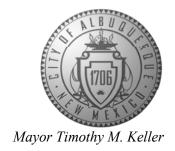
CITY OF ALBUQUERQUE

Planning Department Alan Varela, Director



August 29, 2023

Diane Hoelzer, P.E. Mark Goodwin & Associates PO Box 90606 Albuquerque, NM 87199

RE: Double Eagle Hangars – Phase 1
7401 Atrisco Vista Blvd NE
Grading & Drainage Plan
Engineer's Stamp Date: 08/01/23
Hvdrology File: E05D003

Dear Ms. Hoelzer:

Based upon the information provided in your submittal received 08/02/2023, the Grading & Drainage Plan is approved for Building Permit and Grading Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter.

PO Box 1293

PRIOR TO CERTIFICATE OF OCCUPANCY:

Albuquerque

1. Engineer's Certification, per the DPM Part 6-14 (F): *Engineer's Certification Checklist For Non-Subdivision* is required.

NM 87103

2. Please provide the executed paper Drainage Covenant (latest revision) printed on one-side only with Exhibit A and a check for \$25.00 made out to "Bernalillo County" for the detention ponds per Article 6-15(C) of the DPM to Hydrology for review at Plaza de Sol.

www.cabq.gov

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3995 or <u>rbrissette@cabq.gov</u>.

Sincerely,

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology

Renée C. Brissette

Planning Department



City of Albuquerque

Planning Department Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 11/2018)

Project Title: Double Eagle Hangars	Building Permit #:_	Hydrology File #:	
DRB#:	EPC#:	Work Order#:	
Legal Description: _ract A-1 Plat of Tract A	A-1 & Tract L-1 Parce	els 1-5 Double Eagle Airport	
City Address:7401 Atrisco Vista Blvd. N			
Applicant: Mark Goodwin & Associates, l	PA	Contact: Diane Hoelzer	
Address: PO BOX 90606, Albuquerque,			
Phone#: 505.828.2200	Fax#:	E-mail:diane@goodwinenginee	rs.com
Owner: High Flying Hangars, LLC		Contact:	
Address: PO BOX 25782, Albuquerque, N	N 6 0 7 1 0 7		
Phone#: 505-615-8613		E-mail: kenhinkes@gmai	l.com
TYPE OF SUBMITTAL: PLAT (# OF IS THIS A RESUBMITTAL?: YELDOOR TRAFFIC/ TRANSPORTS TRAFFIC/ TRANSPORT TRAFFIC/	Yes X No		
Check all that Apply: TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERTIFICATION PAD CERTIFICATION CONCEPTUAL G & D PLAN GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT FLOODPLAIN DEVELOPMENT PERMIT ELEVATION CERTIFICATE CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT (TCI TRAFFIC IMPACT STUDY (TIS) OTHER (SPECIFY) PRE-DESIGN MEETING?	APPLIC 2 APPLIC 3 APPLIC 3 APPLIC 4 APPLIC 2 APPLIC 4 APPLIC 5 APPLIC 5 APPLIC 5 APPLIC 5 APPLIC 6 APPLIC	PE OF APPROVAL/ACCEPTANCE SOUGHT BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY PRELIMINARY PLAT APPROVAL SITE PLAN FOR SUB'D APPROVAL SITE PLAN FOR BLDG. PERMIT APPROVA FINAL PLAT APPROVAL SIA/ RELEASE OF FINANCIAL GUARANTE ACCEPTANCE OF APPROVAL GRADING PERMIT APPROVAL SO-19 APPROVAL PAVING PERMIT APPROVAL GRADING/ PAD CERTIFICATION WORK ORDER APPROVAL CLOMR/LOMR FLOODPLAIN DEVELOPMENT PERMIT OTHER (SPECIFY)	ıL
DATE SUBMITTED: 08/01/23	By: Diane Hoelze	er, PE	_
COA STAFE.	ELECTRONIC SURMIT		

FEE PAID:_

Double Eagle Airport Hangars Drainage Management Plan



Prepared by Mark Goodwin & Associates, P.A.

July 2023

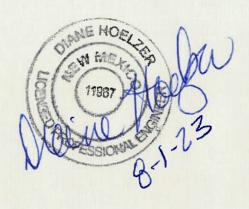


Table of Contents

			0500	DIET	100
1.	PRO.	JECT	DESC	RIPT	IUN

- II DESIGN CRITERIA AND PREVIOUS REPORTS
- III. EXISTING DRAINAGE CONDITIONS
- IV. DEVELOPED DRAINAGE CONDITIONS

EXHIBIT 1 Vicinity Map EXHIBIT 2 Project Site

FIGURE 3 Double Eagle Airport II Grading and Drainage Analysis

EXHIBIT 4 Drainage Plan According to C-130 Plan

EXHIBIT 5 Revised Drainage Plan

APPENDIX A HYDROLOGY

Table 1 Summary of Hydrologic Parameters and Results

AHYMO Input and summary files

Precipitation Data

Leased Survey Information

POCKET 1 GRADING AND DRAINAGE PLAN

DOUBLE EAGLE AIRPORT HANGAR FACILITY GRADING AND DRAINAGE

ANALYSIS- (Record drawing by Molzen Corbin 11/2019)

I. PROJECT DESCRIPTION

The project site is located within the Double Eagle Airport II on an undeveloped parcel of leased land covering an area of approximately 5.80 acres. (Refer to Exhibit 1 Vicinity Map and Exhibit 2 Project Site). The Double Eagle Airport is located on the west mesa, south of Paseo del Norte Blvd. and just west and adjacent to Atrisco Vista Blvd.

