Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller

April 7, 2022

Ronald R. Bohannan, P.E. Tierra West, LLC 5571 Midway Park Place NE Albuquerque, NM 87109

RE: Saranam Golf Course & Montano 4701 Montano Rd NW Grading and Drainage Plan Engineer's Stamp Date: 2/15/2022 Hydrology File: E11D013B

Dear Mr. Bohannan:

Based upon the information provided in your submittal received 2/16/2022, the Grading & Drainage Plan **is not** approved for Building Permit. The following comments need to be addressed for approval of the above referenced project:

General Comments

Albuquerque

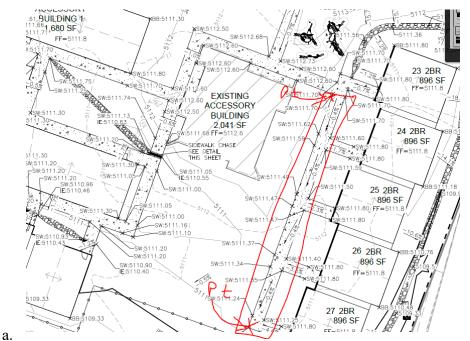
NM 87103

- Will this entrance work? Have you gotten comments from Traffic Section?
 a. Consider utilizing Standard Detail 2426. Geometry work needed.
 TC:5105.42 FL:5104.92 TC:5105.40
- TC:5105.22 FL:5104.90 SW:5106. FL:5104.72 FL:5105.9 CONCRETE TC:5105.37 SWALE @ FL:5105.87 SW:5106.3 :5104.59 SEE CURB -51 TRANSITION 04. L:5105.12 DETAIL CURB SW:5105.83 NSITION C:5105.64 TAIL 5105.14 W:5104.87 FL:5104.42 MATCH SW:5104.78 EXISTING FL:5104.14 MATCH EXISTING 2.
- 3. Practically, during construction so many points are not needed and it makes the plans very hard to review.

Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller



PO Box 1293

Albuquerque

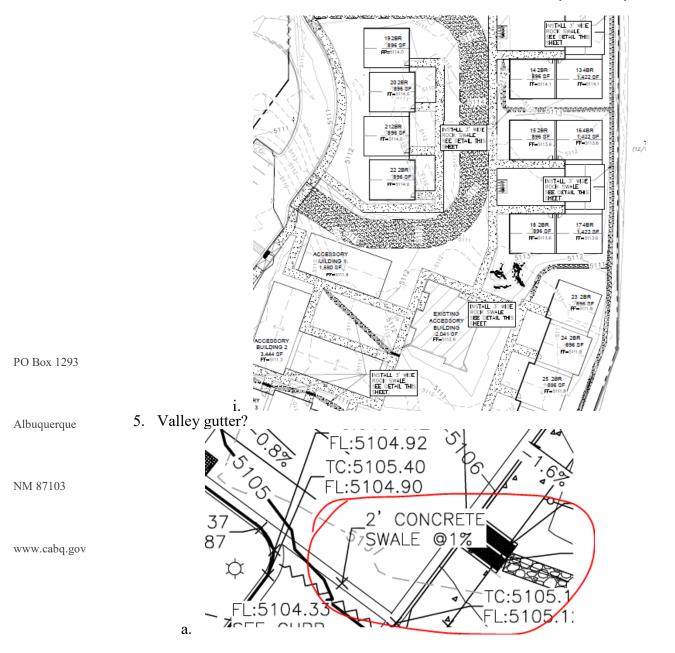
NM 87103

- 4. The drafting is very difficult to wrap my head around as much information is on here and makes the plans difficult to review. Please review drafting standards for company if possible. I feel line weights and work with drafting would help make the plans apparent.
 - a. What I really would like to know is how water is conveying (e.g. low/High points and grade changes, flow lines. Spots for construction where it makes sense. Contours show the big idea where points will show the small details only when needed)
 - b. For example, this section below, I cannot tell what proposed vs. existing contours are without straining my eyes.

Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller



Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, <u>jhughes@cabq.gov</u>, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3695 or <u>dggutierrez@cabq.gov</u>

Sincerely,

Dichut

David G. Gutierrez, P.E. Senior Engineer, Hydrology Planning Department

PO Box 1293

Albuquerque

NM 87103

| | City of Albu Planning Depa | |
|--|--|--|
| | evelopment & Building | |
| DRAINA | GE AND TRANSPORTA | TION INFORMATION SHEET (REV 6/2018) |
| Project Title: Saranam Golf Course & Mo | ntano Building Permit | #: Hydrology File #: |
| DRB#: | EPC#: PR-2020-0 | 003461 Work Order#: |
| Legal Description: TR 27A-2 TAYLOR RAN S-1, S-2 & S-3 TAYLOR | CH REDIV OF TR 27-A INTO TRS 27A-1 27A RANCH SITUATE WITHIN SECTIONS 23, 25 | 203461 Work Order#: |
| | | |
| Applicant: Tierra West, LLC | | Contact: LUIS NORIEGA |
| Address: 5571 MIDWAY PARK PLA | | |
| Phone#: 505-858-3100 | Fax#: | E-mail: LNORIEGA@TIERRAWESTLLC.CO |
| Other Contact: Saranam | | Contact: Tracy Weaver |
| Address: 1028 Eubank NE Suite F, | | |
| Phone#: | Fax#: | E-mail: tweaver@saranamabq.org |
| | | RESIDENCE X DRB SITE ADMIN SITE |
| Check all that Apply: TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERTI PAD CERTIFICATION CONCEPTUAL G & D PLAN GRADING PLAN DRAINAGE REPORT DRAINAGE MASTER PLAN FLOODPLAIN DEVELOPMENT ELEVATION CERTIFICATE CLOMR/LOMR TRAFFIC CIRCULATION LAYO TRAFFIC IMPACT STUDY (TIS) STREET LIGHT LAYOUT | PERMIT APPLIC UT (TCL) | TYPE OF APPROVAL/ACCEPTANCE SOUGHT: BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY PRELIMINARY PLAT APPROVAL SITE PLAN FOR SUB'D APPROVAL SITE PLAN FOR BLDG. PERMIT APPROVAL FINAL PLAT APPROVAL SIA/ RELEASE OF FINANCIAL GUARANTEE FOUNDATION PERMIT APPROVAL GRADING PERMIT APPROVAL APPROVAL PAVING PERMIT APPROVAL GRADING/ PAD CERTIFICATION |
| OTHER (SPECIFY) PRE-DESIGN MEETING? DATE SUBMITTED: 02.16.22 | | WORK ORDER APPROVAL CLOMR/LOMR FLOODPLAIN DEVELOPMENT PERMIT OTHER (SPECIFY) PRIEGA |
| | | |
| COA STAFF: | EI ECTRONIC SUE | MITTAL RECEIVED: |
| COASTAIT. | | |
| | FEE PAID: | |

Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller

February 7, 2022

Ronald R. Bohannan, P.E. Tierra West, LLC 5571 Midway Park Place NE Albuquerque, NM 87109

RE: Saranam Golf Course & Montano 4701 Montano Rd NW **Grading and Drainage Plan** Engineer's Stamp Date: 1/4/2022 Hydrology File: E11D013B

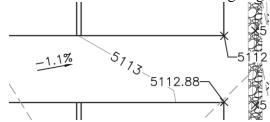
Dear Mr. Bohannan:

Based upon the information provided in your submittal received 1/4/2022, the Grading & Drainage Plan is not approved for Building Permit. The following comments need to be addressed for approval of the above referenced project: PO Box 1293 **General Comments**

- 1. The Basin Map does not appear to reflect the same buildings as the G&D Sheets.
 - a. Additionally, it appears flow is directly differently.
 - b. Ensure cross lot drainage is established (easement) if that is the plan and show how this will be routed to the proper location overall. We need to know where the water ends up ultimately.
- 2. Please utilize new DPM revision Chapter 6.
- 3. It appears the overall flow should end up in the lake and should be retained if possible. Please provide retention calculations (100-year storm event for 10 days) and show how this volume will be contained on-site.
- 4. Check FF vs corner spot elevations. They do not match.
- 5. Call out all pipes and materials.

i.

- a. Show invert elevations.
- 6. Swales
 - a. Provide elevations to show how they flow.
 - b. Provide section and reference to each one.
- 7. Overall the site needs to have a close review to ensure all of the shown elevations work.
 - a. If the elevation here is correct the flow is going right into the building?



www.cabq.gov

Albuquerque

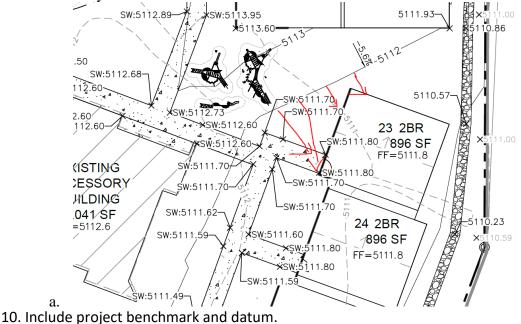
NM 87103

Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller

- 8. Please reference DPM Chaper 6.
- 9. Please take a second look at everything. See below, it appears flow may go directly into the doorways here.



11. The site must show cross lot drainage easement and ensure flow is being properly

PO Box 1293

Albuquerque

NM 87103

SHEET C2.1

www.cabq.gov

1. Please double check the calculations for the tables provided. I checked a couple and did not come up with the same.

conveyed per the easements. Also ensure the pond can handle the additional retention

volume for the site and show existing and proposed volumes for the entire volume the

2. Please include the required volume for retention. (100-year, 10-day storm)

pond is collecting. Show all of this information on this G&D.

