

CITY OF ALBUQUERQUE



February 20, 2012

Don Briggs, P.E.
Grading & Drainage Engineer/Floodplain Administrator
Bernalillo County Public Works Division.
2400 Broadway SE
Albuquerque, NM 87102

Re: Storm Drain Runoff on La Orilla Rd. NW
City of Albuquerque Project Number 706184 (E12/D025)

Dear Mr. Briggs,

PO Box 1293

This letter is to inform you that we have reviewed the construction plans for La Orilla Rd. NW (City of Albuquerque Project Number 706184), and it is acceptable to drain the runoff (just west of Via Corta Del Sur on La Orilla Road) into the City of Albuquerque storm system.

Albuquerque

If you have any questions, you can contact me at 924-3695.

NM 87103

www.cabq.gov

Sincerely,

Shahab Biazar, P.E.
Senior Engineer, Planning Department.
Development and Building Services

Cc: Jon Niski

Capacity of a Double 'A' Storm Drop Inlet

Capacity of the grate:

$$\begin{aligned} L &= 80'' - 2(2'' \text{ ends}) - 14(\frac{1}{2}'' \text{ middle bars}) - 6'' \text{ center piece} \\ &= 63'' \\ &= 5.25' \end{aligned}$$

$$\begin{aligned} W &= 25'' - 13(\frac{1}{2}'' \text{ middle bars}) \\ &= 18.5'' \\ &= 1.54' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 5.25' \times 1.54' \\ &= 8.09 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Effective Area} &= 8.09 - 8.09 (0.5 \text{ clogging factor}) \\ &= 4.04 \text{ ft}^2 \text{ at the grate} \end{aligned}$$

Orifice Equation

$$\begin{aligned} Q &= CA \sqrt{2gH} \\ Q &= 0.6 \times 4.04 \times \sqrt{2 \times 32.2 \times 0.87} \\ Q &= 15.93 \text{ cfs} \end{aligned}$$

0.46
13.19

Capacity of the Throat:

$$L = 11.00'$$

$$\begin{aligned} H &= 10 \frac{3}{4}'' - 4 \frac{1}{2}'' \\ &= 6 \frac{1}{4}'' \\ &= 0.5208' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 11.0' \times 0.5208' \\ &= 5.73 \text{ ft}^2 \text{ at the throat} \end{aligned}$$

0.46
5.06

Weir Equation

$$\begin{aligned} Q &= CLH^{3/2} \\ Q &= 2.95 \times 5.73 \times 0.87^{3/2} \\ Q &= 9.27 \text{ cfs} \end{aligned}$$

5.06
0.46
4.65

Total Capacity:

$$\begin{aligned} Q &= 15.93_{\text{grate}} + 9.27_{\text{throat}} \\ Q &= 25.20 \text{ cfs} \end{aligned}$$

