# CITY OF ALBUQUERQUE

Planning Department Brennon Williams, Interim Director



August 30, 2019

Robert Fierro, P.E. Fierro & Company 6300 Montano Rd. NW Albuquerque, NM 87120

RE: **Academy Parkway Self-Storage** 

3605 Osuna Rd. NW

**Grading and Drainage Plan** Engineer's Stamp Date: 08/22/19 Hydrology File: E17D001W

Dear Mr. Fierro:

Albuquerque

www.cabq.gov

Based upon the information provided in your submittal received 08/22/2019, the Grading & PO Box 1293

Drainage Plan and Drainage Report are approved for Building Permit.

Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Prior to approval in support of Permanent Release of Occupancy

by Hydrology, Engineer Certification per the DPM checklist will be required.

As a reminder, if the project total area of disturbance (including the staging area and any work NM 87103 within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control

> (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Dough Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior

to any earth disturbance.

Also as a reminder, please provide a Drainage Covenant for the proposed stormwater quality pond per Chapter 17 of the DPM prior to Permanent Release of Occupancy. Please submit this on the 4th floor of Plaza de Sol. A \$25 fee will be required.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology

Renée C. Brissette

Planning Department



## City of Albuquerque

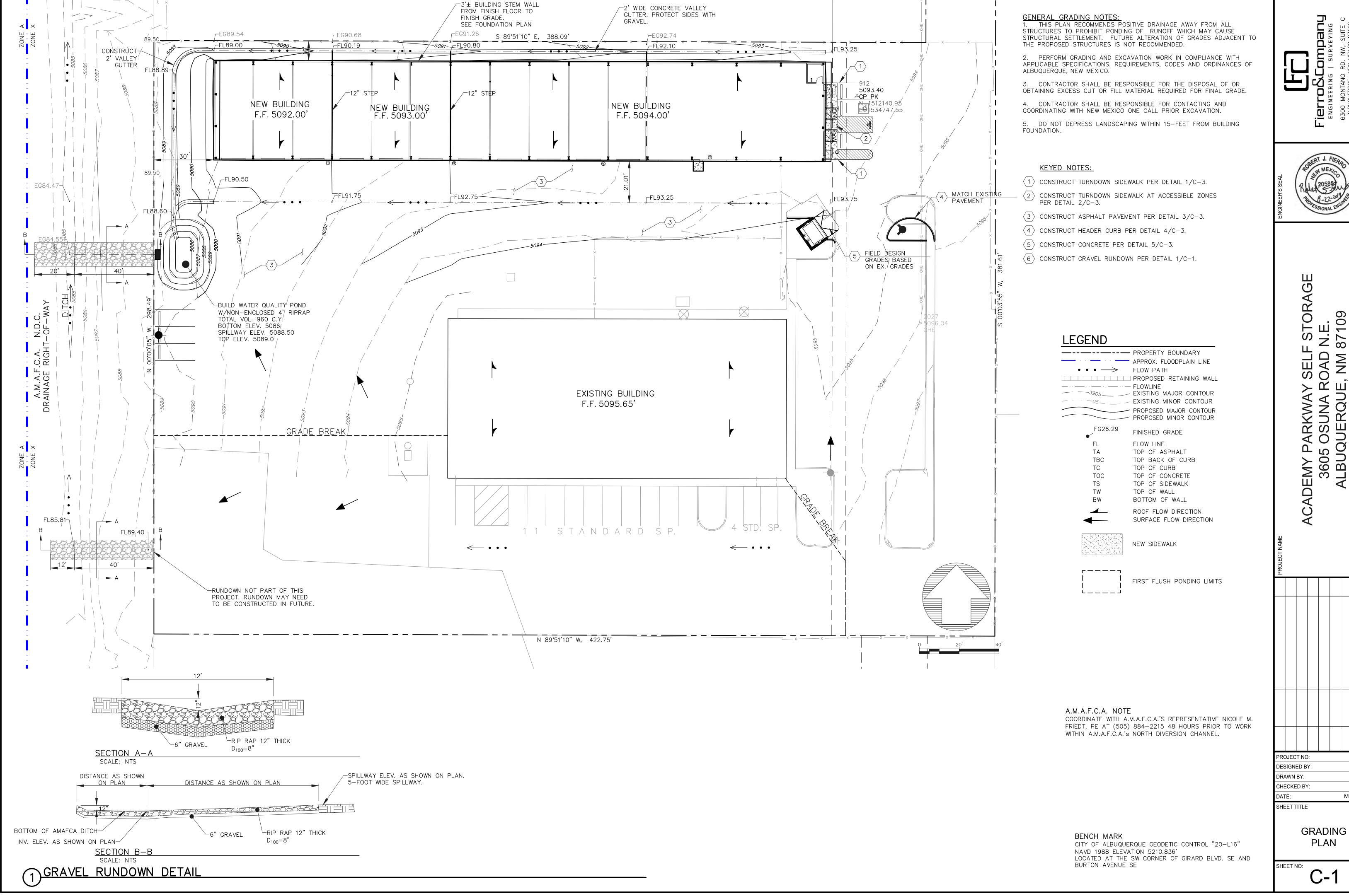
## Planning Department

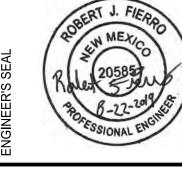
### Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 3/2018)

| Project Title:   | Building Pe  | ermit #: Hydrology File #:   |
|--|--|--|
| PRB#:EPC#:   |  | Work Order#:   |
| Legal Description:   |  |  |
|  |  |  |
| Applicant:   |  | Contact:   |
| Address:   |  |  |
