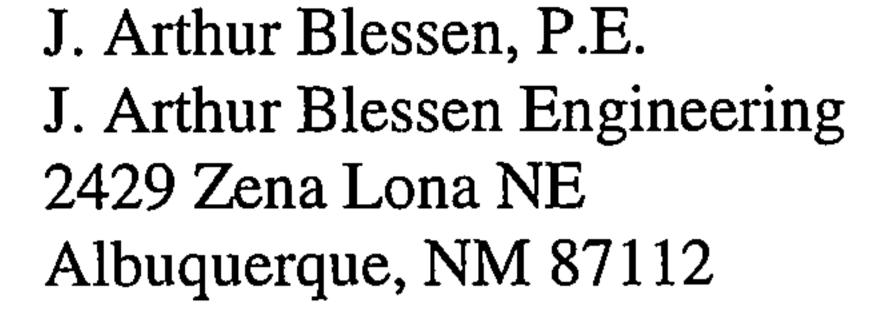
CITY OF ALBUQUERQUE

May 18, 2015



RE: Array Technologies Paving 3901 Midway PL NE Grading and Drainage Plan

Engineers Stamp Date 4/22/2015 (E17D034C)

Dear Mr. Blessen,

Based upon the information provided in your submittal received 4/22/2015, the above referenced Grading and Drainage Plan is approved for Grading Permit and Paving Permit.

Please attach a copy of this approved plan, dated 3-10-15, to the construction sets in the permitting process prior to sign-off by Hydrology.

When completed please provide a copy of the as-build for our records.

PO Box 1293

If you have any questions please contact me at 924-3999 or Rudy Rael at 924-3977.

Albuquerque

New Mexico 87103

www.cabq.gov

RR/SB

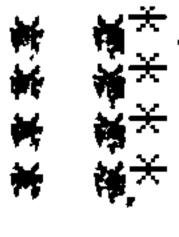
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Sincerely,

Shahab Biazar, P.E.

City Engineer

City of Albuquerque



DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 01/06 - KDM)

PROJECT TITLE: Array Technologies Paving	ZONE MAP: <u>E17/D034C</u>
DRB#: <u>13DRB-70553</u> EPC#: <u>1001360</u>) WORK ORDER#:
LEGAL DESCRIPTION: Lot B1B1A1, Albuquero	ue Industrial Park
CITY ADDRESS: 3901 Midway Place, N.E.	
ENGINEERING FIRM: J. Arthur Blessen Enginee	ering CONTACT: Art Blessen
ADDRESS: 2429 Zena Lona	PHONE: (505) 293-1477
CITY, STATE: Albuquerque, NM	ZIP CODE: 87112
OWNER: Array Technologies	CONTACT: David Mead
ADDRESS: 3901 Midway Place, N.E.	PHONE: (505) 881-7567
CITY, STATE: <u>Albuquerque, NM</u>	ZIP CODE: <u>87109</u>
ARCHITECT:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
SURVEYING FIRM:	LICENSED SURVEYOR:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
PROFESSIONAL LICENSED SURVEYOR SIGNATU	RE LICENSE NO. DATE
CONTRACTOR: Klinger Constructors, LLC	CONTACT: Joel Loes
CONTRACTOR: Klinger Constructors, LLC ADDRESS: 8701 Washington, N.E.	CONTACT: <u>Joel Loes</u> PHONE: <u>(505) 856-9990</u>
CONTRACTOR: Klinger Constructors, LLC ADDRESS: 8701 Washington, N.E. CITY, STATE: Albuquerque, N.M.	CONTACT: <u>Joel Loes</u> PHONE: <u>(505) 856-9990</u> ZIP CODE: <u>87113</u>
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 01/06 - KDM)

PROJECT TITLE: Array Technologies Paving	ZONE MAP: <u>E17/D034C</u>	
DRB#: 13DRB-70553 EPC#: 1001360	WORK ORDER#:	
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LEGAL DESCRIPTION: Lot B1B1A1, Albuquerq	ue Industrial Park	
CITY ADDRESS: 3901 Midway Place, N.E.	do illudotitut i uit	
CITT ADDRESS. 3901 Midway Flace, N.E.		.
	ring CONTACT: Art Blessen -	And bite
ENGINEERING FIRM: J. Arthur Blessen Enginee	ring CONTACT: An Blessen	TYCHITC
ADDRESS: 2429 Zena Lona	PHONE: (505) 293-1477	401-414
CITY, STATE: <u>Albuquerque, NM</u>	ZIP CODE: <u>87112</u>	
OWNER: Array Technologies	CONTACT: David Mead	
ADDRESS: 3901 Midway Place, N.E.	PHONE: (505) 881-7567	
CITY, STATE: Albuquerque, NM	ZIP CODE: 87109	
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A DOUTEOT.	CONTACT:	
ARCHITECT:		
ADDRESS:	PHONE:	
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SURVEYING FIRM:	LICENSED SURVEYOR:	
ADDRESS:	PHONE:	
CITY, STATE:	ZIP CODE:	
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PROFESSIONAL LICENSED SURVEYOR SIGNATU	RE LICENSE NO. DAT	TE
<u> </u>	RE LICENSE NO. DATE CONTACT: Joel Loes	TE .
CONTRACTOR: Klinger Constructors, LLC		
CONTRACTOR: Klinger Constructors, LLC ADDRESS: 8701 Washington, N.E.	CONTACT: <u>Joel Loes</u> PHONE: <u>(505) 856-9990</u>	TE
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