$$4.65 + 5.06$$

$$13.19 + 4.65 = 17.84 \text{ gpm}$$

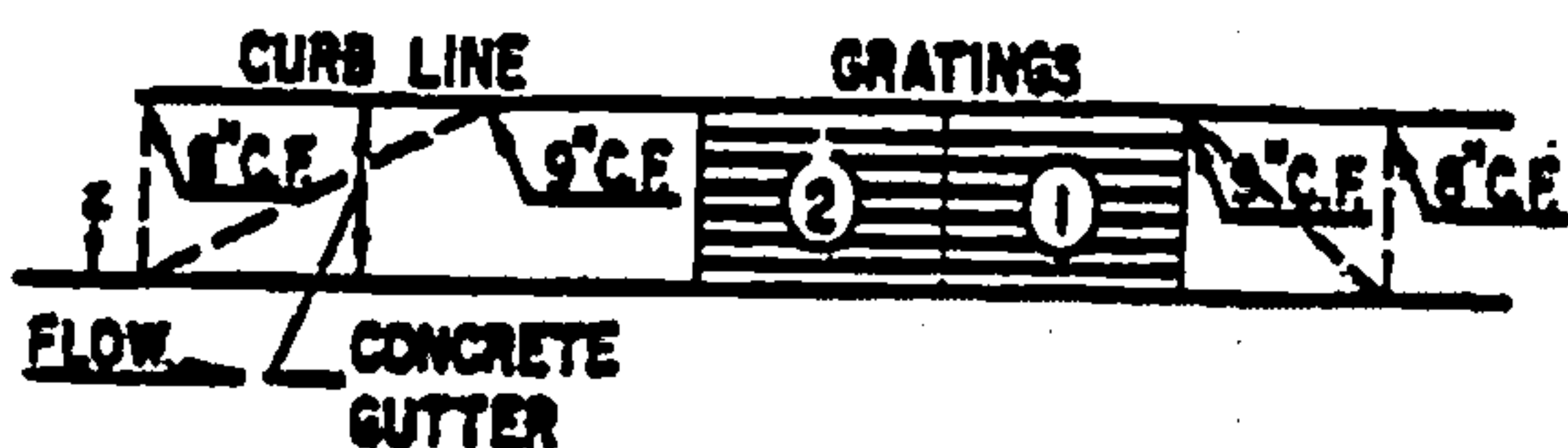
$$52.494 - 17.84 = 34.65 \text{ gpm}$$

$$15.93 \text{ gpm}$$

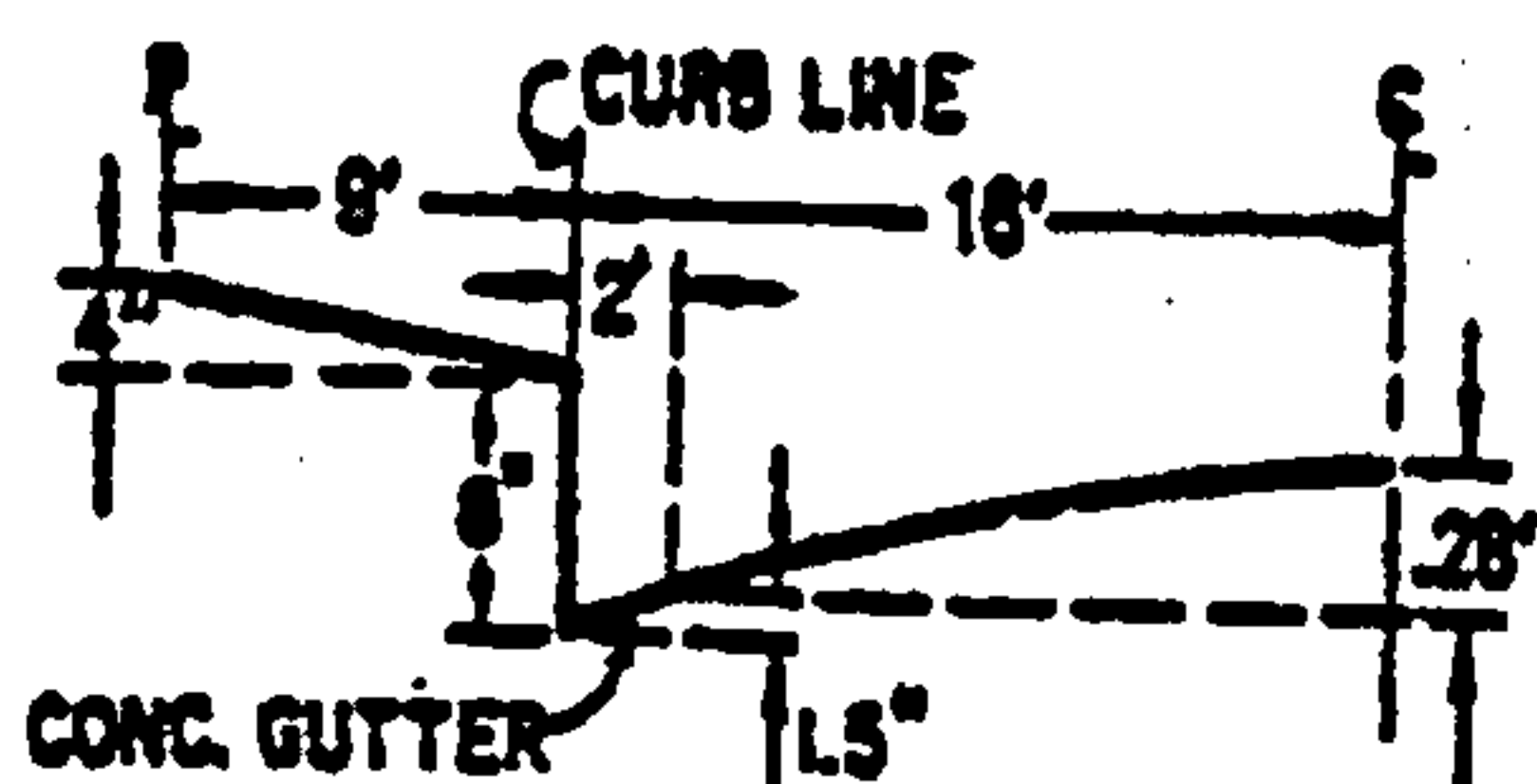
$$17.33 \text{ gpm}$$

$$16.25 + 4.65$$

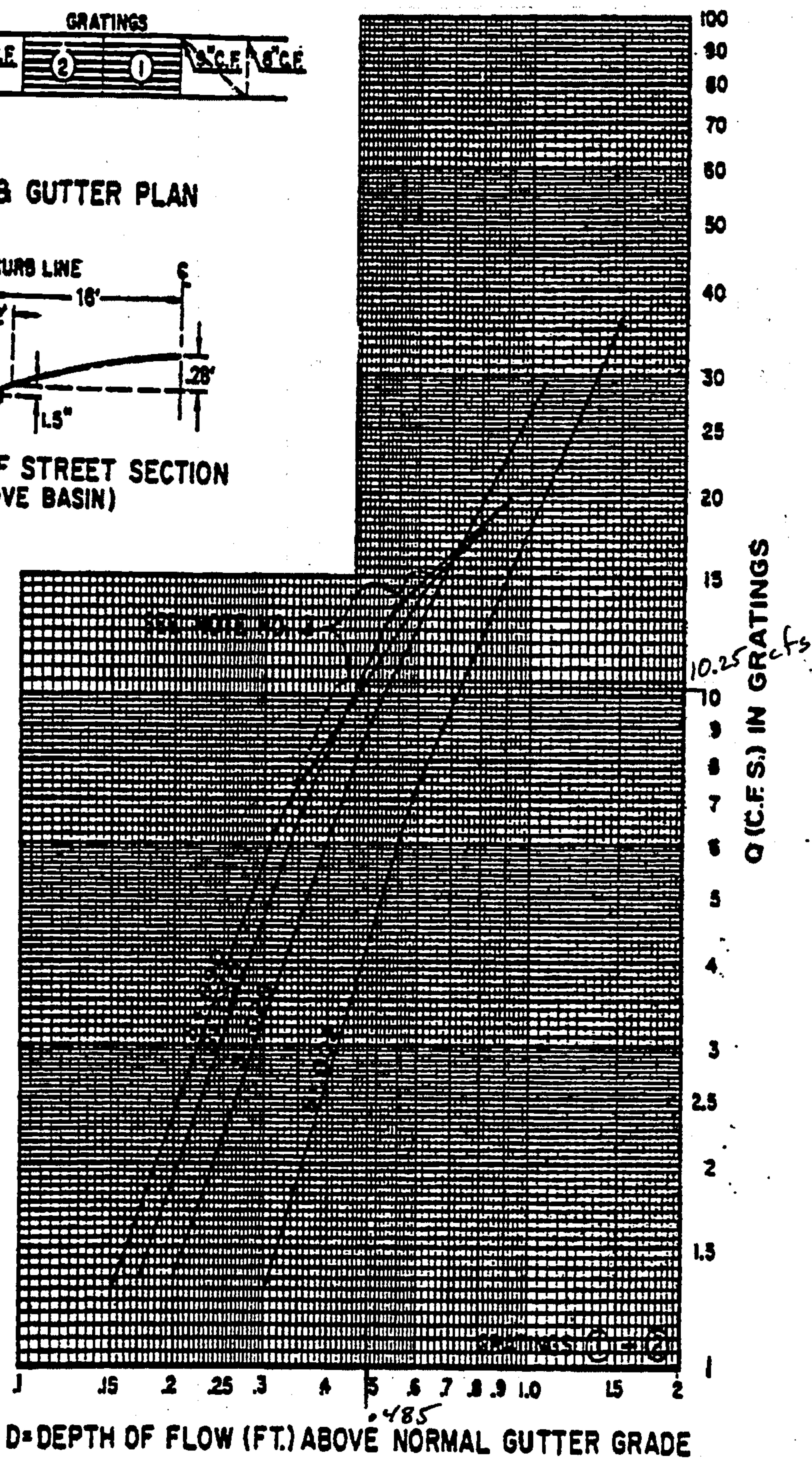
GRATING CAPACITIES FOR TYPE DOUBLE "C," AND "D"



GRATING & GUTTER PLAN



TYPICAL HALF STREET SECTION
(ABOVE BASIN)



Capacity of a Single 'A' Storm Drop Inlet

Capacity of the grate:

$$\begin{aligned} L &= 40'' - 2(2''_{\text{ends}}) - 7(\frac{1}{2}''_{\text{middle bars}}) \\ &= 32 \frac{1}{2}'' \\ &= 2.7083' \end{aligned}$$

$$\begin{aligned} W &= 25'' - 13(\frac{1}{2}''_{\text{middle bars}}) \\ &= 18.5'' \\ &= 1.54' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 2.7083' \times 1.54' \\ &= 4.18 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Effective Area} &= 4.18 - 4.18 (0.5_{\text{clogging factor}}) \\ &= 2.09 \text{ ft}^2 \text{ at the grate} \end{aligned}$$

