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Jefferson Plaza Office Development
(Jefferson Plaza West of Jefferson St.)

Traffic Impact Study

Draft – June 22, 2007
Final – February 2, 2009

Presented to:

City of Albuquerque
Transportation Development Section

Prepared for:

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Jefferson Office Plaza
(Jefferson Plaza west of Jefferson St)
TRAFFIC IMPACT STUDY

STUDY PURPOSE

The study is being conducted in conjunction with a request for approval of an office development plan for the property located on the north side of Jefferson Plaza west of Jefferson St. The purpose of this study is to identify the impact of the Development on the adjacent transportation system, and to make recommendations to mitigate any significant adverse impact on the adjacent transportation system resulting from the implementation of the facility. This report is being prepared to meet the requirements of the City of Albuquerque Transportation Development Section in association with the development of the proposed project associated with this site plan. (see Appendix Page A-2).

STUDY PROCEDURES

A scoping meeting was held on April 26, 2007 with City of Albuquerque staff (Tony Loyd and John Hartmann) to discuss scope and methodology to be utilized within that report.

The basic procedure followed is described as follows:

- 1) Calculate the generated trips for the proposed office development consisting of a proposed 76,000 S.F. of general office space.
- 2) Calculate trip distribution for the newly generated trips of this development. The new office trips will be distributed based on year 2009 citywide population. (See Pages A-7 thru A-11 in Appendix).
- 3) Determine Trip Assignments for the newly generated trips based on the results of the Trip Distribution Analysis and logical routing to and from the site. (See Pages A-12 thru A-13 in Appendix).
- 4) Obtain AM Peak Hour and PM Peak Hour turning movement traffic counts at the intersection Osuna Rd. / Jefferson St., Singer Rd / Jefferson St., and Jefferson Plaza / Jefferson St (See Pages A-60 thru A-67 in Appendix).
- 5) Calculate Historic Growth Rates for each of the approaches to the intersections targeted for analysis where the historic data was available. (See Pages A-14 thru A-22 in Appendix).
- 6) Determine 2009 NO BUILD intersection volumes by growing the data from the existing traffic counts at the calculated historic growth rate to the analysis year (2009), then add in traffic volumes generated by nearby recently approved undeveloped projects. (See Pages A-23 thru A-37 in Appendix).
- 7) Add in data from Trip Assignments Maps and Tables to the 2009 NO BUILD Volumes to obtain 2009 BUILD Volumes for this project. (See Pages A-23 thru A-37 in Appendix).

- 8) Provide signalized and unsignalized intersection analyses for the following intersections:

INTERSECTION	TYPE CONTROL	NO BUILD	BUILD
Osuna Rd. / Jefferson St.	Traffic Signal	2009	2009
Singer Rd / Jefferson St.	Traffic Signal	2009	2009
Jefferson Plaza / Jefferson St.	Stop Sign	2009	2009
Jefferson Plaza / Driveway "A"	Stop Sign	N/A	2009
Jefferson Plaza / Driveway "B"	Stop Sign	N/A	2009

PREVIOUS RELATED TRAFFIC IMPACT STUDIES

There was one previous related Traffic Impact Study to consider in this study: The Vista del Norte Commercial Development traffic was included in the projected 2009 background traffic volumes utilized in this study. However, it should be noted that the Vista del Norte Commercial Development proposal was denied at the City of Albuquerque Environmental Planning Commission on Thursday, June 14, 2007.

GENERAL AREA CHARACTERISTICS

Surrounding land uses consist of some commercial and mostly office uses. This project is located within a moderately active development area.

AREA STREET NETWORK

Osuna Rd. is classified as a Principal Arterial roadway on the Long Range Roadway Plan for the Albuquerque Metropolitan Area. It is currently a paved urban four-lane facility with raised medians and curbs and gutters on both sides of the street. The posted speed limit on Osuna Rd. from I-25 to 2nd St. is 45 M.P.H.

Jefferson St. is classified as a Minor Arterial roadway on the Long Range Roadway Plan for the Albuquerque Metropolitan Area. It is an urban four lane paved roadway from I-25 north to Alameda Blvd. with raised medians and curbs and gutters on both sides of the street. The posted speed limit on Jefferson St is 35 M.P.H.

Singer Rd. to the west of Jefferson St. is classified as a Collector street on the Long Range Roadway System Map for the Albuquerque Metropolitan Planning Area. It is a four-lane urban roadway with raised medians and curbs & gutters on both sides of the street. The posted speed limit on Singer Rd is 30 M.P.H.

FUTURE C.I.P. IMPROVEMENTS TO TRANSPORTATION SYSTEM

The City of Albuquerque has plans to widen Osuna Rd. from Edith Blvd. to Jefferson St. to provide three thru lanes eastbound and westbound. The project is targeted for construction in 2011. However, funding may limit the scope of what can be constructed at that time. The City of Albuquerque's Ten Year Plan designates \$300,000 for the year 2008 and \$3,000,000 for the year 2011.

EXISTING TRAFFIC VOLUMES

2005 Average Weekday Traffic Volumes (AWDT) for major streets in the site plan area are shown on Page A-3 in the Appendix.

Existing AM and PM peak hour turning movement counts for the years 2004 to 2005 were provided by the City of Albuquerque for the following intersections:

Osuna Rd. / Jefferson St. (2004)
Singer Rd / Jefferson St. (2005)

The existing traffic counts are included Appendix Pages A-61 thru A-67.

Traffic volumes for the intersection of Jefferson Plaza / Jefferson St were recently counted by the consulting engineer performing this study.

EXISTING LEVELS OF SERVICE

The Highway Capacity Manual defines Level of Service (LOS) for signalized intersections in terms of average controlled delay per vehicle as follows:

LOS A	10.0" or less	Most Vehicles do not stop
LOS B	10.1 to 20.0"	Some Vehicles stop
LOS C	20.1 to 35.0"	Significant number of vehicles stop
LOS D	35.1 to 55.0"	Many vehicles stop.
LOS E	55.1 to 80.0"	Limit of acceptable delay.
LOS F	> 80.0"	Unacceptable delay.

Level of Service D is generally considered acceptable in urban areas and is the desirable base condition for analysis in a traffic study. In addition to consideration of the overall level-of-service of the signalized intersection, the levels-of-service of each individual movement should be considered also.

Existing levels-of-service were not provided in this study since the implementation year is only two years from now. The implementation year NO BUILD analysis should approximate the existing levels-of-service.

PROPOSED DEVELOPMENT

The development plan is a proposed 76,000 S.F. office development. The land uses utilized for this analysis should be representative of the type of uses that will result from the proposed development. Should the development occur in such a manner that the actual number of trips generated significantly exceed that projected in this study, the City of Albuquerque may require an updated Traffic Impact Study.

Access is provided into the proposed facility via two full access driveways onto Jefferson Plaza. The proposed driveways are designated as Driveway "A" and Driveway "B" in this study.

TRIP GENERATION

Projected trips were calculated from data in the Institute of Transportation Engineers Trip Generation report (7th Edition, 2003). Trips for the development were determined based on land uses projected to be associated with the zone change request for this property.

The resulting number of trips generated for the proposed development is summarized in the following tables:

Jefferson Office Plaza (Jefferson Plaza West of Jefferson St.) Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME	A. M. PEAK HOUR		P. M. PEAK HOUR		
		GROSS	ENTER	EXIT	ENTER	EXIT
Units						
General Office Building (710)	76.00	1,080	133	18	28	136

(See Page A-6 in the Appendix of this report for Trip Generation Worksheets and Summary Table.)

TRIP DISTRIBUTION

Primary and Diverted Linked Trips:

Trips were distributed as follows:

Office Land Use

Primary and diverted linked trips for residential development have been distributed proportionally to the 2009 projected population of Subareas area wide. Population data for 2005 and 2009 were taken from the 2025 Socioeconomic Forecasts for Data Analysis Subzones for the Mid-Region of New Mexico, S-03-01 (April, 2003), Appendix B, supplied by the Mid-Region Council of Governments (MRCOG). Employment Data was interpolated linearly to obtain 2009 values and adjusted for distance from the proposed new facility. The trip distribution worksheets and associated map of subareas are shown on Appendix Pages A -7 thru A -10.

TRIP ASSIGNMENT

Trip assignments are first made on a percentage basis derived from data established in the trip distribution determination process and logical routing. Those percentages are then applied to the projected trips to determine individual traffic movements. Percentage trip assignments are shown in the Appendix, Pages A -12 thru A -13. No pass-by trip reduction was applied to this development.

BACKGROUND TRAFFIC GROWTH

Background traffic growth rates were considered for each individual approach to an intersection that was targeted for analysis based on data from the 2001, 2002, 2003, 2004, and 2005 Traffic Flow maps prepared by the Mid-Region Council of Governments. Almost all of the Traffic Flow Data for the years 2001 thru 2005 taken from the MRCOG Traffic Flow Maps were Standard Data. The data from those years for each approach was plotted on a graph and a linear "regression trend line" calculated using the equation format $y=mx+b$. The growth rate was determined by calculating the average volume increase per year during the time period considered and dividing that volume into the most recent AWDT used in the analysis from which future volumes will be calculated. The rate of growth of that trend line was utilized as the growth rate for each approach if that calculated rate appeared feasible. However, there were some instances where the rate indicated a negative growth trend. In those cases, an appropriate growth rate from an adjacent segment of the same roadway was used or a shorter time span was used to determine the growth rate, or a generic 3% growth rate was utilized. Due to the potential for growth in the area, it was believed that a zero percent growth rate was inappropriate for this study. Additionally, if the R^2 value of the trend line was low, other means of establishing a probable growth rate from the data accumulated was considered. Historical Growth Rate Graphs with linear regression trendlines are shown on Appendix Pages A-14 thru A-21. Additionally, the growth rate utilized for each approach to an intersection is printed at the top of the Turning Movement sheets for each intersection (Appendix Pages A-23 thru A-34).

PROJECTED PEAK HOUR TURNING MOVEMENTS FOR 2009 BUILDOUT

The calculated annual growth rates were applied to the recent (2004 or 2005) peak hour traffic counts furnished by the City of Albuquerque or counted by the consultant to establish the 2009 background traffic volumes. To these volumes, the generated trips based on implementation of the proposed assumed land uses were added to obtain the 2009 BUILD volumes for the intersection analyses. See Appendix Pages A-23 thru A-34 for further information regarding turning movement counts for this project. 2009 NO BUILD Volumes Map, Trips Generated Map, and 2009 BUILD Volumes Map for this project are on Pages A-35 thru A-37 in the Appendix.

Two scenarios are evaluated in this study:

2009 NO BUILD Volumes
2009 BUILD Volumes

INTERSECTION CAPACITY ANALYSIS

Intersection capacity analyses were performed in accordance with the procedures for signalized and unsignalized intersections in the Highway Capacity Manual, Special Report 209, Transportation Research Board, 2000, using TEAPAC's Signal 2000 Software for signalized intersections and HiCAP Version 2 for unsignalized intersections. For signalized intersections, the operational method of analysis was used for 2009 conditions (NO BUILD and BUILD). In addition to utilizing the operational analysis for the intersections, the 1985 planning method may also be used to provide additional information at the intersection to help define critical lane volumes and to help analyze a solution.

Capacity analyses were performed for the following traffic conditions.

- ⇒ 2009 without development of the subject property (NO BUILD)
- ⇒ 2009 with development (BUILD)

The results of the 2009 NO BUILD and the 2009 BUILD capacity analyses are summarized in the following sections - *Results and Discussion of Intersection Capacity Analyses*.

RESULTS OF SIGNALIZED INTERSECTION CAPACITY ANALYSES

IMPLEMENTATION YEAR (2009)

Intersection #1 - Osuna Rd. / Jefferson St. - Pages A-38 thru A-44

The results of the 2009 implementation year analysis of the signalized intersection of Osuna Rd. / Jefferson St. are summarized in the following table:

Osuna Rd. / Jefferson St.	2009 No Build		2009 BUILD	
	A.M.	P.M.	A.M.	P.M.
Existing Geometry	D - 45.8	<i>F - 125</i>	<i>D - 48.2</i>	<i>F - 129</i>
Exist. Geom. – ADD EB.WB Thru, SB RT Lane		<i>E - 67.5</i>		<i>E - 70.2</i>

D - 38.3 – Bold Italicized LOS / Delay designates that one or more turning movements operate at LOS "E" or worse.

Existing Geometry (Osuna Rd. / Jefferson St.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Osuna Rd.	2	0	2	1	0
WB Osuna Rd.	2	0	2	0	1
NB Jefferson St.	1	0	2	0	1
SB Jefferson St..	1	0	1	1	0

The intersection of Osuna Rd. / Jefferson St. is operating beyond capacity during the projected PM Peak Hour NO BUILD Conditions. Mitigation of the capacity shortfall at the intersection of Osuna Rd. / Jefferson St. consists of construction of a fourth eastbound thru lane on Osuna Rd., a third westbound thru lane on Osuna Rd., and a southbound right turn lane on Jefferson St. It is questionable whether or not sufficient right-of-way exists to construct this level of improvements. The City of Albuquerque should consider a minimum geometry similar to the one proposed for mitigation if at all possible as part of the Osuna widening project to provide capacity at the intersection in the future.

The trips generated by the proposed Jefferson Office Plaza comprises only about 1% of the overall 2009 projected BUILD traffic volumes at the intersection of Osuna Rd. / Edith Blvd.

The Queuing Analysis for this intersection results in the lanes length changes summarized in the following table:

Queueing Analysis Summary Sheet

Project:
Intersection:

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Osuna Rd / Jefferson St

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	2	236	190				2	1,081	Cont	0	154	0
AM NO BUILD Queue	2	299	200				2	1,355	700	0	186	225
AM BUILD Queue	2	299	200				2	1,355	700	0	191	225
<i>Existing Lane Length</i>	2	248	190				2	1,487	Cont	0	81	0
PM NO BUILD Queue	2	331	275				2	1,907	>1,000	0	101	175
PM BUILD Queue	2	331	275				2	1,907	>1,000	0	102	175

Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	2	182	190				2	921	Cont	0	262	999
AM NO BUILD Queue	2	203	150				2	1,101	600	0	292	325
AM BUILD Queue	2	213	150				2	1,101	600	0	292	325
<i>Existing Lane Length</i>	2	88	190				2	1,016	Cont	0	201	999
PM NO BUILD Queue	2	98	100				2	1,256	825	0	224	325
PM BUILD Queue	2	100	125				2	1,256	825	0	224	325

Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	122	150				2	227	Cont	1	42	150
AM NO BUILD Queue	1	161	200				2	296	200	1	55	100
AM BUILD Queue	1	162	200				2	301	200	1	56	100
<i>Existing Lane Length</i>	1	291	150				2	557	Cont	1	264	150
PM NO BUILD Queue	1	384	500				2	727	525	1	345	475
PM BUILD Queue	1	389	500				2	763	550	1	355	475

Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	197	175				2	374	Cont	0	160	0
AM NO BUILD Queue	1	227	275				2	430	275	0	204	250
AM BUILD Queue	1	227	275				2	465	300	0	204	250
<i>Existing Lane Length</i>	1	335	175				2	342	Cont	0	303	0
PM NO BUILD Queue	1	385	500				2	393	325	0	381	500
PM BUILD Queue	1	385	500				2	400	325	0	381	500

AM **PM**
 Cycle Length: 100 130

It is usually acceptable to divide the calculated right turn queue length by 2 to account for right turns on red and right turn overlaps.

Intersection #2 - Singer Rd / Jefferson St - Pages A-45 thru A-49

The results of the 2009 implementation year analysis of the signalized intersection of Singer Rd / Jefferson St are summarized in the following table:

Singer Rd / Jefferson St.	2009 No Build		2009 BUILD	
	A.M.	P.M.	A.M.	P.M.
Existing Geometry	C - 29.0	D - 45.2	C - 29.5	D - 49.7

D - 38.3 – Bold Italicized LOS / Delay designates that one or more turning movements operate at LOS "E" or worse.

Existing Geometry (Singer Rd / Jefferson St)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Singer Rd	1	0	1	0	1
WB Singer Rd	2	0	1	1	0
NB Jefferson St	1	0	2	0	1
SB Jefferson St	1	0	1	1	0

The analysis of the intersection of Singer Rd / Jefferson St indicates that the intersection will operate at satisfactory levels-of-service for all conditions analyzed in this study, and that the implementation of the proposed Jefferson Office Plaza will have minimal impact on the signalized intersection.

The Queuing Analysis for this intersection results in the lanes length changes summarized in the following table:

Queueing Analysis Summary Sheet

Project:
Intersection:

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Singer Blvd / Jefferson St

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	145	175				1	28	Cont	1	201	999
AM NO BUILD Queue	1	162	200				1	31	75	1	225	275
AM BUILD Queue	1	163	200				1	31	75	1	225	275
<i>Existing Lane Length</i>	1	212	175				1	63	Cont	1	428	999
PM NO BUILD Queue	1	237	350				1	71	125	1	479	600
PM BUILD Queue	1	237	350				1	71	125	1	479	600
Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	2	56	175				2	19	Cont	0	21	0
AM NO BUILD Queue	2	63	75				2	21	25	0	24	50
AM BUILD Queue	2	63	75				2	21	25	0	25	50
<i>Existing Lane Length</i>	2	193	175				2	35	Cont	0	53	0
PM NO BUILD Queue	2	216	200				2	39	50	0	59	125
PM BUILD Queue	2	216	200				2	39	50	0	59	125
Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	411	125				2	671	Cont	1	283	280
AM NO BUILD Queue	1	421	450				2	687	400	1	290	325
AM BUILD Queue	1	421	450				2	768	450	1	290	325
<i>Existing Lane Length</i>	1	210	125				2	386	Cont	1	156	280
PM NO BUILD Queue	1	215	325				2	395	325	1	160	250
PM BUILD Queue	1	215	325				2	412	325	1	160	250
Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	34	130				2	387	Cont	0	134	0
AM NO BUILD Queue	1	44	75				2	495	300	0	172	225
AM BUILD Queue	1	44	75				2	506	325	0	172	225
<i>Existing Lane Length</i>	1	28	130				2	593	Cont	0	134	0
PM NO BUILD Queue	1	36	75				2	759	550	0	172	275
PM BUILD Queue	1	37	75				2	842	600	0	173	275

Cycle Length: **AM** **PM**
 100 130

It is usually acceptable to divide the calculated right turn queue length by 2 to account for right turns on red and right turn overlaps.

RESULTS OF UNSIGNALIZED INTERSECTION CAPACITY ANALYSES

IMPLEMENTATION YEAR (2009)

Intersection #3 - Jefferson Plaza / Jefferson St - Pages A-50 thru A-55

The results of the analysis of the unsignalized intersection of Jefferson Plaza / Jefferson St. are summarized in the following table:

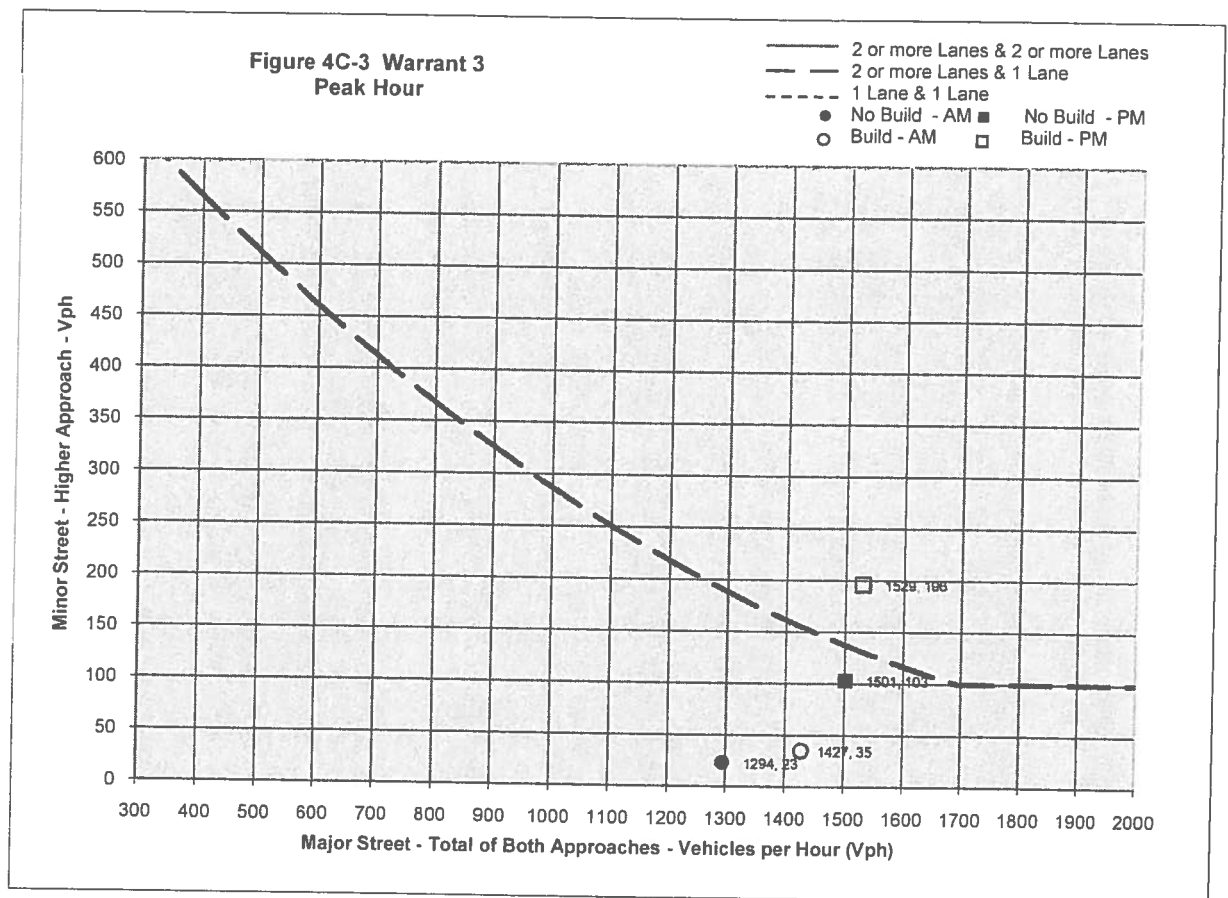
	2009 NO BUILD		2009 BUILD w/Park	
	AM	PM	AM	PM
Jefferson Plaza / Jefferson St.				
Minor Street (Jefferson Plaza)				
EB Left	C - 15.9	D - 27.5	C - 18.9	F - 86.4
EB Thru	C - 15.9	D - 27.5	C - 18.9	F - 86.4
EB Right	C - 15.9	D - 27.5	C - 18.9	F - 86.4
Minor Street (Jefferson Plaza)				
WB Left	C - 19.3	C - 16.7	D - 26.3	C - 18.3
WB Thru	C - 19.3	C - 16.7	D - 26.3	C - 18.3
WB Right	C - 19.3	C - 16.7	D - 26.3	C - 18.3
Major Street (Jefferson St)				
NB Left	A - 9.7	A - 9.3	B - 10.7	A - 9.5
SB Left	A - 8.7	A - 9.8	A - 8.7	A - 9.8

This analysis indicates that the intersection of Jefferson Plaza / Jefferson St. will operate at satisfactory levels-of-service for all conditions analyzed in this study except for the projected 2009 PM Peak Hour BUILD Condition. The long delays expected during the PM Peak Hour BUILD Conditions give cause to evaluate the intersection to see if the warrants are met for a traffic signal based on the Peak Hour Warrant Analysis in the Manual on Uniform Traffic Control Devices. The following graph demonstrates the projected 2009 NO BUILD and BUILD Volumes plotted on the Peak Hour Warrant graph utilizing a 50% reduction for the right turn movements:

Project Name
Jefferson Plaza Office Development
Intersection
Jefferson Plaza / Jefferson St.
Analysis Year
2009

Number of Lanes
Major St. 2
Minor St. 1

Analysis Year Traffic Volumes					
AM	Major	Minor	PM	Major	Minor
No Build	1294	23	No Build	1501	103
Build	1427	35	Build	1529	196



Technically, the preceding graph demonstrates that there will be sufficient volume at the intersection to satisfy the Peak Hour Warrant criteria. However, with a projected delay on the side street of approximately 86 seconds, this study does not recommend a traffic signal at this location. The Peak Hour Warrant is primarily for side streets with excessive delays during the peak hour. This intersection is not characterized by excessive delays on the side street. There are likely other driveways on Jefferson St. in this area with long delays that do not have traffic signals.