Revised Drainage Report

for

Array Tech Paving

3901 Midway Place NE Albuquerque, New Mexico

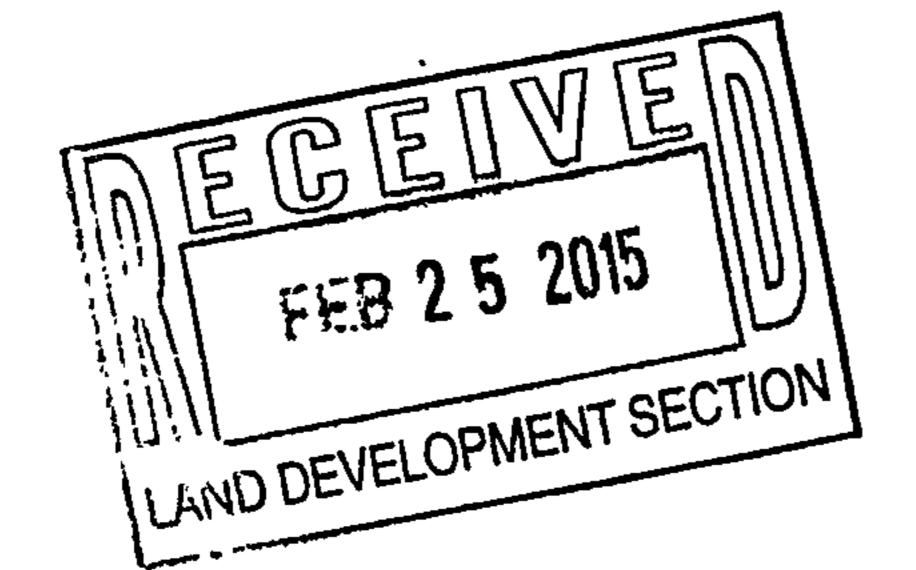
February 23, 2015

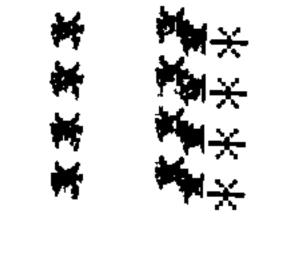
File: E17/D034C

by

J Arthur Blessen Engineering







Location:

34

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Tract B-1-B-1A-1A-1 Albuquerque Industrial Park, Bernalillo County New Mexico 3901 Midway Place NE.

Zone map E-17-Z

Located east of Office Blvd. at the east end of Midway Place, south of the Bear Arroyo North Diversion Channel.

Site Area: 6.16 Acres

Precipitation Zone 2

Flood Zone:

The subject site does not lie within a flood hazard zone (Map 35001C0138H panel 138 of 825). The site slopes from east to west. The Bear Arroyo is located to the north of the site. The lands to the west and south are lower than the site. The existing drainage channel along the east property line block flow from that direction. Therefore offsite flows are considered negligible.

Proposed Improvements:

The proposed development includes the paving of the existing gravel storage yard in Basin B-3 and a portion of Basin C-1.

Existing Conditions:

The existing 6.16 acre site is developed. The previous developments included the a warehouse, and asphalt parking lots and access road, landscaping, and detention ponds. The previously approved drainage plan (COA File #E17/D034C) directs the runoff from the site to two existing detention ponds, and to Midway Place.

The existing master drainage plan "Overall Drainage Plan" for the Nabisco Inc Warehouse (1990) established a discharge rate from the site of 2.1 cfs /acre (2.1cfs/acre x 6.16 acre = 12.9 cfs). The previously approved drainage plan for the site was prepared by Bohannan-Houston Inc (1-6-95) and divided the site into six sub-basins:

Basin A-1

A 0.582 ac portion of the site located at the southwest corner of the site. Runoff from the basin discharges to existing detention pond B. The previously approved drainage plan established the a discharged rate from pond B of 0.3 csf. The volume of the pond is 1077 cf at an elevation of 5128.6 ft. The proposed development will not effect basin A-1.

Basin A-2

A 0.446 ac portion of the site located north of basin A-1. Runoff from the basin discharges to Midway Place. The previously approved drainage plan established free discharge from the basin at a rate of 1.7 cfs. The proposed development will not effect basin A-2.



Basin B-1

The basin includes the existing warehouse and parking lot located to the west of the facility. Runoff from the basin flows to the existing catch basin located in the parking lot, and is routed to the existing detention pond located at the northwest corner of the site via a 21" dia culvert at a rate of 8 cfs. The proposed development will not effect basin B-1.

Basin B-2

The basin includes the existing service drive and landscaping area located at the southeast corner of the site. Runoff from the basin flows to an existing catch basin located east corner of the existing warehouse, and is routed to the existing detention pond located at the northwest corner of the site via a pipe culvert at a rate of 3.3 cfs. The proposed development will not effect basin B-2.