Orifice Equation

$$Q = CA \sqrt{2gH}$$

$$Q = 0.6 \times 2.09 \times \sqrt{2 \times 32.2 \times 0.67}$$

$$Q = 8.24 \text{ cfs}$$

Handwritten: 6.85 cfs, 0.46

Capacity of the Throat:

$$L = 6.50'$$

$$\begin{aligned} H &= 10 \frac{3}{4}'' - 4 \frac{1}{2}'' \\ &= 6 \frac{1}{4}'' \\ &= 0.5208' \end{aligned}$$

Handwritten: 0.46

$$\begin{aligned} \text{Area} &= 6.50' \times 0.5208' \\ &= 3.39 \text{ ft}^2 \text{ at the throat} \end{aligned}$$

Handwritten: 2.99

Weir Equation

$$Q = CLH^{(3/2)}$$

$$Q = 2.95 \times 3.39 \times 0.67^{(3/2)}$$

$$Q = 5.48 \text{ cfs}$$

Handwritten: 2.75 cfs, 0.46

Total Capacity:

$$Q = 8.24_{\text{grate}} + 5.48_{\text{throat}}$$

$$Q = 13.72 \text{ cfs}$$

Handwritten: 6.85 + 2.75 = 9.6 cfs (circled), 2.99

Street Capacity Calculations

La Orilla Road

56' F-F Street Section with 8" curb

Slope= 0.0344

For water depths less than 0.125 feet

Y= Water depth
 Area = $8 \cdot Y^2$
 P= $\text{SQRT}(257 \cdot Y^2) + Y$
 n= 0.017

Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.001	0.170	0.005	0.000	0.001	0.455	0.005	0.801	0.007
0.02	0.003	0.341	0.009	0.002	0.005	0.722	0.014	0.899	0.017
0.04	0.013	0.681	0.019	0.015	0.029	1.146	0.046	1.010	0.041
0.06	0.029	1.022	0.028	0.043	0.086	1.501	0.090	1.080	0.066
0.08	0.051	1.362	0.038	0.093	0.186	1.819	0.146	1.133	0.094
0.1	0.080	1.703	0.047	0.169	0.338	2.111	0.211	1.176	0.124
0.12	0.115	2.044	0.056	0.275	0.549	2.383	0.286	1.212	0.154
0.125	0.125	2.129	0.059	0.306	0.612	2.449	0.306	1.221	0.162

For water depths greater than 0.125 ft but less than 0.405 ft

Y1= Y-0.125
 A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
 P2= $P1 + \text{SQRT}(2501 \cdot Y1^2)$

Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.13	0.136	2.384	0.057	0.325	0.650	2.398	0.312	1.172	0.160
0.16	0.226	3.914	0.058	0.546	1.092	2.419	0.387	1.066	0.174
0.20	0.416	5.955	0.070	1.142	2.285	2.748	0.550	1.083	0.222
0.24	0.686	7.995	0.086	2.162	4.323	3.153	0.757	1.134	0.283
0.28	1.036	10.035	0.103	3.694	7.388	3.567	0.999	1.188	0.351
0.32	1.466	12.076	0.121	5.825	11.649	3.974	1.272	1.238	0.423
0.35	1.793	13.423	0.134	7.598	15.196	4.237	1.468	1.269	0.472
0.39	2.411	15.647	0.154	11.232	22.464	4.659	1.817	1.315	0.556
0.41	2.645	16.412	0.161	12.700	25.399	4.801	1.945	1.330	0.585

For water depths greater than 0.405 ft but less than 0.667 ft

Y2= Y - 0.405
 A3= $A2 + Y2^2 \cdot 28$
 P3= $P2 + Y2$

Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.41	2.785	16.417	0.170	13.837	27.673	4.968	2.037	1.367	0.614
0.44	3.625	16.447	0.220	21.444	42.888	5.916	2.603	1.572	0.782
0.46	4.094	16.463	0.249	26.247	52.494	6.411	2.928	1.672	0.875
0.50	5.305	16.507	0.321	40.354	80.708	7.607	3.803	1.896	1.114
0.55	6.705	16.557	0.405	59.502	119.004	8.874	4.881	2.109	1.388
0.59	7.951	16.601	0.479	78.909	157.818	9.924	5.900	2.268	1.633
0.63	8.945	16.637	0.538	95.890	191.779	10.720	6.754	2.380	1.829
0.67	9.981	16.674	0.599	114.935	229.869	11.515	7.681	2.485	2.034

For water depths greater than 0.667 ft but less than 0.847 ft

Y3= Y - 0.667
 A4= $A3 + 28 \cdot Y3 + 25 \cdot Y3^2$
 P4= $P3 + \text{SQRT}(2501 \cdot Y3^2)$

Street Capacity Calculations

La Orilla Road

56' F-F Street Section with 8" curb

Slope= 0.0344

For water depths less than 0.125 feet

Y= Water depth
Area = $8 \cdot Y^2$
P= $\text{SQRT}(257 \cdot Y^2) + Y$
n= 0.017

Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.010	0.001	0.170	0.005	0.000	0.001	0.455	0.005	0.801	0.007
0.020	0.003	0.341	0.009	0.002	0.005	0.722	0.014	0.899	0.017
0.040	0.013	0.681	0.019	0.015	0.029	1.146	0.046	1.010	0.041
0.060	0.029	1.022	0.028	0.043	0.086	1.501	0.090	1.080	0.066
0.080	0.051	1.362	0.038	0.093	0.186	1.819	0.146	1.133	0.094
0.100	0.080	1.703	0.047	0.169	0.338	2.111	0.211	1.176	0.124
0.120	0.115	2.044	0.056	0.275	0.549	2.383	0.286	1.212	0.154
0.125	0.125	2.129	0.059	0.306	0.612	2.449	0.306	1.221	0.162

For water depths greater than 0.125 ft but less than 0.645 ft

Y1= Y-0.125
A2= $A1 + 2 \cdot Y1 + 28 \cdot Y1^2$
P2= $P1 + \text{SQRT}(2501 \cdot Y1^2) + Y1$

Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.130	0.136	2.384	0.057	0.326	0.651	2.399	0.312	1.173	0.160
0.200	0.433	5.955	0.073	1.221	2.441	2.822	0.564	1.112	0.230
0.300	1.333	11.056	0.121	5.271	10.542	3.956	1.187	1.273	0.410
0.400	2.793	16.157	0.173	14.048	28.095	5.030	2.012	1.402	0.618
0.450	3.733	18.707	0.200	20.662	41.324	5.536	2.491	1.454	0.727
0.485	4.467	20.477	0.218	26.245	52.491	5.875	2.848	1.487	0.805
0.550	6.033	23.808	0.253	39.162	78.323	6.492	3.570	1.543	0.956
0.600	7.393	26.359	0.280	51.351	102.703	6.946	4.168	1.580	1.074
0.645	8.736	28.654	0.305	64.160	128.319	7.344	4.737	1.612	1.182