Phase 1 entails the construction of the southern hangar building consisting of an approximate area of 31,330 square feet. The adjacent pavement on the north and south side of the building will also be constructed at this time along with the two adjacent depression ponds located on either side of the building and a 4' wide valley gutter.

Future phases will include the construction of three additional buildings and adjacent pavement and depression ponds. This drainage management plan includes all phases of development. The timing of the future phases is not yet known.

II. DESIGN CRITERIA AND PREVIOUS DEVELOPMENT

The drainage design criteria is in accordance with Chapter 6 of the C.O.A. Development Process Manual, latest version. The 100-year 6-hour storm event was analyzed to determine peak runoff in the valley gutters located between the hangars and the volume of runoff routed through the six depression ponds. The onsite Land Treatment values used were based on Table 6.2.9, in the DPM.

This drainage management plan is based on and follows closely to the C.O.A. Aviation Department's Double Eagle II Airport Hangar Facility-Grading and Drainage Analysis (RECORD DRAWING dated 11/2019), sheet C-103 prepared by Molzer Corbin. A copy of this drawing is included in this report.

III. EXISTING DRAINAGE CONDITIONS

Under existing conditions, the project site is surrounded on all four sides by an existing paved road (refer to Exhibit 2). The project site is relatively flat. In general, the road drops off from west to east and from north to south. Runoff from the existing building and pavement on the west side flows towards the project site and is designed to be intercepted by three inlets and a storm drain as shown on the Molzen Corbin C-103 Plan (refer to Exhibit 3). According to this plan, most of the onsite runoff is supposed to drain towards the south and east and eventually be intercepted by a drainage channel that connects to a detention pond (refer to Exhibit 4). Runoff from the existing pavement on the north side flows towards the project site and then eastward. Runoff from the existing pavement on the east and south side flows away from the project site and down to the existing drainage channel. All the runoff in this area is ultimately intercepted by a drainage channel and conveyed to an existing detention pond.

IV. DEVELOPED DRAINAGE CONDITIONS

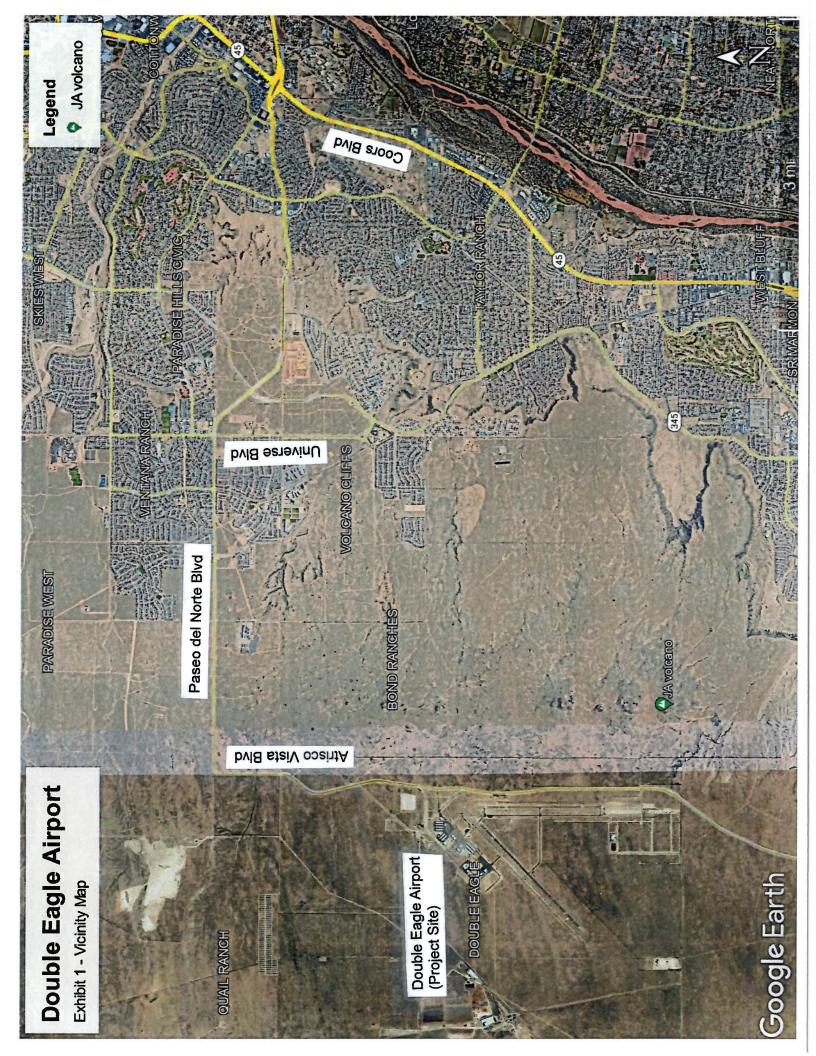
Given the relative flatness of the site, the existing elevations on the adjacent paved roads and the