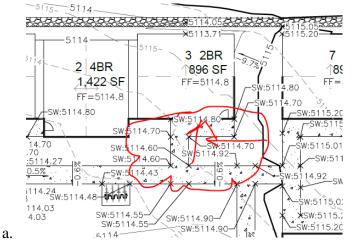
SHEET C2.3

3. Check this area. Appears maybe some ponding and may flow right to the doorway area here.

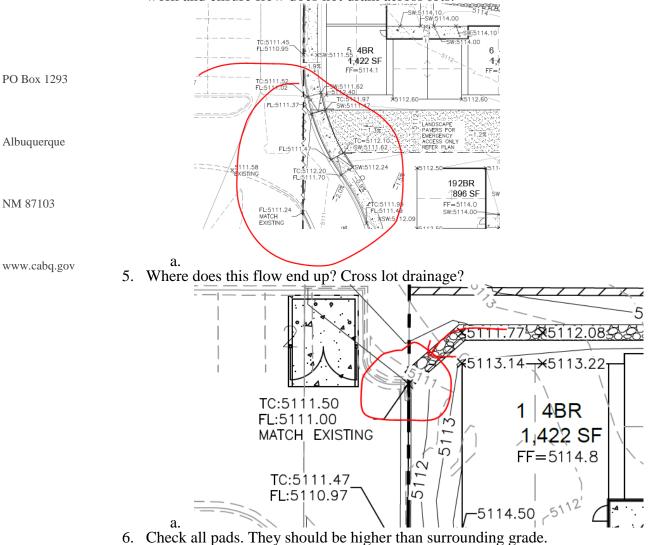
Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller



4. Is this a water block? If so, please provide elevations showing how these grades will work and ensure flow does not drain across lots.



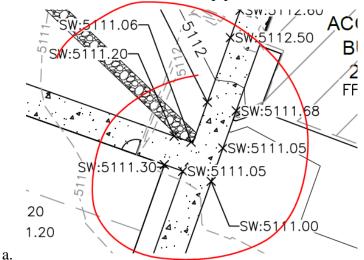
Find Hydrology forms and information at: <u>cabq.gov/planning/development-review-services/hydrology-section</u>

Planning Department Alan Varela, Interim Director



Mayor Timothy M. Keller

7. How will this work? Will there be a pipe under sidewalk?



b. Please show all of these details. As of now it appears this is just a ponding area.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

Albuquerque

If you have any questions, please contact me at 924-3695 or <u>dggutierrez@cabq.gov</u>

NM 87103

200 Gut

Sincerely,

David G. Gutierrez, P.E. Senior Engineer, Hydrology Planning Department

TIERRA WEST, LLC

February 15, 2022

Mr. David Gutierrez, P.E. City of Albuquerque, Planning Department PO BOX 1293 Albuquerque, NM 87103

RE: Saranam Golf Course & Montano 4701 Montano Rd NW Grading and Drainage Plan

Dear Mr. Gutierrez,

Per the correspondence dated February 7, 2022, please find the following responses addressing the comments listed below:

- 1. The Basin Map does not appear to reflect the same buildings as the G&D Sheets.
 - a. Additionally, it appears flow is directly differently.

Ensure cross lot drainage is established (easement) if that is the plan and show how this will be routed to the proper location overall. We need to know where the water ends up ultimately.
 Response: See updated sheet C2.1, basin maps updated to reflect latest changes to site plan and grading. Tract 27-A-2A (Project site) is to drain to the large retention pond located just west of site via an 18" HDPE pipe located within a 20' drainage easement granted under DOC# 202101455 (attached).

- 2. Please utilize new DPM revision Chapter 6. Response: Hydrology calculations updated to new DPM chapter 6 requirements.
- It appears the overall flow should end up in the lake and should be retained if possible. Please provide retention calculations (100-year storm event for 10 days) and show how this volume will be contained onsite.

Response: See updated sheet C2.1, the proposed 100 yr. - 10 day storm volume was calculated and is the design storm. According to as-builts under hydro file E11_D13A (Attached) the existing retention pond/lake west of the site has enough capacity to retain the proposed design storm volume.

4. Check FF vs corner spot elevations. They do not match.

Response: See typical unit section detail, sheets C2.3 or C2.4. The housing units are to be built on piers and 1.67' steel beams. Therefore the corner spots may be lower than the FF due to the design of the housing units,

- 5. Check out all pipes and materials.
 - a. Show invert elevations.

Response: Only one pipe proposed on the southwest side of site see sheet C2.2. Pipe material, slope and invert elevations called out.

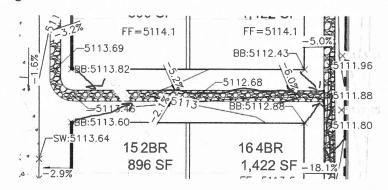
6. Swales

- a. Provide elevations to show how they flow.
- b. Provide section and reference to each one.

Response: Swale section and call outs are shown on sheet C2.2, swale elevations shown on sheets C2.3 and C2.4.

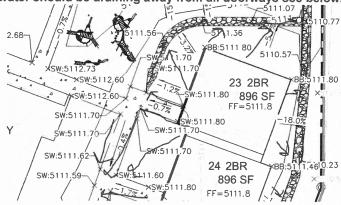
7. Overall the site needs to have a close review to ensure all of the shown elevations work.

a. If the elevation here is correct the flow is going right into the building? Response: Site analyzed to ensure shown elevations work, swales added to areas which may be prone to ponding see below.



- 8. Please reference DPM Chapter 6. Response: Referenced on sheet C2.1.
- 9. Please take a second look at everything. See below, it appears flow may go directly into the doorways here.

Response: Storm water should be draining away from all doorways see below.



10. Include project benchmark and datum. Response: See sheets C2.1 or C2.2 for datum.



11. The site must show cross lot drainage easement and ensure flow is being properly conveyed per the easements. Also ensure the pond can handle the additional retention volume for the site and show existing and proposed volumes for the entire volume the pond is collecting. Show all of this information on this G&D.

Response: See updated sheet C2.1, pond data. Tract 27-A-2A (Project site) is to drain to the large retention pond located just west of site via an 18" HDPE pipe located within a 20' drainage easement granted under DOC# 202101455 (attached).

Sheet C2.1

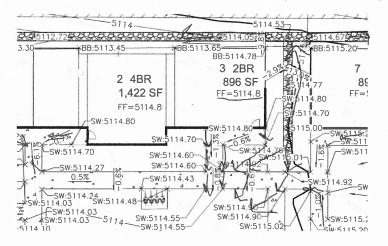
1. Please double check the calculations for the tables provided. I checked a couple and did not come up with the same.

Response: Calculations updated to new DPM chapter 6 requirements.

2. Please include the required volume for retention. (100-year, 10-day storm) Response: Required volume included in sheet C2.1.

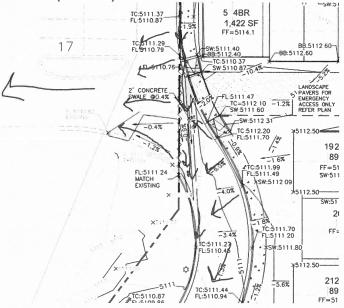
Sheet C2.3

3. Check this area. Appears maybe some ponding and may flow right to the doorway area here. Response: Storm water should be draining away from all doorways and does not pond see below.



4. Is this a water block? If so, please provide elevations showing how these grades will work and ensure flow does not drain across lots.

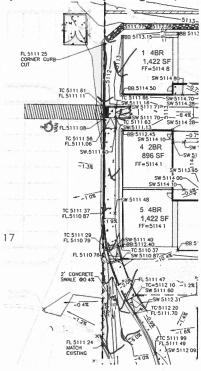
Response: Yes there is a water block; flows from the project site are routed south towards the proposed parking area. Flows from the tract to the west flows to the west towards the existing retention pond per As-Builts (Attached).



5. Where does this flow end up? Cross lot drainage?

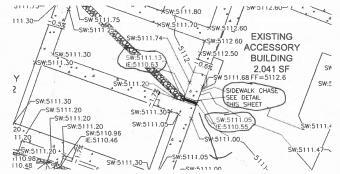
-Seer - ---

Response: Flows from the project site are intersected by the curb and gutter pan running along the property line towards the proposed southern parking area, no cross lot drainage needed.



- 6. Check all pads. They should be higher than surrounding grade. **Response: Acknowledged and checked.**
- 7. How will this work? Will there be a pipe under sidewalk?
 - a. Picture
 - b. Please show all of these details. As of now it appears this is just a ponding area.

Response: Acknowledged a sidewalk chase culvert was proposed in these types of areas see below.