For water depths greater than 0.645 ft but less than 0.667 ft

Y2= Y - 0.645
A3= A2 + Y2*28
P3= P2 + Y2

Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.646	8.764	28.895	0.303	64.144	128.287	7.319	4.728	1.605	1.178
0.647	8.792	28.896	0.304	64.484	128.968	7.334	4.745	1.607	1.182
0.650	8.876	28.899	0.307	65.510	131.019	7.380	4.797	1.613	1.193
0.652	8.932	28.901	0.309	66.197	132.394	7.411	4.832	1.617	1.201
0.657	9.072	28.906	0.314	67.927	135.854	7.487	4.919	1.628	1.219
0.660	9.156	28.909	0.317	68.974	137.948	7.533	4.972	1.634	1.230
0.665	9.296	28.914	0.322	70.732	141.465	7.609	5.060	1.644	1.249
0.667	9.352	28.916	0.323	71.441	142.881	7.639	5.095	1.648	1.257

Street Capacity Calculations

La Orilla Road

56' F-F Street Section with 8" curb

Slope= 0.0344

For water depths less than 0.125 feet

Y= Water depth
Area = $8 \cdot Y^2$
P= $\text{SQRT}(257 \cdot Y^2) + Y$
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Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.001	0.170	0.005	0.000	0.001	0.455	0.005	0.801	0.007
0.02	0.003	0.341	0.009	0.002	0.005	0.722	0.014	0.899	0.017
0.04	0.013	0.681	0.019	0.015	0.029	1.146	0.046	1.010	0.041
0.06	0.029	1.022	0.028	0.043	0.086	1.501	0.090	1.080	0.066
0.08	0.051	1.362	0.038	0.093	0.186	1.819	0.146	1.133	0.094
0.1	0.080	1.703	0.047	0.169	0.338	2.111	0.211	1.176	0.124
0.12	0.115	2.044	0.056	0.275	0.549	2.383	0.286	1.212	0.154
0.125	0.125	2.129	0.059	0.306	0.612	2.449	0.306	1.221	0.162

For water depths greater than 0.125 ft but less than 0.405 ft

56' F-F
32' F-F
0.645

Y1= Y - 0.125
A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
P2= $P1 + \text{SQRT}(2501 \cdot Y1^2)$

Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
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0.645

For water depths greater than 0.405 ft but less than 0.667 ft

Y2= Y - 0.405
A3= $A2 + Y2^2 \cdot 28$
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Depth (ft)	Area (ft ²)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
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0.44	3.625	16.447	0.220	21.444	42.888	5.916	2.603	1.572	0.782
0.46	4.094	16.463	0.249	26.247	52.494	6.411	2.928	1.672	0.875
0.50	5.305	16.507	0.321	40.354	80.708	7.607	3.803	1.896	1.114
0.55	6.705	16.557	0.405	59.502	119.004	8.874	4.881	2.109	1.388
0.59	7.951	16.601	0.479	78.909	157.818	9.924	5.900	2.268	1.633
0.68	8.945	16.637	0.538	95.890	191.779	10.720	6.754	2.380	1.829
0.67	9.981	16.674	0.599	114.935	229.869	11.515	7.681	2.485	2.034

For water depths greater than 0.667 ft but less than 0.847 ft

Y3= Y - 0.667
A4= $A3 + 28 \cdot Y3 + 25 \cdot Y3^2$
P4= $P3 + \text{SQRT}(2501 \cdot Y3^2)$