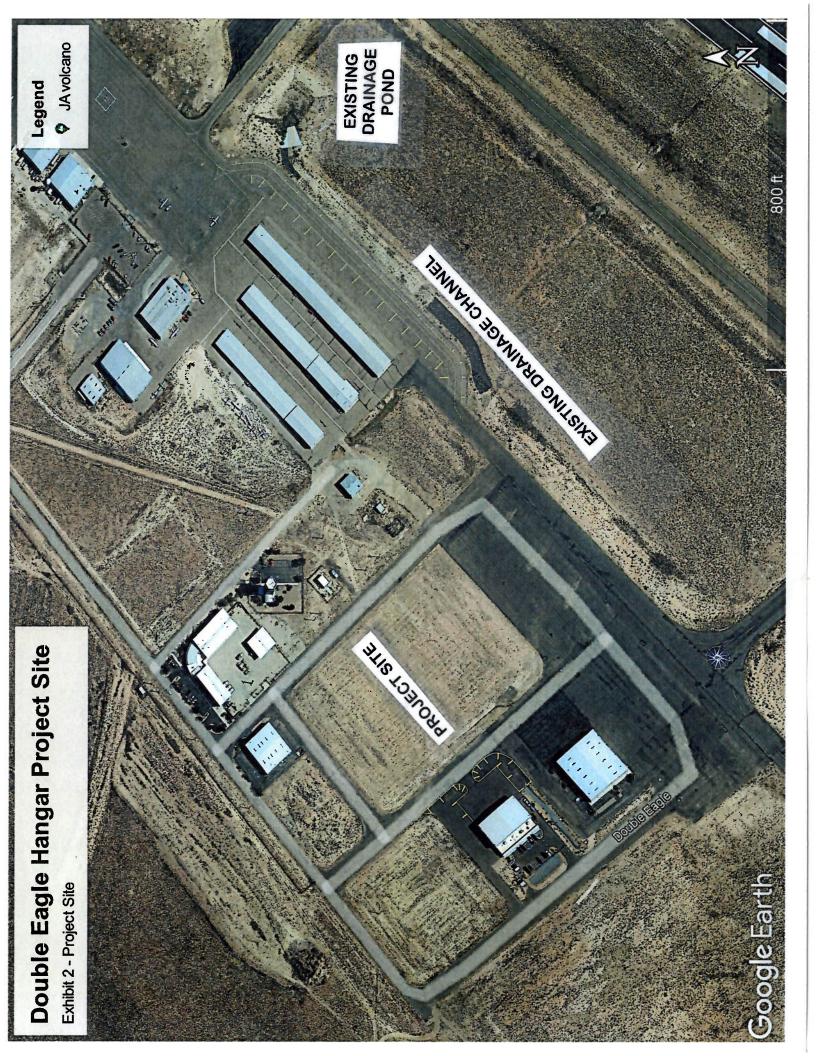
orientation of the proposed hangar buildings, it is not possible to drain the project site in the manner as designed on record drawing C-103.

To adequately drain the site while maintaining the maximum slopes allowed in the paved areas where planes are moved in and out of the hangars, a high point had to be created partly into the paved areas that separate each of the buildings as shown on Exhibit 5. Runoff from sub basins 1-SW, 2-SW and 3-SW which includes half of the building's roof and a portion of the pavement will be directed to one of 3 depression ponds through a 4' wide valley gutter as shown. These ponds have capacity to retain runoff for the 100-yr 6-hr storm event. If any of these depression ponds should spill over, the overflow shall continue in a southeast direction towards the existing drainage channel as intended from the previously approved drainage plan.

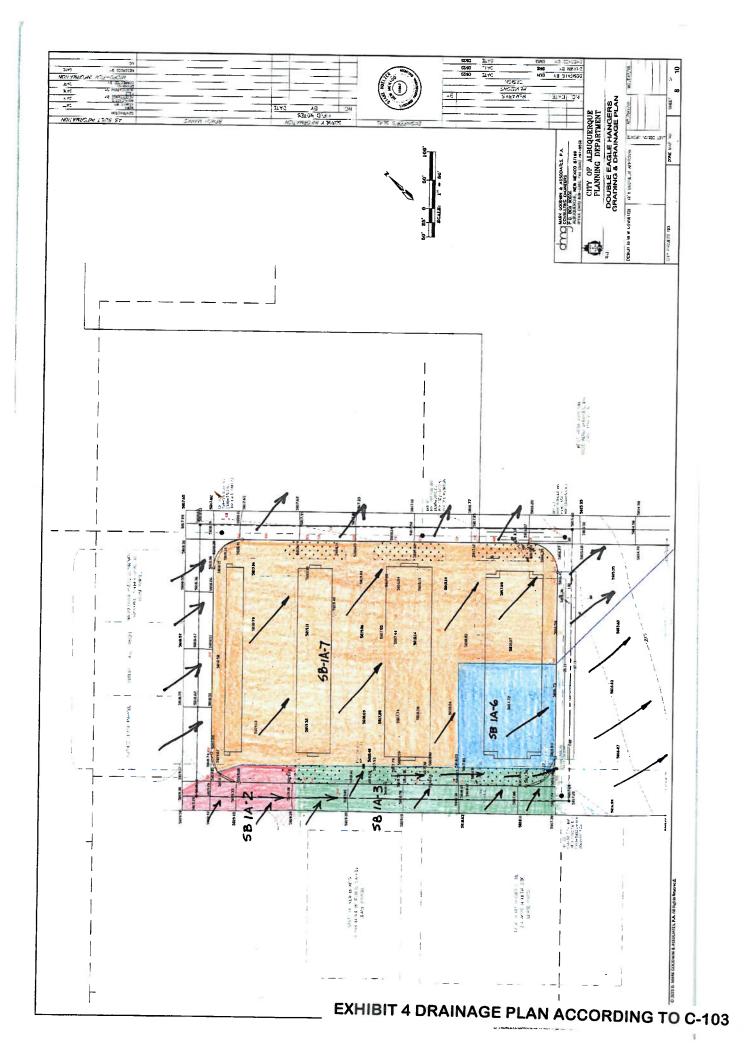
Runoff from sub basins 1-NE, 2-NE and 3-NE will drain to the depression ponds on the northeast side of the buildings through a 4' wide valley gutter. These depression ponds will retain 50-75% of the 100-yr 6-hr storm event. Should any of these depression ponds overtop, the runoff should flow across the road and into a drainage channel as intended and shown on the approved, record drawing C-103.