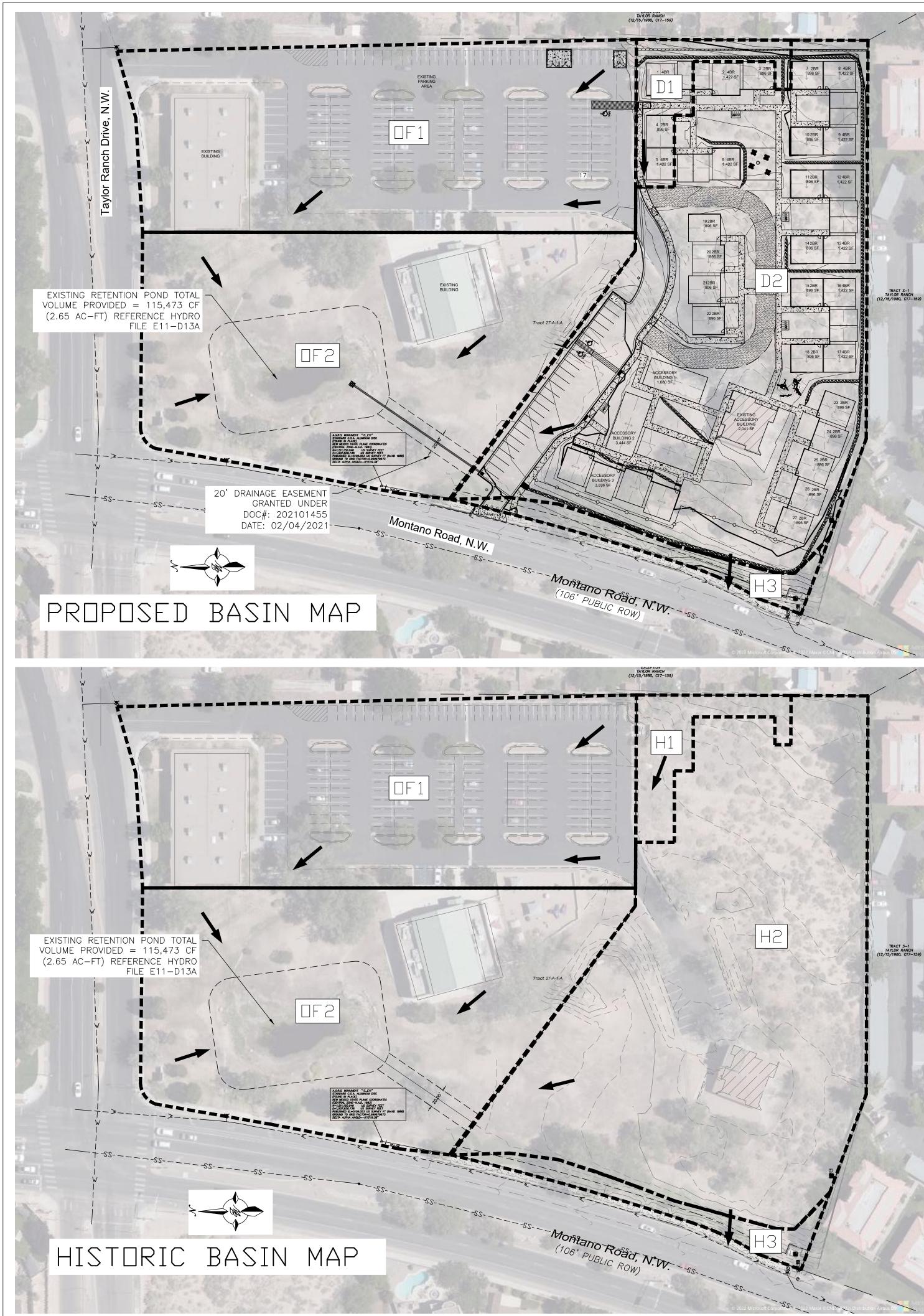


If you have any questions or need additional information regarding this matter, please do not hesitate to contact me.

Sincerely 10 C

Ronald R. Bohannan, P.E

JN: 2019080 RRB/In/mc



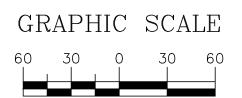
INTRODUCTION & REFERENCE FILES FILE #: E11D013A FLOOD PLAIN

THE PROJECT AREA IS INCLUDED ON FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAP (FIRM) PANEL 35001C0114H DATED AUGUST 16, 2012 AND IS SHOWN ON THIS PAGE. THE MAP INDICATES THE SITE LIES WITHIN FLOOD ZONE X, AN AREA OF MINIMAL FLOOD HAZARD. HISTORIC DRAINAGE:

THERE IS NO OFFSITE FLOWS INTO THE PARCEL. THE HISTORIC DRAINAGE RUNOFF IS RETAINED ONSITE WITH THE PARCEL DRAINING TO THE EXISTING RETENTION POND AT THE SOUTHWEST CORNER. IT APPEARS FROM A CURSORY REVIEW OF THE AVAILABLE DOCUMENTS THE RETENTION POND WAS SIZED TO ACCOMMODATE THE ENTIRE PARCEL IN A DEVELOPED STATE, AS DETAILED ON THE SITE DEVELOPMENT PLAN. THE LAKE IS AT LEAST 6 FEET DEEP AND THERE IS AN EXISTING MARSH/WETLAND AREA THAT WAS PLANTED AND IS IN GOOD HEALTH FOR ALBUQUERQUE. A SMALL PORTION OF THE EMBANKMENT ALONG THE FRONTAGE DRAINS DIRECTLY INTO MONTANO RD. THIS SLOPED LANDSCAPED AREA WILL REMAIN IN THE DEVELOPED CONDITION AND FREELY DISCHARGE. PROPOSED DRAINAGE:

THE WEIGHTED E METHOD FROM THE "CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL CH 6 WAS USED TO CALCULATE THE RUNOFF AND VOLUME FOR THE SITE. THE HYDROLOGY TABLES ARE SHOWN ON THIS PAGE. THE SITE WAS DIVIDED INTO 5 BASINS WITH THE APPROPRIATE LAND TREATMENT DETERMINED AS SHOWN IN THE DRAINAGE TABLE. THE MAJORITY OF THE SITE WILL SHEET FLOW AND BE DIRECTED BY CURB AND GUTTER TO A 3'X3' GRATE INLET IN THE PARKING LOT IN THE SOUTH WEST CORNER OF THE SITE WHICH HAS THE INLET CAPACITY FOR THE EXPECTED FLOW. ROOF DRAINS FROM THE RESIDENTIAL BUILDINGS SHALL BE DIRECTED INTO THE PARKING LOT AREA AND SHEET FLOW ACROSS THE PROPERTY. THE RUNOFF SHALL THEN BE DISCHARGED DIRECTLY TO THE EXISTING RETENTION POND VIA AN 18-INCH HDPE PIPE THAT HAS THE CAPACITY TO CONVEY THE DESIGN FLOWS. THE LANDSCAPED EMBANKMENT ALONG MONTANO WILL FREELY DISCHARGE INTO THE STREET AND DOES NOT CONTRIBUTE SIGNIFICANT FLOWS. A SMALL PORTION OF THE DRIVEWAY ENTRANCE SHALL ALSO FLOW INTO MONTANO RD. AS SHOWN IN THE HYDROLOGY CALCULATIONS THE EXISTING RETENTION POND HAS ENOUGH CAPACITY TO RETAIN THE 100 YR - 10 DAY STORM AND ANY REQUIRED STORM WATER QUALITY VOLUME.





SCALE: 1"=60'

DPM Weighted E Method Chapter 6 Precipitation Zone 1

East of Mesa View United Methodist Church 4701 Montaño Rd NW, Albuquerque, NM 87120 2/10/2022 TWLLC Date

<u>Equations:</u>

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area) Volume = Weighted E * Total Area Flow = Qa*Aa + Qb*Ab + Qc*Ac + Qd*Ad

| Basin Descriptions | | | | | | | | | | | | 100-Year, 10-Day | | | |
|--------------------|----------|---------|---------|------------|---------|---------|-------|---------|------|----------|--------|------------------|------------|---------|--------|
| Basin | Tract | Area | Area | Area | Treatmo | ent A | Treat | ment B | Trea | atment C | Treatm | ent D | Weighted E | Volume | Flow |
| ID | Tract | (sf) | (acres) | (sq miles) | % | (acres) | % | (acres) | % | (acres) | % | (acres) | (in) | (ac-ft) | cfs |
| H1 | 27-A-2-A | 9,700 | 0.22 | 0.00035 | 85% | 0.189 | 0% | 0.000 | 10% | 0.022 | 5% | 0.011 | 0.675 | 0.014 | 0.40 |
| H2 | 27-A-2-A | 132,562 | 3.04 | 0.00476 | 70% | 2.130 | 0% | 0.000 | 20% | 0.609 | 10% | 0.304 | 0.799 | 0.246 | 6.28 |
| *H3 | 27-A-2-A | 7,636 | 0.18 | 0.00027 | 0% | 0.000 | 0% | 0.000 | 100% | 0.175 | 0% | 0.000 | 0.950 | 0.014 | 0.50 |
| OF1 | 27-A-1-A | 96,500 | 2.22 | 0.00346 | 0% | 0.000 | 5% | 0.111 | 15% | 0.332 | 80% | 1.772 | 1.971 | 0.619 | 8.49 |
| OF2 | 27-A-1-A | 102,500 | 2.35 | 0.00368 | 4% | 0.094 | 0% | 0.000 | 80% | 1.882 | 16% | 0.376 | 1.140 | 0.278 | 7.10 |
| Total | | 142,262 | 8.01 | 0.01251 | | 2.414 | | 0.111 | | 3.021 | | 2.464 | | 1.158 | 22.276 |

*BASIN H3 NATURALLY FLOWS TO CITY ROW AND IS NOT INCLUDED IN TOTAL VOLUME/FLOW

Proposed Conditions

| | Basin Descriptions | | | | | | | | | | 100-Year, 10-Day | | | | |
|-------|--------------------|---------|---------|------------|-------------|---------|------------|---------|-------------|---------|------------------|---------|------------|---------|--------|
| Basin | Tract | Area | Area | Area | Treatment A | т | reatment B | 5 | Treatment C | | Treatment D | | Weighted E | Volume | Flow |
| ID | | (sf) | (acres) | (sq miles) | % | (acres) | % | (acres) | % | (acres) | % | (acres) | (in) | (ac-ft) | cfs |
| H1 | 27-A-2-A | 9,700 | 0.22 | 0.00035 | 0% | 0.000 | 25% | 0.056 | 25% | 0.056 | 50% | 0.111 | 1.540 | 0.045 | 0.74 |
| H2 | 27-A-2-A | 132,562 | 3.04 | 0.00476 | 0% | 0.000 | 20% | 0.609 | 20% | 0.609 | 60% | 1.826 | 1.680 | 0.689 | 10.58 |
| *H3 | 27-A-2-A | 7,636 | 0.18 | 0.00027 | 0% | 0.000 | 0% | 0.000 | 100% | 0.175 | 0% | 0.000 | 0.950 | 0.014 | 0.50 |
| OF1 | 27-A-1-A | 96,500 | 2.22 | 0.00346 | 0% | 0.000 | 5% | 0.111 | 15% | 0.332 | 80% | 1.772 | 1.971 | 0.619 | 8.49 |
| OF2 | 27-A-1-A | 102,500 | 2.35 | 0.00368 | 4% | 0.094 | 0% | 0.000 | 80% | 1.882 | 16% | 0.376 | 1.140 | 0.278 | 7.10 |
| | | | | | | | | | | | | | | | |
| Total | | 142,262 | 8.01 | 0.01251 | | 0.094 | | 0.775 | | 3.054 | | 4.086 | | 1.631 | 26.916 |