|  |  | E-mail:  |
| Other Contact:   |  | Contact:   |
| Address:   |  |  |
|  |  | E-mail:  |
| Check all that Apply:  |  | IS THIS A RESUBMITTAL?: Yes No   |
| DEPARTMENT:  HYDROLOGY/ DRAINAC TRAFFIC/ TRANSPORTA  TYPE OF SUBMITTAL:  ENGINEER/ARCHITECT OF PAD CERTIFICATION CONCEPTUAL G & D PLACE OF THE PARTY | TION CERTIFICATION IN IN IENT PERMIT APPLIC E LAYOUT (TCL) (TIS) | TYPE OF APPROVAL/ACCEPTANCE SOUGHT:  BUILDING PERMIT APPROVAL  CERTIFICATE OF OCCUPANCY  PRELIMINARY PLAT APPROVAL  SITE PLAN FOR SUB'D APPROVAL  SITE PLAN FOR BLDG. PERMIT APPROVAL  FINAL PLAT APPROVAL  SIA/ RELEASE OF FINANCIAL GUARANTEE  FOUNDATION PERMIT APPROVAL  GRADING PERMIT APPROVAL  SO-19 APPROVAL  PAVING PERMIT APPROVAL  GRADING/ PAD CERTIFICATION  WORK ORDER APPROVAL  CLOMR/LOMR  FLOODPLAIN DEVELOPMENT PERMIT |
| DATE SUBMITTED:  | P <sub>ve</sub>  | OTHER (SPECIFY)  |

FEE PAID:

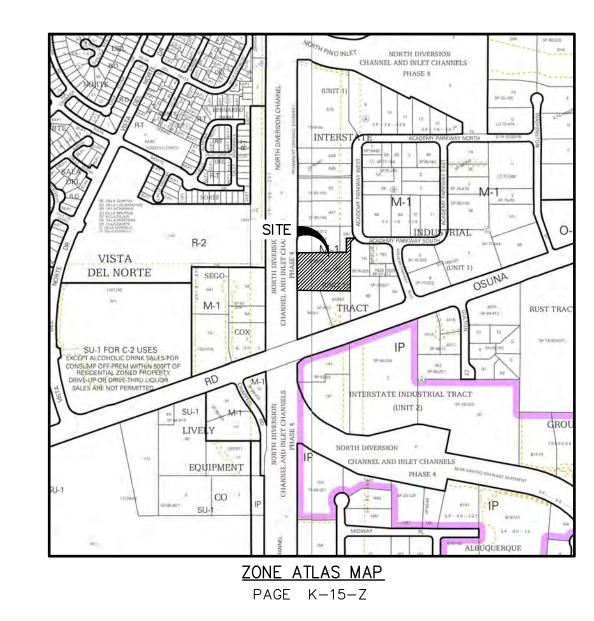




MAY 2019



FLOOD INSURANCE RATE MAP MAP NO. 35001C0136G EFFECTIVE DATE: 09/26/2008



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PARKWAY SELF OSUNA ROAD QUERQUE, NM

S6 ALB

A 15,370 sq.ft. RV storage building is being proposed at 3605 Osuna Rd. NE directly north of the Cox Tire and Automotive building. The proposed building will be built on the same lot as the Cox Tire and Automotive building, which is Lot 6A-1, Block A, Interstate Industrial Tract, Unit 1. The existing site's grading and drainage plan is file E15/D54 as recorded under City of Albuquerque's hydrology department. The purpose of this Grading & Drainage Plan is to 1) provide hydrologic and hydraulic analysis of the existing and proposed condition, 2) satisfy the first flush requirement, 3) seek building approval, and 4)seek approval to perform construction within AMAFCA's right-of-way.