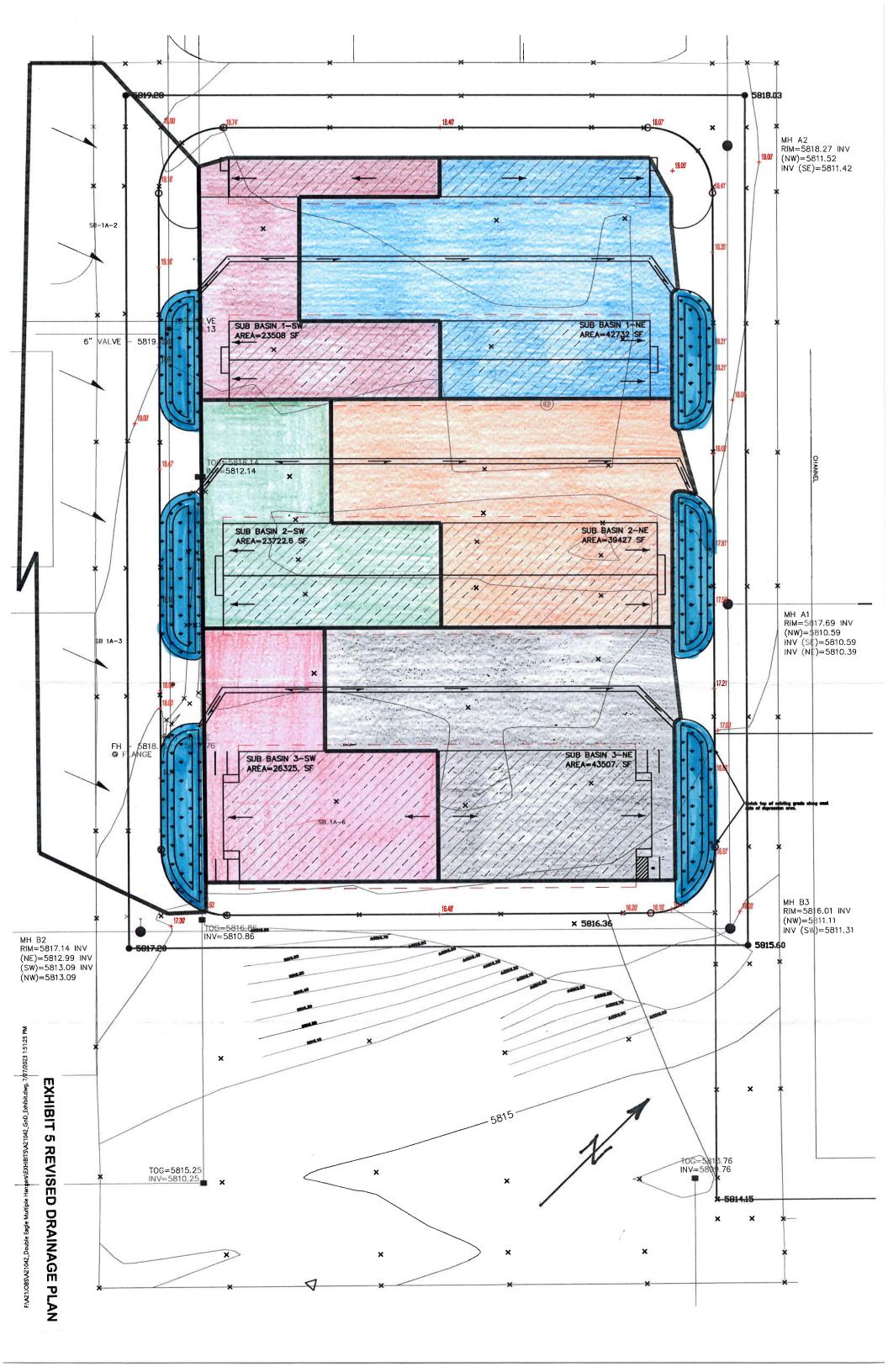
All of the depression ponds shall have a side slope of 5:1 and be 2' deep as indicated on the grading plan. Each of the depression pond shall be lined with 6" cobblestone or porous pavers. All of the depression ponds will have a slight low area where overflow spillage will occur as indicated on the grading plan by arrows.