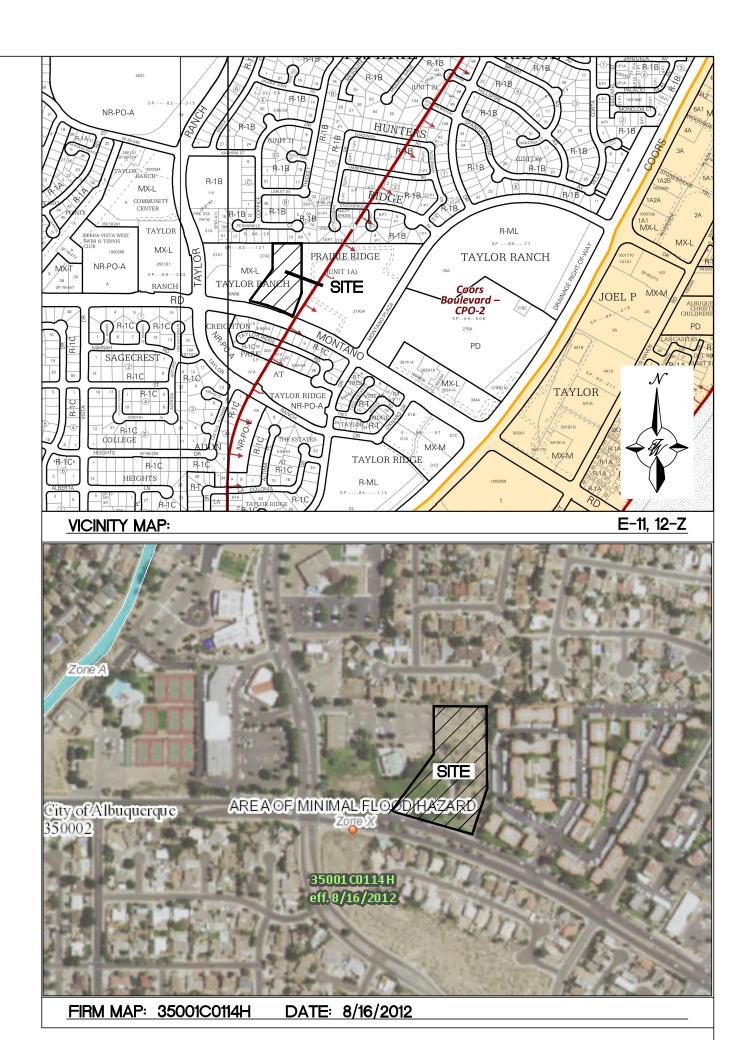
*BASIN H3 NATURALLY FLOWS TO CITY ROW AND IS NOT INCLUDED IN TOTAL VOLUME/FLOW

| Excess Precipitation, E (in.) | | | Peak Dis | scharge (cf | s/acre) |
|-------------------------------|----------|---------|----------|-------------|---------|
| Zone 1 | 100-Year | 10-Year | Zone 1 | 100-Year | 10-Year |
| Ea | 0.55 | 0.08 | Qa | 1.54 | 0.3 |
| Eb | 0.73 | 0.22 | Qb | 2.16 | 0.81 |
| Ec | 0.95 | 0.44 | Qc | 2.87 | 1.46 |
| Ed | 2.24 | 1.24 | Qd | 4.12 | 2.57 |

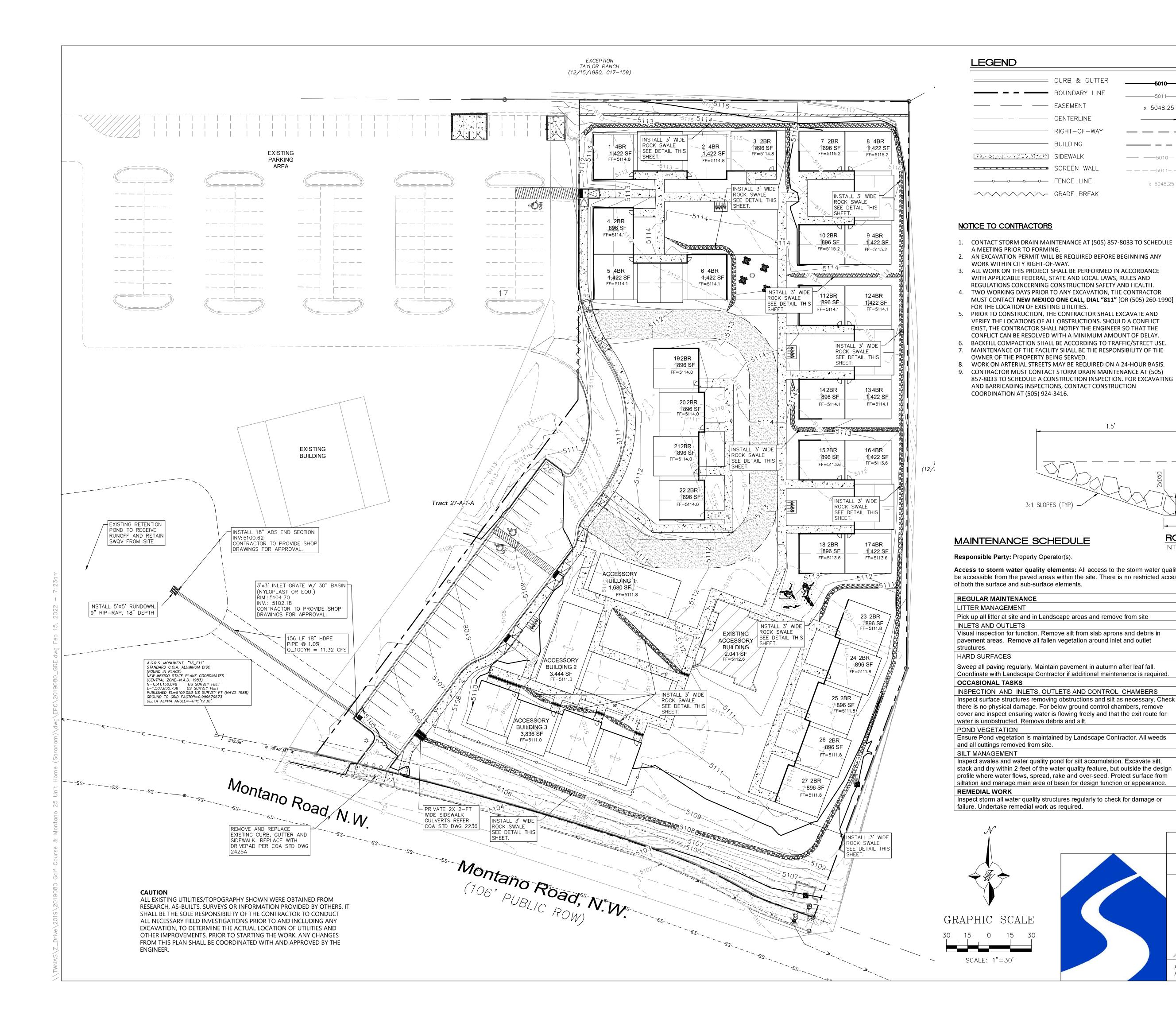
EXISTING POND ELEVATION FT VOLUME (CF) AREA SF

| ENSTING FOND | ELE VATION FT | AREA SF | |
|---------------------|------------------------|----------------------|--------------------|
| | 5100 | 326 | 0 |
| | 5101 | 4486 | 2406 |
| | 5102 | 8728 | 9013 |
| | 5103 | 12123 | 19439 |
| | 5104 | 19494 | 35247 |
| | 5105 | 41166 | 65577 |
| OVERFLOW | 5106 | 58625 | 115473 |
| 100-YR 10-DAY PO | ND DATA | VOLUME AC-FT | VOLUME CF |
| Historic Pond Volum | ıe | 1.158 | 50,442.40 |
| Proposed Pond Volu | ime Required | 1.631 | 71,046.25 |
| Existing Pond volum | e provided (Max) | 2.65 | 115,473.00 |
| * Existing po | nd volume obtained fro | m As-builts file E11 | _D13A (11/22/2002) |

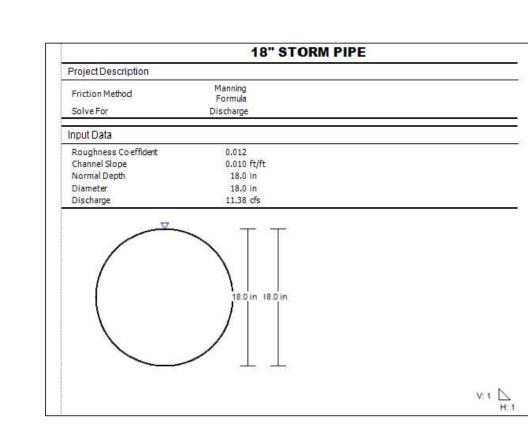
Existing Conditions



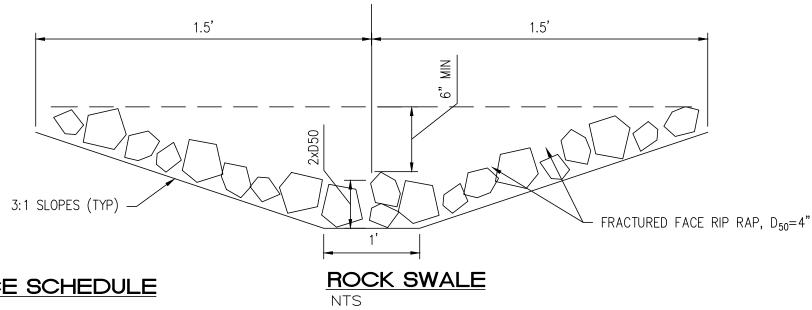
| ENGINEER'S SEAL | SARANAM AT | DRAWN BY LN |
|----------------------------------|--|---------------------------|
| NUD R. BOHA | 4701 MONTANO RD NW | <i>DATE</i> 10/20/2021 |
| AL METICO | CONCEPTIONAL GRADING & DRAINAGE PLAN | |
| PROFILESSIONALENGIN | | SHEET # |
| Pu 10/15/2022 | 5571 MIDWAY PARK PLACE NE Albuquerque, NM 87109 | C2.1 |
| RONALD R. BOHANNAN P.E. #7868 | (505) 858-3100 www.tierrawestllc.com | јов # 2019080 |



| _ | CURB & GUTTER | 5010 | CONTOUR MAJOR |
|---|---------------|--------------------|-------------------------|
| | BOUNDARY LINE | | CONTOUR MINOR |
| | | x 5048.25 | |
| | CENTERLINE | > | FLOW ARROW |
| | RIGHT-OF-WAY | | EXISTING CURB & GUTTER |
| | BUILDING | | EXISTING BOUNDARY LINE |
| | SIDEWALK | 5010 | EXISTING CONTOUR MAJOR |
| | SCREEN WALL | — — — —5011– — — – | EXISTING CONTOUR MINOR |
| | FENCE LINE | x 5048.25 | EXISTING SPOT ELEVATION |
| | | | |