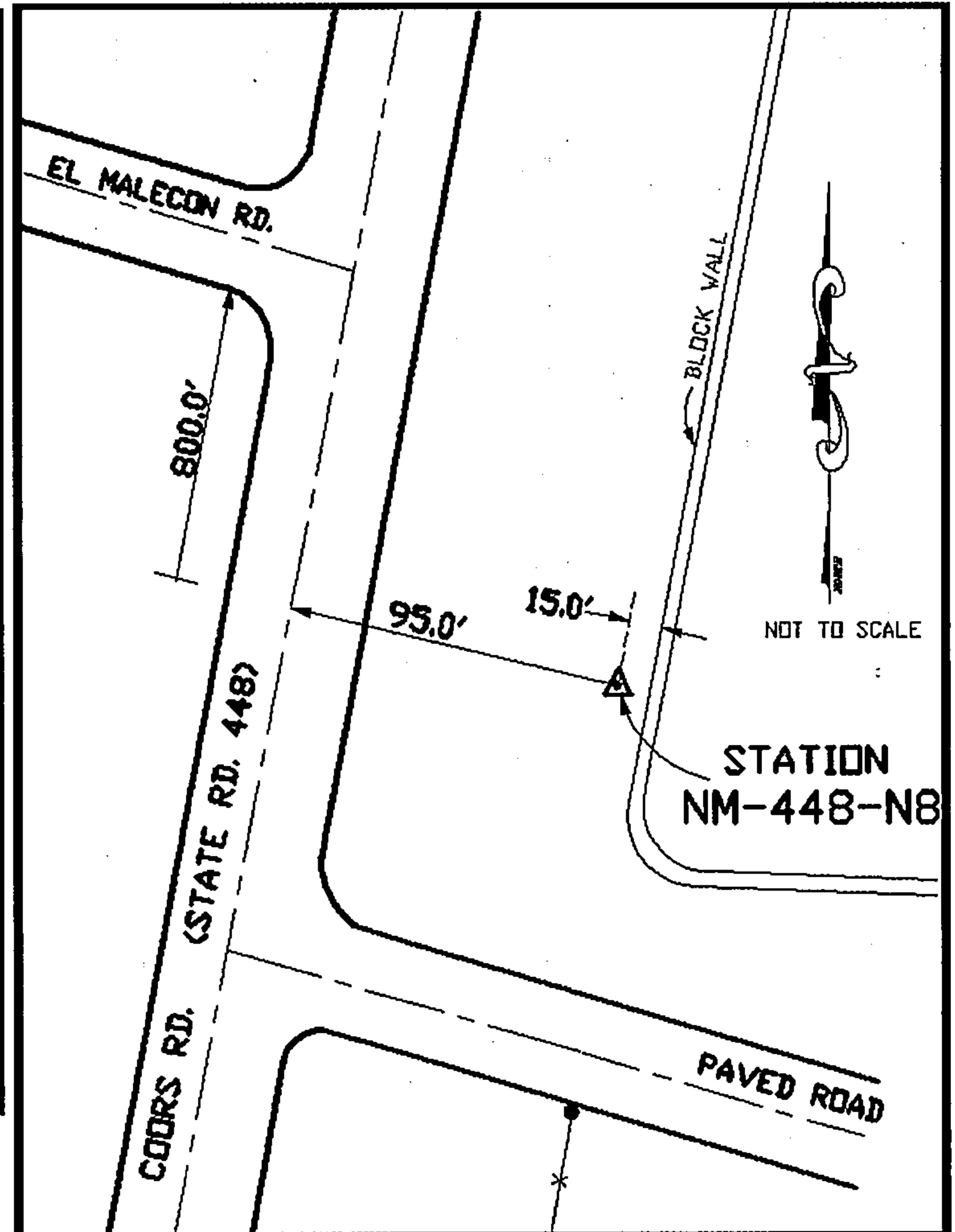
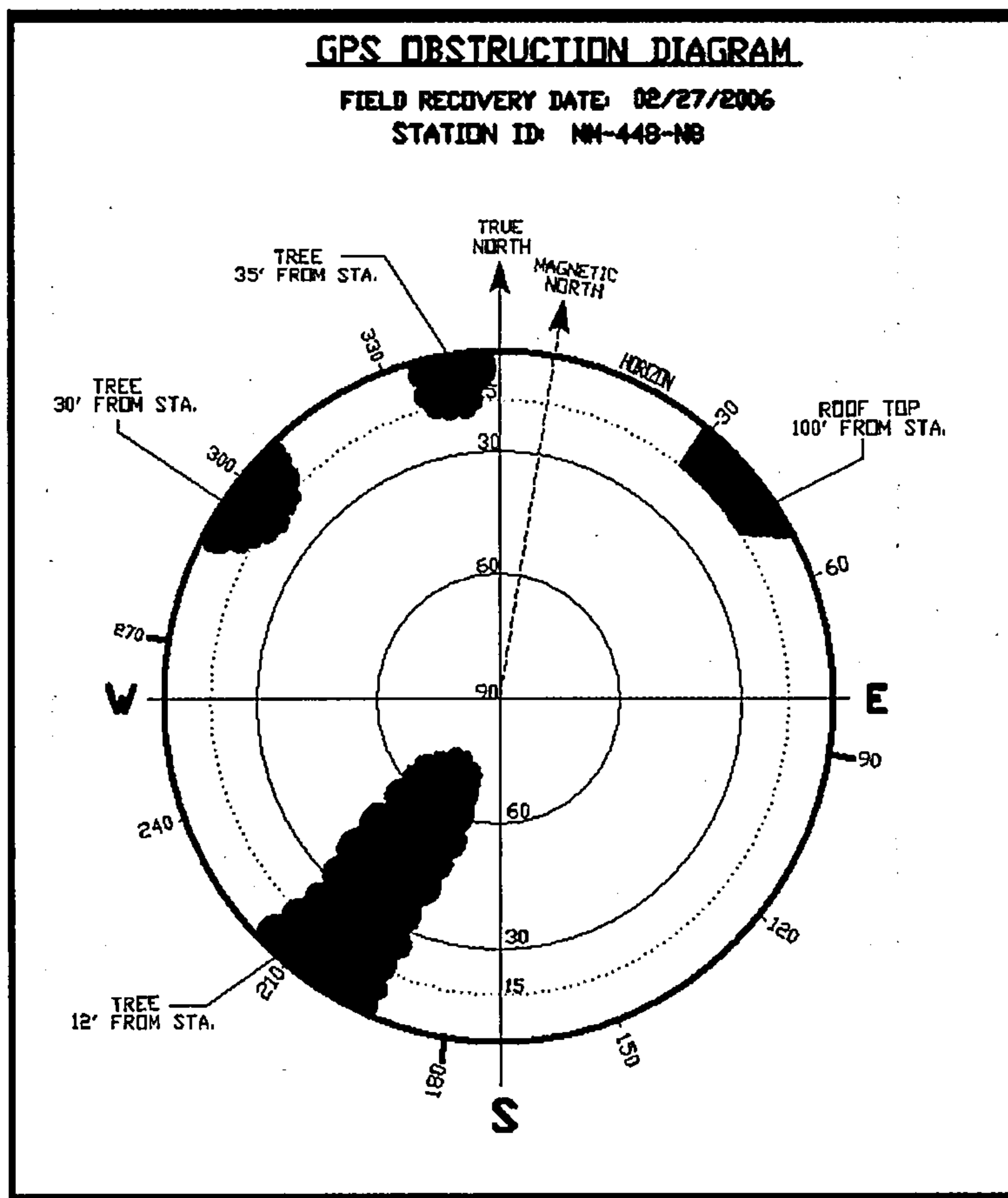