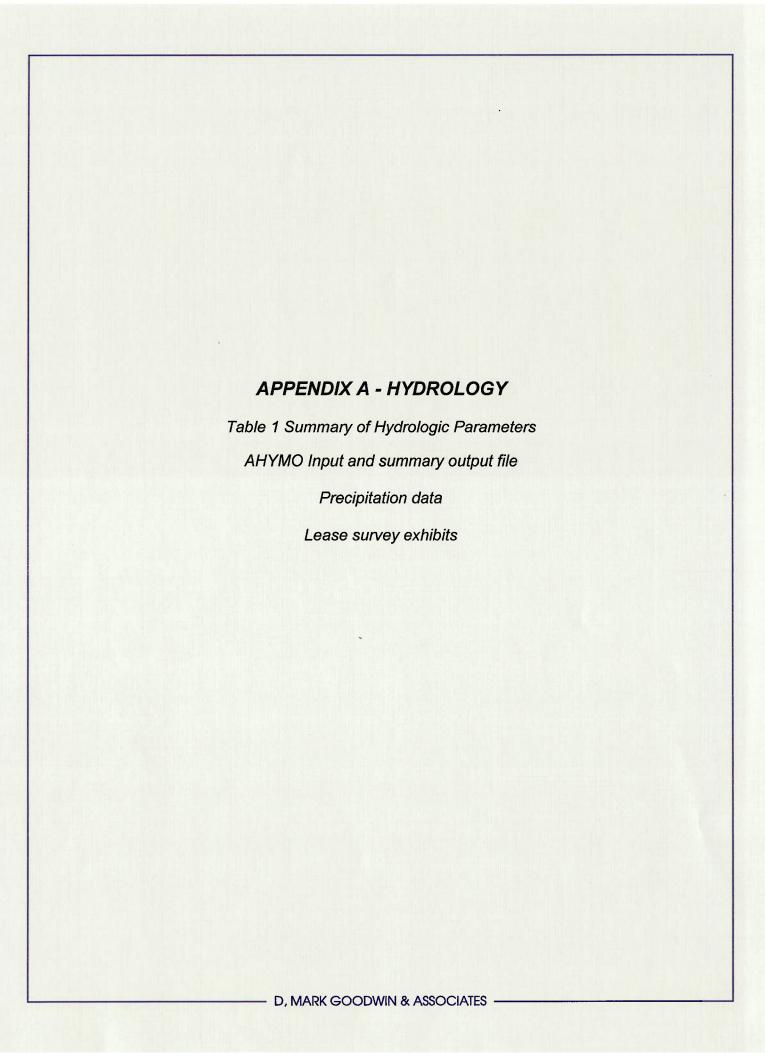












		3		L						_							
		Volume	(CuFt)	3,859	3,903	4,317	7,026	6,473	7,166		32,744						
		Volume	(Acre-Ft)	0.089	060.0	0.099	0.161	0.149	0.165		0.752						
yr-6 hr)		Discharge	CFS	2.45	2.48	2.75	4.46	4.11	4.55		20.80						
MO 100		s	۵	100.0	100.0	100.0	100.0	100.0	100.0								
TABLE 1 Summary of Hydrologic Parameters and Results (AHYMO 100 yr-6 hr) DOUBLE EAGLE AIRPORT HANGARS Albuquerque	nent Value	ပ	0.0	0.0	0.0	0.0	0.0	0:0									
	Land Treatment Values	m	0.0	0.0	0.0	0.0	0.0	0.0									
	7	4	0.0	0.0	0.0	0.0	0.0	0.0									
	DOUBLE EAGLE	DOUBLE EAG	DOUBLE EAG	DOUBLE EAGL Alb	DOUBLE EAGI	DOUBLE EA	DOUBLE E	Area	(sq.mi.)	0.00084	0.00085	0.00094	0.00153	0.00141	0.00156		
		Area	(acre)	0.54	0.54	09.0	0.98	0.91	1.00		4.57						
	Area	(Sq.Ft.)	23,508.00	23,722.60	26,325.00	42,732.00	39,427.00	43,507.00		199,221.60							
		DESCRIPTION	hanger SW	hanger SW	hanger SW	hanger NE	hanger NE	hanger NE									
		SUB BASIN	ID	1-SW	2-SW	3-SW	1-NE	2-NE	3-NE								

WQ Treatmnt Vol (CF)