### Methodology

Hydrologic procedures presented in the Hydrology Section of the DMP, Section 22.2, revised April 7, 1993 were followed.

The site currently outlets runoff from two discharge points. Basin 1 discharges via a shallow ditch located along the north property line. Basin 2 discharges as sheet flow into's AMAFCA's Right-of-Way. Both oultets are released into an AMAFCA's ditch. Said AMAFCA's ditch connects to a North Diversion Channel outfall, which is located approximately 550 feet north of the site. The approved Grading & Drainage plan under file E15/D54 allows free discharge into's AMAFCA's right-of-way with it being 100% impervious (Land Treatment D). The site is currently 98% impervious and includes a 13,700 sq.ft. building and asphalt parking lot.

Improvements includes a 15,370 sq.ft. RV storage building and additional parking. The proposed drainage pattern will not alter from the approved grading plan filed under E15/D54. The first flush volume will be retained via a water quality pond. The first flush volume from Basin 1 is calculated to be 754 cubic feet as computed below. The new impervious area includes the RV storage building, new parking, and a 28-foot drive aisle along the self-storage building. The outlet from Basin 1 is proposed to be a gravel rundown as shown in detail 1/C-1. The gravel rundown was originally proposed in the G&D plan under file E15/D54. Currently the gravel rundown does not exist and has not been maintained. Hydrologic and hydraulic analysis is included on this Sheet.

The total area of disturbance is 39, 500 sq.ft., which is less then the 1-acre threshold for an Erosion and Sediment Control Plan and Owner's certified Notice of Intent (NOI).

### Conclusion

The City's requirements have been satisfied under this grading & drainage plan. The contractor shall use these civil sheets for construction, and will need a drainage certification in order to obtain a Close-out. This drainage report seeks approval for building permit and work within AMAFCA's right-of-way.

## LEGEND

| SWALE EXISTING MAJOR CONTOUR EXISTING MINOR CONTOUR   |
|---|
| PROPOSED MAJOR CONTOUR<br>PROPOSED MINOR CONTOUR<br>SURFACE FLOW DIRECTION<br>ROOF FLOW DIRECTION |
| EXISTING BASIN MAP<br>PROPOSED BASIN MAP<br>APPROXIMATE FLOOD PLAIN                               |

| Basin 1 - Existing/Propo | sec | 1         |                 | Basin 2 - Existing/Propo | sec | 1        |                 |
|--------------------------|-----|-----------|-----------------|--------------------------|-----|----------|-----------------|
| Area of Treatment A      | =   | 0.000     | ft <sup>2</sup> | Area of Treatment A      | =   | 0.000    | ft <sup>2</sup> |
|                          |     | 0         | ac              |                          |     | 0        | ac              |
| Area of Treatment B      | =   | 2146.00   | ft <sup>2</sup> | Area of Treatment B      | =   | 515.00   | ft <sup>2</sup> |
|                          |     | 0.049     | ac              |                          |     | 0.012    | ac              |
| Area of Treatment C      | =   | 0.00      | ft <sup>2</sup> | Area of Treatment C      | =   | 0.00     | ft <sup>2</sup> |
|                          |     | 0.000     | ac              |                          |     | 0.000    | ac              |
| Area of Treatment D      | =   | 100675.00 | ft <sup>2</sup> | Area of Treatment D      | =   | 36214.00 | ft <sup>2</sup> |
|                          |     | 2.311     | ac              |                          |     | 0.831    | ac              |
| Total Area               | =   | 102821.00 | ft <sup>2</sup> | Total Area               | =   | 36729.00 | ft <sup>2</sup> |
|                          |     | 2.360     | ac              |                          |     | 0.843    | ac              |
|                          |     | 0.003688  |                 |                          |     | 0.001317 |                 |
| Volumetric Flow          |     |           |                 | Volumetric Flow          |     |          |                 |
| Weighted E               | =   | 2.092     | inches          | Weighted E               | =   | 2.101    | inches          |
| Volume (6hr)             | =   | 0.412     | acre-ft         | Volume (6hr)             | =   | 0.148    | acre-ft         |
| Volume (24hr)            | =   | 0.489     | acre-ft         | Volume (24hr)            | =   | 0.175    | acre-ft         |
| Volume (4days)           |     | 0.594     | acre-ft         | Volume (4days)           | =   | 0.213    | acre-ft         |
| Volume (10days)          | =   | 0.720     | acre-ft         | Volume (10days)          | =   | 0.258    | acre-ft         |
| Peak Rate of Discharge   |     |           |                 | Peak Rate of Discharge   |     |          |                 |
| Q <sub>100</sub>         | =   | 11.0      | cfs             | Q <sub>100</sub>         | =   | 3.934    | cfs             |