Basin B-3

The basin includes the gravel storage yard and asphalt service drive located at the northeast corner of the site. Runoff from the basin flows to an existing catch basin located on the east side of the existing loading dock, and is routed to the existing detention pond located at the northwest corner of the site via a 15" dia culvert at a rate of 3.9 cfs. The proposed development includes the paving of the existing gravel storage yard. The proposed new rate of discharge from basin B-3 is 4.6 cf. when combined with basin B-2 the flow through the 15" culvert is 7.9 cfs.

Basin C-1 (previously unnamed)

The basin includes the existing parking lot on the north side of the facility and the north section of the site. Runoff form the site discharges to the existing detention pond (pond A) located within the site. The proposed development includes the paving of a portion on the site, however the additional paving are (2077 sf) does not significantly increase the runoff from the site.

The combined runoff from basins B-1, B-2, B-3, and A is discharged to existing pond A at a rate of 23.5 cfs. The previously approved drainage plan established a discharge rate from the pond at a rate of 10.6 cfs via a 16.9 orifice to the existing asphalt swale within the drainage easement running parallel to the Bear Arroyo. The existing pond volume was established as 16,000 cf at an elevation of 5126.0.

The combined existing discharge from the site is (0.3cfs + 1.7cfs + 10.6cfs = 12.6cfs).

Note: The elevations have been adjusted to correspond to the current bench mark reference.

Developed Conditions:

The proposed development includes the paving of the gravel storage yard and a portion of basin C-1. The proposed additions will be confined to basins B-3 and C-1. The remainder of the site will remain unchanged. The runoff from the proposed addition will be directed to the existing detention pond A.

The combined area of basins B-1, B-2, B-3, and C-1 is 5.132 ac. and produces a combined runoff of 21.5 cfs. The existing orifice in the outlet pipe for pond A limits the discharge to 10.6 cfs. The required pond volume for the combined basis in 12,069 cfs. The existing pond A has a capacity of 12,124 cf at a elevation of 5125.78 ft, therefore no modification to the pond is required.

Summary:

As shown by the attached calculations the existing runoff for the site will be decreased form the existing due to the additional landscaping. The existing detention pond with a controlled discharge will control the addition runoff from the site.

Basin A-1 A-2	Historic 0.9 cfs 0.7 cfs	Existing 1.5 cfs 1.7 cfs	Proposed no change no change	to existing pond B discharge 0.3 cfs to Midway Place
B-1	2.5 cfs	7.0 cfs	no change	
B-2	1.1 cfs	3.0 cfs	no change	
B-3	1.6 csf	3.9 cfs	4.6 cfs	
C -1	2.8 cfs	6.9 cfs	6.9 cfs	
Total t	to pond	20.8 cfs	21.5 cfs	to existing pond A discharge 10.6 cfs

To follow are the calculations analyze the historic and proposed conditions for the 6-HOUR, 100 year event. The analysis is in. accordance with the City of Albuquerque Development Process Manual Volume II.

City of Albuquerque DPM 1997 edition

Basin A1

Precipitation Zone

Basin Area = 0.582 acres

Historic	Improved Condition	
Treatment	Treatment	

1 Octilionic					
Area of $A =$	25338 sf	100%	Area of A =	sf	0%
Area of $B =$	0 sf	0%	Area of $B =$	22709 sf	90%
Area of $C =$	0 sf	0%	Area of C =	0 sf	0%
Area of $D =$	0 sf	0%	Area of D =	2629 sf	10%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditions				Improved Conditions			
Treatment	% of Area		Ξn	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.90 x	0.78 =	0.70
С	0.00 x	1.13 =	0.00	С	0.00 x	1.13 =	0.00
D	0.00 x	2.12 =	0.00	D	0.10 x	2.12 =	0.22
		E =	0.53			E =	0.92
Volume V = E	A / 12						
Ve	= 0.530 x	0.5817 /	12 =	0.026	acre ft	1119 cf	
Vi	= 0.919 x	0.5817 /	12 =	0.045	acre ft	1941 cf	

Discharge Rate, Q (cfs / acre) 100 vr storm table A-9

Biodinargo i tato	$, \alpha (0.0) \alpha 0.0$	y. 0.0					
Treatment	% of Area		2	Treatment	% of Area	1	Q
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.90 x	2.28 =	2.04
С	0.00 x	3.14 =	0.00	С	0.00 x	3.14 =	0.00
D	0.00 x	4.7 =	0.00	D	0.10 x	4.7 =	0.49
		q =	1.56			q =	2.53

Peak Rate Qp = q A

$$Qp(e) = 1.56 \times 0.5817 = 0.91 \text{ cfs}$$

 $Qp(i) = 2.53 \times 0.5817 = 1.47 \text{ cfs}$

Excess Volume = 0.019 acre ft
Excess Rate = 0.56 cfs

tc = 0.2 hr

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.739 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.265 hr

Discharge Rate 0.30 cfs 0.52 cfs/ac

Volume 2027 cf Discharged - 720 cf Pond Voulme 1308 cf



Basin A1-Pond

	elevation	depth ft	area sf	volume cf
top of pond	66.75	0.75	7034	4737
	66	1	5598	4691
	65	0.5	3784	1243
bottom	64.5		1188	
				10671