822.8 830.3 921.4 1,495.6 1,379.9 1,522.7 6,972.8

7/26/2023

DLH

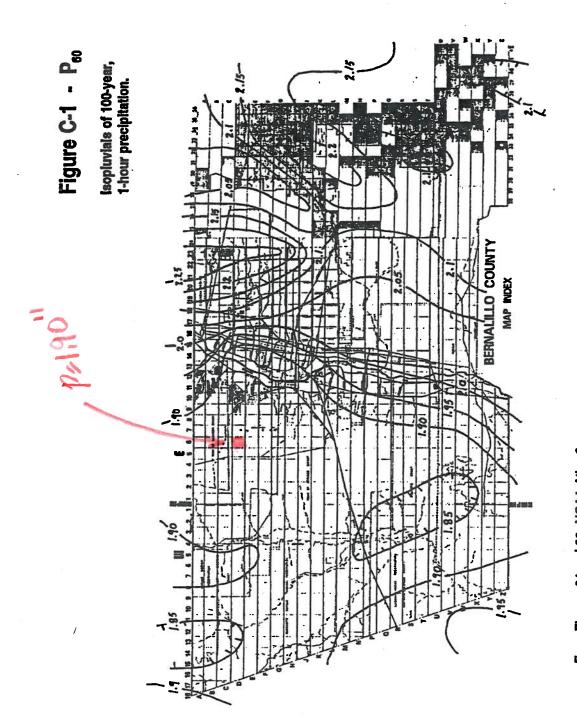
WQ Treatment Volume = $(0.42"/12) \times (Area) \times (\% Tr.D)$

f./projects/21042/Table 1 Hydro Sum (dmg 21042)

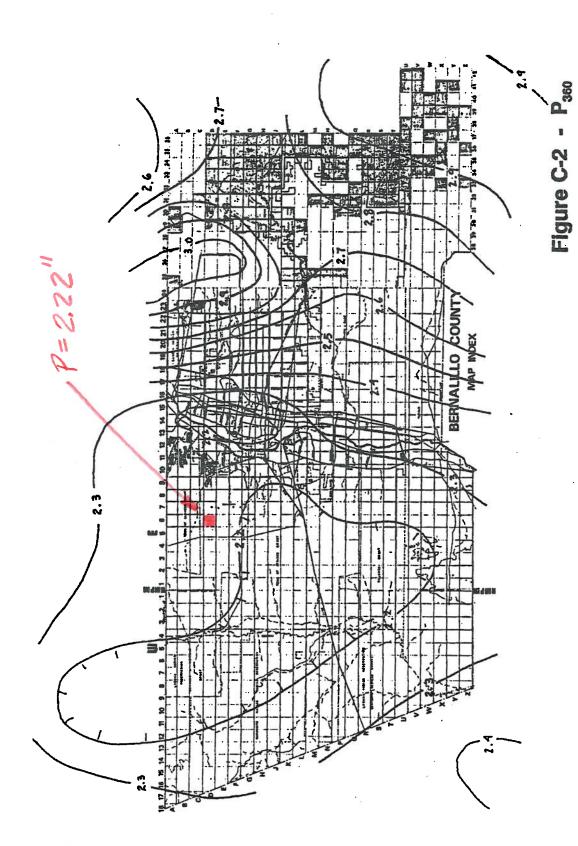
	Depresssion Ponds	Ponds	
Depression	AREA 1	AREA 2	VOLUME
POND	ВОТТОМ	TOP	CU.FT.
1-SW	950.0	3166.2	3900.3
2-SW	1191.0	3806.3	4750.9
3-SW	1474.0	4346.2	5267.5
1-NE	951.24	3166.15	3901.9
2-NE	1191.01	3806.27	4751.0
3-NE	1140.15	3954.76	4812.2

(MON/DAY/YR) =07/27/2023 M-GoodwinNMSiteA90075759	CFS PAGE = 1 PER ACRE NOTATION	TIME= 0.00 RAING= 2.220	4.564 PER IMP= 100.00	4.564 PER IMP= 100.00	4.564 PER IMP= 100.00	4.556 PER IMP= 100.00	4.558 PER IMP= 100.00	4.556 PER IMP= 100.00
RUN DATE (M USER NO.= M-	TIME TO PEAK (HOURS)		1.500	1.500	1.500	1.500	1.500	1.500
D	RUNOFF (INCHES)		1.97661	1,97661	1.97661	1.97661	1.97661	1,97661
- Ver. S4.01a, Rel: 01a	RUNOFF VOLUME (AC-FT)		0.089	0.090	660.0	0.161	0.149	0.164
- Ver.	PEAK DISCHARGE (CFS)	(07-26-23)	2.45	2.48	2.75	4.46	4.11	4.55
.DAT	AREA I (SQ MI)	er Development NT FILE: DBLEAGLE.DAT (07-26-23) ************************************	**********	**********	**************************************	0.00153	0.00141 ***************	***************************************
(AHYMO-S4) Files\DBLEAGLE	OM TO ID O. NO.	13der Devel VENT FILE: ************************************	* * * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * *	* * * * * * * * * * * * * * * * * * *
PROGRAM SUMMARY TABLE (AHYMO-S4) FILE = F:\DIANE\AHYMO Files\DBLEAGLE.DAT	FROM HYDROGRAPH ID IDENTIFICATION NO.	Double Eagle Airport Hanger 100 YEAR 6-HOUR STORM EVENT TYPE= 1 NOAA 14 ***********************************	1.00 1	7.00	**************************************	- * * * * * * * * * * * * * * * * * * *	2.00	3.00
AHYMO PROGRAM SUMMARY TABLE INPUT FILE = F:\DIANE\AHYMO	COMMAND IDEN	ALL **** UBLE ***B	*S ***********************************	**S **********************************	**C **********************************	COMPUTE NM HYD 1.00 - 1 0.00153 *\$ ***********************************	*S ***********************************	*S ***********************************
AH	CO	* \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	* CO * * 4	* * * G * * * 4	* * * * * Q w w n	. O * * +	* * * O * *	*S COI FII