(A6) HYDROLOGY CALCULATIONS

## FIRST FLUSH STORAGE:

WATER QUALITY STORAGE NEEDED=26,600 SQ.FT.\*(.34")\*(1'/12")=754 CU.FT.

| Triangular        |              | Highlighted         |         |
|-------------------|--------------|---------------------|---------|
| Side Slopes (z:1) | = 6.00, 6.00 | Depth (ft)          | = 0.66  |
| Total Depth (ft)  | = 1.00       | Q (cfs)             | = 11.00 |
|                   |              | Area (sqft)         | = 2.61  |
| Invert Elev (ft)  | = 100.00     | Velocity (ft/s)     | = 4.21  |
| Slope (%)         | = 4.00       | Wetted Perim (ft)   | = 8.03  |
| N-Value           | = 0.033      | Crit Depth, Yc (ft) | = 0.74  |
|                   |              | Top Width (ft)      | = 7.92  |
| Calculations      |              | EGL (ft)            | = 0.94  |
| Compute by:       | Known Q      |                     |         |
| Known Q (cfs)     | = 11.00      |                     |         |

### DACINI 1 CHANNEL

| Triangular        |              | Highlighted         |        |
|-------------------|--------------|---------------------|--------|
| Side Slopes (z:1) | = 6.00, 6.00 | Depth (ft)          | = 0.45 |
| Total Depth (ft)  | = 1.00       | Q (cfs)             | = 4.00 |
|                   |              | Area (sqft)         | = 1.21 |
| Invert Elev (ft)  | = 100.00     | Velocity (ft/s)     | = 3.29 |
| Slope (%)         | = 4.00       | Wetted Perim (ft)   | = 5.47 |
| N-Value           | = 0.033      | Crit Depth, Yc (ft) | = 0.49 |
|                   |              | Top Width (ft)      | = 5.40 |
| Calculations      |              | EGL (ft)            | = 0.62 |
| Compute by:       | Known Q      | ~ /                 |        |
| Known Q (cfs)     | = 4.00       |                     |        |

(A8) HYDRAULIC CALC. - BASIN 2 CHANNEL

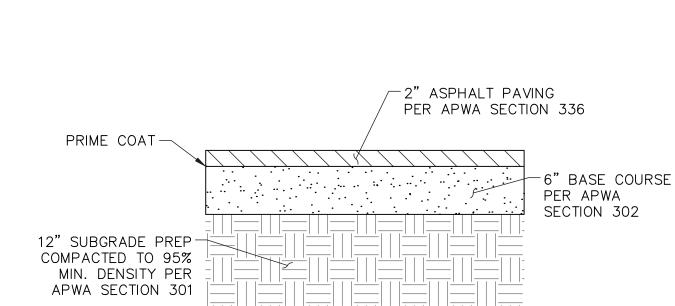
PROJECT NO: **DESIGNED BY:** DRAWN BY: CHECKED BY: MAY 2019 SHEET TITLE DRAINAGE PLAN

C-2

CURB GENERAL NOTES

- 1. ANY DEVIATIONS FROM THESE STANDARDS SHALL BE SUBMITTED TO THE CITY ENGINEER FOR PRIOR APPROVAL
- 2. ALL WORK IN PUBLIC RIGHT-OF-WAY SHALL BE CONSTRUCTED BY A LICENSED CONTRACTOR AND REQUIRES PERMIT AND APPROVAL BY THE DEPT OF PUBLIC WORKS.
- 3. SUBGRADE SHALL BE COMPACTED TO 95% ASTM D 1557, MIN.
- 4. CURB SHALL BE PORTLAND CEMENT CONCRETE. PORTLAND CEMENT CONCRETE SHALL BE 3000 PSI @ 28 DAYS w/CLASS F FLY ASH AND 7% + /- 2% AIR ENTRAINMENT. (MAX 20% FLY ASH BY WEIGHT).
- 5. FOR CONCRETE CURB CONSTRUCT TRANSVERSE JOINTS AS FOLLOWS:
- TOOLED CONTRACTION JOINTS AT 5' INTERVALS. - 1/2" PRE-MOLDED BITUMINOUS EXPANSION JOINTS AT 15' INTERVALS. - SEALED EXPANSION JOINTS AT 90' INTERVALS.
- 6. DIMENSIONS AT ROUNDED CORNERS MEASURED TO INTERSECTION OF STRAIGHT LINES.