Intersection #4 - Jefferson Plaza / Driveway 'A' - Pages A-56 thru A-58

The results of the analysis of the unsignalized intersection of Jefferson Plaza / Driveway 'A'. are summarized in the following table:

	2009 BUILD w/Park	
	AM	PM
Jefferson Plaza / Driveway 'A'.		
Minor Street (Driveway 'A')		
SB Left	B - 10.5	B - 10.5
SB Right	B - 10.5	B - 10.5
Major Street (Jefferson Plaza)		
EB Left	A - 8.0	A - 7.3

This analysis indicates that the intersection of Jefferson Plaza / Driveway 'A' will operate at satisfactory levels-of-service for the all conditions analyzed in this study.

Intersection #5 - Jefferson Plaza / Driveway "B" - Pages A-59 thru A-60a

The results of the analysis of the unsignalized intersection of Jefferson Plaza / Driveway "B" are summarized in the following table:

	2009 BUILD w/Park	
	AM	PM
Jefferson Plaza / Driveway "B"		
Minor Street (Driveway "B")		
SB Left	A - 9.9	A - 9.7
SB Right	A - 9.9	A - 9.7
Major Street (Jefferson Plaza)		
EB Left	A - 7.7	A - 7.3

This analysis indicates that the intersection of Jefferson Plaza / Driveway 'B' will operate at satisfactory levels-of-service for the all conditions analyzed in this study.

It should be noted that Levels of Service (LOS) for unsignalized intersections cannot be compared directly with Levels of Service for signalized intersections. LOS for unsignalized intersections is based on reserve capacity, which is converted to generalized levels of delay; LOS for signalized intersections is based on actual delay in seconds..

LEVEL-OF-SERVICE CRITERIA FOR UNSIGNALIZED INTERSECTIONS

<u>Average Delay</u> <u>(secs)</u>	<u>Level-of-Service</u>
≤ 10	A
> 10 and ≤ 15	B
> 15 and ≤ 25	C
> 25 and ≤ 35	D
> 35 and ≤ 50	E
> 50	F

Generally speaking, a Level-of-Service D or better is an acceptable parameter for design purposes.

CONCLUSIONS

This analysis was conducted using the following methodology: Trip Generation was established using the Institute of Transportation Engineers' (ITE's) Trip Generation Manual (7th Edition). Generated Trips were distributed proportionately based on the Population Data Analysis Subareas citywide. Growth rates of background traffic volumes were established from historical data from 2001 through 2005; and the intersection analyses were performed in accordance with the 2000 Highway Capacity Manual, Special Report 209. The Traffic Impact Study showed a moderate increase in traffic congestion for the adjacent transportation network based on 100% buildout of the proposed project.

There were some capacity shortfalls noted, especially at the intersection of Osuna Rd. / Jefferson St. that should be improved when the City constructs the Osuna Rd. widening project in 2011 to implement a third eastbound and a third westbound thru lane on Osuna Rd. from Edith Blvd. through Jefferson St. The City's improvement plan for the intersection of Osuna Rd. / Jefferson St. should meet the minimum recommendations of this study if at all possible.

In summary, the proposed 2009 development plan for the Jefferson Office Plaza facility at Jefferson Plaza / Jefferson St will present no significant adverse impact to the adjacent transportation system provided that the following recommendations are followed:

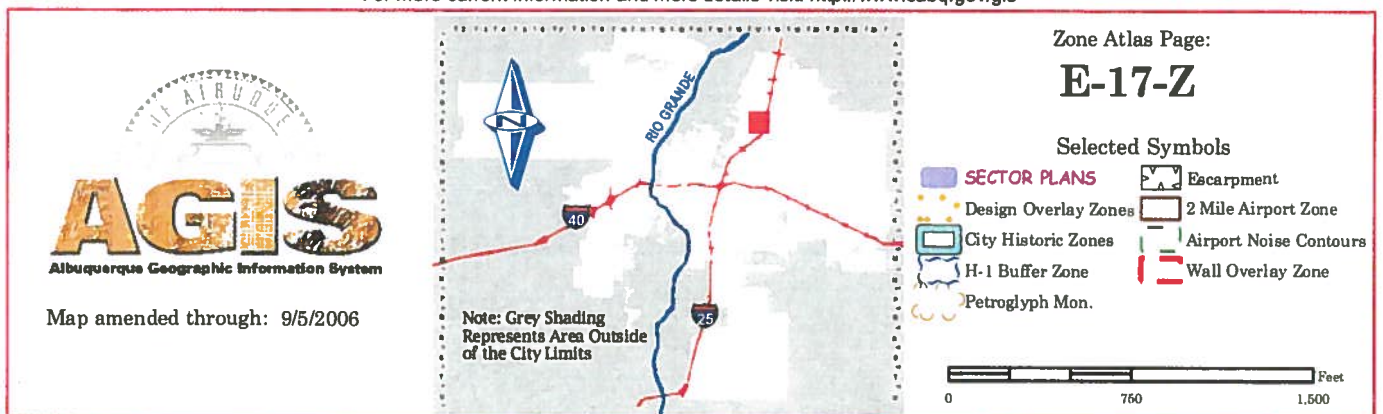
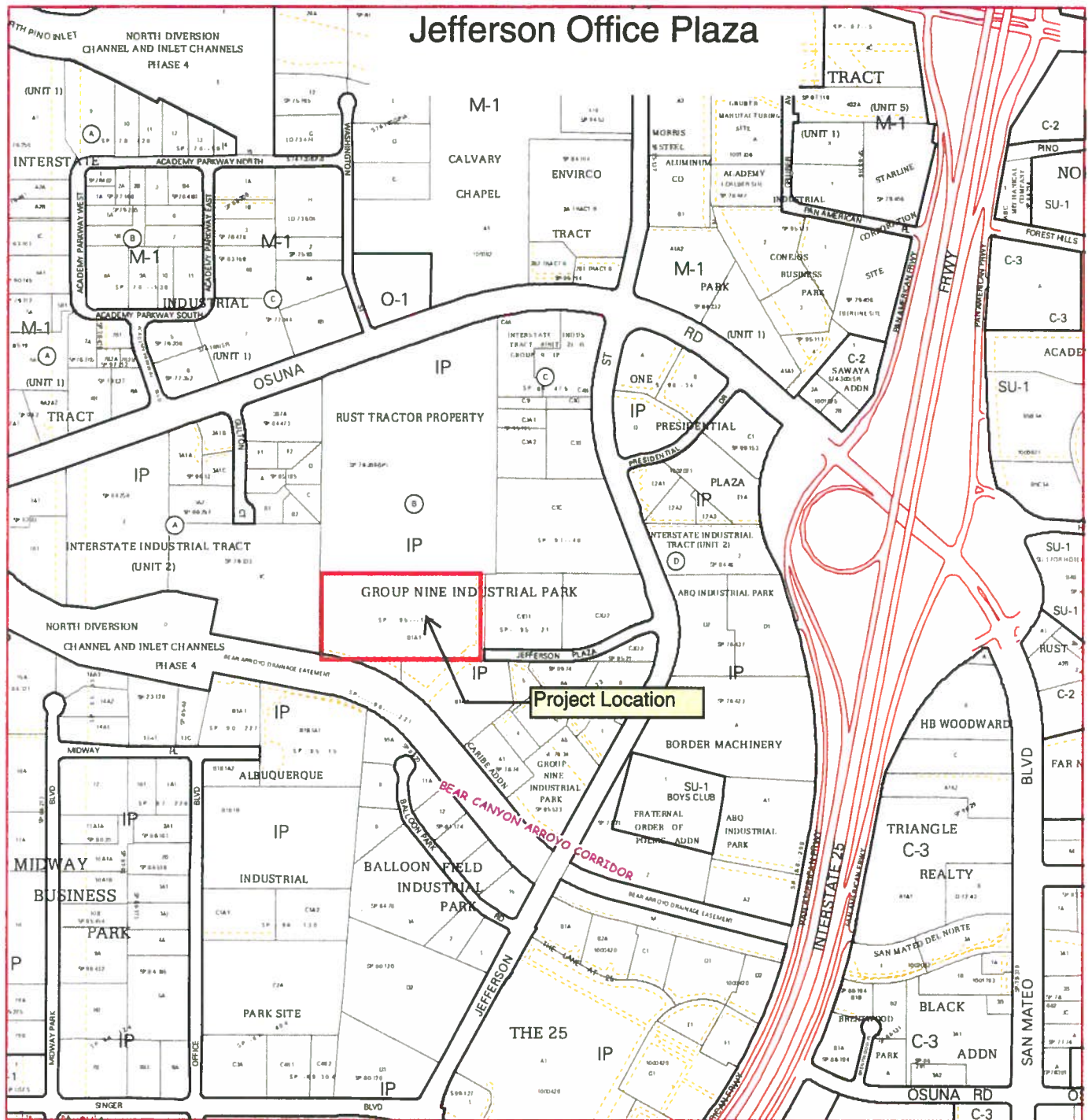
RECOMMENDATIONS

FROM IMPLEMENTATION YEAR (2009) ANALYSIS

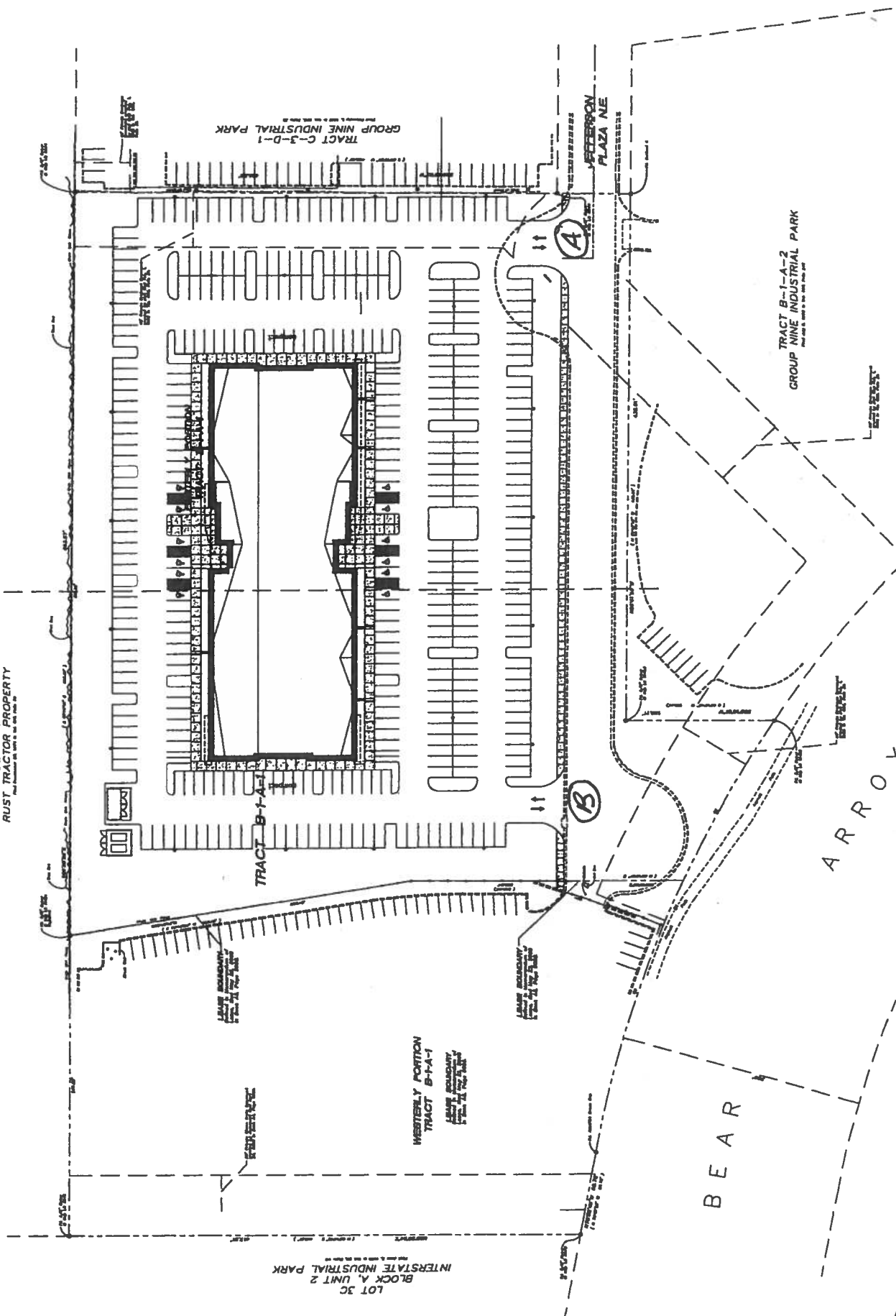
- Design and construction of the proposed development should insure that adequate sight distances are maintained to the extent possible at all proposed driveways and intersections, and at existing intersections contingent to this site.
- **Osuna Rd. / Jefferson St.** – Construct a fourth eastbound thru lane on Osuna Rd. (to create a thru/right turn lane), a third westbound thru lane on Osuna Rd., and a southbound right turn lane on Jefferson St. (See discussion on Page 6). The trips generated by the proposed Jefferson Office Plaza comprises only about 1% of the overall 2009 projected BUILD traffic volumes at the intersection of Osuna Rd. / Jefferson St.
- **Jefferson Plaza / Jefferson St.** – This study does not recommend that a traffic signal be constructed at the intersection of Jefferson Plaza / Jefferson St. However, to mitigate the moderately long delays anticipated for the eastbound traffic on Jefferson Plaza at Jefferson St., it is recommended that the eastbound approach be striped to incorporate an exclusive eastbound left turn lane and a thru / right turn lane if sufficient right-of-way exists. The eastbound left turn lane should be constructed to a length of 100 feet plus transition. The projected eastbound left turn queue length is less than 100 feet.
- **Driveway “A”** – Driveway “A” is recommended to be a full access unsignalized driveway on Jefferson Plaza. Driveway “A” may be constructed with one exiting lane (for left turns and right turns) and one entering lane.
- **Driveway “B”** – Driveway “B” is recommended to be a full access unsignalized driveway on Jefferson Plaza. Driveway “B” may be constructed with one exiting lane (for left turns and right turns) and one entering lane.
- All driveways should be constructed utilizing 25 feet curb returns or larger if needed to accommodate large delivery trucks.
- All driveways shall be designed and constructed to meet the City of Albuquerque Development Process Manual (D.P.M) standards.

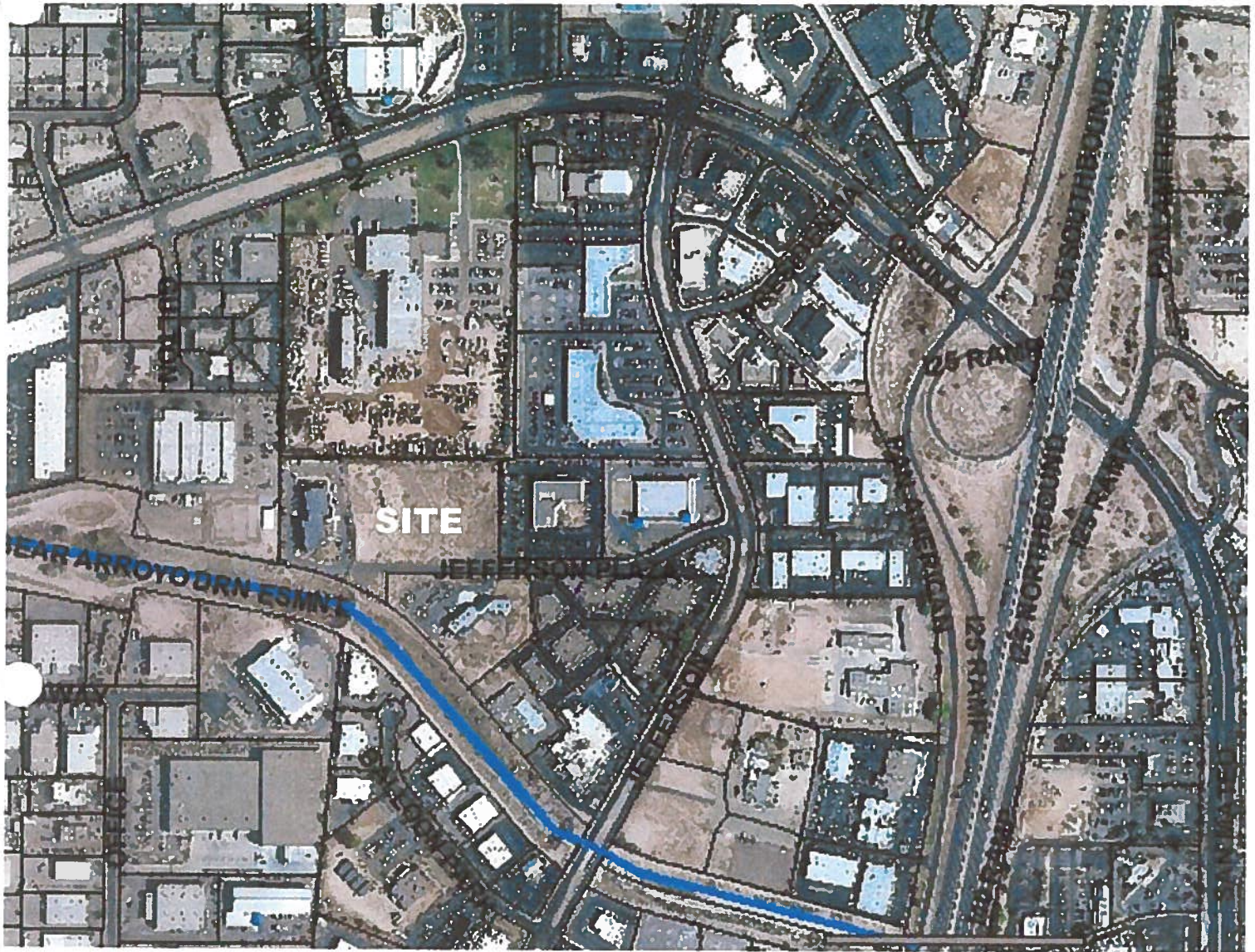
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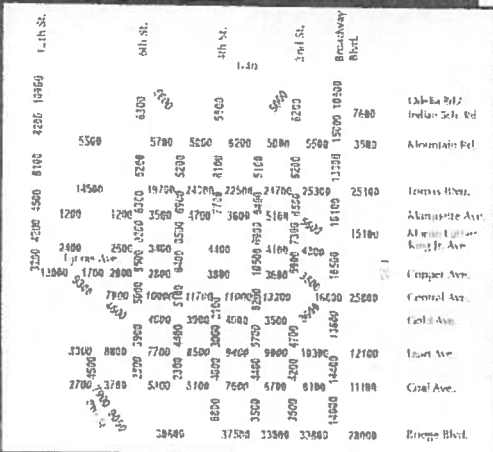
BLOCK B
RUST TRACT PROPERTY



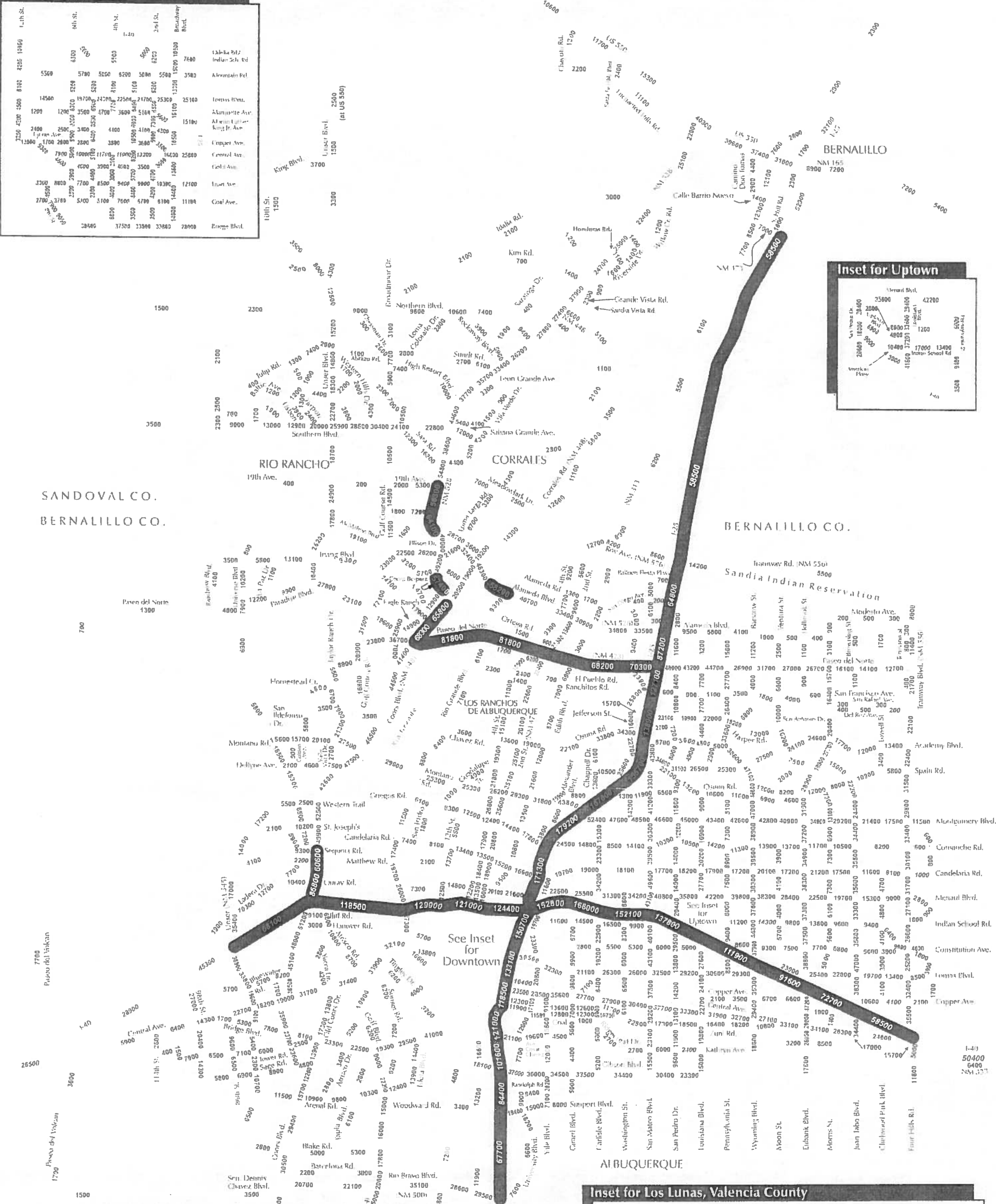
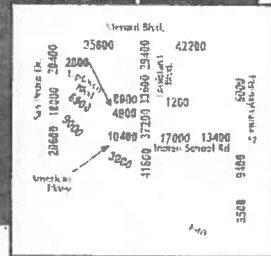