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*S
      Double Eagle Airport Hanger Development
*S
      100 YEAR 6-HOUR STORM EVENT FILE: DBLEAGLE.DAT (07-26-23)
*S
START
               0.0 HRS PUNCH CODE=0 PRINT LINES=-6
RAINFALL
               TYPE=1 RAIN QUARTER=0.0 RAIN ONE=1.90
               RAIN SIX=2.22 RAIN DAY=2.75 DT=.05
*S *********************************
*S *********************************
*S DOUBLE EAGLE AIRPORT
*S *********************************
*S SUB BASIN 1-SW
ID=1 HYD=1 DA=.00084 SQ MI
COMPUTE NM HYD
               PER A=0 B=0 C=0 D=100
               TP=.133 HRS
                             RAIN=-1
PRINT HYD
               ID=1 CODE=1
*5 ********************************
*S SUB BASIN 2-SW
ID=1 HYD=2 DA=.00085 SQ MI
COMPUTE NM HYD
               PER A=0 B=0 C=0 D=100
               TP=.133 HRS
PRINT HYD
               ID=1 CODE=1
*S *********************************
*S SUB BASIN 3-SW
*S **********************************
               ID=1 HYD=3 DA=.00094 SO MI
COMPUTE NM HYD
               PER A=0 B=0 C=0 D=100
               TP=.133 HRS
                             RAIN=-1
PRINT HYD
               ID=1 CODE=1
*S ****************
*S SUB BASIN 1-NE
ID=1 HYD=1 DA=.00153 SQ MI
COMPUTE NM HYD
               PER A=0 B=0 C=0 D=100
               TP=.133 HRS
                             RAIN=-1
PRINT HYD
               ID=1 CODE=1
*S *********************************
*S SUB BASIN 2-NE
* 5 ******************************
               ID=1 HYD=2 DA=.00141 SQ MI
COMPUTE NM HYD
               PER A=0 B=0 C=0 D=100
               TP=.133 HRS
                             RAIN=-1
PRINT HYD
               ID=1 CODE=1
*S **********************************
*S SUB BASIN 3-NE
*$ **************************
COMPUTE NM HYD
               ID=1 HYD=3 DA=.00156 SQ MI
               PER A=0 B=0 C=0 D=100
               TP=.133 HRS
                            RAIN=-1
PRINT HYD
               ID=1 CODE=1
FINISH
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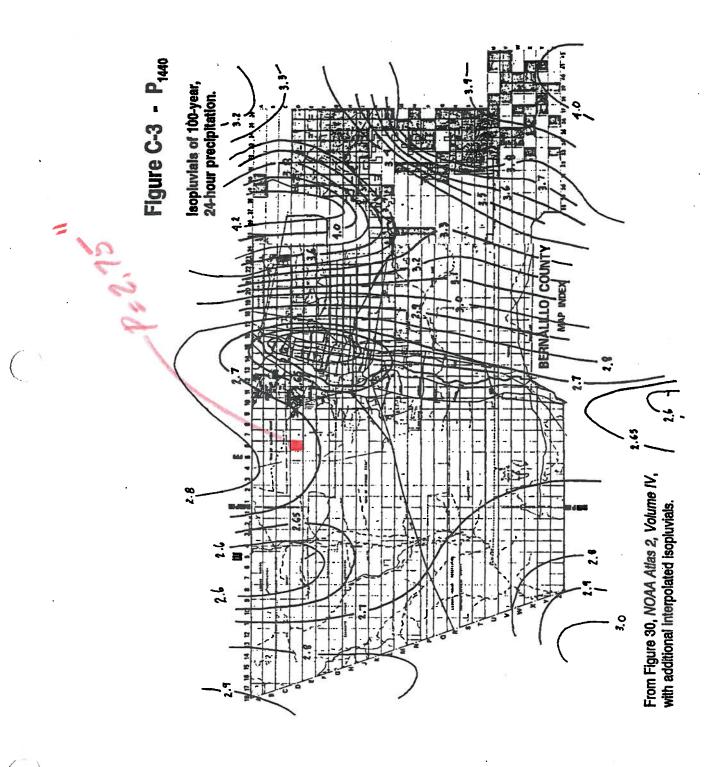


From Figures 24 and 30, NOAA Atlas 2, Volume IV, and $Y_{i\infty}$ from equation on Table 11



