	.46400	869.97	33.399	1.34962	1.567	2.930	PER IMP=	.00
*S ROUTE 510.1 THRU 510.2 (PA-2 THROUGH BASIN PA-15)											
ROUTE MCUNGE	510.81	1	5	.46400	852.72	33.322	1.34652	1.633	2.871	CCODE =	.1
*S ADD 510.81 AND 510.80											
ADD HYD	510.90	6&5	11	3.64200	3701.81	261.704	1.34732	1.866	1.588		
*S ROUTE 510.90 THRU 510.2											
ROUTE MCUNGE	510.82	11	7	3.64200	3696.06	261.540	1.34648	1.900	1.586	CCODE =	.1
COMPUTE NM HYD	510.20	-	1	.27000	448.49	12.184	.84608	1.500	2.595	PER IMP=	.00
*S ADD 510.9 AND 510.2 (ROUTED PA-1 TO PA-16)											
ADD HYD	510.91	1&7	12	3.91200	3745.17	273.724	1.31194	1.900	1.496		
*S ROUTE SUM OF 510.0 TO 510.2 THRU 510.3(PA-1 & PA-16 THROUGH BASIN PA-17)											
ROUTE MCUNGE	510.83	12	7	3.91200	3723.21	273.118	1.30904	2.000	1.487	CCODE =	.1
RAINFALL TYPE= 2											
COMPUTE NM HYD	510.30	-	1	.27800	513.58	15.171	1.02326	1.500	2.887	RAIN24= 3.650 PER IMP=	6.00
*S SEPARATE 510.3 (PA-17) INTO NORTH AND SOUTH DRAINAGE											
DIVIDE HYD	510.3N	1	11	.16124	297.88	8.799	1.02326	1.500	2.887		
	510.3S	AND	12	.11676	215.70	6.372	1.02326	1.500	2.887		
*S SEPARATE 510.3S INTO A (FLOW TO S. EAGLE RIDGE DR.) AND B (FLOW TO CV-D23-47)											
DIVIDE HYD	510.3SA	12	13	.06422	118.64	3.505	1.02326	1.500	2.887		
	510.3SB	AND	14	.05254	97.07	2.867	1.02326	1.500	2.887		
*S ADD FLOW TO CV-D23-47 BACK TO P17N (DS FROM N. EAGLE RIDGE DR.)											
ADD HYD	510.3N	14&11	11	.21378	394.94	11.667	1.02326	1.500	2.887		
*S TOTAL DRAINAGE AT END OF BASIN 510.3 (PA-17) IN MAIN ARROYO											
ADD HYD	510.92	11&7	11	4.12578	3762.44	284.812	1.29435	2.000	1.425		
*S SEPARATE 510.3SA(P17A) INTO FLOW TO 510.3SC (CV-D23-49) AND 510.3SD (DS-D23-1											
DIVIDE HYD	510.3SC	13	14	.04559	84.23	2.488	1.02326	1.500	2.887		
	510.3SD	AND	15	.01862	34.40	1.016	1.02326	1.500	2.887		
*S ADD FLOW DS FROM EAGLE ROCK PLACE											
ADD HYD	510.93	14&15	14	.06422	118.64	3.505	1.02326	1.500	2.887		
*S ROUTE 510.93(P17C&D) THROUGH NATURAL CHANNEL TO CONF. W/MAIN PINO											
ROUTE MCUNGE	510.84	14	7	.06422	111.75	3.448	1.00673	1.667	2.719	CCODE =	.1
*S ROUTE SUMS 510.92(PA-1 THRU PA-17) THROUGH BASIN 510.4(PA-19)											
ROUTE MCUNGE	510.85	11	8	4.12578	3741.54	284.503	1.29295	2.100	1.417	CCODE =	.1
COMPUTE NM HYD	510.40	-	1	.22100	530.73	21.147	1.79410	1.500	3.752	PER IMP=	35.00
*S ADD ROUTED FLOWS TO 510.4											

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 4 NOTATION
ADD HYD	510.94	18	19	4.34678	3806.63	305.650	1.31843	2.067	1.368	
*S 510.99=TOTAL MAIN PINO FLOW AT TRAMWAY LANE										
ADD HYD	510.99	19	20	4.41100	3824.12	309.098	1.31389	2.067	1.355	
FINISH										