Jefferson Office Plaza
**Jefferson Plaza / Jefferson St
Aerial Photo**

Inset for Downtown



Inset for Uptown



Average Weekday Traffic Flows

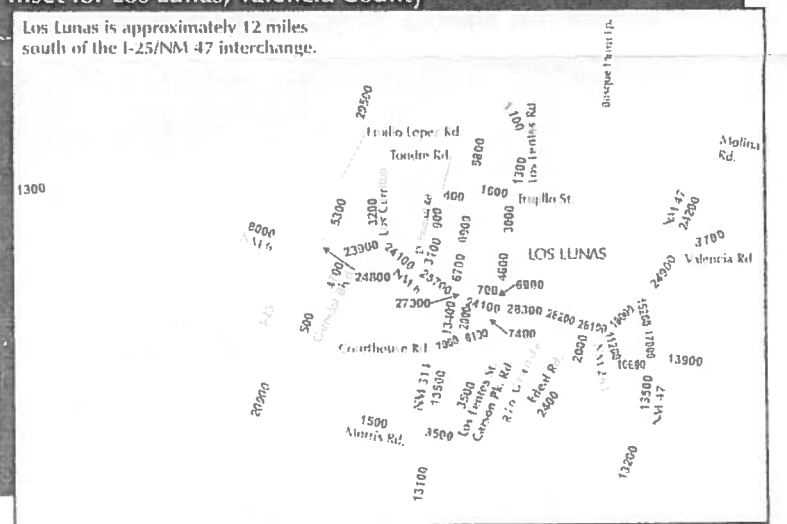
- 0 - 900
- 1000 - 4900
- 5000 - 14900
- 15000 - 24900
- 25000 - 34900
- 35000 - 44900
- 45000 - 54900
- 55000 - 104900

Standard Data
Link Volume is based on traffic count data accepted by the NM Department of Transportation Traffic Monitoring System (TMS) as standard in accordance with the New Mexico State Traffic Monitoring Standards (NMSTMS).

Non-Standard Data
Link Volume is based either on traffic count data not in compliance with the NMSTMS or on professional judgement. NMDOT recommends that nonstandard data be used with caution.

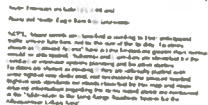
0 1 2 3 4 Miles

Inset for Los Lunas, Valencia County



2005 Traffic Flows for the Greater Albuquerque Area

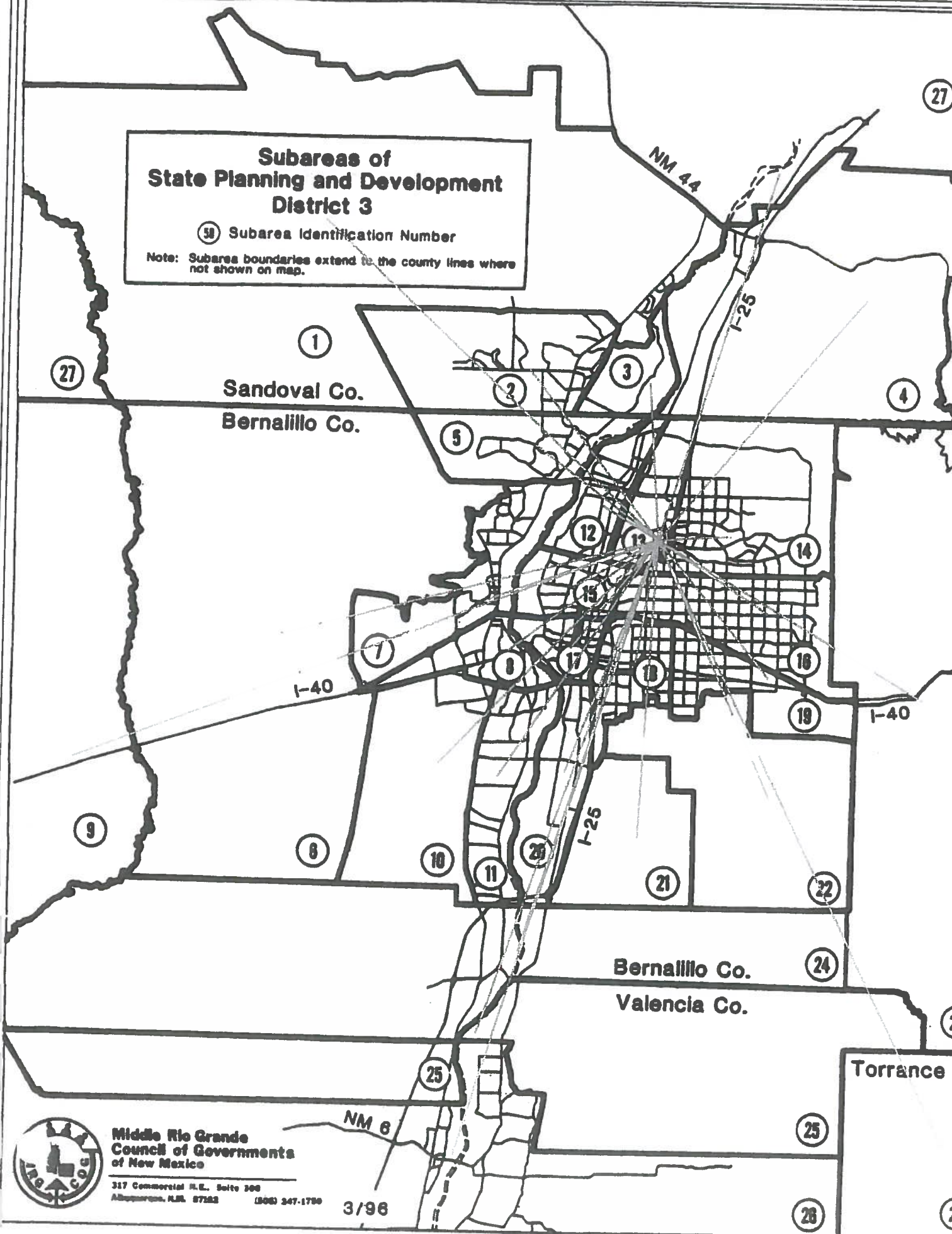
Map prepared by the Mid-Region Council of Governments in cooperation with the New Mexico Department of Transportation, the local governments in the Albuquerque Metropolitan Planning Area, and the U.S. Department of Transportation, Federal Highway Administration.



Subareas of State Planning and Development District 3

③ Subarea Identification Number

Note: Subarea boundaries extend to the county lines where not shown on map.



Middle Rio Grande
Council of Governments
of New Mexico

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3/96

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Trin Distribution Subarea Map

Figure

Trip Distribution Table

Jefferson Office Plaza

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development
2000 and 2025 Data Taken from Mid-Region Council of Governments' 2025 Socioeconomic
2025 Socioeconomic Forecasts by Date Analysis Subzones for the Mid-Region of New Mexico (S-03-01)

Sub Area I.D.#	% Sub Area in Study	2005 Population	2010 Population	Interpolated Population for the Year 2009	Population in Study	Dist. (Mi.)	Population / Distance	% Population / Distance	% Utilizing	(JN)			(OE)			(JS)		
										Jefferson St. North			Owens Rd. East			Jefferson St. South		
		2005	2010	2009						% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing
1	100%	26,972	39,738	37,185	2,009	22.71	88	0.08%	20%	0.02%	18	80%	0.06%	71	0%	0.00%	0	0%
2	100%	39,348	40,810	40,358	37,185	10.52	3,535	3.19%	30%	2.55%	2,828	20%	0.84%	707	0%	0.00%	0	0%
3	100%	7,865	8,728	8,555	8,555	7.71	1,110	1.00%	80%	0.80%	888	20%	0.20%	222	0%	0.00%	0	0%
4	100%	13,387	14,838	14,826	14,826	15.61	937	0.85%	20%	0.17%	187	80%	0.68%	750	0%	0.00%	0	0%
5	100%	35,998	44,203	42,556	42,556	7.57	5,621	5.07%	31%	4.06%	4,497	20%	1.01%	1,124	0%	0.00%	0	0%
6	100%	2,784	3,950	3,717	3,717	19.10	195	0.18%	0%	0.00%	0	10%	0.02%	19	90%	0.16%	175	0%
7	100%	48,565	59,815	57,405	57,405	8.78	6,540	5.90%	0%	0.00%	0	10%	0.59%	654	90%	5.31%	5,886	0%
8	100%	27,546	28,553	28,352	28,352	9.11	3,111	2.81%	0%	0.00%	0	10%	0.28%	311	90%	2.53%	2,800	0%
9	100%	1,878	1,888	1,846	1,846	30.49	61	0.05%	0%	0.00%	0	10%	0.01%	6	90%	0.05%	54	0%
10	100%	39,532	4,822	11,764	11,764	15.75	747	0.67%	0%	0.00%	0	10%	0.07%	75	90%	0.81%	872	0%
11	100%	32,051	33,202	32,972	32,972	13.94	2,366	2.14%	50%	2.22%	2,459	0%	0.21%	237	90%	1.92%	2,129	0%
12	100%	16,144	16,146	16,146	16,146	3.28	4,918	4.44%	50%	4.34%	4,807	0%	0.80%	883	11%	0.97%	1,078	0%
13	100%	8,715	10,146	9,860	9,860	1.01	9,811	8.86%	49%	11.73%	12,997	0%	0.00%	0	50%	11.73%	12,987	0%
14	100%	93,104	94,278	94,044	94,044	3.62	25,993	23.47%	0%	0.00%	0	0%	0.00%	0	100%	4.77%	5,288	0%
15	100%	24,691	25,282	25,148	25,148	4.76	5,286	4.77%	0%	0.00%	0	0%	0.00%	0	100%	14.91%	18,518	0%
16	100%	108,882	108,353	108,459	108,459	6.57	16,618	14.91%	0%	0.00%	0	0%	0.00%	0	100%	2.54%	2,812	0%
17	100%	20,920	21,196	21,141	21,141	6.77	3,124	2.82%	0%	0.00%	0	10%	0.28%	312	90%	5.56%	6,163	0%
18	100%	42,078	41,670	41,752	41,752	6.10	6,848	6.18%	0%	0.00%	0	10%	0.61%	671	90%	5.45%	6,041	0%
19	100%	59,027	58,888	58,916	58,916	8.78	8,713	8.06%	0%	0.00%	0	10%	0.81%	871	90%	0.64%	713	0%
20	100%	8,482	8,988	8,658	8,658	12.19	792	0.71%	0%	0.00%	0	10%	0.07%	79	90%	0.00%	0	0%
21	100%	6	6	6	6	14.67	0	0.00%	0%	0.00%	0	10%	0.00%	0	90%	0.00%	0	0%
22	100%	4,231	3,629	3,749	3,749	13.87	270	0.24%	0%	0.00%	0	10%	0.02%	27	90%	0.22%	243	0%
23	100%	18,140	20,380	19,940	19,940	15.28	1,305	1.18%	0%	0.00%	0	10%	0.12%	131	90%	1.06%	1,175	0%
24	100%	2,393	2,554	2,522	2,522	21.17	118	0.11%	0%	0.00%	0	10%	0.01%	12	90%	0.10%	107	0%
25	100%	1,008	1,082	1,051	1,051	25.66	41	0.04%	0%	0.00%	0	10%	0.00%	4	90%	0.03%	37	0%
26	100%	75,506	85,654	83,624	83,624	31.62	2,644	2.39%	0%	0.00%	0	10%	0.24%	264	90%	2.15%	2,380	0%
27	100%	20,955	22,276	22,012	22,012	18.89	1,165	1.05%	20%	0.21%	233	80%	0.84%	832	0%	0.00%	0	0%
28	100%	19,524	21,690	21,257	21,257	32.90	846	0.68%	0%	0.00%	0	10%	0.08%	85	90%	0.52%	582	0%
29	100%	11,360	13,771	13,289	13,289	49.98	266	0.24%	20%	0.05%	53	80%	0.19%	213	0%	0.00%	0	0%
		811,863	836,916	831,905	793,557		110,772	100.00%		28.15%	28,967		7.63%	8,454		61.25%	87,851	
											26.15%			7.63%			61.25%	

* - Subarea in which the site is located.

Trip Distribution Table

Jefferson Office Plaza

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development

2000 and 2025 Data Taken from Mid-Region Council of Governments' 2025 Socioeconomic

2025 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico (S-03-01)

Sub Area I.D.#	% Sub Area in Study	2005		2010		Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	(SW)			(OW)		
		2005	2010	2005	2010					% Utilizing	% Population / Dist Utilizing	Population	% Utilizing	% Population / Dist Utilizing	Population
1	100%	26,972	39,738			37,185	2,009	22.71	88	0%	0.00%	0	0%	0.00%	0
2	100%	36,348	40,610			40,358	37,185	10.52	3,535	0%	0.00%	0	0%	0.00%	0
3	100%	7,865	8,728			8,555	8,555	7.71	1,110	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,938			14,626	14,626	15.61	937	0%	0.00%	0	0%	0.00%	0
5	100%	35,968	44,203			42,556	42,556	7.57	5,821	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,950			3,717	3,717	18.10	195	0%	0.00%	0	0%	0.00%	0
7	100%	48,585	59,615			57,405	57,405	8.78	6,540	0%	0.00%	0	0%	0.00%	0
8	100%	27,546	28,553			28,352	28,352	9.11	3,111	0%	0.00%	0	0%	0.00%	0
9	100%	1,678	1,888			1,846	1,846	30.49	61	0%	0.00%	0	0%	0.00%	0
10	100%	39,532	4,822			11,764	11,764	15.75	747	0%	0.00%	0	0%	0.00%	0
11	100%	32,051	33,202			32,972	32,972	13.94	2,386	0%	0.00%	0	0%	0.00%	0
12	100%	16,144	16,146			16,146	16,146	3.28	4,918	0%	0.00%	0	0%	0.00%	0
13	100%	8,715	10,146			9,860	9,860	1.01	9,811	0%	0.53%	589	0%	2.22%	2,459
14	100%	93,104	94,279			94,044	94,044	3.82	25,993	0%	0.00%	0	0%	0.00%	0
15	100%	24,691	25,282			25,148	25,148	4.76	5,286	0%	0.00%	0	0%	0.00%	0
16	100%	108,882	108,353			108,459	108,459	6.57	16,518	0%	0.00%	0	0%	0.00%	0
17	100%	20,920	21,196			21,141	21,141	6.77	3,124	0%	0.00%	0	0%	0.00%	0
18	100%	42,078	41,670			41,752	41,752	8.10	6,848	0%	0.00%	0	0%	0.00%	0
19	100%	59,027	58,888			58,916	58,916	8.78	6,713	0%	0.00%	0	0%	0.00%	0
20	100%	9,482	9,699			9,656	9,656	12.19	792	0%	0.00%	0	0%	0.00%	0
21	100%	6	6			6	6	14.67	0	0%	0.00%	0	0%	0.00%	0
22	100%	4,231	3,629			3,749	3,749	13.87	270	0%	0.00%	0	0%	0.00%	0
23	100%	18,140	20,390			19,940	19,940	15.28	1,305	0%	0.00%	0	0%	0.00%	0
24	100%	2,393	2,554			2,522	2,522	21.17	119	0%	0.00%	0	0%	0.00%	0
25	100%	1,009	1,062			1,051	1,051	25.66	41	0%	0.00%	0	0%	0.00%	0
26	100%	75,506	85,654			83,624	83,624	31.82	2,844	0%	0.00%	0	0%	0.00%	0
27	100%	20,955	22,276			22,012	22,012	18.99	1,165	0%	0.00%	0	0%	0.00%	0
28	100%	19,524	21,690			21,257	21,257	32.90	646	0%	0.00%	0	0%	0.00%	0
29	100%	11,360	13,771			13,289	13,289	49.98	266	0%	0.00%	0	0%	0.00%	0
		811,863	836,916			831,905	793,557	110,772			0.53%	589		3.64%	4,029
												0.53%			3,64%

* - Subarea in which the site is located.

Trip Distribution Table

Jefferson Office Plaza

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development

2000 and 2025 Data Taken from Mid-Region Council of Governments' 2025 Socioeconomic

2025 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico (S-03-01)

Sub Area I.D.#	% Sub Area in Study	2005 Population	2010 Population	Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	(JC)			(SE)		
								% Utilizing	% Population / Dist Utilizing	Population	% Utilizing	% Population / Dist Utilizing	Population
1	100%	28,972	39,738	37,185	2,009	22.71	88	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,810	40,358	37,185	10.52	3,535	0%	0.00%	0	0%	0.00%	0
3	100%	7,865	8,728	8,555	8,555	7.71	1,110	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,938	14,826	14,826	15.61	937	0%	0.00%	0	0%	0.00%	0
5	100%	35,868	44,203	42,558	42,558	7.57	5,621	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,950	3,717	3,717	19.10	195	0%	0.00%	0	0%	0.00%	0
7	100%	48,565	59,815	57,405	57,405	8.78	6,540	0%	0.00%	0	0%	0.00%	0
8	100%	27,546	28,553	28,352	28,352	9.11	3,111	0%	0.00%	0	0%	0.00%	0
9	100%	1,678	1,888	1,846	1,846	30.49	61	0%	0.00%	0	0%	0.00%	0
10	100%	39,532	4,822	11,784	11,784	15.75	747	0%	0.00%	0	0%	0.00%	0
11	100%	32,051	33,202	32,972	32,972	13.94	2,366	0%	0.00%	0	0%	0.00%	0
12	100%	16,144	16,148	16,146	16,146	3.28	4,918	0%	0.00%	0	0%	0.00%	0
13*	100%	8,715	10,148	9,880	9,880	1.01	9,811	4%	0.35%	392	5%	0.44%	491
14	100%	93,104	94,279	94,044	94,044	3.62	25,993	0%	0.00%	0	0%	0.00%	0
15	100%	24,691	25,262	25,148	25,148	4.78	5,286	0%	0.00%	0	0%	0.00%	0
16	100%	108,882	108,353	108,459	108,459	6.57	16,618	0%	0.00%	0	0%	0.00%	0
17	100%	20,820	21,196	21,141	21,141	6.77	3,124	0%	0.00%	0	0%	0.00%	0
18	100%	42,078	41,670	41,752	41,752	6.10	6,848	0%	0.00%	0	0%	0.00%	0
19	100%	59,027	58,888	58,916	58,916	8.78	6,713	0%	0.00%	0	0%	0.00%	0
20	100%	9,482	9,699	9,656	9,656	12.19	792	0%	0.00%	0	0%	0.00%	0
21	100%	6	6	6	6	14.87	0	0%	0.00%	0	0%	0.00%	0
22	100%	4,231	3,829	3,749	3,749	13.87	270	0%	0.00%	0	0%	0.00%	0
23	100%	18,140	20,390	19,940	19,940	15.28	1,305	0%	0.00%	0	0%	0.00%	0
24	100%	2,393	2,554	2,522	2,522	21.17	119	0%	0.00%	0	0%	0.00%	0
25	100%	1,009	1,062	1,051	1,051	25.86	41	0%	0.00%	0	0%	0.00%	0
26	100%	75,506	85,654	83,824	83,824	31.82	2,644	0%	0.00%	0	0%	0.00%	0
27	100%	20,955	22,276	22,012	22,012	18.89	1,165	0%	0.00%	0	0%	0.00%	0
28	100%	19,524	21,690	21,257	21,257	32.90	646	0%	0.00%	0	0%	0.00%	0
29	100%	11,360	13,771	13,289	13,289	49.98	266	5%	0.35%	392	0%	0.00%	0
		811,863	836,916	831,905	793,557	110,772				392			491
										0.35%		0.44%	0.44%

* - Subarea in which the site is located.