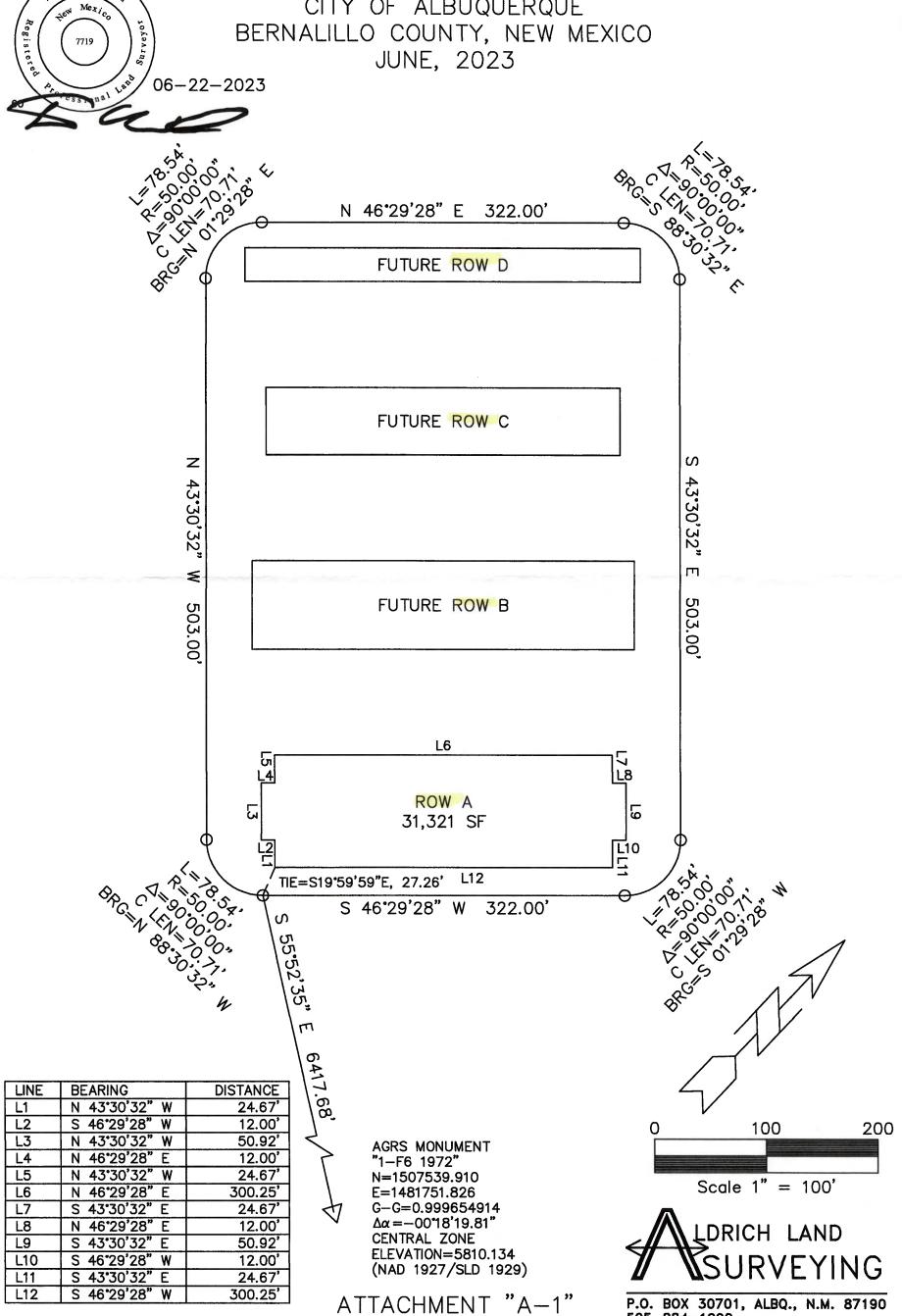
From Figure 24, NOAA Atlas 2, Volume IV, with additional interpolated isopluvials.

Isopluvials of 100-year, 6-hour precipitation.



LEASE SURVEY HIGH FLYING HANGERS - ROW A DOUBLE EAGLE II AIRPORT WITHIN THE

SECTIONS 25 & 26 TOWNSHIP 11 NORTH, RANGE 1 EAST, N.M.P.M. CITY OF ALBUQUERQUE BERNALILLO COUNTY, NEW MEXICO

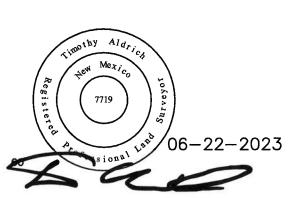


505-884-1990

LEASE SURVEY HIGH FLYING HANGERS — ROW A DOUBLE EAGLE II AIRPORT

WITHIN THE
SECTIONS 25 & 26
TOWNSHIP 11 NORTH, RANGE 1 EAST, N.M.P.M.
CITY OF ALBUQUERQUE
BERNALILLO COUNTY, NEW MEXICO
JUNE, 2023

	00 25.46 07'	25.4	16' 500.	
24.6	• •		24.	67'
60.125	10-A 3,319 SF	72.125	1-A 3,319 SF	60.125
	50.13'		50.13'	
60.00	9-A 3,007.5 SF	60.00	2-A 3,007.5 SF	60.00
	50.13'		50.13'	
60.00	8-A 2,007.5 SF	60.00′	3-A 3,007.5 SF	60.00
60.00	7-A 2.007.5 SF	60.00	4-A 3,007.5 SF	60.00
60.125,	6–A 12.00° 3,319 SF 52° 3,319 SF	72.125,	Ŏ.	60.125



Scale 1" = 40'

Scale 1" = 40'

LDRICH LAND

SURVEYING

P.O. BOX 30701, ALBQ., N.M. 87190
505-884-1990