City of Albuquerque DPM 1997 edition

Basin A2

Precipitation Zone

Basin Area = 0.422 acres

Historic			Improved Condition	ons	
Treatment			Treatment		
Area of $A =$	18384 sf	100%	Area of A =	sf	0%
Area of $B =$	0 sf	0%	Area of $B =$	3384 sf	18%
Area of C =	0 sf	0%	Area of $C =$	0 sf	0%
Area of D =	0 sf	ი%	Area of D =	15000 sf	82%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Condit	ions			Improved Cond	ditions		
Treatment	% of Area	E	Ξn	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.18 x	0.78 =	0.14
С	0.00 x	1.13 =	0.00	C	0.00 x	1.13 =	0.00
Đ	0.00 x	2.12 =	0.00	D	0.82 x	2.12 =	1.73
		E =	0.53			E =	1.87
Volume V = E	A/12						
Ve =	= 0.530 x	0.422 /	12 =	0.019 ad	cre ft	812 cf	
Vi =	= 1.873 x	0.422 /	12 =	0.066 ad	cre ft	2870 cf	

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9 % of Area % of Area Treatment Treatment 0.00 x1.56 =1.56 0.00 1.00 x 1.56 =Α Α 0.18 x2.28 =0.42 2.28 = 0.00 x0.00 3.14 =0.00 0.00 x3.14 =0.00 x0.00 4.7 = 0.82 x4.7 = 3.83 0.00 x0.00 4.25 1.56 q =

Peak Rate Qp = q A $Qp(e) = 1.56 \times 0.422 = 0.66 \text{ cfs}$ $Qp(i) = 4.25 \times 0.422 = 1.80 \text{ cfs}$

Excess Volume = 0.047 acre ft Excess Rate = 1.14 cfs

tc = 0.2 hr

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.724 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.205 hr

Discharge Rate 1.70 cfs 4.03 cfs/ac

Volume 2999 cf Discharged - 2924 cf

City of Albuquerque DPM 1997 edition

Basin B1

Precipitation Zone 2 Basin Area = 1.570 acres

Historic			Improved Condition	ons ·	
Treatment			Treatment		
Area of $A =$	68379 sf	100%	Area of $A =$	sf	0%
Area of $B =$	0 sf	0%	Area of B =	6822 sf	10%
Area of $C =$	0 sf	0%	Area of C =	0 sf	0%
Area of $D =$	0 sf	0%	Area of D =	61557 sf	90%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Condition	S			Improved Cond	ditions		
Treatment	% of Area	E	Ξn	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.10 x	0.78 =	0.08
С	0.00 x	1.13 =	0.00	C	0.00 x	1.13 =	0.00
D	0.00 x	2.12 =	0.00	D	0.90 x	2.12 =	1.91
		E =	0.53			Ε =	1.99
Volume V = E A /	12						
Ve =	0.530 x	1.5698 /	12 =	0.069 a	cre ft	3020 cf	
Vi =	1.986 x	1.5698 /	12 =	0.260 a	cre ft	11319 cf	

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9 % of Area % of Area Treatment Treatment 0.00 x1.56 =0.00 1.00 x 1.56 =1.56 Α Α В 0.23 2.28 = 0.10 x0.00 x2.28 =0.00 3.14 =0.00 0.00 x0.00 0.00 x3.14 =4.7 =4.23 0.90 x4.7 = 0.00 0.00 x4.46 1.56 q =q =

Peak Rate Qp = q A $Qp(e) = 1.56 \times 1.5698 = 2.45 \text{ cfs}$ $Qp(i) = 4.46 \times 1.5698 = 7.00 \text{ cfs}$

Excess Volume = 0.191 acre ft Excess Rate = 4.55 cfs

> tc = 0.2 hr tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.714 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.198 hr

Discharge Rate 7.00 cfs 4.46 cfs/ac

Volume 11825 cf Discharged - 11825 cf

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Drainage Calculation

City of Albuquerque DPM 1997 edition

Basin B2

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Precipitation Zone

Basin Area = 0.723 acres

Historic	Improved Conditions
Treatment	Treatment

100% Area of A =31500 sf Area of A =0% Area of B =Area of B = 0 sf 3652 sf 12% 0% Area of C = 0 sf Area of C =6924 sf 22% 0% Area of D =0 sf Area of D =0% 20924 sf 66%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditions Improved Conditions

	· 						
Treatment	% of Area	E	<u>En</u>	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.12 x	0.78 =	0.09
C	0.00 x	1.13 =	0.00	С	0.22 x	1.13 =	0.25
D	0.00 x	2.12 =	0.00	D	0.66 x	2.12 =	1.41
		E =	0.53			Ε =	1.75
1/-1 1/ F A /	40						

Volume V = EA/12

$$Ve = 0.530 \times 0.7231 / 12 = 0.032 \text{ acre ft}$$
 1391 cf $Vi = 1.747 \times 0.7231 / 12 = 0.105 \text{ acre ft}$ 4586 cf