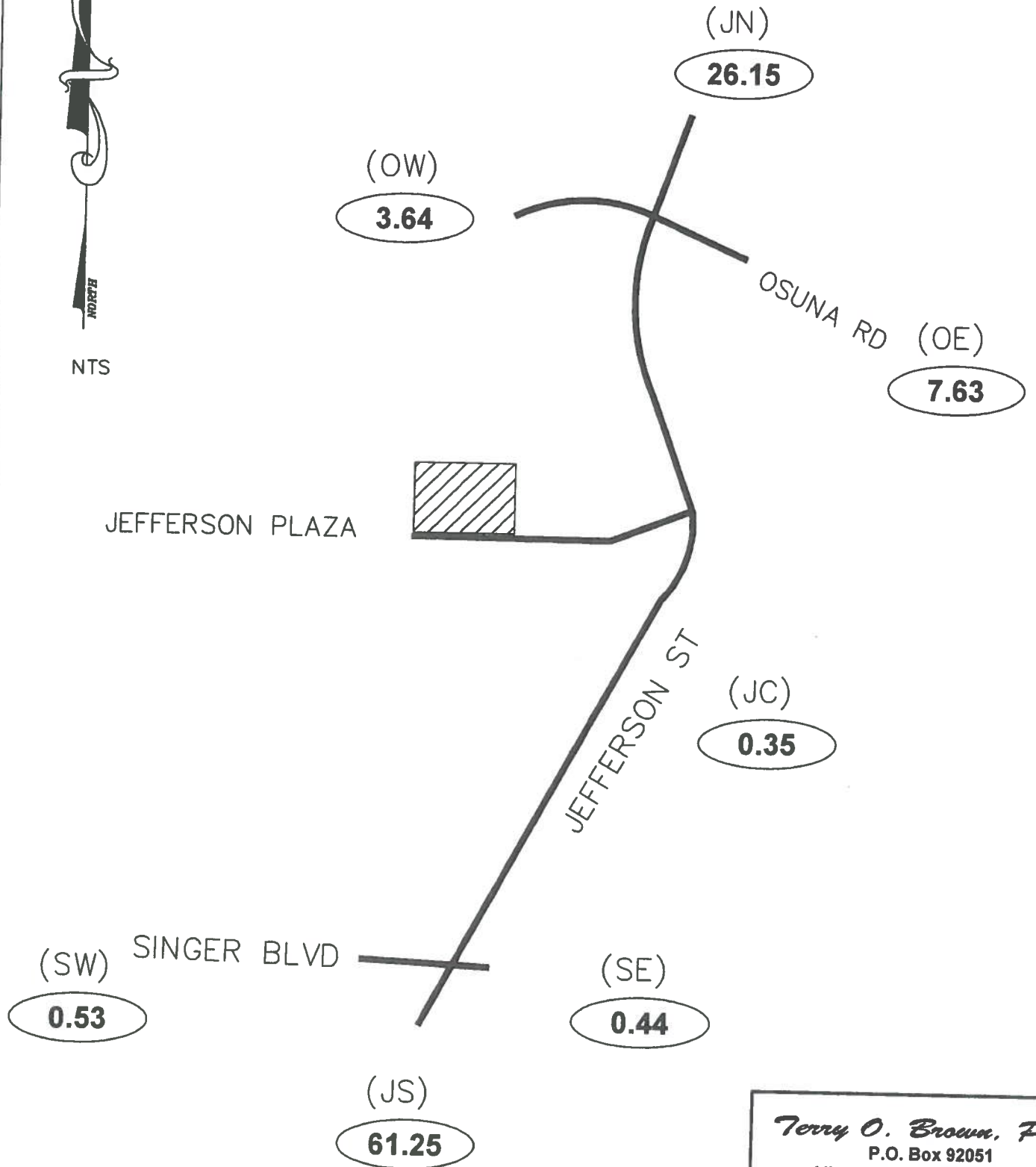
Jefferson Office Plaza

(Jefferson St / Jefferson Plaza)

Trip Distribution Map (%)



NTS



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A-11

Jefferson Office Plaza

(Jefferson St / Jefferson Plaza)

Trip Assignments (% Entering)



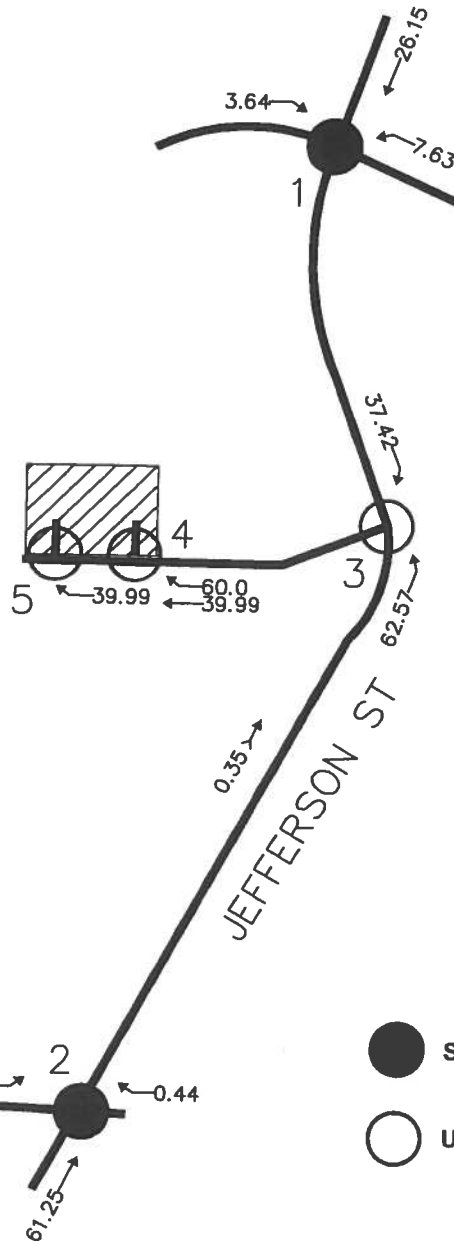
NTS

JEFFERSON PLAZA

SINGER BLVD

OSUNA RD

JEFFERSON ST



SIGNALIZED INTERSECTION



UNSIGNALIZED INTERSECTION

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Jefferson Office Plaza

(Jefferson St / Jefferson Plaza)

Trip Assignments (% Exiting)



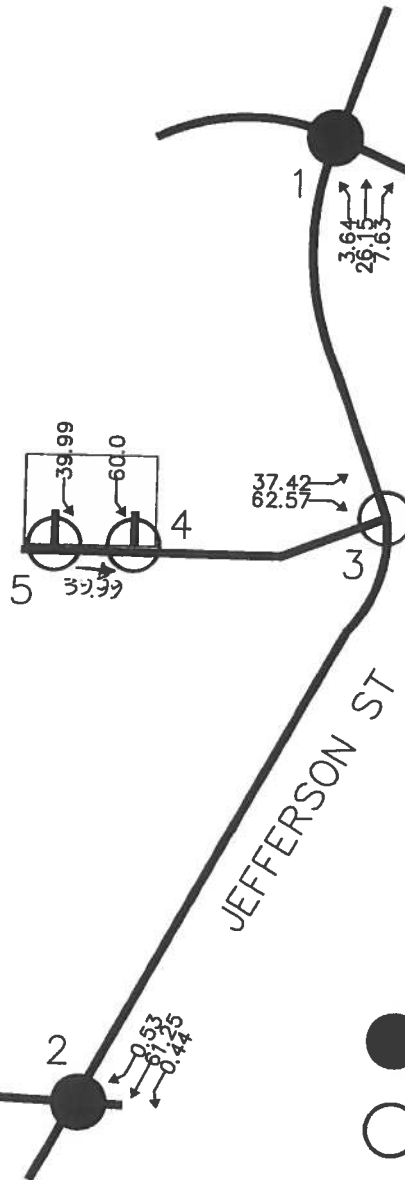
NTS

JEFFERSON PLAZA

SINGER BLVD

OSUNA RD

JEFFERSON ST



SIGNALIZED INTERSECTION



UNSIGNALIZED INTERSECTION

Terry O. Brown, P.E.

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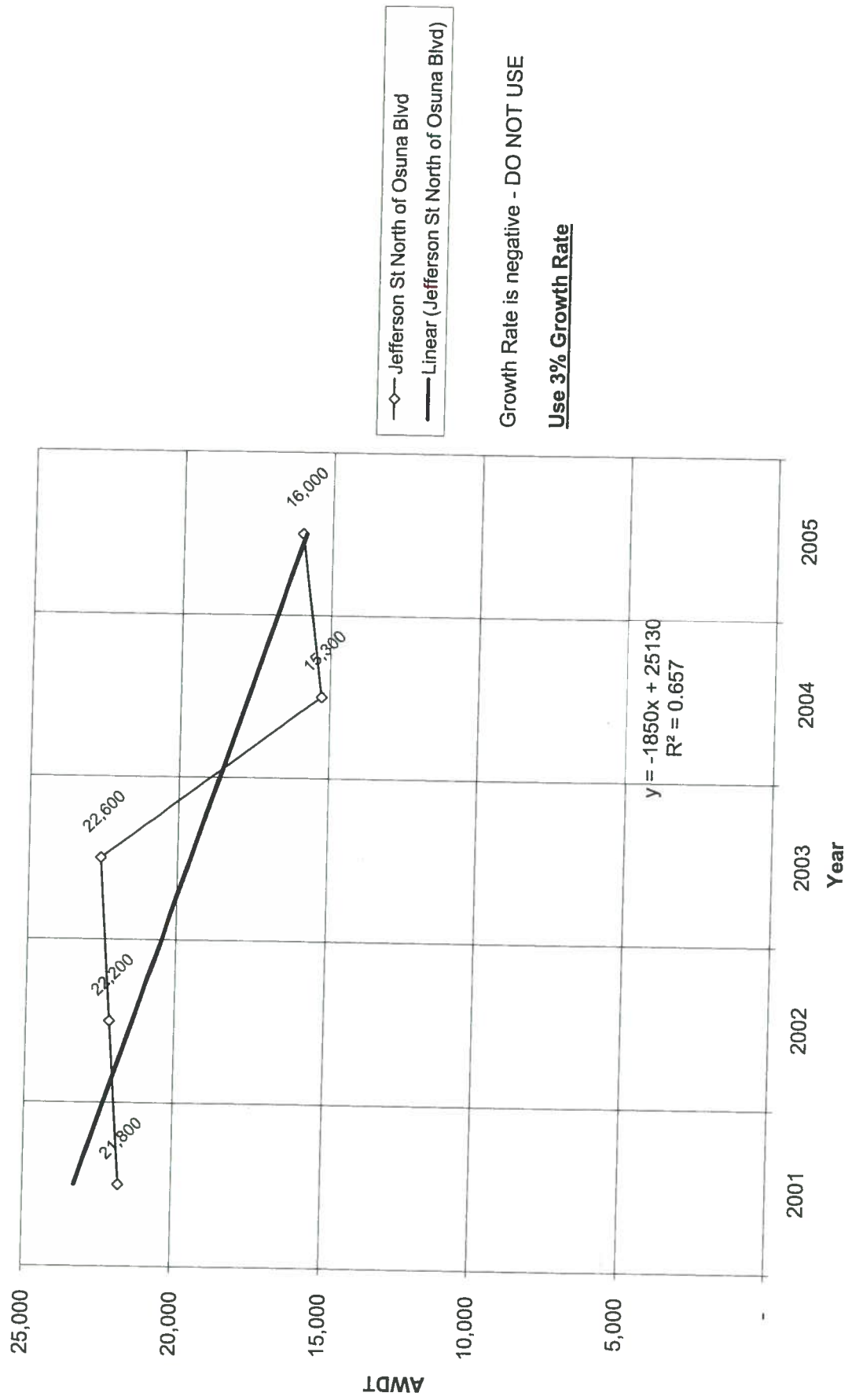


Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Historic Growth Rate Table

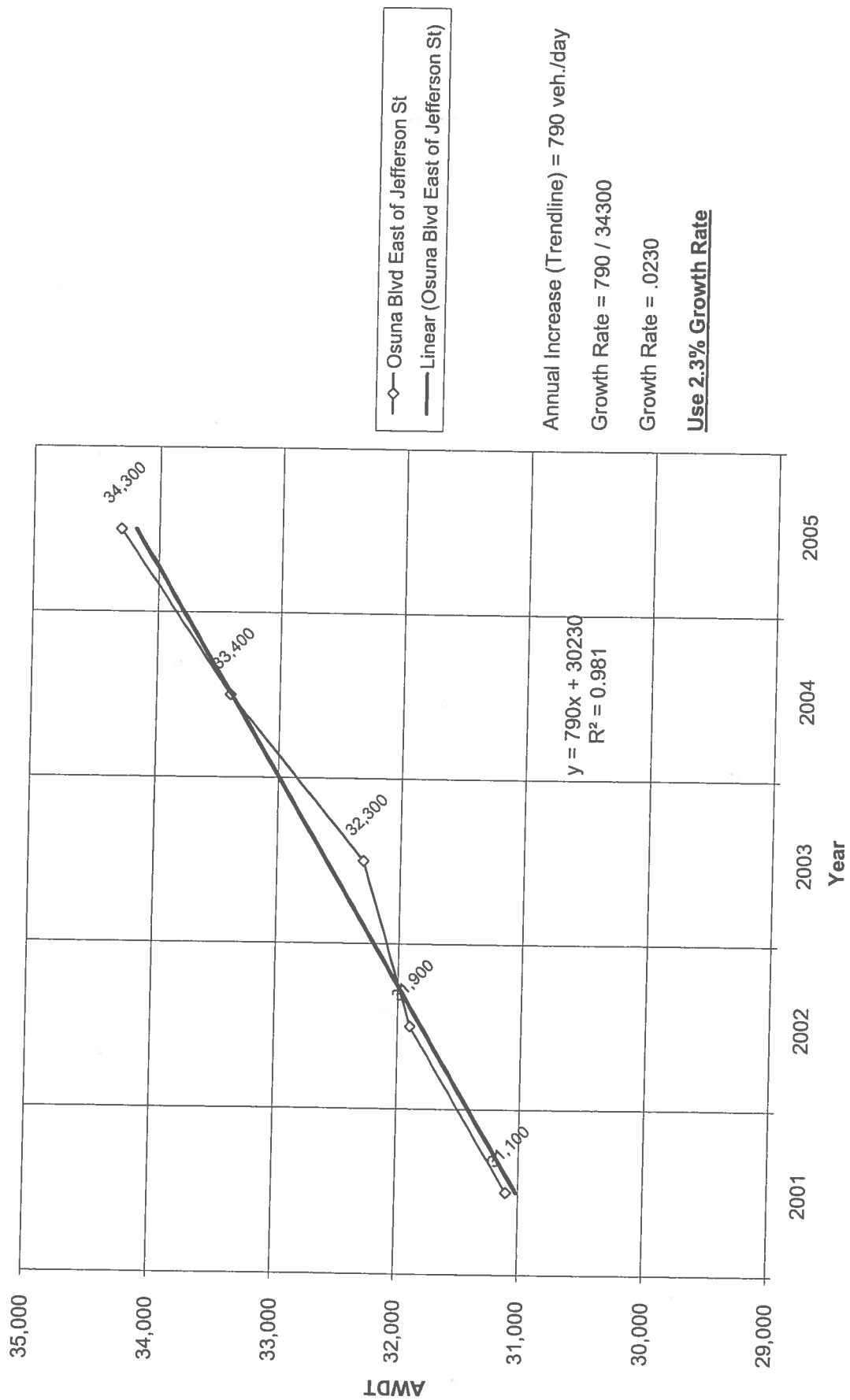
Traffic Flows from MRCOG Map

	2001	2002	2003	2004	2005
Jefferson St North of Osuna Blvd	21,800	22,200	22,600	15,300	16,000
Osuna Blvd East of Jefferson St	31,100	31,900	32,300	33,400	34,300
Jefferson St South of Osuna Blvd	16,100	15,000	15,200	15,700	22,700
Jefferson St North of Singer Rd	19,200	19,600	19,900	24,800	25,600
Singer Rd West of Jefferson St	-	-	10,700	10,600	10,500
Osuna Blvd West of Jefferson St	28,700	29,300	29,700	32,300	33,800
Jefferson St South of Singer Rd	13,800	14,000	14,200	14,100	14,200

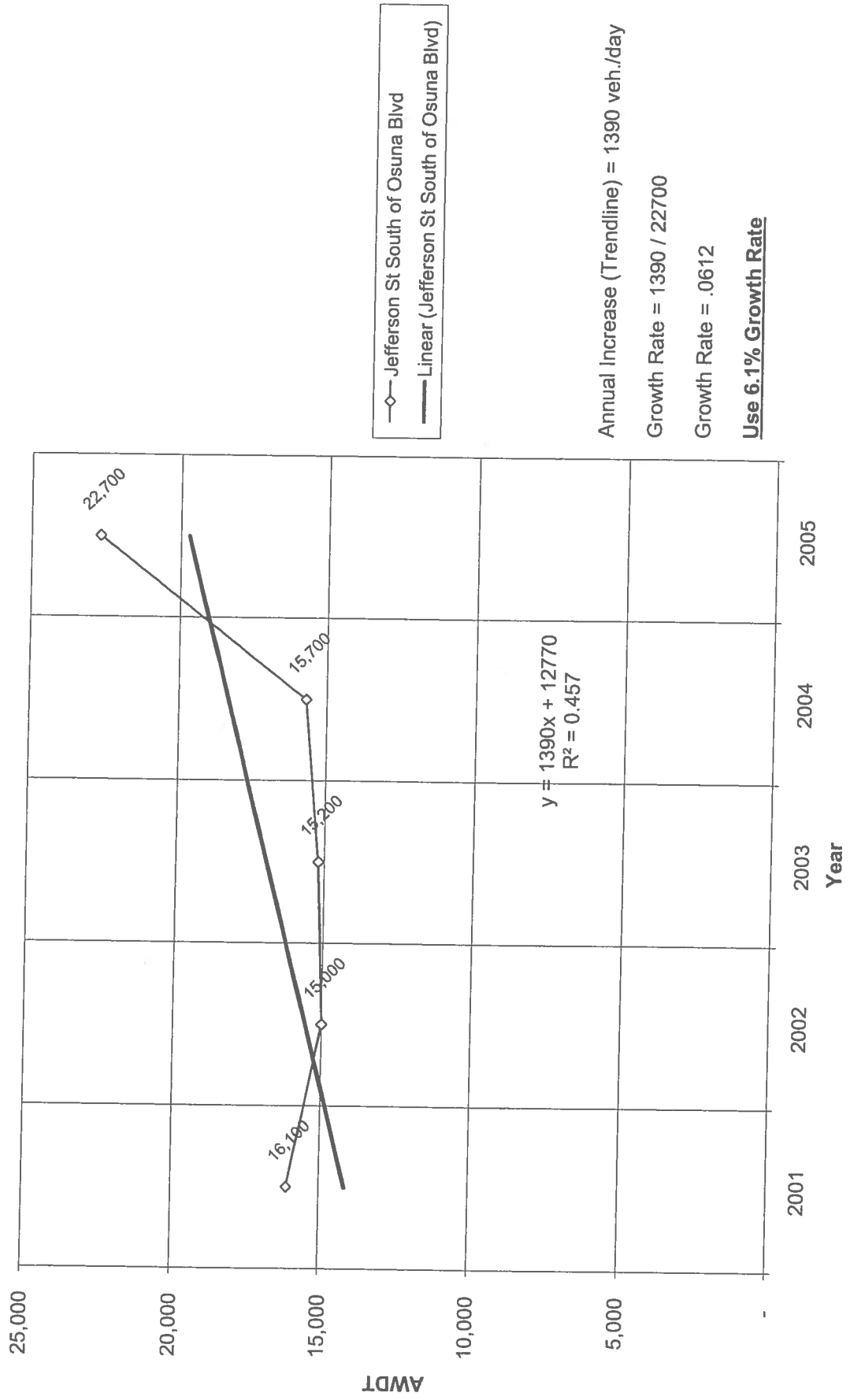
Historic Growth Chart Jefferson St North of Osuna Blvd (2001-2005)



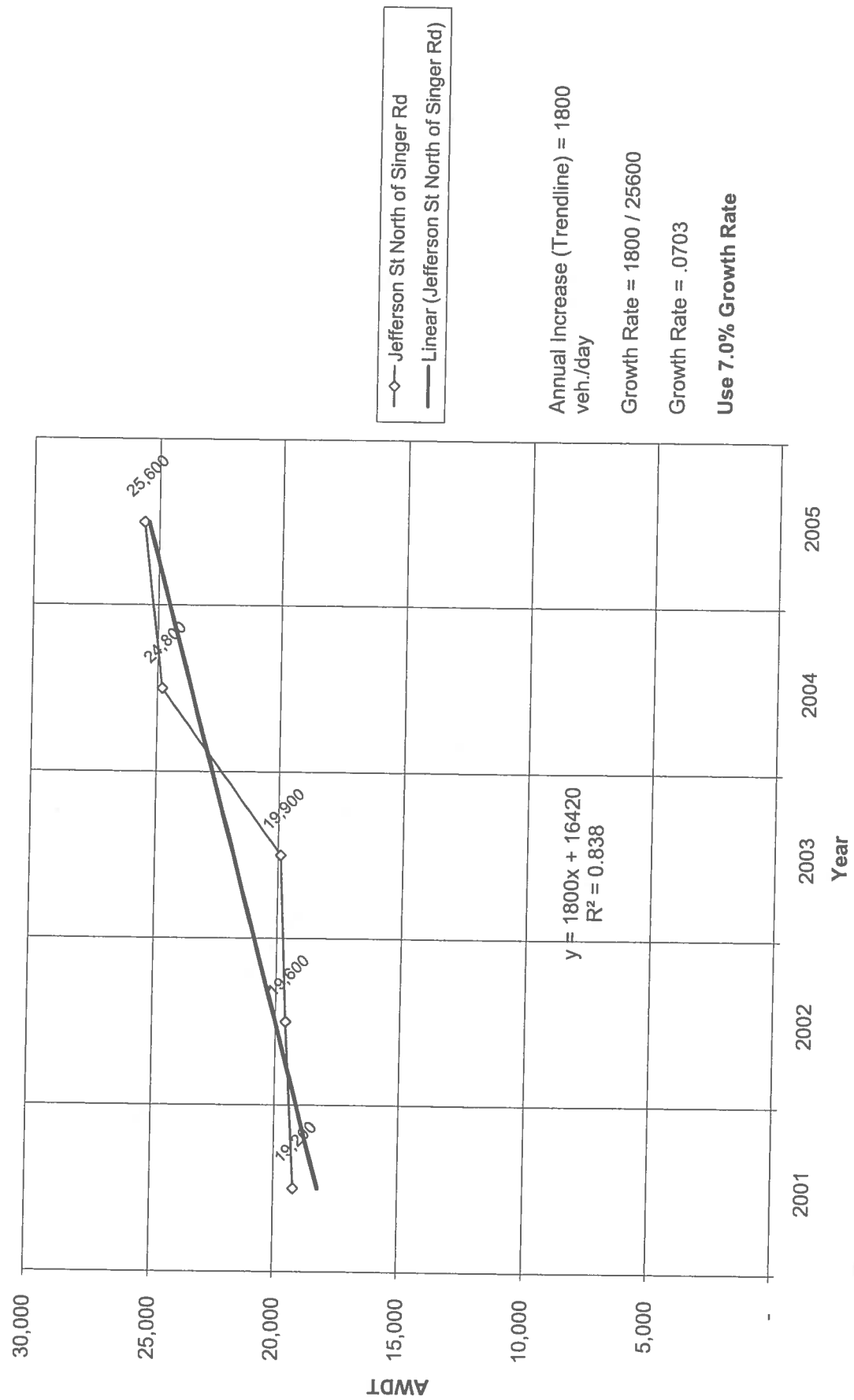
Historic Growth Chart Osuna Blvd East of Jefferson St (2001-2005)