Control Station Data

Station Name: NM_448_N8

Status: Active

City of Albuquerque, New Mexico
Department of Municipal Development
Construction Services Division
Survey Section

**Description:**

The station is located 5.4 miles north of downtown Albuquerque.

To reach the station from the intersection of Montano Road and Coors Boulevard NW travel north on Coors Boulevard 1.05 miles to the station on the right.

The station mark is an NMSHC survey control brass disc set in a concrete post flush with the ground and is stamped "STA NM448-N8".

For more information contact the Geodetic Surveyor Cliff Wilkie at 505-768-3609 or Cwilkie@cabq.gov.

NAD 83 Position**Datum:** NAD 83**Projection:** New Mexico State Plane**Zone:** Central**Latitude:** 35 - 9 - 46.90266**Longitude:** 106 - 40 - 37.51231**Ellipsoidal Height (meters):** 1509.196**Order:** 1 **Class:** 1**NAVD 1988 Elevation****Datum:** NAVD 1988**Orthometric Height (US survey feet):** 5021.651**Order:** 2 **Class:** 1**Ground to Grid Factor:** 0.999682367**Mapping Angle:** -0_14_45.47**Northing (US survey feet):** 1514900.643**Easting (US survey feet):** 1512770.226

Northing (meters): 461742.639

Easting (meters): 461093.287

Azimuth Orientation

No azimuth data available

Capacity of a Single 'A' Storm Drop Inlet

Capacity of the grate:

$$\begin{aligned} L &= 40'' - 2(2''_{\text{ends}}) - 7(\frac{1}{2}''_{\text{middle bars}}) \\ &= 32 \frac{1}{2}'' \\ &= 2.7083' \end{aligned}$$

$$\begin{aligned} W &= 25'' - 13(\frac{1}{2}''_{\text{middle bars}}) \\ &= 18.5'' \\ &= 1.54' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 2.7083' \times 1.54' \\ &= 4.18 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Effective Area} &= 4.18 - 4.18 (0.5_{\text{clogging factor}}) \\ &= 2.09 \text{ ft}^2 \text{ at the grate} \end{aligned}$$

Orifice Equation

$$\begin{aligned} Q &= CA \sqrt{2gH} \\ Q &= 0.6 \times 2.09 \times \sqrt{2 \times 32.2 \times 0.67} \\ Q &= 8.24 \text{ cfs} \end{aligned}$$

Capacity of the Throat:

$$L = 6.50'$$

$$\begin{aligned} H &= 10 \frac{3}{4}'' - 4 \frac{1}{2}'' \\ &= 6 \frac{1}{4}'' \\ &= 0.5208' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 6.50' \times 0.5208' \\ &= 3.39 \text{ ft}^2 \text{ at the throat} \end{aligned}$$

Weir Equation

$$\begin{aligned} Q &= CLH^{(3/2)} \\ Q &= 2.95 \times 3.39 \times 0.67^{(3/2)} \\ Q &= 5.48 \text{ cfs} \end{aligned}$$

Total Capacity:

$$\begin{aligned} Q &= 8.24_{\text{grate}} + 5.48_{\text{throat}} \\ Q &= 13.72 \text{ cfs} \end{aligned}$$

Capacity of a Double 'A' Storm Drop Inlet

Capacity of the grate:

$$\begin{aligned} L &= 80'' - 2(2''_{\text{ends}}) - 14(\frac{1}{2}''_{\text{middle bars}}) - 6''_{\text{center piece}} \\ &= 63'' \\ &= 5.25' \end{aligned}$$

$$\begin{aligned} W &= 25'' - 13(\frac{1}{2}''_{\text{middle bars}}) \\ &= 18.5'' \\ &= 1.54' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 5.25' \times 1.54' \\ &= 8.09 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Effective Area} &= 8.09 - 8.09 (0.5_{\text{clogging factor}}) \\ &= 4.04 \text{ ft}^2 \text{ at the grate} \end{aligned}$$