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9

Treatment	% of Area	Ć	2	Treatment	% of Area	1	Q
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.12 x	2.28 =	0.26
С	0.00 x	3.14 =	0.00	С	0.22 x	3.14 =	0.69
D	0.00 x	4.7 =	0.00	D	0.66 x	4.7 =	3.12
		q =	1.56			q =	4.08

Peak Rate Qp = q A

$$Qp(e) = 1.56 \times 0.7231 = 1.13 \text{ cfs}$$

 $Qp(i) = 4.08 \times 0.7231 = 2.95 \text{ cfs}$

Excess Volume = 0.073 acre ft Excess Rate = 1.82 cfs

tc = 0.2 hr

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.737 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.218 hr

Discharge Rate 2.95 cfs 4.08 cfs/ac

Volume 4791 cf Discharged - 4791 cf



Basin B3

Precipitation Zone

Basin Area = 1.040 acres

Historic			Improved Condition	ons	
Treatment			Treatment		
Area of A =	45296 sf	100%	Area of A =	sf	0%
Area of $B =$	0 sf	0%	Area of $B =$	sf	0%
Area of C =	0 sf	0%	Area of C =	8266 sf	18%
Area of $D =$	0 sf	0%	Area of D =	37030 sf	82%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditions	S			Improved Cond	ditions		
Treatment	% of Area	E	≣n	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.00 x	0.78 =	0.00
С	0.00 x	1.13 =	0.00	C	0.18 x	1.13 =	0.21
D	0.00 x	2.12 =	0.00	D	0.82 x	2.12 =	1.73
		E =	0.53			E =	1.94
Volume V = E A /	12						
Ve =	0.530 x	1.0399 /	12 =	0.046 a	cre ft	2001 cf	
Vi =	1.939 x	1.0399 /	12 =	0.168 a	cre ft	7320 cf	

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9

α (0.0) wolo, is	, j. 0.0					
% of Area		2	Treatment	% of Area		Q
1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
0.00 x	2.28 =	0.00	В	0.00 x	2.28 =	0.00
0.00 x	3.14 =	0.00	С	0.18 x	3.14 =	0.57
0.00 x	4.7 =	0.00	D	0.82 x	4.7 =	3.84
	q =	1.56			q =	4.42
	% of Area 1.00 x 0.00 x 0.00 x	% of Area 1.56 = 0.00 x 2.28 = 0.00 x 3.14 = 0.00 x 4.7 =	$1.00 \times 1.56 = 1.56$ $0.00 \times 2.28 = 0.00$ $0.00 \times 3.14 = 0.00$ $0.00 \times 4.7 = 0.00$	% of Area Q Treatment 1.00 x 1.56 = 1.56 A 0.00 x 2.28 = 0.00 B 0.00 x 3.14 = 0.00 C 0.00 x 4.7 = 0.00 D	% of Area Q Treatment % of Area $1.00 \times 1.56 = 1.56$ A $0.00 \times 0.00 \times 2.28 = 0.00$ B $0.00 \times 0.00 \times 3.14 = 0.00$ C $0.18 \times 0.00 \times 4.7 = 0.00$ D $0.82 \times 0.00 \times 0.0$	% of Area Q Treatment % of Area 1.00 x 1.56 = 1.56 A 0.00 x 1.56 = 0.00 x 2.28 = 0.00 B 0.00 x 2.28 = 0.00 x 3.14 = 0.00 C 0.18 x 3.14 = 0.00 x 4.7 = 0.00 D 0.82 x 4.7 =

Peak Rate Qp = q A

$$Qp(e) = 1.56 \times 1.0399 = 1.62 \text{ cfs}$$

 $Qp(i) = 4.42 \times 1.0399 = 4.59 \text{ cfs}$

Excess Volume = 0.122 acre ft 5319.8 CF Excess Rate = 2.97 cfs

tc = 0.2 hr

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.721 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.205 hr

Discharge Rate 4.59 cfs 4.42 cfs/ac

Volume 7648 cf Discharged - 7648 cf

City of Albuquerque DPM 1997 edition

Basin C1

Precipitation Zone

Basin Area = 1.798 acres

Historic	Improved Conditions
Treatment	Treatment

Treatment			Treatment		
Area of A =	78336 sf	100%	Area of A =	sf	0%
Area of B =	0 sf	0%	Area of B =	24113 sf	31%
Area of $C =$	0 sf	0%	Area of $C =$	4935 sf	6%
Area of D =	0 sf	0%	Area of $D =$	49288 sf	63%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditions			Improved Conditions				
Treatment	% of Area	E	n	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.31 x	0.78 =	0.24
С	0.00 x	1.13 =	0.00	С	0.06 x	1.13 =	0.07
D	0.00 x	2.12 =	0.00	D	0.63 x	2.12 =	1.33
		E =	0.53			E =	1.65
Volume V = E A	/12						

12 =

12 =

0.079 acre ft

0.247 acre ft

3460 cf

10740 cf

Discharge Rate, O (cfs / acre) 100 vr storm table A-9

0.530 x 1.7983 /

1.645 x 1.7983 /

Discharge Hate	, G (CIS / acre) Toc	yı stolili ta	DIC A 3				
Treatment	% of Area)	Treatment	% of Area	1	Q
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	· B	0.31 x	2.28 =	0.70
С	0.00 x	3.14 =	0.00	С	0.06 x	3.14 =	0.20
D	0.00 x	4.7 =	0.00	D	0.63 x	4.7 =	2.96
		a =	1.56			a =	3.86