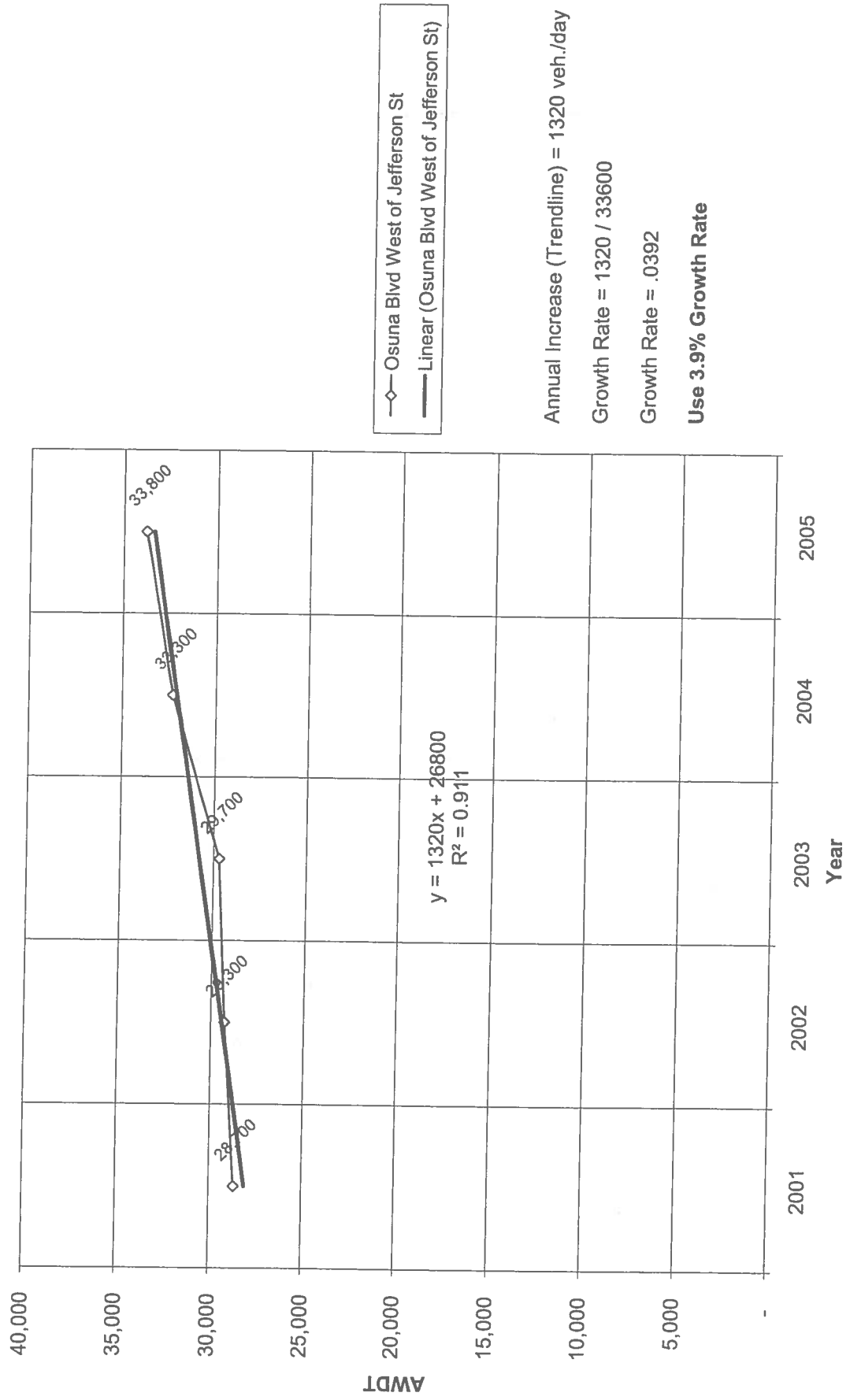
Historic Growth Chart Jefferson St South of Osuna Blvd (2001-2005)



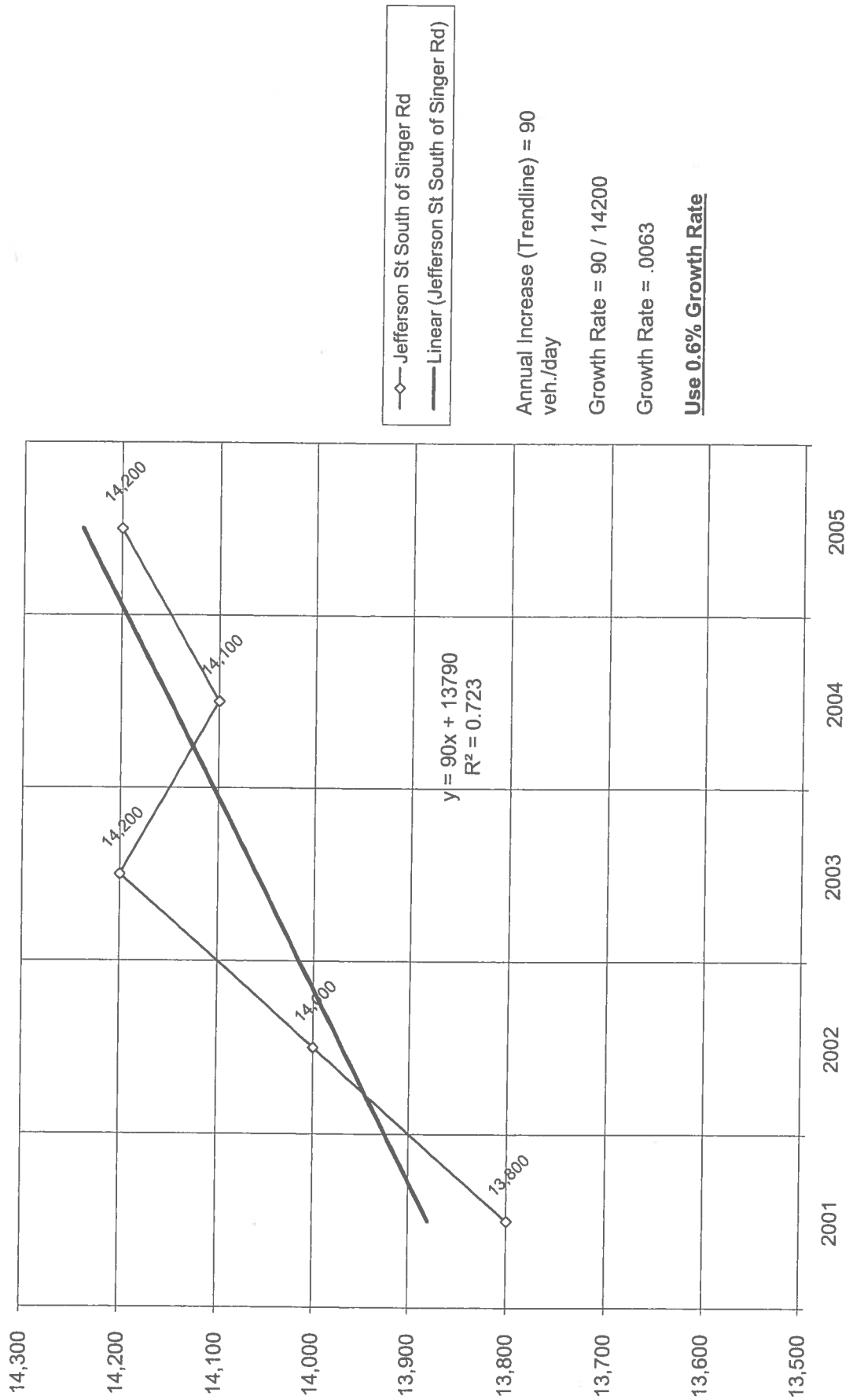
Historic Growth Chart Jefferson St North of Singer Rd (2001-2005)



Historic Growth Chart Osuna Blvd West of Jefferson St (2001-2005)



Historic Growth Chart Jefferson St South of Singer Rd (2001-2005)



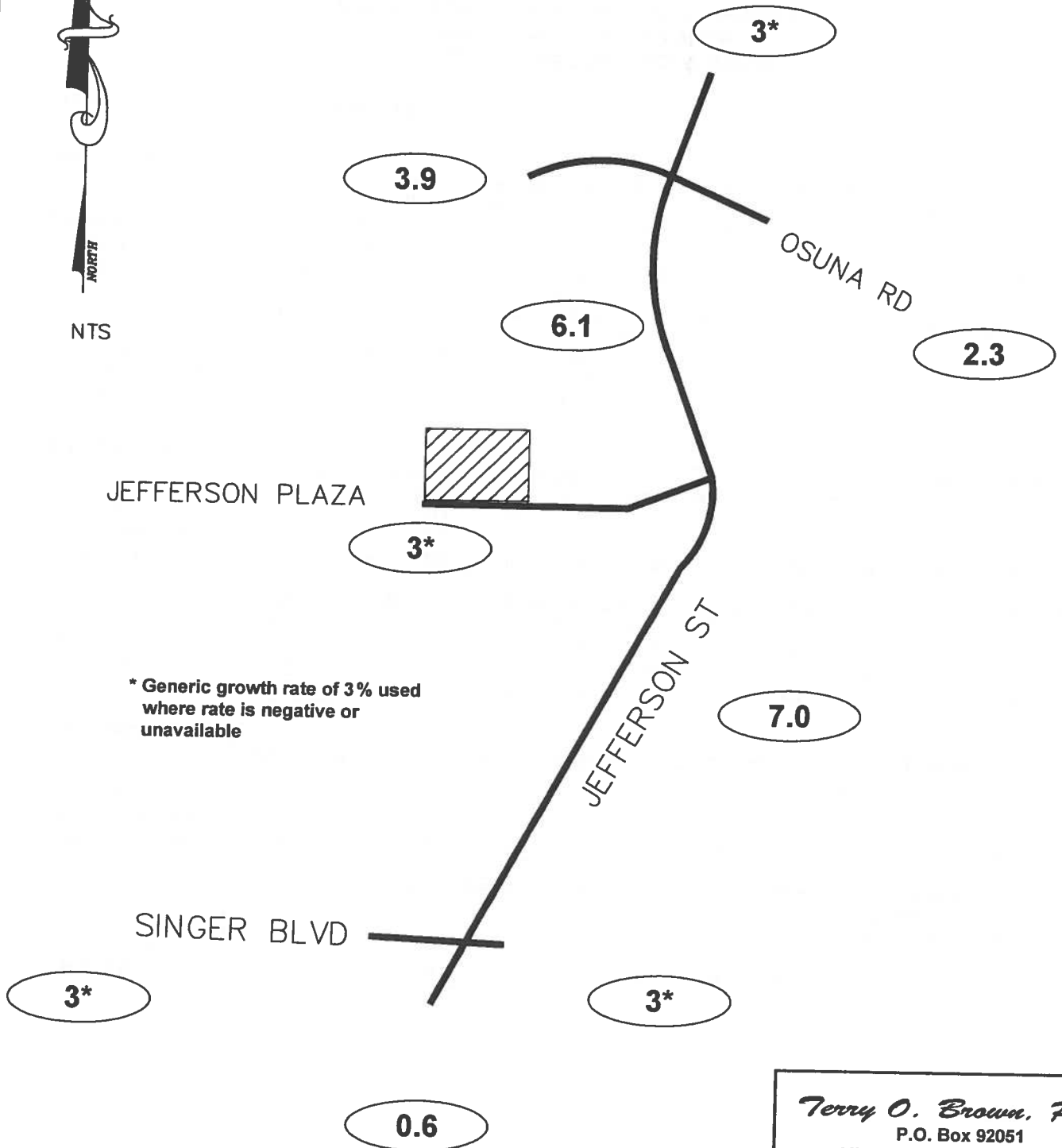
Jefferson Office Plaza

(Jefferson St / Jefferson Plaza)

Growth Rate Map (%)



NTS



* Generic growth rate of 3% used
where rate is negative or
unavailable

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Jefferson Office Plaza (Jefferson Plaza / Jefferson St)

Projected Turning Movements SUMMARY PROPOSED DEVELOPMENT (2009) - 100% Development

INTERSECTION: Summary

Jefferson Plaza / Driveway 'A'

(4)	0.88			0.88			0.85			0.85			PHF
	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'A')			Southbound (Driveway 'A')			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
	0	28	0	0	153	0	0	0	0	0	0	0	
	0	30	0	0	162	0	0	0	0	0	0	0	
	0	37	0	0	215	80	0	0	0	11	0	0	
0.87													

Existing (2007)
2009 (NO BUILD - P.M.)
2009 (BUILD - P.M.)

0.97			0.97			0.85			0.85			PHF
Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'A')			Southbound (Driveway 'A')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	122	0	0	23	0	0	0	0	0	0	0	0
0	129	0	0	24	0	0	0	0	0	0	0	0
0	183	0	0	35	17	0	0	0	82	0	0	0

Jefferson Plaza / Driveway 'B'

(5) 3.0% Truck Existing (2007) 2009 (NO BUILD - A.M.) 2009 (BUILD - A.M.)	0.88			0.88			0.85			0.85			PHF
	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'B')			Southbound (Driveway 'B')			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
	0	28	0	0	153	0	0	0	0	0	0	0	
	0	30	0	0	162	0	0	0	0	0	0	0	
	0	30	0	0	162	53	0	0	0	7	0	0	

Existing (2007)
2009 (NO BUILD - P.M.)
2009 (BUILD - P.M.)

0.97			0.97			0.85			0.85			PHF
Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'B')			Southbound (Driveway 'B')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	122	0	0	23	0	0	0	0	0	0	0	0
0	129	0	0	24	0	0	0	0	0	0	0	0
0	129	0	0	24	11	0	0	0	54	0	0	0

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Projected Turning Movements Worksheet
Osuna Rd / Jefferson St

INTERSECTION:

E-W Street: **Osuna Rd** (1)
N-S Street: **Jefferson St**

Year of Existing Counts: 2004
Implementation Year: 2009

Growth Rates

	3.90%			2.30%			6.10%			3.00%		
	Eastbound (Osuna Rd)			Westbound (Osuna Rd)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	236	1,081	154	182	921	262	122	227	42	197	374	160
Background Traffic Growth	46	211	30	21	106	30	37	69	13	30	56	24
Subtotal	282	1,292	184	203	1,027	292	159	296	55	227	430	184
Vista del Norte Comm. Dev.	17	63	2	0	74	0	2	0	0	0	0	20
Subtotal (NO BUILD - A.M.)	299	1,355	186	203	1,101	292	161	296	55	227	430	204
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	3.64%	7.63%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	26.15%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	3.64%	26.15%	7.63%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	5	10	0	0	1	5	1	0	35	0
Total AM Peak Hour BUILD Volumes	299	1,355	191	213	1,101	292	162	301	56	227	465	204

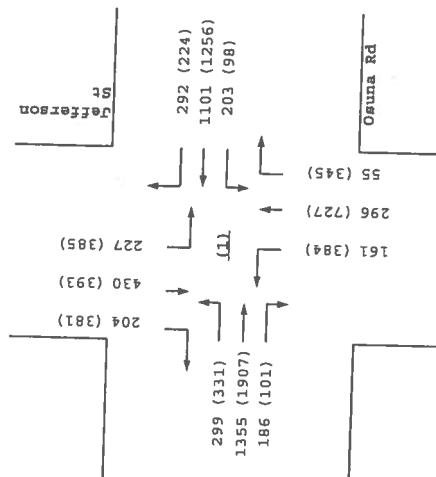
	Eastbound (Osuna Rd)			Westbound (Osuna Rd)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	248	1,487	81	88	1,016	201	291	557	264	335	342	303
Background Traffic Growth	48	290	16	10	117	23	89	170	81	50	51	45
Subtotal	296	1,777	97	98	1,133	224	380	727	345	385	393	348
Vista del Norte Comm. Dev.	35	130	4	0	123	0	4	0	0	0	0	33
Subtotal (NO BUILD - P.M.)	331	1,907	101	98	1,256	224	384	727	345	385	393	381
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	3.64%	7.63%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	26.15%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	3.64%	26.15%	7.63%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	1	2	0	0	5	36	10	0	7	0
Total PM Peak Hour BUILD Volumes	331	1,907	102	100	1,256	224	389	763	355	385	400	381

Number of Commercial Trips Generated

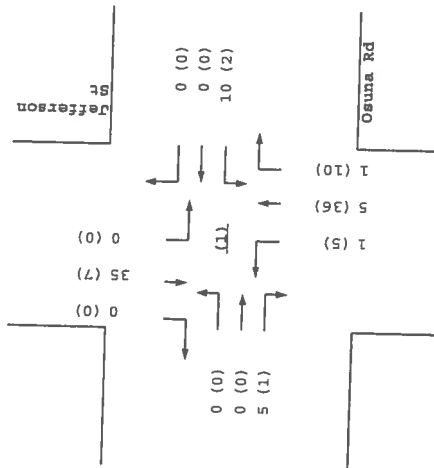
Entering	133	18	A.M.	100% Commercial Development
Exiting	28	136	P.M.	

	Eastbound (Osuna Rd)			Westbound (Osuna Rd)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	264	1,207	172	195	985	280	144	269	50	215	408	174
2007 PM Peak Hr. Volumes	277	1,661	90	94	1,086	215	344	659	312	365	373	330

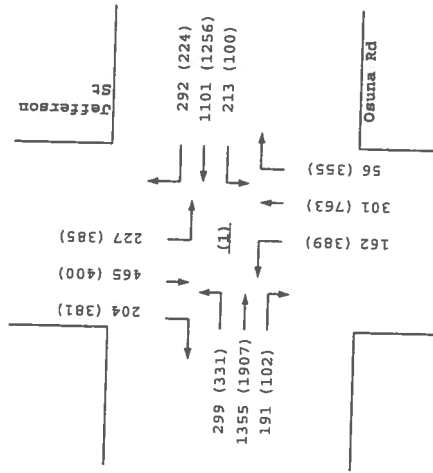
2009
NO BUILD



Trips



2009
BUILD



Osuna Rd / Jefferson St

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Projected Turning Movements Worksheet

Singer Blvd / Jefferson St

INTERSECTION: E-W Street: **Singer Blvd** (2)

N-S Street: **Jefferson St**

Year of Existing Counts 2005

Implementation Year 2009

Growth Rates

	3.00%			3.00%			0.60%			7.00%		
	Eastbound (Singer Blvd)			Westbound (Singer Blvd)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	145	28	201	56	19	21	411	671	283	34	387	134
Background Traffic Growth	17	3	24	7	2	3	10	16	7	10	108	38
Subtotal (NO BUILD - A.M.)	162	31	225	63	21	24	421	687	290	44	495	172
Percent Commercial Trips Generated(Entering)	0.53%	0.00%	0.00%	0.00%	0.00%	0.44%	0.00%	61.25%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.44%	61.25%	0.53%
Total Trips Generated	1	0	0	0	0	1	0	81	0	0	11	0
Total AM Peak Hour BUILD Volumes	163	31	225	63	21	25	421	768	290	44	506	172

	Eastbound (Singer Blvd)			Westbound (Singer Blvd)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	212	63	428	193	35	53	210	386	156	28	593	134
Background Traffic Growth	25	8	51	23	4	6	5	9	4	8	166	38
Subtotal (NO BUILD - P.M.)	237	71	479	216	39	59	215	395	160	36	759	172
Percent Commercial Trips Generated(Entering)	0.53%	0.00%	0.00%	0.00%	0.00%	0.44%	0.00%	61.25%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.44%	61.25%	0.53%
Total Trips Generated	0	0	0	0	0	0	0	17	0	1	83	1
Total PM Peak Hour BUILD Volumes	237	71	479	216	39	59	215	412	160	37	842	173

Number of Commercial Trips Generated

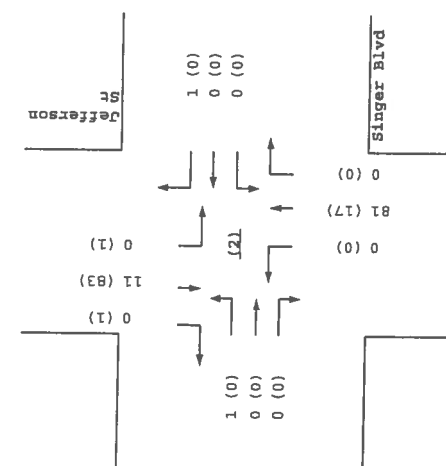
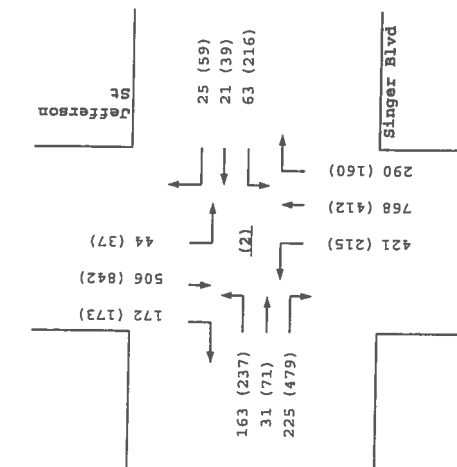
Entering	133	18	A.M.	100% Commercial Development
Exiting	28	136	P.M.	

	Eastbound (Singer Blvd)			Westbound (Singer Blvd)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	154	30	213	59	20	22	416	679	286	39	441	153
2007 PM Peak Hr. Volumes	225	67	454	205	37	56	213	391	158	32	676	153

2009
BUILD

Trips

2009
NO BUILD



Singer Blvd / Jefferson St

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Projected Turning Movements Worksheet
Jefferson Plaza / Jefferson St

INTERSECTION: E-W Street: Jefferson Plaza (3)

N-S Street: Jefferson St

Year of Existing Counts 2007

Implementation Year 2009

Growth Rates

	3.00%			3.00%			7.00%			6.10%		
	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	14	0	14	2	7	4	62	468	9	6	515	84
Background Traffic Growth	1	0	1	0	0	0	9	66	1	1	63	10
Subtotal (NO BUILD - A.M.)	15	0	15	2	7	4	71	534	10	7	578	94
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	62.57%	0.00%	0.00%	0.00%	0.00%	37.42%
Percent Commercial Trips Generated(Exiting)	37.42%	0.00%	62.57%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	7	0	11	0	0	0	83	0	0	0	0	50
Total AM Peak Hour BUILD Volumes	22	0	26	2	7	4	154	534	10	7	578	144

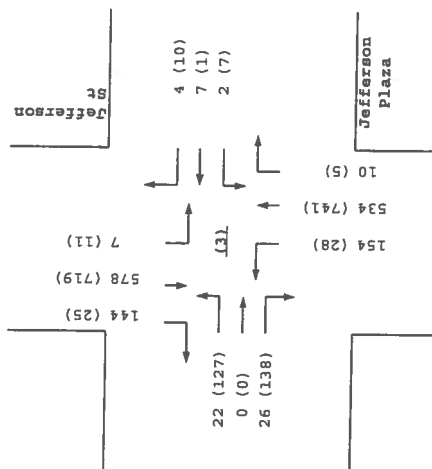
	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	72	0	50	7	1	9	9	650	4	10	641	13
Background Traffic Growth	4	0	3	0	0	1	1	91	1	1	78	2
Subtotal (NO BUILD - P.M.)	76	0	53	7	1	10	10	741	5	11	719	15
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	62.57%	0.00%	0.00%	0.00%	0.00%	37.42%
Percent Commercial Trips Generated(Exiting)	37.42%	0.00%	62.57%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	51	0	85	0	0	0	18	0	0	0	0	10
Total PM Peak Hour BUILD Volumes	127	0	138	7	1	10	28	741	5	11	719	25

Number of Commercial Trips Generated

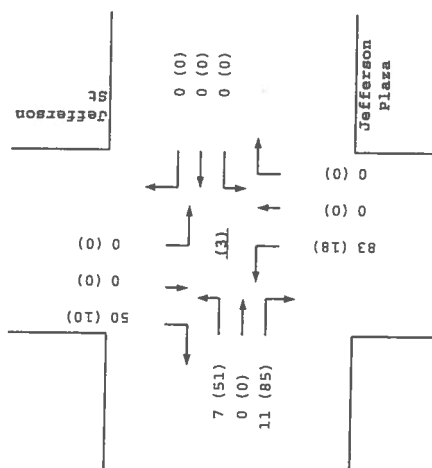
Entering	133	18	A.M.	100% Commercial Development
Exiting	28	136	P.M.	