Orifice Equation

$$\begin{aligned} Q &= CA \sqrt{2gH} \\ Q &= 0.6 \times 4.04 \times \sqrt{2 \times 32.2 \times 0.67} \\ Q &= 15.93 \text{ cfs} \end{aligned}$$

Capacity of the Throat:

$$L = 11.00'$$

$$\begin{aligned} H &= 10 \frac{3}{4}'' - 4 \frac{1}{2}'' \\ &= 6 \frac{1}{4}'' \\ &= 0.5208' \end{aligned}$$

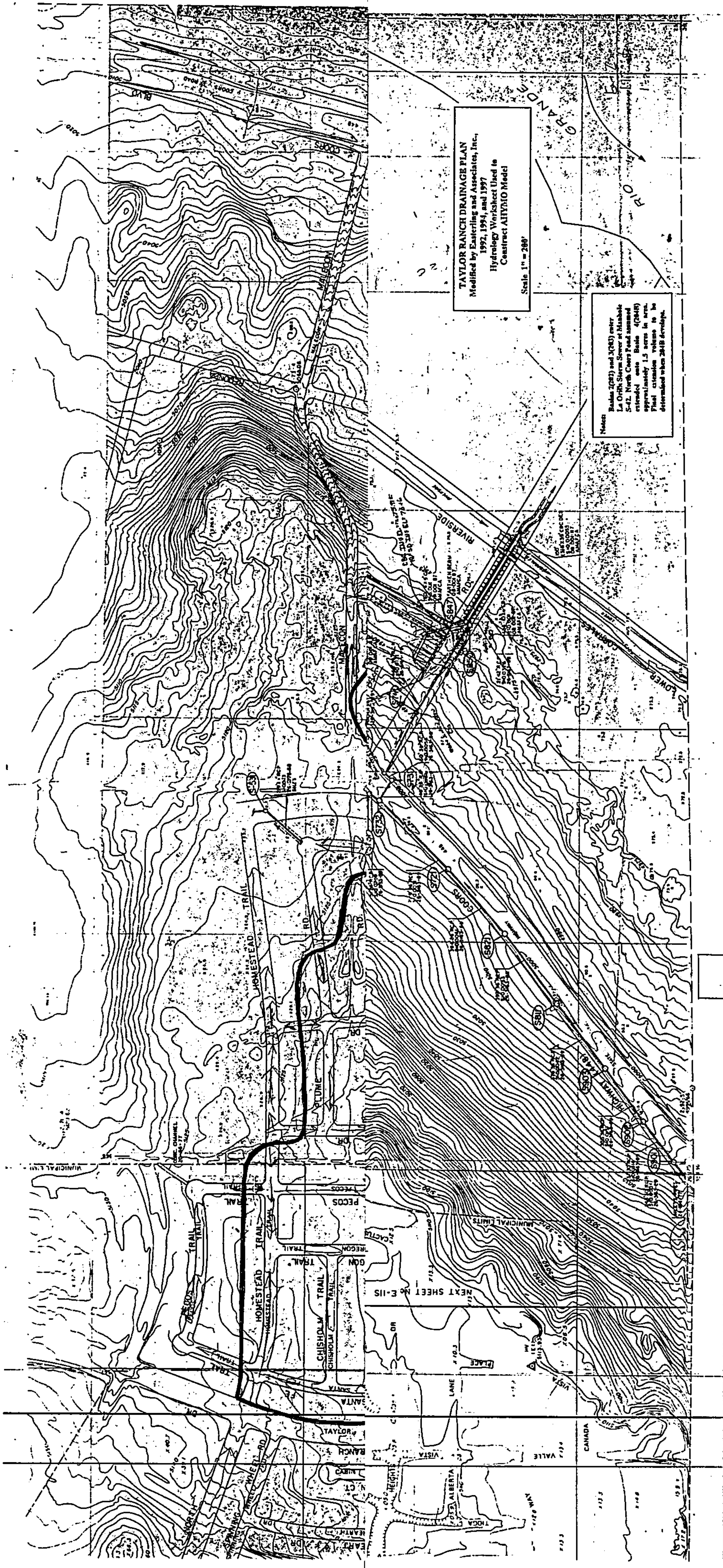
$$\begin{aligned} \text{Area} &= 11.0' \times 0.5208' \\ &= 5.73 \text{ ft}^2 \text{ at the throat} \end{aligned}$$

Weir Equation

$$\begin{aligned} Q &= CLH^{(3/2)} \\ Q &= 2.95 \times 5.73 \times 0.67^{(3/2)} \\ Q &= 9.27 \text{ cfs} \end{aligned}$$

Total Capacity:

$$\begin{aligned} Q &= 15.93_{\text{grate}} + 9.27_{\text{throat}} \\ Q &= 25.20 \text{ cfs} \end{aligned}$$



TAYLOR RANCH DRAINAGE PLAN
Modified by Eastling and Associates, Inc.,
1992, 1994, and 1997
Hydrology Worksheet Used in
Contract AHYMO Model
Scale 1" = 200'

Notes
Basin (2007) and (2008) enter
La Orita Storm Sewer at N. 10th St.
S-42 North Coast Road tunnel
crossed under Basin (2007)
Flow direction is indicated by
arrow. Flow volume is to be
determined when 2008 develops.