HEADER CURB DETAIL SCALE: NTS

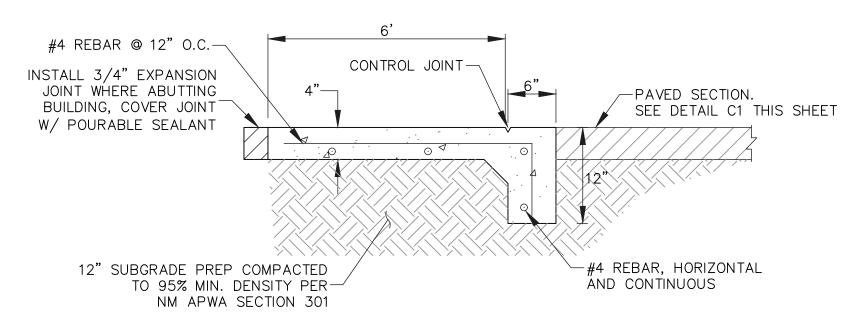


### CONSTRUCTION NOTES

1. PRIOR TO CONSTRUCTION, CONTRACTOR TO OBTAIN PAVEMENT DESIGN FROM A MATERIAL LAB WITH A LICENSED PROFESSIONAL ENGINEER. USE PAVEMENT SECTION RECOMMENDED UNDER PAVEMENT DESIGN FROM MATERIAL LAB. THIS DETAIL IS PROVIDED AS A BASES FOR COST ESTIMATING PURPOSES.

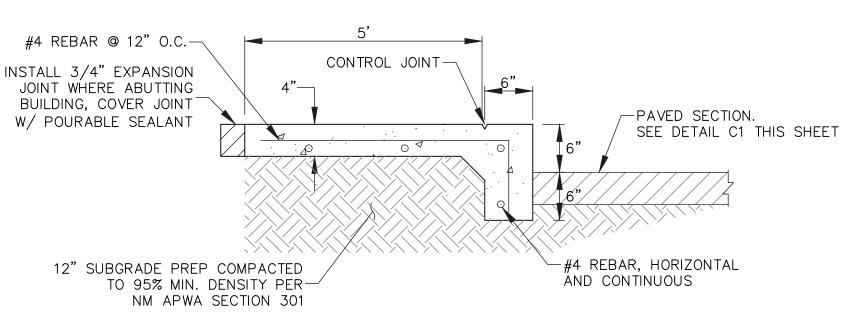
TYPICAL ASPHALT PAVEMENT SECTION SCALE: NTS

EXISTING MATERIAL -



- 1. CONTROL JOINTS SHALL BE PLACED @ 5' O.C.
- 2. EXPANSION JOINTS SHALL BE PLACED @ 20' O.C.
- 3. 4000 PSI CONCRETE W/ BRUSH FINISH.

TURN DOWN SIDEWALK AT ACCESSIBLE ZONES SCALE: 1" = 1'



- 1. CONTROL JOINTS SHALL BE PLACED @ 5' O.C.
- 2. EXPANSION JOINTS SHALL BE PLACED @ 20' O.C.
- 3. 4000 PSI CONCRETE W/ BRUSH FINISH.

TURN DOWN SIDEWALK SCALE: 1" = 1'

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PROJECT NO: DESIGNED BY:

DRAWN BY: CHECKED BY: MAY 2019

SHEET TITLE

**DETAILS** 

@ 18" O.C. EACH WAY 12" SUBGRADE PREP COMPACTED TO 95% MIN. DENSITY -EXISTING MATERIAL

-F'C=4000 PSI @ 28 DAYS PER

NM APWA SECTION 340

FINISH GRADE-

POND OVERFLOW WEIR SECTION

1'- 0" 1'- 0"

6" CONCRETE (4000 PSI) REINFORCED WITH #4 REBAR

TYPICAL 6" PCC SCALE: NTS

**CONSTRUCTION NOTES:** WHEN ABUTTING TO VERICAL WALLS, BENCHES OR BUILDINGS, INSTALL 1/2" BITUMINOUS EXPANSION JOINT. RECESS 1/4" VERTICALLY. INSTALL SIKA-FLEX POLYMER SEALANT OR APPROVED EQUAL.

-NON-ENCLOSED RIPRAP LINING

4" MIN. ROCK DIA. PLACED OVER

NON-WOVEN GEOTEXTILE FILTER FABRIC

- 2. INSTALL CONTRACTION JOINTS @ 6'-0" O.C.
- 3. LIGHT BROOM FINISH CONCRETE SURFACE REQUIRED.

**C-3**