Peak Rate Qp = q A

Ve =

$$Qp(e) = 1.56 \times 1.7983 = 2.81 \text{ cfs}$$

 $Qp(i) = 3.86 \times 1.7983 = 6.94 \text{ cfs}$

Excess Volume = 0.167 acre ft Excess Rate = 4.13 cfs

tc = 0.2 hr

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.741 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.221 hr

Discharge Rate 6.94 cfs 3.86 cfs/ac

Volume 11221 cf Discharged - 11221 cf

City of Albuquerque DPM 1997 edition

Basins B-1, B-2, B-3 and C-1 (to pond A)

Precipitation Zone

2

Basin Area = 5.132 acres

Historic			Improved Conditi	ions	
Treatment			Treatment		
Area of A =	223563 sf	100%	Area of $A =$	sf	0%
Area of $B =$	0 sf	0%	Area of B =	39587 sf	18%
Area of $C =$	0 sf	0%	Area of $C =$	22125 sf	10%
Area of $D =$	0 sf	0%	Area of $D =$	161851 sf	72%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Condition	ons			Improved Cond	litions		
Treatment	% of Area	E	∃n	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.18 x	0.78 =	0.14
С	0.00 x	1.13 =	0.00	C	0.10 x	1.13 =	0.11
D	0.00 x	2.12 =	0.00	D	0.72 x	2.12 =	1.53
		E =	0.53			E =	1.78
Volume V = E A	/ 12					•	
Ve =	0.530 x	5.1323 /	12 =	0.227 ac	cre ft	9874 cf	
Vi =	1.785 x	5.1323 /	12 =	0.763 ad	cre ft	33250 cf	

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9

Biodilaigo itato	$, \alpha (0.0) \alpha 0.0$, j. 0.0a.	 •				
Treatment	% of Area)	Treatment	% of Area	1	Q
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.18 x	2.28 =	0.40
С	0.00 x	3.14 =	0.00	С	0.10 x	3.14 =	0.31
D	0.00 x	4.7 =	0.00	D	0.72 x	4.7 =	3.40
		q =	1.56			q =	4.12

Peak Rate Qp = q A

$$Qp(e) = 1.56 \times 5.1323 = 8.01 \text{ cfs}$$

 $Qp(i) = 4.12 \times 5.1323 = 21.13 \text{ cfs}$

Excess Volume = 0.537 acre ft
Excess Rate = 13.12 cfs

tc = 0.2 hr

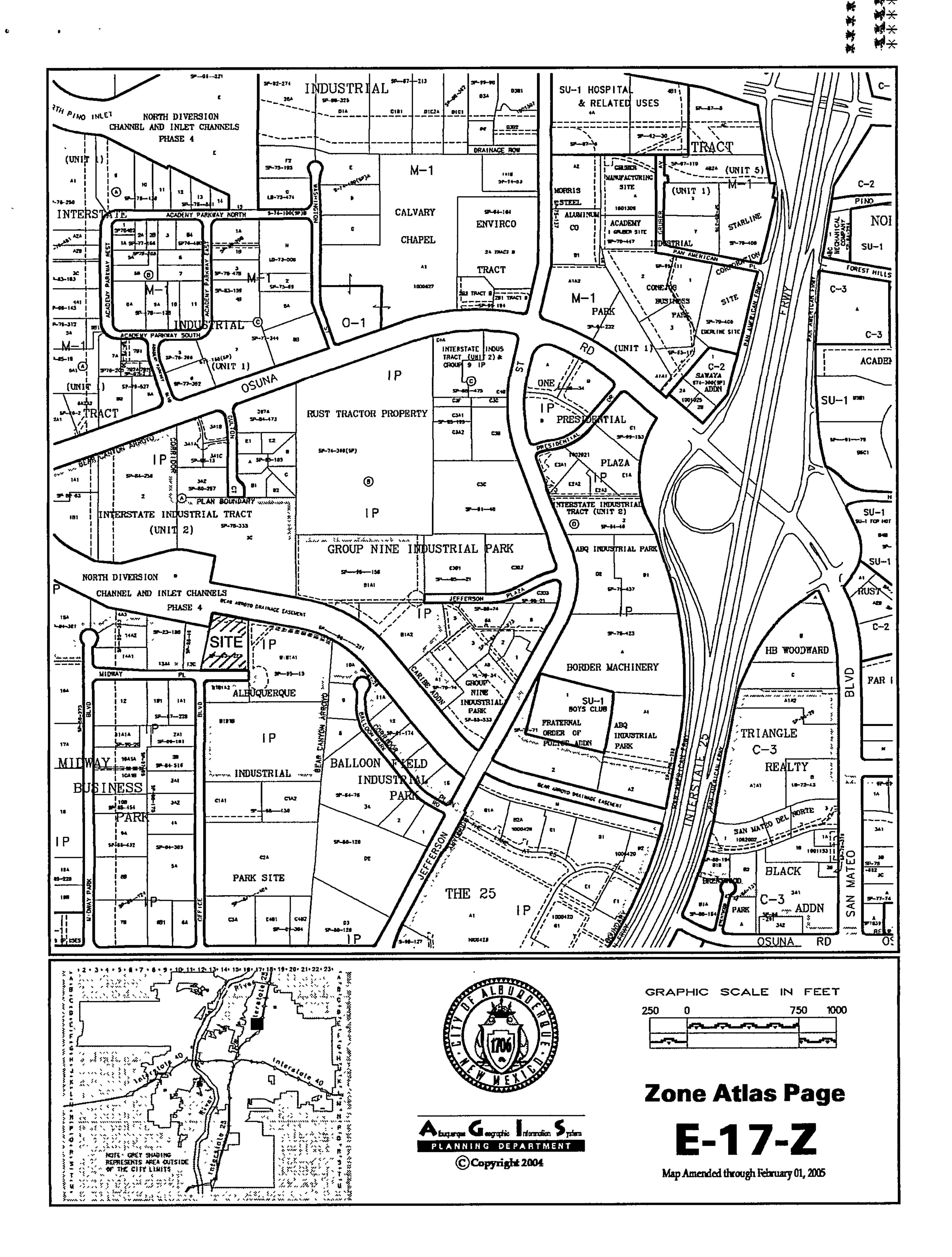
tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.732 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.213 hr