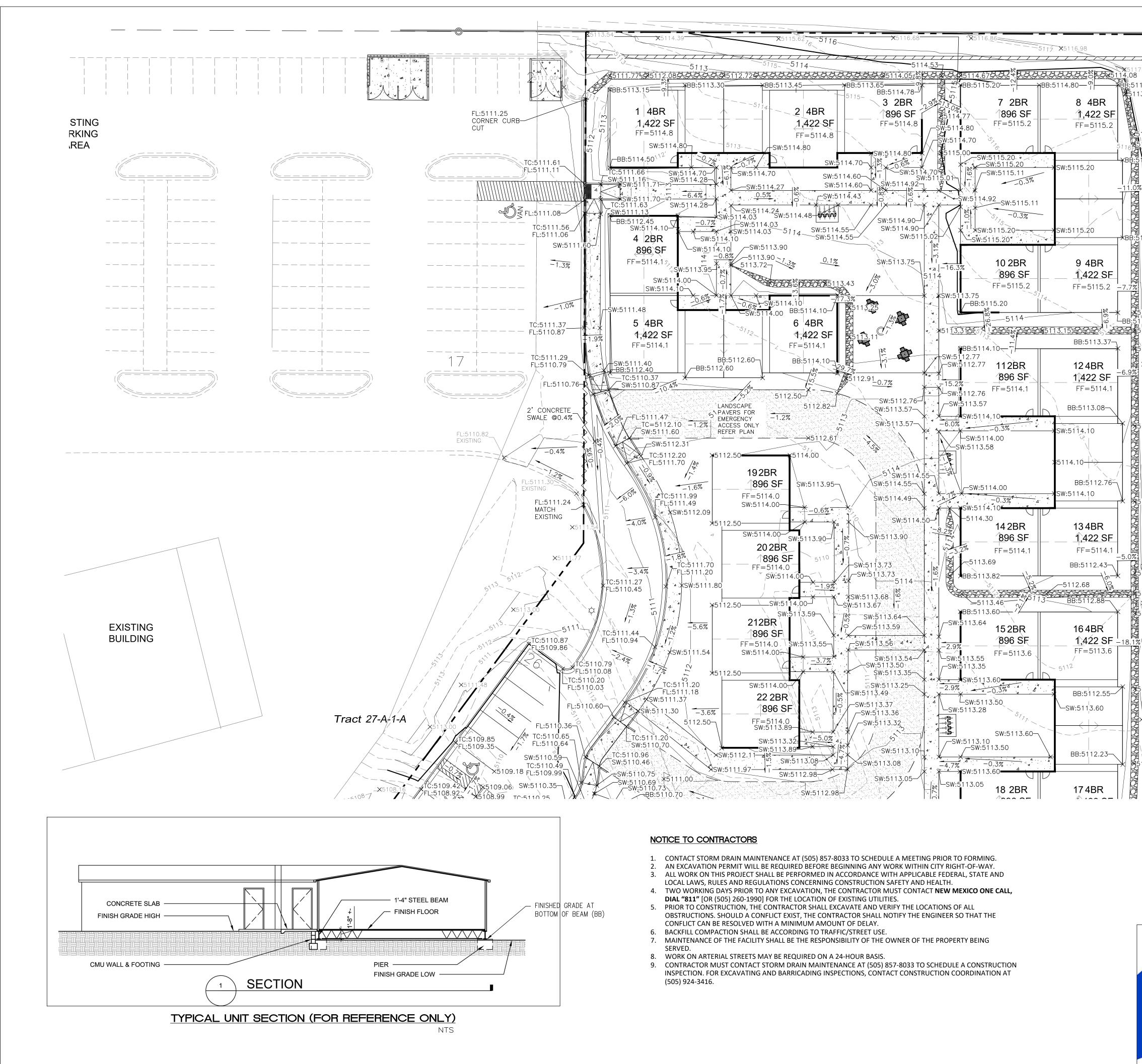
18" STORM DRAIN CAPACITY CALC

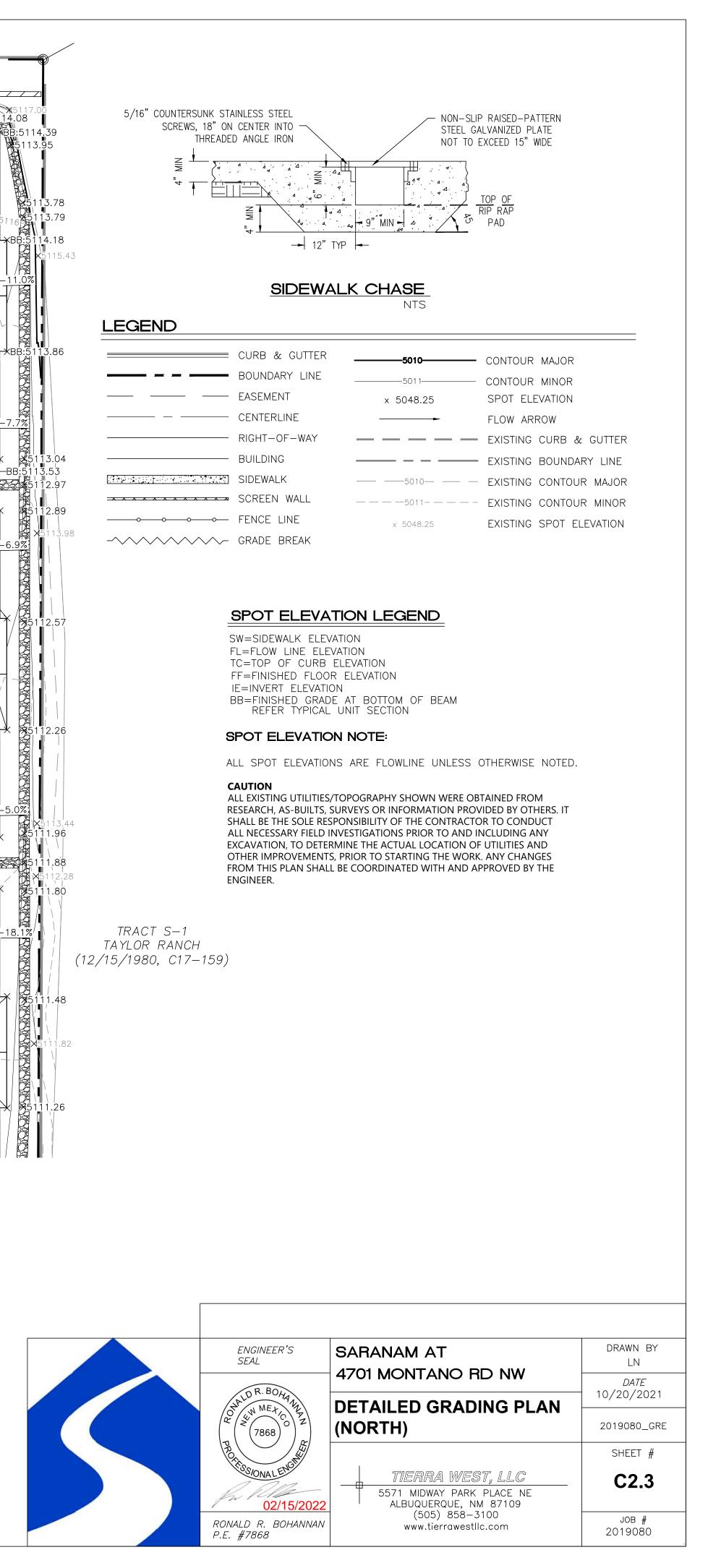


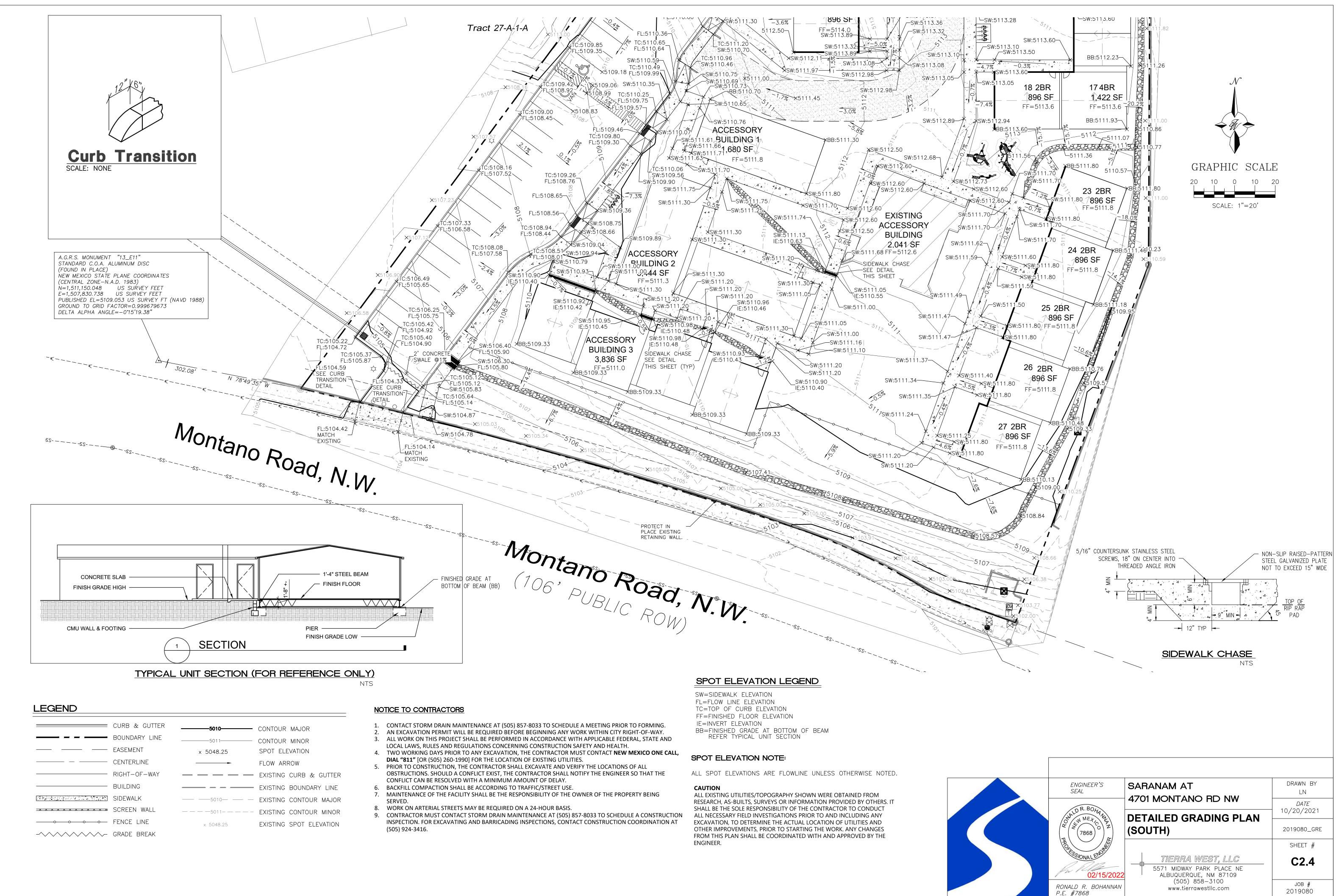
Access to storm water quality elements: All access to the storm water quality elements shall be accessible from the paved areas within the site. There is no restricted access to the location

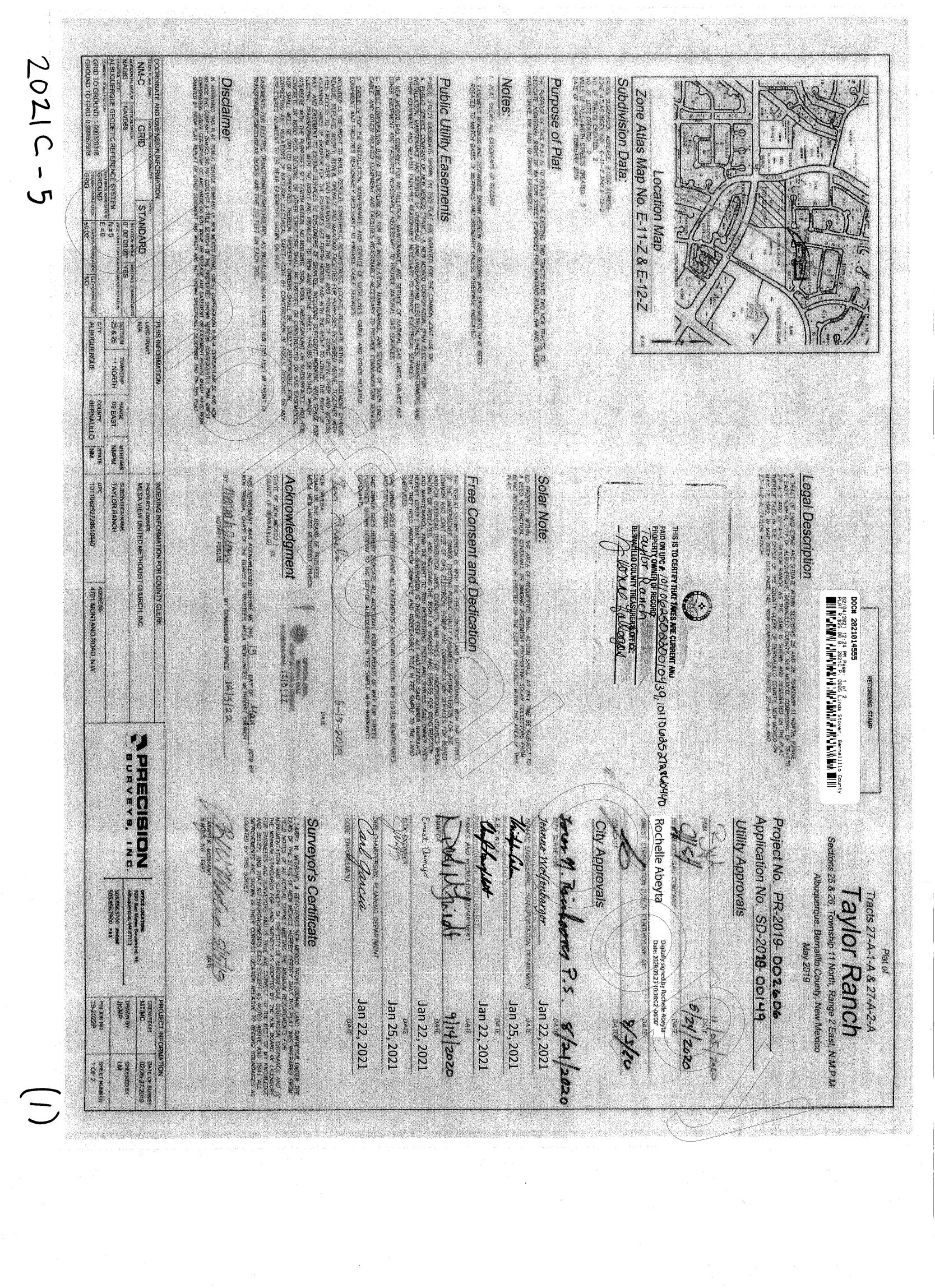
| | FREQUENCY |
|--|-------------|
| | |
| andscape areas and remove from site | Daily |
| | - |
| Remove silt from slab aprons and debris in | |
| allen vegetation around inlet and outlet | |
| | Monthly |
| | |
| intain pavement in autumn after leaf fall. | |
| ntractor if additional maintenance is required. | As required |
| | FREQUENCY |
| OUTLETS AND CONTROL CHAMBERS | |
| oving obstructions and silt as necessary. Check | |
| or below ground control chambers, remove | |
| ter is flowing freely and that the exit route for elevents and silt. | Yearly |
| | really |
| ntained by Landscape Contractor. All weeds | |
| site. | As required |
| | |
| ty pond for silt accumulation. Excavate silt, | |
| ne water quality feature, but outside the design | |
| ad, rake and over-seed. Protect surface from | |
| a of basin for design function or appearance. | Yearly |
| | FREQUENCY |
| structures regularly to check for damage or | |
| rk as required. | Yearly |
| | |