	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Jefferson St)			Southbound (Jefferson St)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	14	0	14	2	7	4	62	468	9	6	515	84
2007 PM Peak Hr. Volumes	72	0	50	7	1	9	9	650	4	10	641	13

2009
BUILD

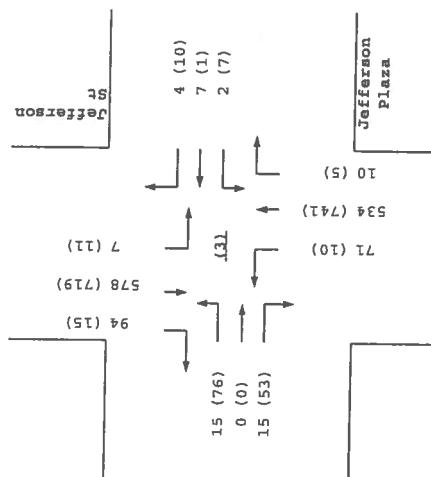


Trips



Jefferson Plaza / Jefferson St

2009
NO BUILD



Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
 Projected Turning Movements Worksheet
Jefferson Plaza / Driveway 'A'

INTERSECTION: E-W Street: Jefferson Plaza (4)
 N-S Street: Driveway 'A'
 Year of Existing Counts 2007
 Implementation Year 2009
 Growth Rates

	3.00%			3.00%			3.00%			3.00%		
	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'A')			Southbound (Driveway 'A')		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	28	0	0	153	0	0	0	0	0	0	0
Background Traffic Growth	0	2	0	0	9	0	0	0	0	0	0	0
Subtotal (NO BUILD - A.M.)	0	30	0	0	162	0	0	0	0	0	0	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	39.99%	60.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	39.99%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	60.00%	0.00%	0.00%
Total Trips Generated	0	7	0	0	53	80	0	0	0	11	0	0
Total AM Peak Hour BUILD Volumes	0	37	0	0	215	80	0	0	0	11	0	0

	3.00%			3.00%			3.00%			3.00%		
	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'A')			Southbound (Driveway 'A')		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	122	0	0	23	0	0	0	0	0	0	0
Background Traffic Growth	0	7	0	0	1	0	0	0	0	0	0	0
Subtotal (NO BUILD - P.M.)	0	129	0	0	24	0	0	0	0	0	0	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	39.99%	60.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	39.99%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	60.00%	0.00%	0.00%
Total Trips Generated	0	54	0	0	11	17	0	0	0	82	0	0
Total PM Peak Hour BUILD Volumes	0	183	0	0	35	17	0	0	0	82	0	0

Number of Commercial Trips Generated

Entering	133	18	A.M.
Exiting	28	136	P.M.

100% Commercial Development

	Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'A')			Southbound (Driveway 'A')		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	0	28	0	0	153	0	0	0	0	0	0	0
2007 PM Peak Hr. Volumes	0	122	0	0	23	0	0	0	0	0	0	0

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
 Projected Turning Movements Worksheet
Jefferson Plaza / Driveway 'B'

INTERSECTION:

E-W Street: Jefferson Plaza (5)

N-S Street: Driveway 'B'

Year of Existing Counts

2007

Implementation Year

2009

Growth Rates

Existing Volumes

Background Traffic Growth

Subtotal (NO BUILD - A.M.)

Percent Commercial Trips Generated(Entering)

Percent Commercial Trips Generated(Exiting)

Total Trips Generated

Total AM Peak Hour BUILD Volumes

3.00%			3.00%			3.00%			3.00%		
Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'B')			Southbound (Driveway 'B')		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
0	28	0	0	153	0	0	0	0	0	0	0
0	2	0	0	9	0	0	0	0	0	0	0
0	30	0	0	162	0	0	0	0	0	0	0
0.00%	0.00%	0.00%	0.00%	0.00%	39.99%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	39.99%	0.00%	0.00%
0	0	0	0	0	53	0	0	0	7	0	0
0	30	0	0	162	53	0	0	0	7	0	0

Existing Volumes

Background Traffic Growth

Subtotal (NO BUILD - P.M.)

Percent Commercial Trips Generated(Entering)

Percent Commercial Trips Generated(Exiting)

Total Trips Generated

Total PM Peak Hour BUILD Volumes

3.00%			3.00%			3.00%			3.00%		
Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'B')			Southbound (Driveway 'B')		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
0	122	0	0	23	0	0	0	0	0	0	0
0	7	0	0	1	0	0	0	0	0	0	0
0	129	0	0	24	0	0	0	0	0	0	0
0.00%	0.00%	0.00%	0.00%	0.00%	39.99%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	39.99%	0.00%	0.00%
0	0	0	0	0	11	0	0	0	54	0	0
0	129	0	0	24	11	0	0	0	54	0	0

Number of Commercial Trips Generated

Entering Exiting

133 18 A.M.
28 136 P.M.

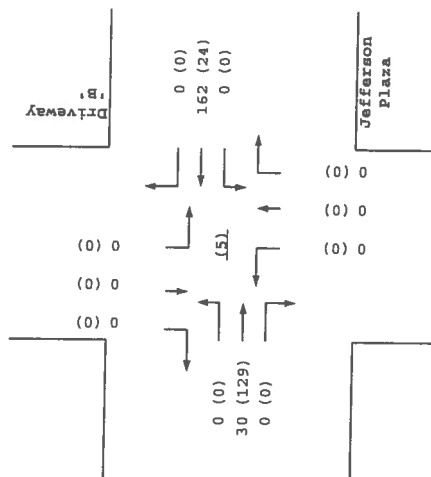
100% Commercial Development

2007 AM Peak Hr. Volumes

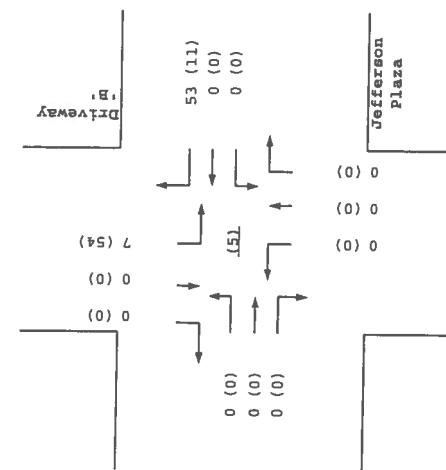
2007 PM Peak Hr. Volumes

Eastbound (Jefferson Plaza)			Westbound (Jefferson Plaza)			Northbound (Driveway 'B')			Southbound (Driveway 'B')		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
0	28	0	0	153	0	0	0	0	0	0	0
0	122	0	0	23	0	0	0	0	0	0	0

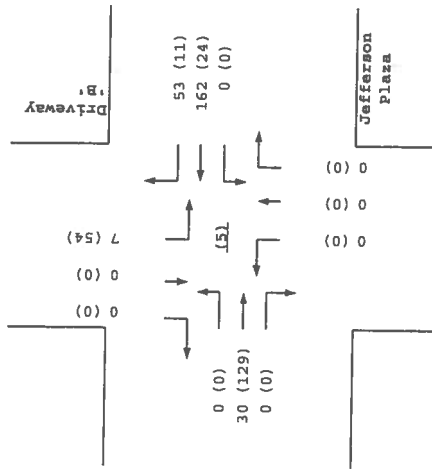
2009
NO BUILD



Trips



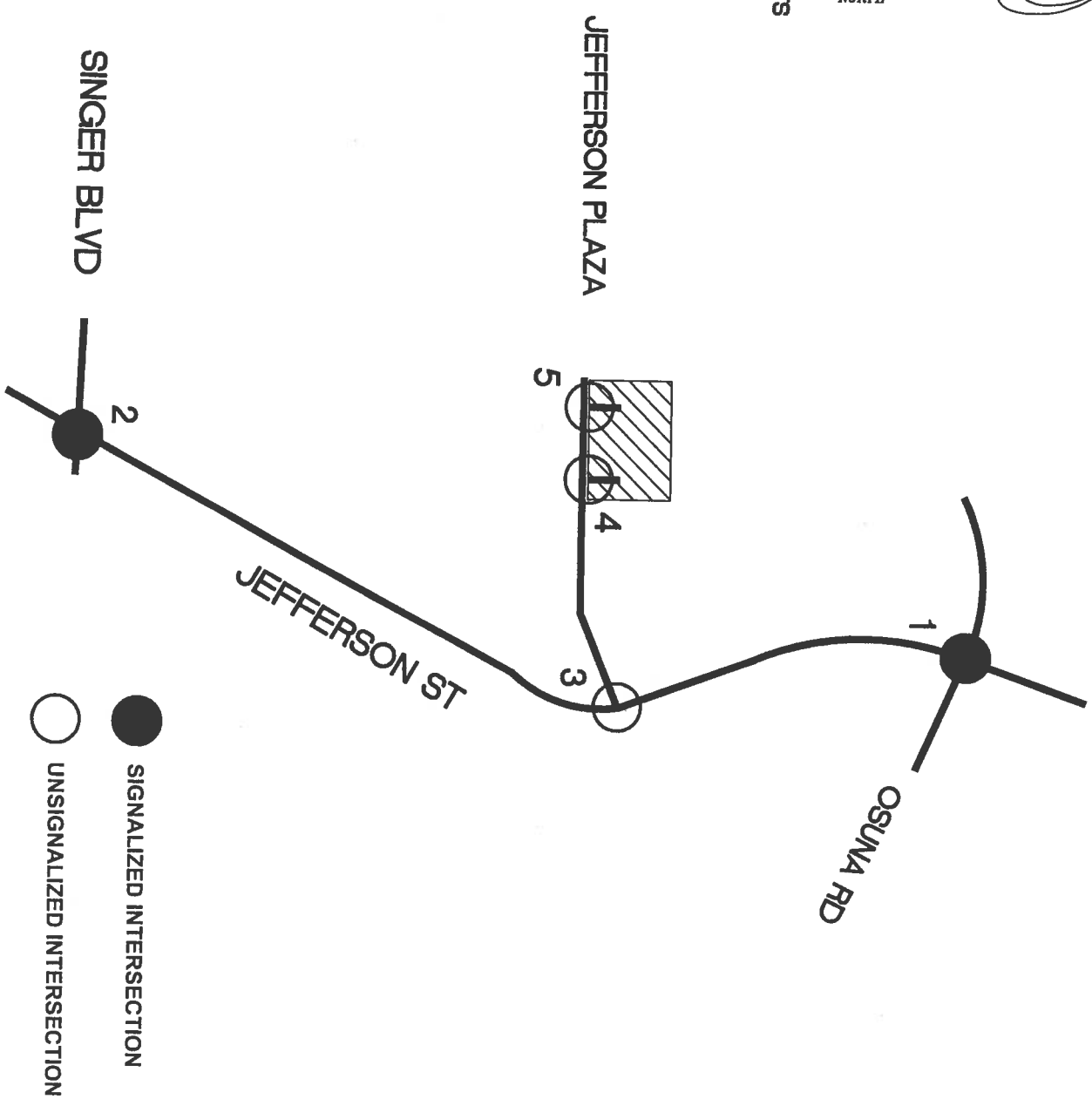
2009
BUILD

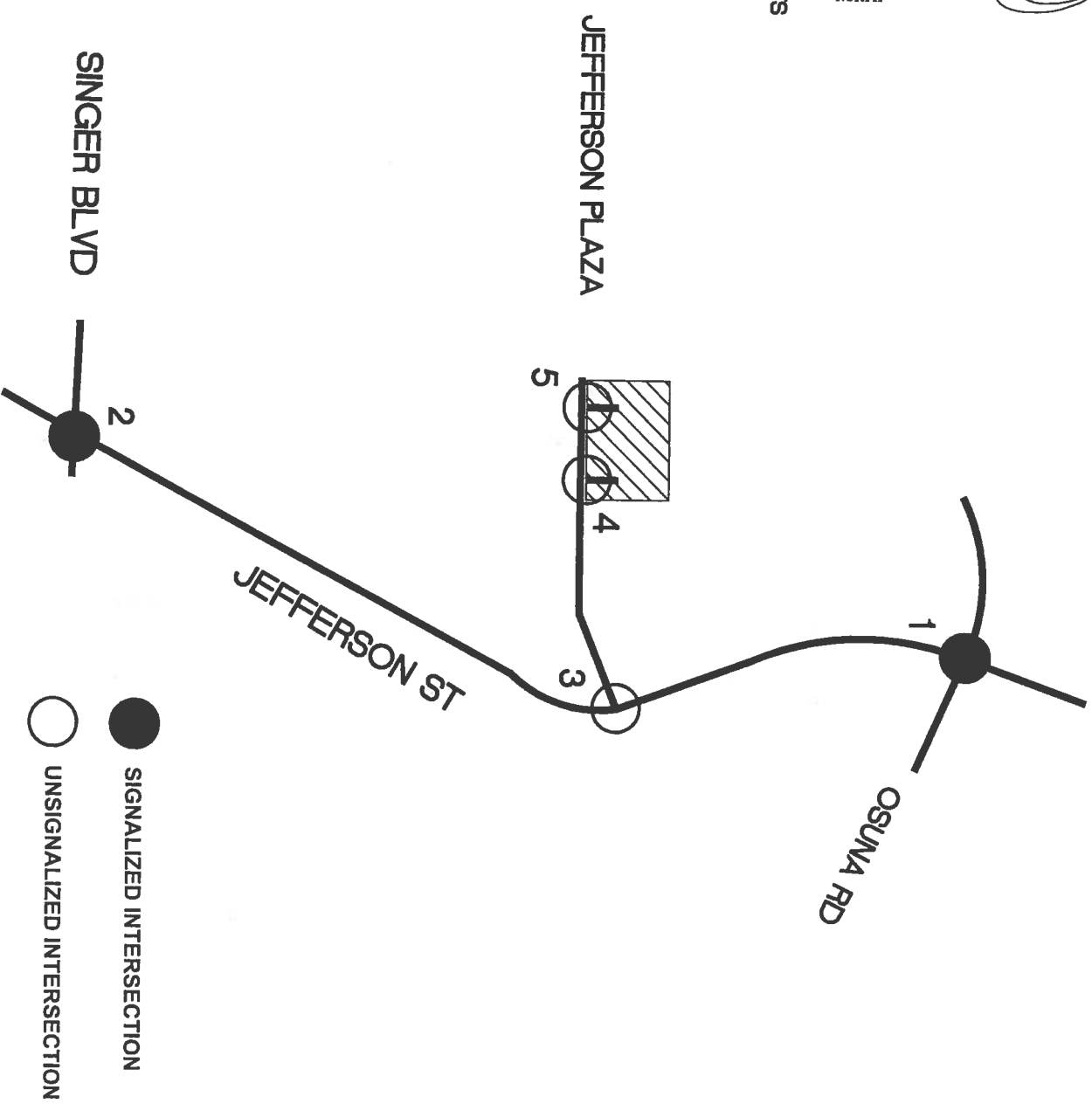


Jefferson Plaza / Driveway 'B'



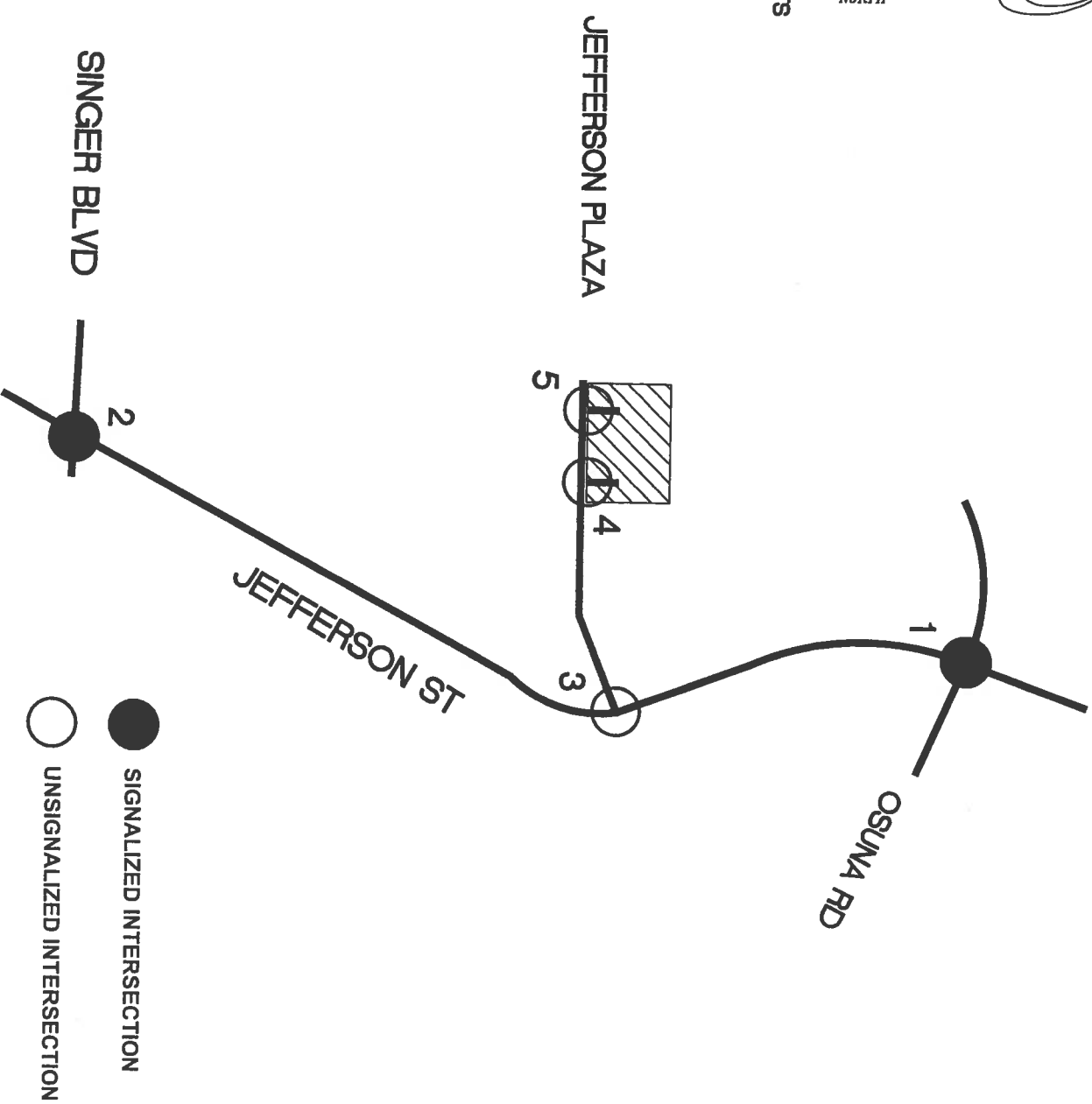
NTS







NTS



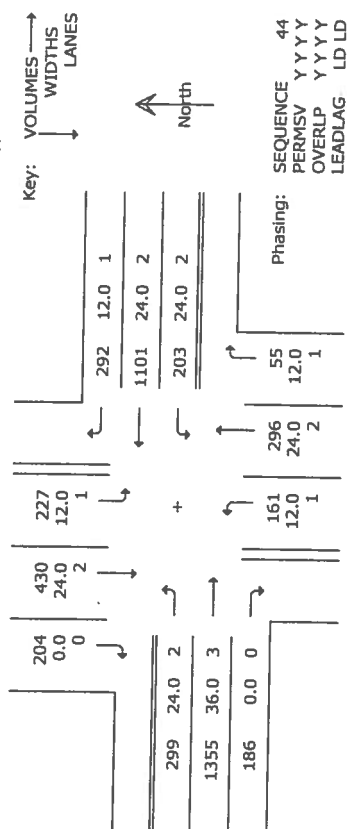
Analysis of Intersection #1

Osuna Rd / Jefferson St

SIGNAL2000/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 1 -

Area Location Type: NONCBD



	SB			WB			NB			EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HVV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.87	.87	.87	.87	.87	.87	.91	.91	.91	.88	.88	.88
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Strup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 44 LD/LD	←	←	←	←	←	←
North	↑	↑	↑	↑	↑	↑
C=100"	G= 7.5" Y+R= 5.0"	G= 23.6" Y+R= 5.0"	G= 12.3" Y+R= 5.0"	G= 36.6" Y+R= 5.0"	G= 0.0" Y+R= 0.0"	G= 0.0" Y+R= 0.0"

SIGNAL2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int # 1 -

V/C 0.849 (Critical V/C 0.942)

Control Delay 45.8

Level of Service D

	Phase 1	Phase 2	Phase 3	Phase 4
Sq 44 LD/LD	←	←	←	←
North	↑	↑	↑	↑
G/C=0.075 G= 7.5" Y+R= 5.0" Off= 0.0%	G/C=0.236 G= 23.6" Y+R= 5.0" Off=12.5%	G/C=0.123 G= 12.3" Y+R= 5.0" Off=41.1%	G/C=0.366 G= 36.6" Y+R= 5.0" Off=58.4%	

C=100 sec G= 80.0 sec = 80.0% Y=20.0 sec = 20.0% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	g/C	Reqd	Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	Queue Model 1
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SB Approach

RT+TH LT	24/2 12/1	0.234 0.050	735 309	789 343	728 261	0.923 0.761	53.6 36.7	*D *D	545 320
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NB Approach

RT TH LT	12/1 24/2 12/1	0.064 0.409 0.114	617 829 179	641 829 200	60 325 177	0.094 0.392 0.859	18.2 32.5 54.0	B *D *D	49 188 252
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WB Approach

RT TH LT	12/1 24/2 12/1	0.247 0.363 0.090	763 1277 344	770 1285 407	336 1266 233	0.436 0.985 0.556	16.9 53.0 42.9	B *D *D	281 952 154
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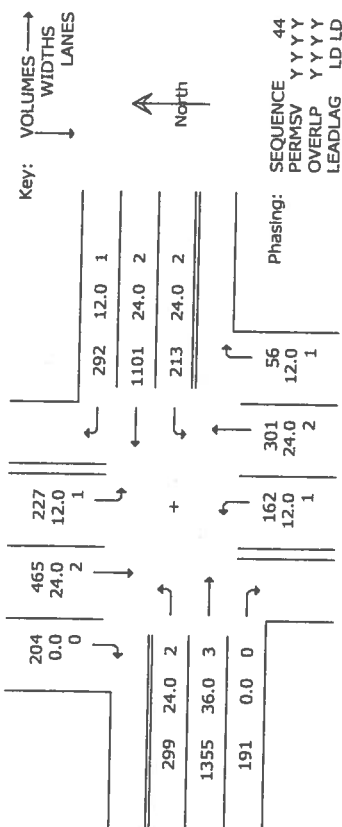
EB Approach