Discharge Rate 10.60 cfs 2.07 cfs/ac

Volume 34740 cf Discharged - 22670 cf Pond Voulme 12069 cf

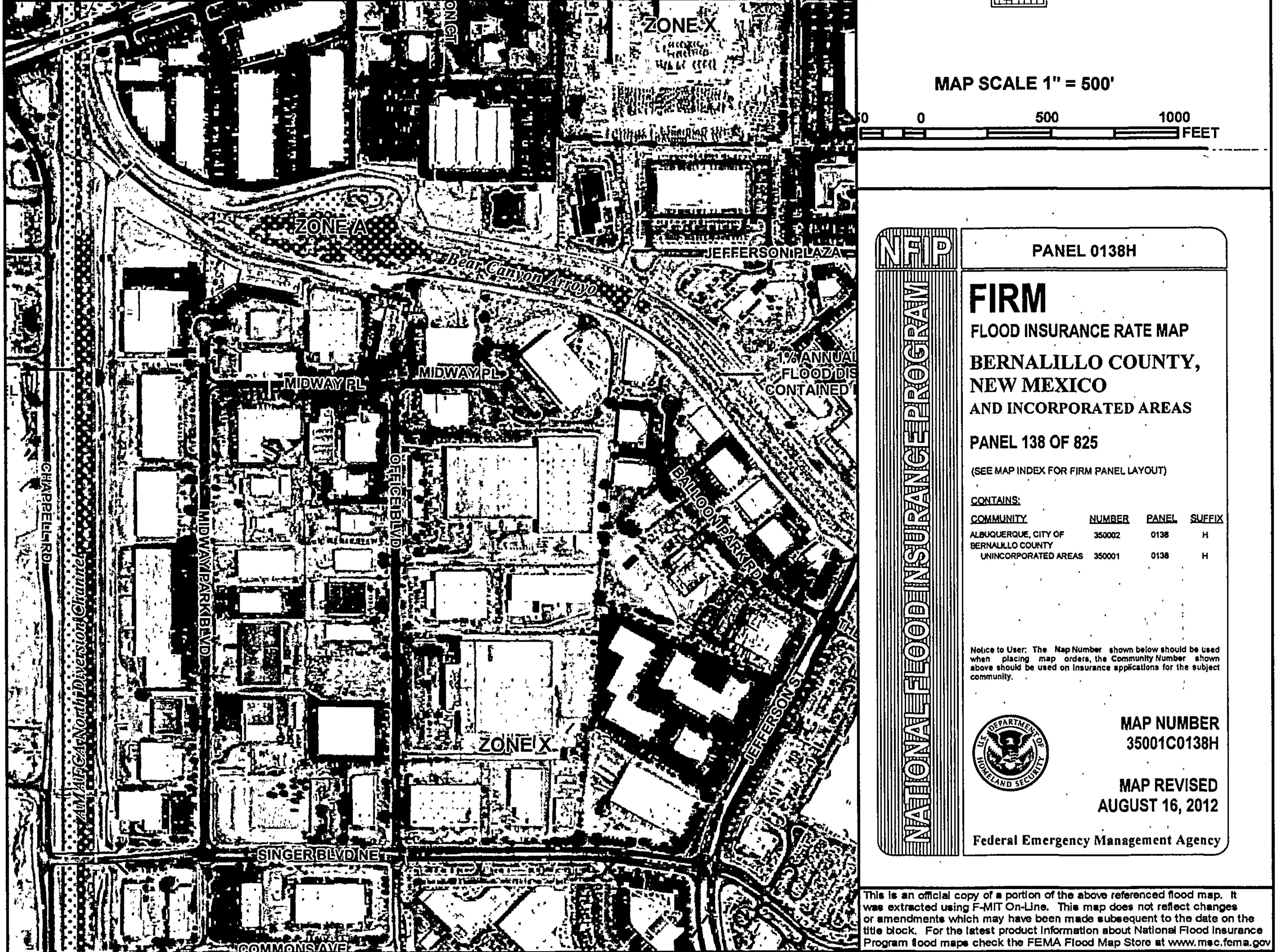


top of pond	elevation 25.78 25	depth ft 0.78 1	area sf 6305 5582	volume cf 4635.93 4936	6809
L _ 11	24	1.19	4290	2552.55	
bottom	22.81		0	12124.48	