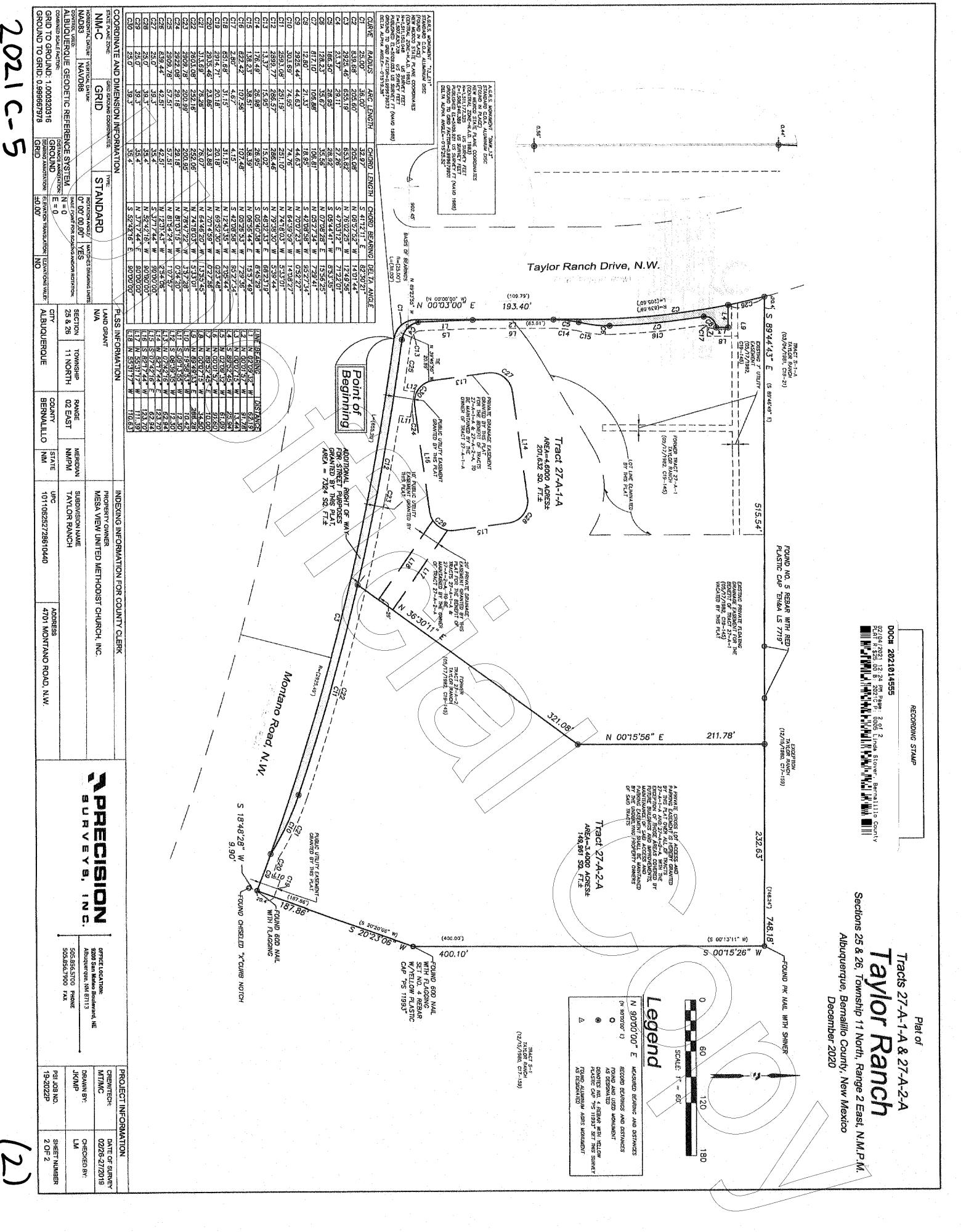
| ENGINEER'S SEAL | SARANAM AT | DRAWN BY LN |
|----------------------------------|--|---------------------------|
| DR. BOW | 4701 MONTANO RD NW | <i>DATE</i> 10/20/2021 |
| NU WETCONFU | | |
| ((7868)) | (OVER ALL) | 2019080_GRE |
| PROFILESSIONALENGIN | | SHEET # |
| 02/15/2022 | 5571 MIDWAY PARK PLACE NE ALBUQUERQUE, NM 87109 | C2.2 |
| RONALD R. BOHANNAN P.E. #7868 | (505) 858-3100 www.tierrawestllc.com | јов # 2019080 |

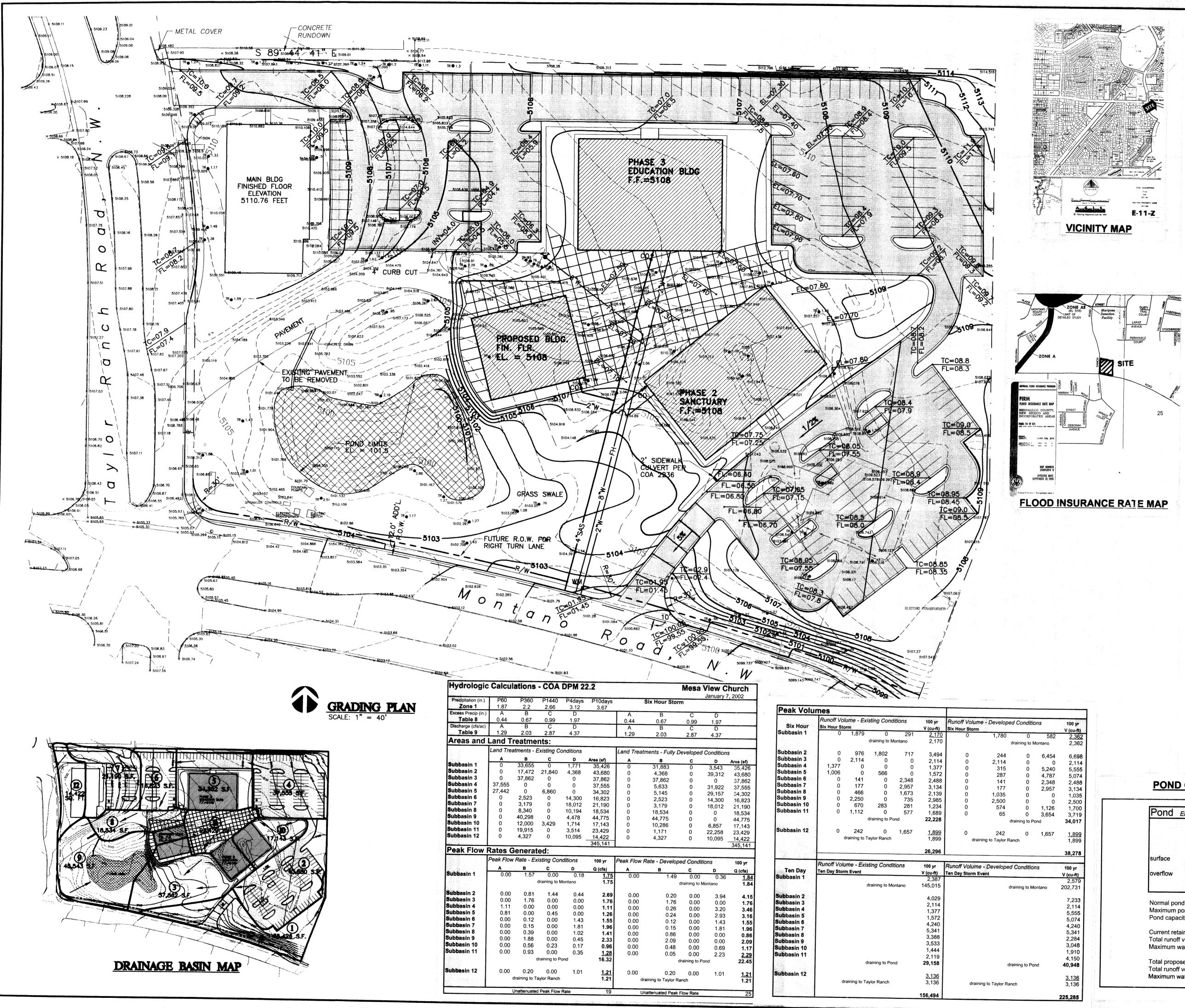












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Site Location - As shown by the Vicinity Map (Zone Atlas Map E-11), the 8.07-acre site on the west side of Albuquerque is located northeast of the intersection of Montano Road and Taylor Ranch Drive and about 2,200 feet west of Coors Boulevard. At present, the site is developed with a 10,000 square foot church building, a 2,500 square foot outbuilding, and associated paved driveway and parking facilities. A large portion of the undeveloped property on the site is landscaped as a park with a permanent pond as shown. The vast majority of the surrounding area is currently developed, thereby making this a modification to an existing site within an infill area. The proposed improvements consist of a 7,020 square foot Sunday school building with adjacent asphalt paving and drainage facilities for Phase One, a 12,600 square foot sanctuary building with associated parking for Phase Two, and a 13,042 square foot education building with associated parking for Phase Three. The existing driveway from Taylor Ranch Drive is to be abandoned and all access is to be routed to the existing entrance located north of the existing church under Phase One conditions. The proposed driveway located on Montano Road in proposed to be constructed under the Phase Two construction.

Address- Mesa View United Methodist Church, 4701 Montano Road NW, Albuquerque, New Mexico 87120.

Legal Description - Tracts 27-A1 and 27-A2, Taylor Ranch, New

Temporary Benchmark - A temporary benchmark for use in construction of the site will be the finish floor elevation of the main church building at elevation 5,110.76 feet. For horizontal datum, a chiseled "X" on the block wall near the northwesterly corner of the site. According to the plat, ACS control station "3-E10" is located at a bearing of N.83deg00'13" and distance of 8,805.16 feet.

Flood Zone - As shown by Panel 114 of 825 of the National Flood Insurance Program Flood Insurance Rate Maps (FIRM) for the City of Albuquerque, New Mexico, dated September 20, 1996, this site does not lie within a designated flood hazard zone.

Existing Conditions - Currently, the properties to the north and east of the project site drain to either Taylor Ranch Drive or Montano Road. No offsite runoff impacts the site. The street slopes are on the order of one-percent and support adequate capacity to carry the fully developed 100-year design flow runoff from the site. The existing pond on the project site acts as a retention facility during significant rainfall events. Most of the subject property discharges through the park area into the pond. The normal water surface of the pond is maintained at elevation 5,101-feet storing approximately 35,247 cubic-feet of water. The maximum capacity of the pond before overtopping occurs was estimated as 115,473 cubic-feet of water at elevation 5,103 feet. All overflow will discharge to Montano Road. The existing 100-year runoff was estimated as 64,405 cubic-feet at an elevation of 5,101.96-feet.

Proposed Grading - The Grading and Drainage Plan shows 1) existing and proposed grades indicated by spot elevations and contours at one-foot intervals with continuity between existing and proposed grades; 2) the limit of existing and proposed improvements. The proposed building pads will be set to correspond to finish floor elevations of 5,108-feet to maintain positive drainage around the buildings. The additional northern parking area on both sides of the Phase Three building will be graded to drain from both ends to the proposed four-foot curb cut and then to the south across the grass-lined swale to the pond as shown on the plan. The grading of the site will maintain the drainage to the existing pond. The proposed condition 100-year rainfall runoff will generate approximately 76,195-cubic feet of volume in the pond at a maximum elevation of 5,102.35-feet. The increase in water surface elevation due to the proposed construction was estimated to be less than five-inches. This will leave about 0.65-foot of freeboard constituting 39,278 cubic-feet of additional storage volume before overtopping.

The Phase Two construction will include the construction of the additional driveway as shown on the Plan. The runoff from this parking facility will discharge through a sidewalk culvert into the pond. The increase in discharge to Montano Road was estimated to be approximately 0.11-cfs.