RT+TH LT	36/3 24/2	0.353 0.122	1806 344	1806 407	1751 340	0.970 0.811	45.8 54.2	D *D	900 250
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SIGNAL2000/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 1 -

Area Location Type: NONCBD



	SB			WB			NB			EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.87	.87	.87	.87	.87	.87	.91	.91	.91	.88	.88	.88
Predimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival byp, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park minvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

Sq 44 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
C=110"	G= 8.7" Y+R= 5.0"	G= 27.3" Y+R= 5.0"	G= 13.6" Y+R= 5.0"	G= 40.5" Y+R= 5.0"	G= 0.0" Y+R= 0.0"	G= 0.0" Y+R= 0.0"

SIGNAL2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int. # 1 -

V/C 0.846 (Critical V/C 0.939)

Control Delay 48.2

Level of Service D

Sq 44 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4
	G/C=0.079 G= 8.7" Y+R= 5.0" Off= 0.0%	G/C=0.248 G= 27.3" Y+R= 5.0" Off=12.4%	G/C=0.124 G= 13.6" Y+R= 5.0" Off=41.8%	G/C=0.368 G= 40.5" Y+R= 5.0" Off=58.7%

C=110 sec G= 90.0 sec = 81.8% Y=20.0 sec = 18.2% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	Reqd g/c	Used g/c	Service Rate @D (vph)	Adj Volume	v/c	HCM Delay	L Queue Model 1
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SB Approach

RT+TH LT	24/2 12/1	0.250 0.050	0.248 0.079	762 317	831 354	768 261	0.924 0.735	56.2 36.1	*E+ D+	616 338
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51.1 D

NB Approach

RT TH LT	12/1 24/2 12/1	0.072 0.123 0.081	0.417 0.248 0.079	623 800 175	654 870 198	62 331 178	0.095 0.380 0.868	19.5 34.6 57.8	B C *E+	55 206 277
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40.2 D+

WB Approach

RT TH LT	12/1 24/2 12/1	0.252 0.367 0.100	0.492 0.368 0.124	758 1268 324	772 1292 402	336 1266 245	0.435 0.980 0.582	18.4 54.6 47.6	B *D D	305 1014 178
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47.1 D

EB Approach

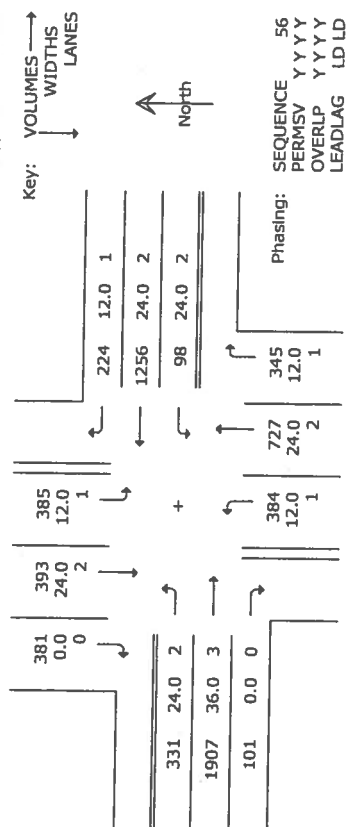
RT+TH LT	36/3 24/2	0.359 0.129	0.368 0.124	1809 324	1815 402	1757 340	0.968 0.808	48.4 58.1	D *E+	969 271
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50.0 D

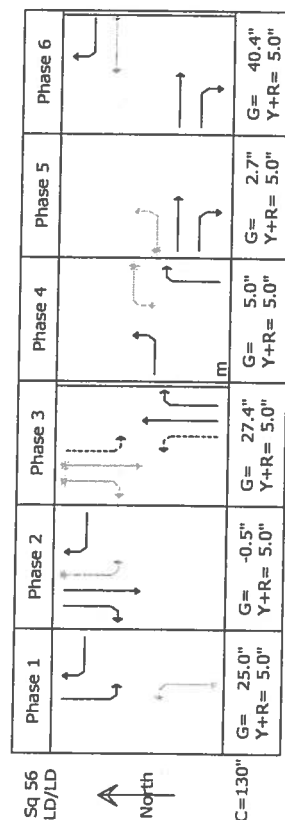
SIGNAL2000/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 1 -

Area Location Type: NONCBD



	SB			WB			NB			EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.79	.79	.79	.91	.91	.91	.90	.90	.90	.94	.94	.94
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



SIGNAL2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int # 1 -

V/C 1.114 (Critical V/C 1.260)

Control Delay 125.2

Level of Service F

Sq 56 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
	G/C=0.193 G= 25.0" Y+R= 5.0" Off= 0.0%	G/C= -0.004 G= -0.5" Y+R= 5.0" Off=23.1%	G/C=0.211 G= 27.4" Y+R= 5.0" Off=26.5%	G/C=0.038 G= 5.0" Y+R= 5.0" Off=51.5%	G/C=0.021 G= 2.7" Y+R= 5.0" Off=59.2%	G/C=0.311 G= 40.4" Y+R= 5.0" Off=65.1%

C=130 sec G=100.0 sec = 76.9% Y=30.0 sec = 23.1% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	Reqd	g/C	Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	L	Queue Model 1
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SB Approach

RT+TH LT	24/2 12/1	0.327 0.274	0.245 0.227	673 403	795 453	979 487	1.228 1.070	162.8 101.9	*F *F	1234 ft 986 ft
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NB Approach

RT TH LT	12/1 24/2 12/1	0.294 0.264 0.240	0.288 0.211 0.193	372 594 346	437 729 390	383 808 427	0.849 1.090 1.081	57.9 111.7 106.0	E+ *F *F	621 ft 896 ft 891 ft
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WB Approach

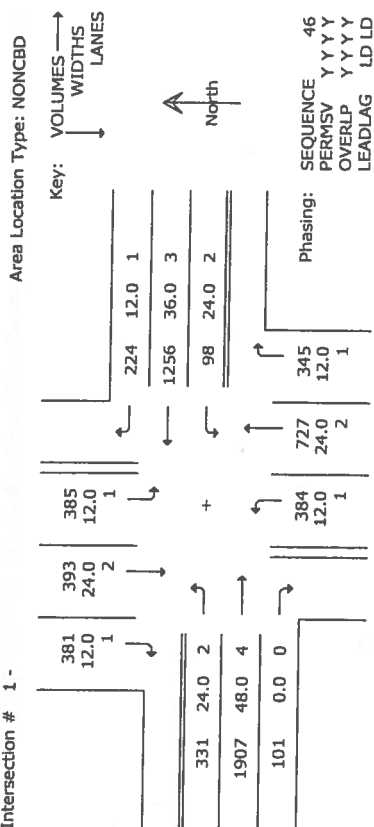
RT TH LT	12/1 24/2 12/1	0.216 0.405 0.099	0.576 0.311 0.038	896 999 1	903 1091 96	246 1380 108	0.272 1.265 0.824	14.0 171.4 95.1	B+ *F *F	208 ft 1745 ft 115 ft
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EB Approach

RT+TH LT	36/3 24/2	0.431 0.155	0.370 0.098	1793 100	1845 292	2136 352	1.158 1.057	118.5 123.8	F *F	1670 ft 411 ft
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SIGNAL 2000/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 1 -



	SB			WB			NB			EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.79	.79	.79	.91	.91	.91	.90	.90	.90	.94	.94	.94
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 46 LD/LD						
G= 34.3"		G= 28.4"	G= 5.0"	G= 3.3"	G= 33.9"	G= 0.0"
Y+R= 5.0"		Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 0.0"

C=130"

SIGNAL 2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int # 1 -

V/C 0.924 (Critical V/C 1.031)

Control Delay 67.5 Level of Service E+

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5
Sq 46 LD/LD					
G/C=0.264		G/C=0.219	G/C=0.038	G/C=0.025	G/C=0.261
G= 34.3"		G= 28.4"	G= 5.0"	G= 3.3"	G= 33.9"
Y+R= 5.0"		Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"
Off= 0.0%		Off=30.3%	Off=56.0%	Off=63.7%	Off=70.0%

C=130 sec G=105.0 sec = 80.8% Y=25.0 sec = 19.2% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	Reqd	g/C Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	L	Queue Model 1
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SB Approach

	RT	TH	LT
12/1	0.349	0.359	0.359
24/2	0.187	0.219	0.219
12/1	0.275	0.264	0.264
500	563	520	520
482	497	487	487
0.856	0.647	0.937	0.937
50.9	48.1	62.0	62.0
D	D	*E+	*E+
750 ft	397 ft	812 ft	812 ft

53.6 D

NB Approach

	RT	TH	LT
12/1	0.294	0.296	0.296
24/2	0.264	0.219	0.219
12/1	0.199	0.264	0.264
387	452	571	571
383	808	427	427
0.825	1.052	0.748	0.748
54.3	97.8	27.7	27.7
D	*F	C	C
607 ft	857 ft	561 ft	561 ft

69.0 E

WB Approach

	RT	TH	LT
12/1	0.216	0.564	0.564
36/3	0.296	0.261	0.261
24/2	0.099	0.038	0.038
884	1312	96	96
246	1380	108	108
0.278	1.052	0.824	0.824
14.8	87.7	95.1	95.1
B+	*F	*F	*F
214 ft	998 ft	115 ft	115 ft

77.8 E

EB Approach

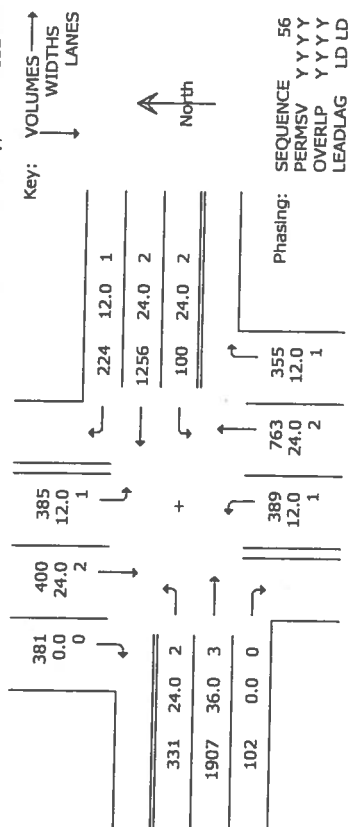
	RT+TH	LT
48/4	0.332	0.155
24/2	0.325	0.102
2080	2160	308
2136	352	1011
0.989	1.011	1.011
60.4	109.5	109.5
E+	*F	*F
1052 ft	391 ft	391 ft

67.4 E+

SIGNAL2000/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 1 -

Area Location Type: NONCBD



	SB			WB			NB			EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.79	.79	.79	.91	.91	.91	.90	.90	.90	.94	.94	.94
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 56 LD/LD						
C=130"	G= 25.2" Y+R= 5.0"	G= -0.9" Y+R= 5.0"	G= 27.8" Y+R= 5.0"	G= 5.0" Y+R= 5.0"	G= 2.7" Y+R= 5.0"	G= 40.2" Y+R= 5.0"

SIGNAL2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int # 1 -

V/C 1.124 (Critical V/C 1.279)

Control Delay 128.8

Level of Service F

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 56 LD/LD						
	G/C=0.194 G= 25.2" Y+R= 5.0" Off= 0.0"	G/C= -0.007 G= -0.9" Y+R= 5.0" Off=-23.2"	G/C=0.214 G= 27.8" Y+R= 5.0" Off=26.4"	G/C=0.038 G= 5.0" Y+R= 5.0" Off=51.6"	G/C=0.021 G= 2.7" Y+R= 5.0" Off=59.3"	G/C=0.309 G= 40.2" Y+R= 5.0" Off=65.2"

C=130 sec G=100.0 sec = 76.9% Y=30.0 sec = 23.1% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	Req'd	g/C Used	Service Rate @D (vph) @E	Adj Volume	V/C	HCM Delay	L	Queue Model 1
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SB Approach

	RT+TH LT	24/2 12/1	0.329 0.274	0.245 0.225	676 400	797 449	988 487	1.237 1.077	166.2 *F	104.4 *F	1254 ft 994 ft
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NB Approach

	RT TH LT	12/1 24/2 12/1	0.300 0.273 0.243	0.291 0.214 0.194	378 607 348	443 742 392	394 848 432	0.864 1.128 1.088	59.4 125.0 110.3	E+ *F	645 ft 976 ft 908 ft
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WB Approach

	RT TH LT	12/1 24/2 12/1	0.216 0.405 0.100	0.573 0.309 0.038	890 992 1	898 1085 96	246 1380 110	0.274 1.128 0.840	14.2 174.6 98.1	B+ *F	210 ft 1757 ft 118 ft
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EB Approach

	RT+TH LT	36/3 24/2	0.431 0.155	0.368 0.098	1783 100	1837 292	2138 352	1.164 1.057	121.2 123.8	F *F	1684 ft 411 ft
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Analysis of Intersection #2

Singer Rd / Jefferson St

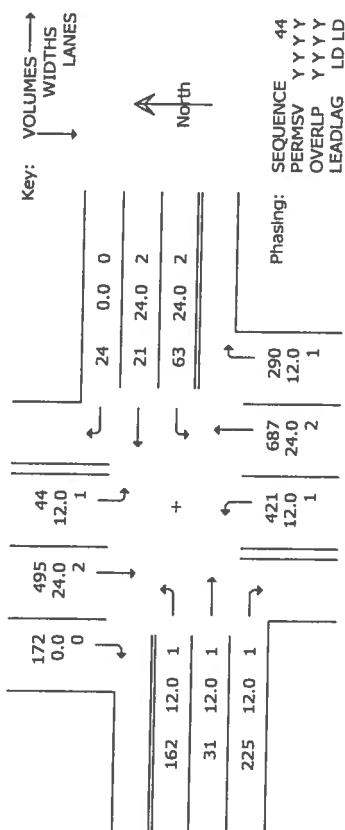
Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Analysis of Singer Rd / Jefferson St - [2_ANX.FOR]
2009 AM Peak NOBUILD Conditions

06/21/07
20:28:12

SIGNAL2000/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 2 -

Area Location Type: NONCBD



	SB			WB			NB			EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.83	.83	.83	.75	.75	.75	.89	.89	.89	.80	.80	.80
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Strutted lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

Sq 44 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
C=110"	G= 31.5" Y+R= 5.0"	G= 39.4" Y+R= 5.0"	G= 12.1" Y+R= 5.0"	G= 7.1" Y+R= 5.0"	G= 0.0" Y+R= 0.0"	G= 0.0" Y+R= 0.0"

Jefferson Office Plaza (Jefferson Plaza / Jefferson St)
Analysis of Singer Rd / Jefferson St - [2_ANX.FOR]
2009 AM Peak NOBUILD Conditions

06/21/07
20:28:12

SIGNAL2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int # 2 -
V/C 0.583 (Critical V/C 0.780)

Control Delay 29.0 Level of Service C

Sq 44 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4
	G/C=0.286 G= 31.5" Y+R= 5.0" Off= 0.0%	G/C=0.358 G= 39.4" Y+R= 5.0" Off=33.2%	G/C=0.110 G= 12.1" Y+R= 5.0" Off=73.5%	G/C=0.064 G= 7.1" Y+R= 5.0" Off=89.0%

C=110 sec G= 90.0 sec = 81.8% Y=20.0 sec = 18.2% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	Reqd g/c	Service Rate @D (vph) @E	Adj Volume	V/c	HCM Delay	L	Queue Model 1
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SB Approach

RT+TH	24/2	0.258	0.358	1180	1209	803	0.664	31.1	*C	494 ft
LT	12/1	0.000	0.286	632	645	53	0.082	7.6	A	27 ft

NB Approach

RT	12/1	0.247	0.513	796	805	326	0.405	16.8	B	283 ft
TH	24/2	0.240	0.358	1230	1258	772	0.614	29.9	C	461 ft
LT	12/1	0.206	0.286	618	634	473	0.746	23.8	*C+	451 ft

WB Approach

RT+TH	24/2	0.038	0.064	126	180	60	0.288	49.8	D	45 ft
LT	24/2	0.046	0.110	276	351	84	0.225	45.0	D	58 ft

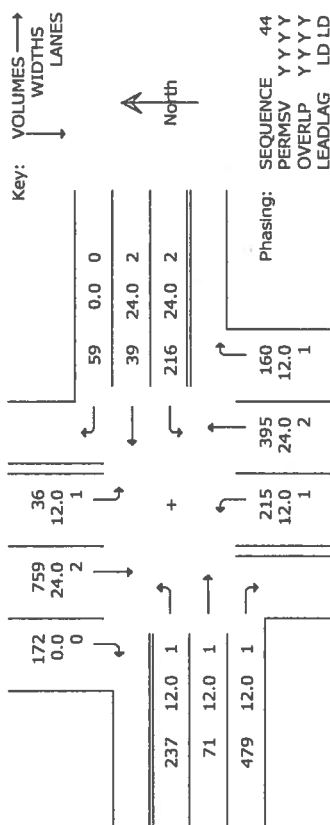
EB Approach

RT	12/1	0.220	0.396	586	621	281	0.452	25.0	C+	291 ft
TH	12/1	0.046	0.064	65	95	39	0.328	50.8	*D	56 ft
LT	12/1	0.079	0.110	258	301	202	0.641	42.3	*D+	270 ft

SIGNAL2000/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 2 -

Area Location Type: NONCBD



	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.86	.86	.86	.75	.75	.75	.91	.91	.91	.75	.75	.75
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park minvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0




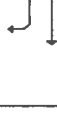

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 44 LD/LD						
North						
C=110"	G= 12.5" Y+R= 5.0"	G= 36.2" Y+R= 5.0"	G= 11.9" Y+R= 5.0"	G= 29.4" Y+R= 5.0"	G= 0.0" Y+R= 0.0"	G= 0.0" Y+R= 0.0"

SIGNAL2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int # 2 -
V/C 0.732 (Critical V/C 0.964)

Control Delay 45.2

Level of Service D

Sq. 44 D/LD	Phase 1	Phase 2	Phase 3	Phase 4
				
				
	G/C=0.114 G= 12.5" Y+R= 5.0" Off= 0.0%	G/C=0.329 G= 36.2" Y+R= 5.0" Off=15.9%	G/C=0.108 G= 11.9" Y+R= 5.0" Off=53.4%	G/C=0.267 G= 29.4" Y+R= 5.0" Off=68.7%

C=110 sec G= 90.0 sec = 81.8% Y=20.0 sec = 18.2% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	Reqd	q/C Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	L Queue Model 1
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SB Approach

	RT+TH LT	24/2 12/1	0.329 0.000	0.329 0.114	1084 436	1124 456	1083 42	0.964 0.092	54.9 15.4	*D B	866 33
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NB Approach

	RT TH LT	12/1 24/2 12/1	0.154 0.151 0.116	0.483 0.329 0.114	741 1117 234	757 1156 261	176 434 236	0.232 0.375 0.884	16.7 28.4 56.8	B C *E+	147 246 352
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WB Approach

	RT+TH LT	24/2 24/2	0.067 0.113	0.267 0.108	790 271	853 345	131 288	0.154 0.783	30.9 58.3	C *E+	76 230
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EB Approach

	RT TH LT	12/1 12/1 12/1	0.426 0.085 0.009	0.426 0.267 0.108	640 433 509	669 492 539	639 95 316	0.955 0.193 0.586	54.8 31.3 24.7	*D C C+	938 106 346
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SIGNAL2000/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Area Location Type: NONCBD



C=110 sec	G= 90.0 sec = 81.8%	Y=20.0 sec = 18.2%	Ped= 0.0 sec = 0.0%
-----------	---------------------	--------------------	---------------------

WB Approach										53.6 D	
RT+TH LT		24/2 24/2	0.067 0.113	0.261 0.102	767 251	832 324	131 288	0.157 0.828	31.4 63.6	C *E+	77 ft 238 ft

EB Approach										48.4 D	
RT		12/1	0.426	0.416	621	652	639	0.980	61.8	*E+	978 ft
TH		12/1	0.085	0.261	420	479	95	0.198	31.9	C	107 ft
LT		12/1	0.015	0.102	488	520	316	0.608	26.3	C+	355 ft

Analysis of Intersection #3

Jefferson Plaza / Jefferson St

CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year AM Peak Hour 2009
 Comment 2009 AM Peak NOBUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson St
 Minor Street Jefferson Plaza

Input Data

Lane Configuration	NB			SB			WB			EB		
Lane 1 (curb)	TR			R			LTR			LTR		
Lane 2	T			T								
Lane 3	L			T								
Lane 4				L								
Lane 5												
Movement	NB			SB			WB			EB		
	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	71	534	10	7	578	94	2	7	4	15	1	15
PHF	0.91	0.91	0.91	0.88	0.88	0.88	0.75	0.75	0.75	0.75	0.75	0.75
Percent of heavy vehicles, HV	3	3	3	3	3	3	3	3	3	3	3	3
Flow rate	78	587	11	8	657	107	3	9	5	20	1	20
Flare storage (# of vehs)												
Median storage (# of vehs)							1			1		
Signal upstream of Movement 2												
Length of study period (h)	0.25											

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
WB	1	LTR	17	269	0.063	0	19.3	C	19.3
	2								
	3							C	
EB	1	LTR	41	372	0.110	0	15.9	C	15.9
	2								
	3							C	
NB	①		78	838	0.093	0	9.7	A	
SB	④		8	968	0.008	0	8.7	A	

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CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year AM Peak Hour 2009
 Comment 2009 AM Peak BUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson St
 Minor Street Jefferson Plaza

Input Data

Lane Configuration	NB			SB			WB			EB		
Lane 1 (curb)	TR			R			LTR			LTR		
Lane 2	T			T								
Lane 3	L			T								
Lane 4				L								
Lane 5												
Movement	NB			SB			WB			EB		
	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	154	534	10	7	578	144	2	7	4	22	1	26
PHF	0.91	0.91	0.91	0.88	0.88	0.88	0.75	0.75	0.75	0.75	0.75	0.75
Percent of heavy vehicles, HV	3	3	3	3	3	3	3	3	3	3	3	3
Flow rate	169	587	11	8	657	164	3	9	5	29	1	35
Flare storage (# of vehs)												
Median storage (# of vehs)							1			1		
Signal upstream of Movement 2												
Length of study period (h)	0.25											

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
WB	1	LTR	17	186	0.091	0	26.3	D	26.3 D
	2								
	3								
EB	1	LTR	65	323	0.201	1	18.9	C	18.9 C
	2								
	3								
NB	①		169	798	0.212	1	10.7	B	
SB	④		8	968	0.008	0	8.7	A	