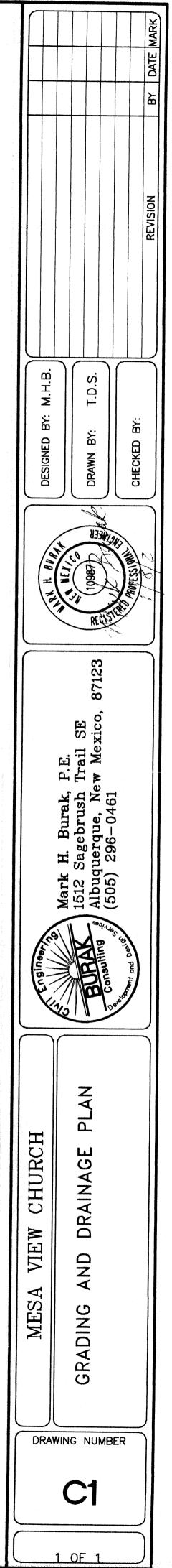
Hydrologic Methods - The drainage basin map shows twelve separate subbasins 1 through 12 to assess peak flow rates at various points around the project site culminating at the pond or Montano Road. The spreadsheet calculations analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The process outlined in the DPM, Section 22.2 was used to quantify the peak flow rates and volumes. As shown by these calculations, the fully developed commercial improvements will result in a minimal increase in runoff generated by the site. Also, by draining the majority of the site toward the existing pond, only a minimal increase in runoff to Montano Road will be found due to the proposed improvements to the site. As noted on the Plan, only subbasin one will continue to discharge onto Montano Road as is the case under current conditions.

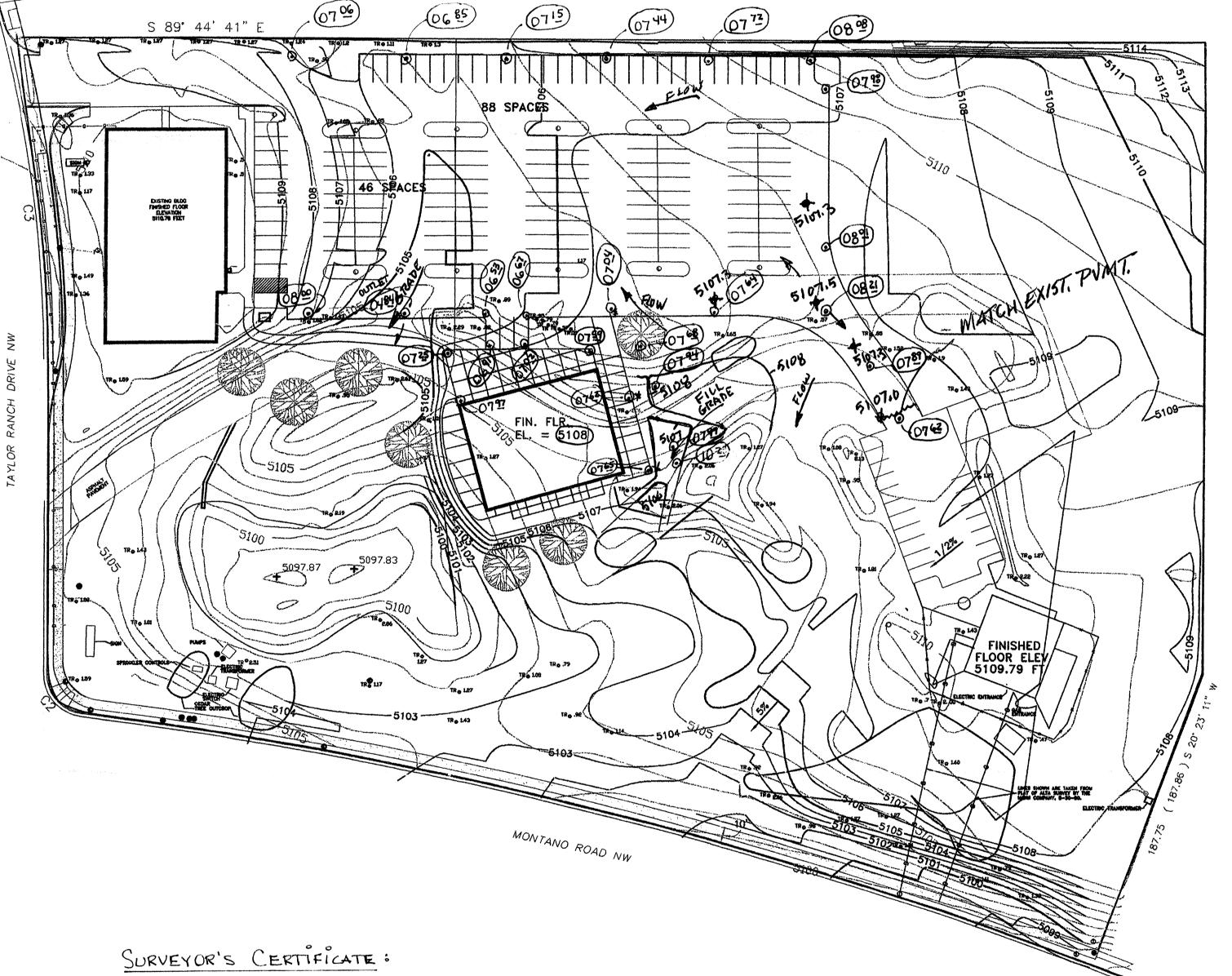
The subject property improvements increase the existing peak runoff by about six cubic feet per second and about 12,000 cubic feet of volume in the Pond as shown on the calculations. A spreadsheet for Precipitation Zone 1 is included on this plan. This spreadsheet outlines the peak runoff and volume generated for each subbasin for existing and proposed fully developed conditions.

Erosion Control Measures - The contractor shall ensure that no soil erodes from the site into public right-of-way or onto private property. This can be achieved by constructing temporary berms at the property lines and wetting the soil to keep it from blowing. The contractor shall promptly clean up any material excavated within the public right-of-way so that the excavated material is not susceptible to being washed down the street. The contractor shall secure "Topsoil Disturbance Permits" prior to beginning construction.

POND CAPACITY DATA

| Pond | Elevation | Area | | Volume | | | |
|---|--|--|--|--|-----------|---|----------------------|
| | 5097 | 326 | 0 | 0 | | | |
| | 5098 | 4,486 | 2,406 | 2,406 | | | |
| | 5099 | 8,728 | 6,607 | 9,013 | | | |
| | 5100 | 12,123 | 10,426 | 19,439 | | | |
| surface | 5101 | 19,494 | 15,809 | 35,247 | | | |
| | 5102 | 41,166 | 30,330 | 65,577 | | | |
| overflow | 5103 | 58,625 | 49,896 | 115,473 | | | |
| | 5104 | | | | | | |
| | | | | | | | |
| Sector Sector | | | | | | | |
| Normal na | and otorood | | ст Г 404 б | | | | |
| Normal po | and storage | e at elevat | ion 5,101 f | et |] | 35,247 | cf |
| Maximum | pond capa | icity to ove | erflow | | | 115,473 | |
| Maximum | pond capa | icity to ove | erflow | eet to overflow | | | |
| Maximum Pond capa Current re | pond capa acity from r tained rund | icity to ove formal wa off in pond | ter surface | to overflow | nt | 115,473 80,226 | cf |
| Maximum Pond capa Current re Total runo | pond capa acity from r tained rund ff volume u | ormal wa ormal wa off in pond under curre | erflow ter surface for 100-yr ent conditio | to overflow rainfall eve | nt | 115,473 80,226 29,158 | cf cf |
| Maximum Pond capa Current re Total runo | pond capa acity from r tained rund ff volume u | ormal wa ormal wa off in pond under curre | erflow ter surface for 100-yr ent conditio | to overflow | nt | 115,473 80,226 29,158 64,405 | cf cf cf |
| Maximum Pond capa Current re Total runo Maximum | pond capa acity from r tained rund ff volume u water surfa | icity to ove normal wa off in pond inder curre ace for exi | erflow ter surface for 100-yr ent conditic sting 100-y | to overflow rainfall eve ons rr rainfall ev | nt ent | 115,473 80,226 29,158 | cf cf |
| Maximum Pond capa Current re Total runo Maximum Total prop | pond capa acity from r tained rund ff volume u water surfa osed retair | ncity to over normal war off in pond under curre ace for exi ned runoff | erflow ter surface for 100-yr ent conditic sting 100-y in pond for | to overflow rainfall eve ons rr rainfall ev | nt ent | 115,473 80,226 29,158 64,405 | cf cf cf |
| Maximum Pond capa Current re Total runo Maximum Total prop Total runo | pond capa acity from r tained rund ff volume u water surfa osed retain ff volume u | ncity to over normal war off in pond under curre ace for exi ned runoff under prop | erflow ter surface for 100-yr ent conditic sting 100-y in pond for osed cond | to overflow rainfall eve ons rr rainfall ev | nt ent | 115,473 80,226 29,158 64,405 5,101.96 | cf cf cf fe |





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BEST OF MY KNOWLEDGE AND BELIEF. Brie K. McCetock BRIAN K. McCLINTOCK, NMPS No 11597 DATED: 11/22/2002

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THE ASBUILT ELEVATIONS, SHOWN HEREON BY THE CIRCLED NUMBERS, WERE TAKEN IN THE FIELD ON NOVEMBER 22ND, 2002. THE ELEVATIONS SHOWN ARE BASED OFF THE BUILDING F/F ELEVATION OF 5108.00 AND ARE TRUE AND CORRECT TO THE



1

Engineer's Certification Mesa View Church Addition - Phase One

This site was surveyed on November 22, 2002 by Brian K. McClintock, NMPS No. 11597 to confirm post-construction design elevations and drainage for the project's first phase . I, Mark H. Burak, P.E. have assessed the "as constructed" conditions and have found the site to be in substantial compliance with the approved plan stamped 01/08/02.

C Durch

Mark H. Burak P.# 10987

December 1, 2002

RECREATION DEPARTMENT.

a U. H. *B.H.*