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CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year PM Peak Hour 2009
 Comment 2009 PM Peak NOBUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson St
 Minor Street Jefferson Plaza

Input Data

Lane Configuration	NB			SB			WB			EB		
Lane 1 (curb)	TR			R			LTR			LTR		
Lane 2	T			T								
Lane 3	L			T								
Lane 4				L								
Lane 5												
Movement	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	10	741	5	11	719	15	7	1	10	76	1	53
PHF	0.86	0.86	0.86	0.97	0.97	0.97	0.75	0.75	0.75	0.75	0.75	0.75
Percent of heavy vehicles, HV	3	3	3	3	3	3	3	3	3	3	3	3
Flow rate	12	862	6	11	741	15	9	1	13	101	1	71
Flare storage (# of vehs)												
Median storage (# of vehs)							1			1		
Signal upstream of Movement 2												
Length of study period (h)	0.25											

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
WB	1	LTR	23	331	0.069	0	16.7	C	16.7
	2								
	3								C
EB	1	LTR	173	329	0.526	3	27.5	D	27.5
	2								
	3								D
NB	①		12	843	0.014	0	9.3	A	
SB	④		11	766	0.015	0	9.8	A	

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CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year PM Peak Hour 2009
 Comment 2009 PM Peak BUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson St
 Minor Street Jefferson Plaza

Input Data

Lane Configuration	NB			SB			WB			EB		
Lane 1 (curb)	TR			R			LTR			LTR		
Lane 2	T			T								
Lane 3	L			T								
Lane 4				L								
Lane 5												
Movement	NB			SB			WB			EB		
	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	28	741	5	11	719	25	7	1	10	127	1	138
PHF	0.86	0.86	0.86	0.97	0.97	0.97	0.75	0.75	0.75	0.75	0.75	0.75
Percent of heavy vehicles, HV	3	3	3	3	3	3	3	3	3	3	3	3
Flow rate	33	862	6	11	741	26	9	1	13	169	1	184
Flare storage (# of vehs)												
Median storage (# of vehs)							1			1		
Signal upstream of Movement 2												
Length of study period (h)	0.25											

Output Data

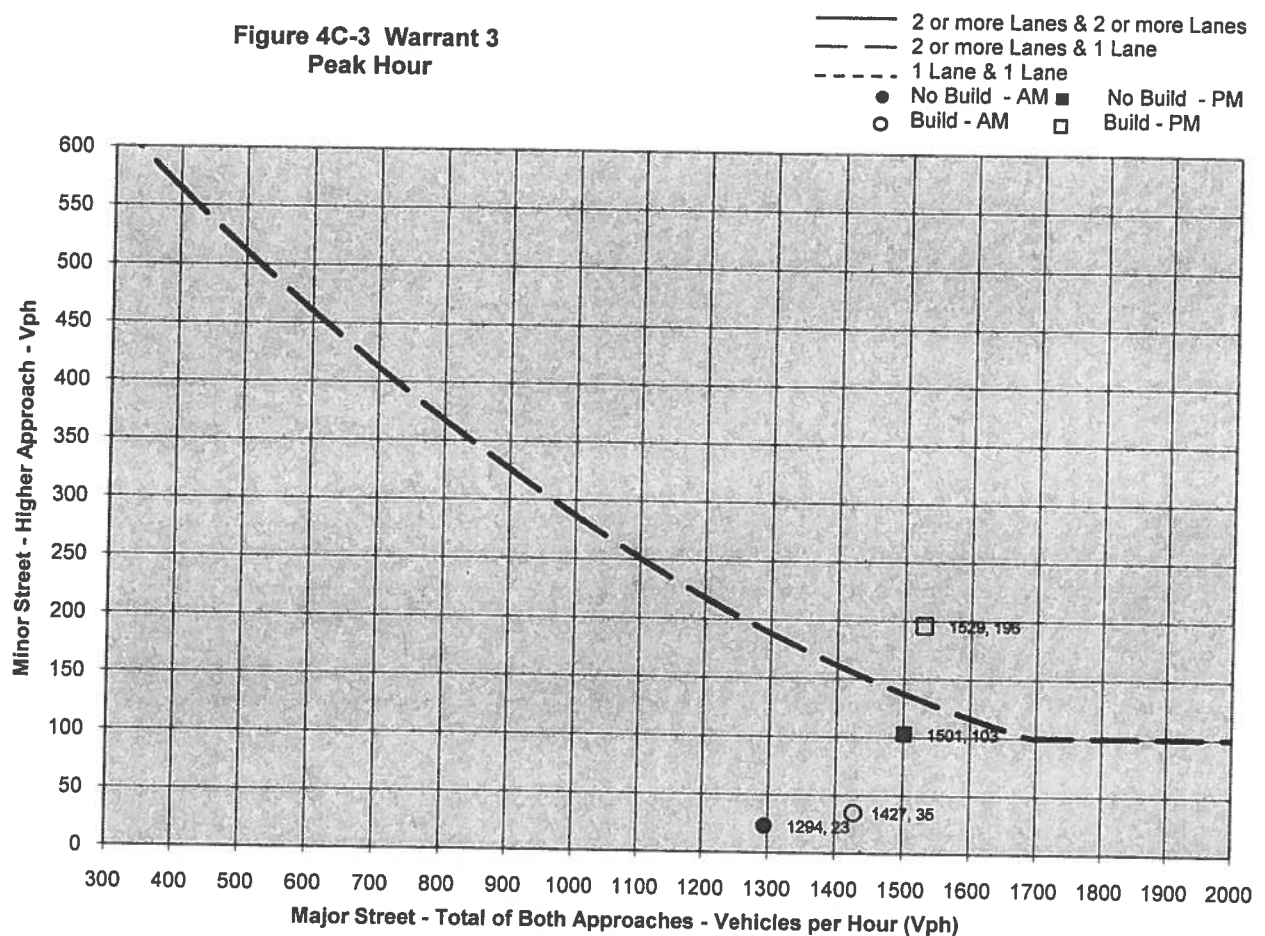
	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
WB	1	LTR	23	294	0.078	0	18.3	C	18.3 C 86.4 F
	2								
	3								
EB	1	LTR	354	350	1.012	12	86.4	F	86.4 F
	2								
	3								
NB	①		33	836	0.039	0	9.5	A	
SB	④		11	766	0.015	0	9.8	A	

Project Name
 Jefferson Plaza Office Development
Intersection
 Jefferson Plaza / Jefferson St.
Analysis Year
 2009

Number of Lanes
 Major St. 2
 Minor St. 1

Analysis Year Traffic Volumes					
AM	Major	Minor	PM	Major	Minor
No Build	1294	23	No Build	1501	103
Build	1427	35	Build	1529	196

**Figure 4C-3 Warrant 3
 Peak Hour**



Comments -

Analysis of Intersection #4

Jefferson Plaza / Driveway 'A'

CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year AM Peak Hour 2009
 Comment 2009 AM Peak BUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson Plaza
 Minor Street Driveway 'A'

Input Data

Lane Configuration	EB			WB			NB			SB		
Lane 1 (curb)	LT			TR						LR		
Lane 2												
Lane 3												
Lane 4												
Lane 5												
Movement	EB			WB			NB			SB		
	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	1	37			215	80				11		1
PHF	0.88	0.88			0.88	0.88				0.85		0.85
Percent of heavy vehicles, HV	3	3			3	3				3		3
Flow rate	1	42			244	91				13		1
Flare storage (# of vehs)												
Median storage (# of vehs)												
Signal upstream of Movement 2												
Length of study period (h)	0.25											

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
NB	1								
	2								
	3								
SB	1	LR	14	664	0.021	0	10.5	B	10.5 B
	2								
	3								
EB	①		1	1218	0.001	0	8.0	A	
WB	④								

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CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year PM Peak Hour 2009
 Comment 2009 PM Peak BUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson Plaza
 Minor Street Driveway 'A'

Input Data

Lane Configuration	EB			WB			NB			SB		
Lane 1 (curb)	LT			TR						LR		
Lane 2												
Lane 3												
Lane 4												
Lane 5												
Movement	EB			WB			NB			SB		
	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	1	183			35	17				82		1
PHF	0.97	0.97			0.97	0.97				0.85		0.85
Percent of heavy vehicles, HV	3	3			3	3				3		3
Flow rate	1	189			36	18				96		1
Flare storage (# of vehs)												
Median storage (# of vehs)												
Signal upstream of Movement 2												
Length of study period (h)	0.25											

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
NB	1								
	2								
	3								
SB	1	LR	97	752	0.129	0	10.5	B	10.5 B
	2								
	3								
EB	①		1	1545	0.001	0	7.3	A	
WB	④								

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Analysis of Intersection #5
Jefferson Plaza / Driveway 'B'

CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year AM Peak Hour 2009
 Comment 2009 AM Peak BUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson Plaza
 Minor Street Driveway 'B'

Input Data

Lane Configuration	EB			WB			NB			SB		
Lane 1 (curb)	LT			TR						LR		
Lane 2												
Lane 3												
Lane 4												
Lane 5												
Movement	EB			WB			NB			SB		
	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	1	30			162	53				7		1
PHF	0.88	0.88			0.88	0.88				0.85		0.85
Percent of heavy vehicles, HV	3	3			3	3				3		3
Flow rate	1	34			184	60				8		1
Flare storage (# of vehs)												
Median storage (# of vehs)												
Signal upstream of Movement 2				ft			Movement 5			ft		
Length of study period (h)				0.25								

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
NB	1								
	2								
	3								
SB	1	LR	9	744	0.012	0	9.9	A	9.9 A
	2								
	3								
EB	①		1	1316	0.001	0	7.7	A	
WB	④								

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CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year PM Peak Hour 2009
 Comment 2009 PM Peak BUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 6/13/2007
 Major Street Jefferson Plaza
 Minor Street Driveway 'B'

Input Data

Lane Configuration	EB			WB			NB			SB		
Lane 1 (curb)	LT			TR						LR		
Lane 2												
Lane 3												
Lane 4												
Lane 5												
Movement	EB			WB			NB			SB		
	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	1	129			24	11				54		1
PHF	0.97	0.97			0.97	0.97				0.85		0.85
Percent of heavy vehicles, HV	3	3			3	3				3		3
Flow rate	1	133			25	11				64		1
Flare storage (# of vehs)												
Median storage (# of vehs)												
Signal upstream of Movement 2												
Length of study period (h)	0.25											

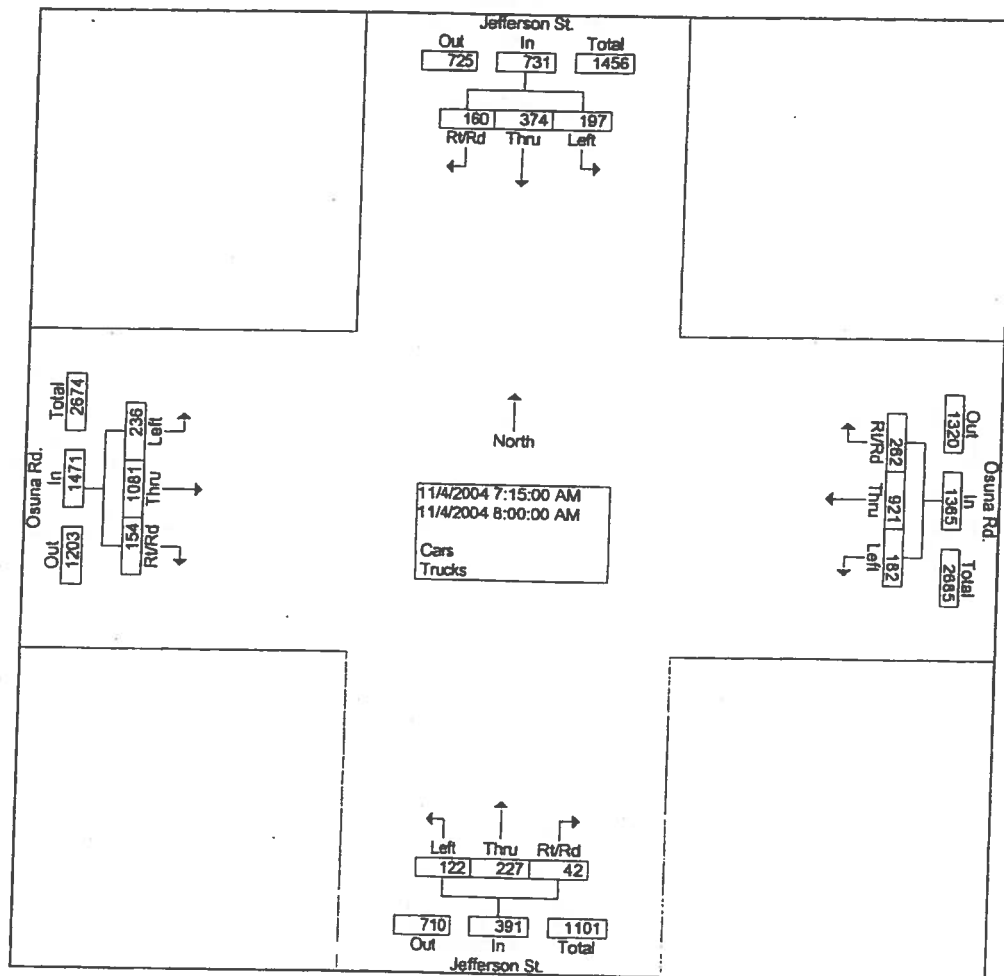
Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
NB	1								
	2								
	3								
SB	1	LR	65	825	0.079	0	9.7	A	9.7 A
	2								
	3								
EB	①		1	1568	0.001	0	7.3	A	
WB	④								

Mid-Region Council of Governments
Intersection Turning Movement Analysis

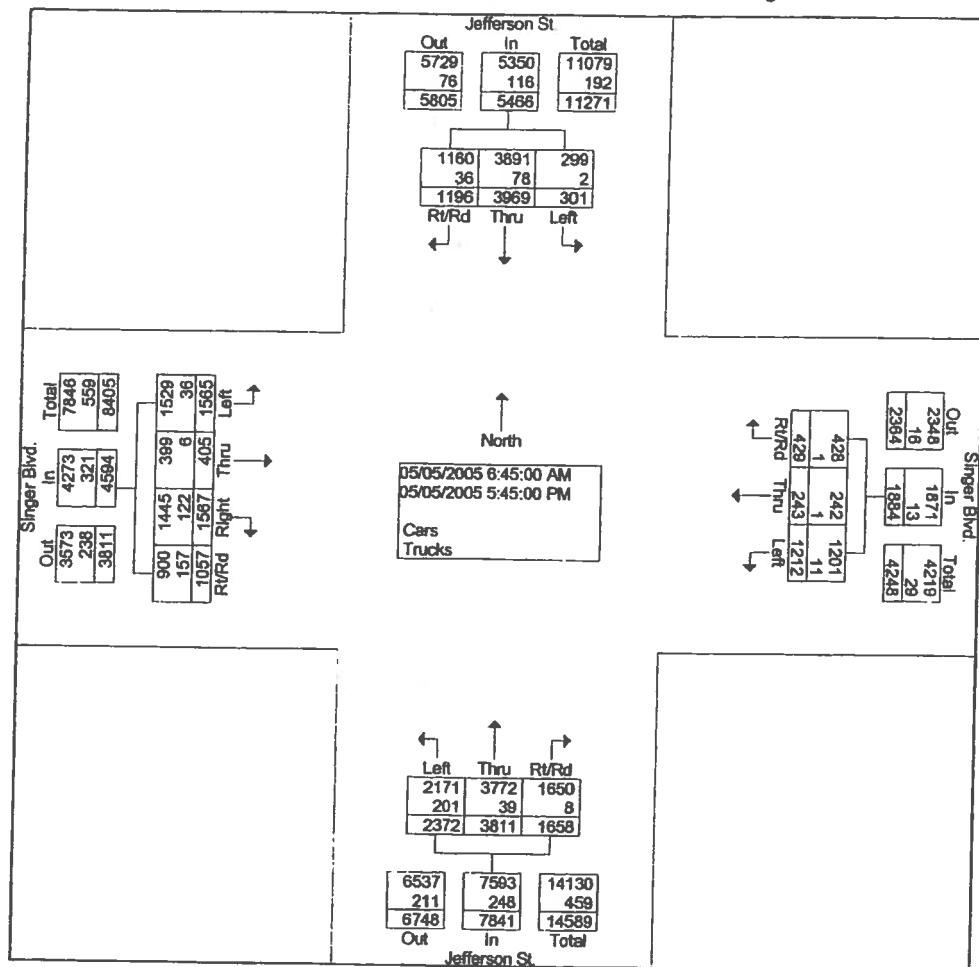
File Name : Osuna Rd. and Jefferson St
Site Code : 00025373
Start Date : 11/04/2004
Page No : 3

Start Time	Jefferson St. From North					Osuna Rd. From East					Jefferson St. From South					Osuna Rd. From West					InL Total
	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	
Peak Hour From 06:45 to 08:30 - Peak 1 of 1																					
Intersection 07:15																					
Volume	197	374	157	3	731	182	921	251	11	1365	122	227	30	12	391	236	1081	147	7	1471	3958
Percent	26.9	51.2	21.5	0.4		13.3	67.5	18.4	0.8		31.2	58.1	7.7	3.1		16.0	73.5	10.0	0.5		
Volume	197	374	157	3	731	182	921	251	11	1365	122	227	30	12	391	236	1081	147	7	1471	3958
Volume	58	105	47	1	211	53	264	74	0	391	34	61	9	4	108	91	285	41	1	418	1128
Peak Factor																					0.877
High Int. 07:45																					
Volume	58	105	47	1	211	53	264	74	0	391	34	61	9	4	108	91	285	41	1	418	
Peak Factor																					0.880



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Intersection Turning Movement Analysis

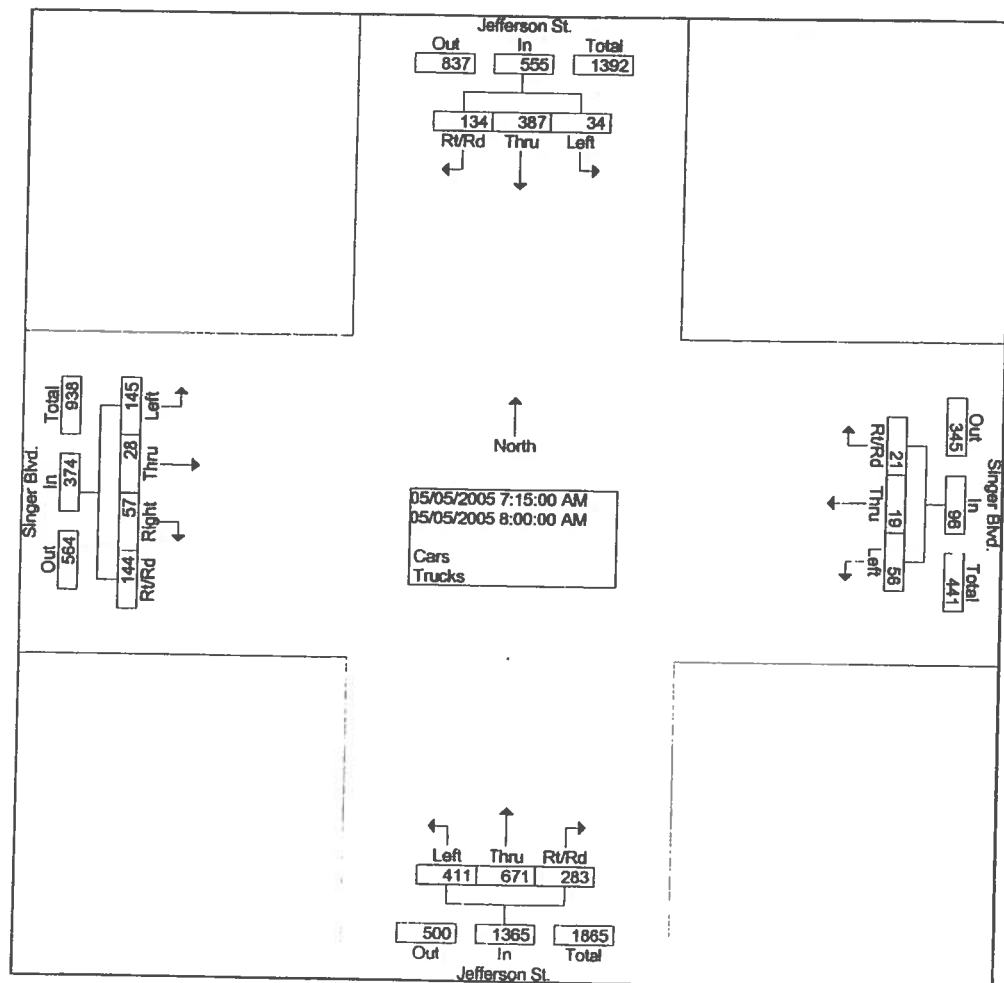
File Name : Singer Blvd. and Jefferson St.
Site Code : 00025880
Start Date : 05/05/2005
Page No : 2



Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Singer Blvd. and Jefferson St
Site Code : 00025880
Start Date : 05/05/2005
Page No : 3

	Jefferson St. From North					Singer Blvd. From East					Jefferson St. From South					Singer Blvd. From West					
Start Time	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Int. Total
Peak Hour From 06:45 to 09:30 - Peak 1 of 1																					
Intersection 07:15																					
Volume	34	387	121	13	555	56	19	21	0	96	411	671	282	1	1365	145	28	57	144	374	2390
Percent	6.1	69.7	21.8	2.3		58.3	19.8	21.9	0.0		30.1	49.2	20.7	0.1		38.8	7.5	15.2	38.5		
Volume	34	387	121	13	555	56	19	21	0	96	411	671	282	1	1365	145	28	57	144	374	2390
Volume	14	107	39	8	168	9	9	3	0	21	108	189	87	0	384	41	10	20	46	117	690
Peak Factor																					0.866
High Int. 07:45						08:00					07:45					07:45					
Volume	14	107	39	8	168	23	8	8	0	39	108	189	87	0	384	41	10	20	46	117	
Peak Factor					0.826					0.615					0.889						0.799



Signalized Intersection Information Sheet

Intersection: Jefferson Plaza / Jefferson St

Speed Limit - E-W Street: 25 M.P.H.

Speed Limit - N-S Street: 35 M.P.H.

Type of Intersection Control: Signalized

Date:

6/12/2007

East Bound Approach:

Jefferson Plaza

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	0	-	1	-	0						
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>				Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO	
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

West Bound Approach:

Jefferson Plaza

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	0	-	1	-	0						
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>				Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO	
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

North Bound Approach:

Jefferson St

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	0	-	1	1	0						
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>				Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO	
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

South Bound Approach:

Jefferson St

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	0	-	2	-	1						
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>				Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO	
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

NOTE: Existing Geometry