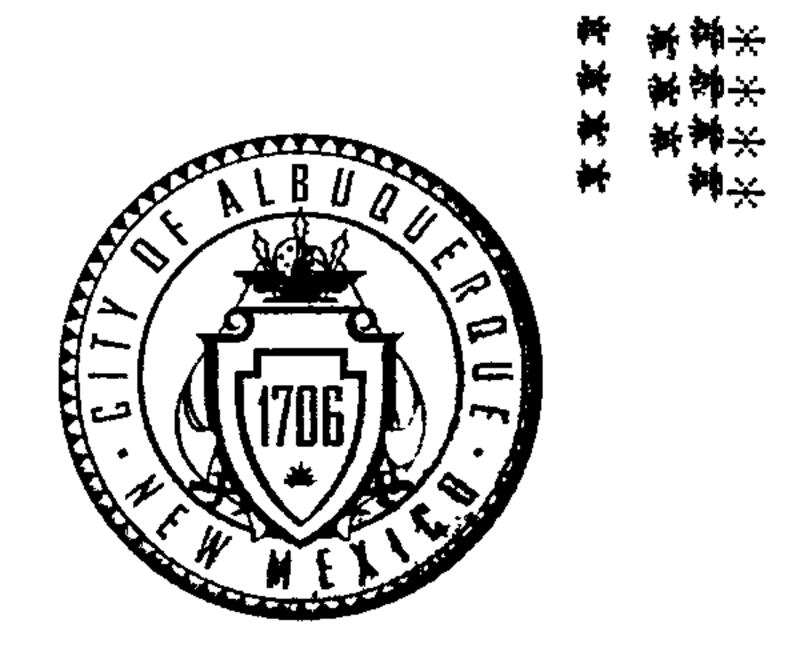


宋末宋宋



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CITY OF ALBUQUERQUE



June 25, 2015

J. Arthur Blessen, P.E.
J. Arthur Blessen Engineering
2429 Zena Lona NE
Albuquerque, NM 87112

Re: Array Technologies Paving

3901 Midway PL NE

Request Permanent CO - Accepted

Engineers Stamp Date 4/22/2015 (E17D034C)

Certification dated: 8-12-14

Dear Mr. Blessen,

Based on the Certification received 6/22/2015, the above named business is acceptable for release of Certificate of Occupancy by Hydrology.

PO Box 1293

If you have any questions, you can contact me at 924-3695 or Rudy Rael at 924-3977.

Albuquerque

New Mexico 87103

Rita Harmon, P.E.

Sincerely,

Principal Engineer, Hydrology

Planning Department

www.cabq.gov

RR/RH

email

C:

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 01/06 - KDM)

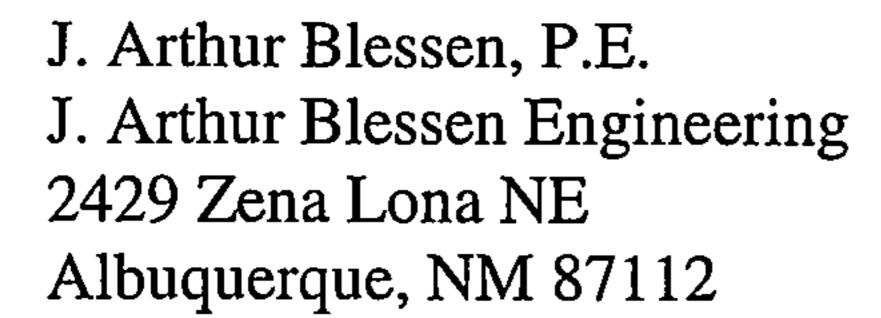
PROJECT TITLE: Array Technologies Paving	ZONE MAP: <u>E17/D034C</u>
DRB#: 13DRB-70553 EPC#: 1001360	WORK ORDER#:
LEGAL DESCRIPTION: Lot B1B1A1, Albuquerq	ue Industrial Park
CITY ADDRESS: 3901 Midway Place, N.E.	
	• • • • • • • • • • • • • • • • • • •
ENGINEERING FIRM: J. Arthur Blessen Engineer	
ADDRESS: 2429 Zena Lona	PHONE: (505) 401-4142
CITY, STATE: <u>Albuquerque, NM</u>	ZIP CODE: <u>87112</u>
OWNER: Array Technologies	CONTACT: David Mead
ADDRESS: 3901 Midway Place, N.E.	PHONE: (505) 881-7567
CITY, STATE: <u>Albuquerque</u> , NM	ZIP CODE: <u>87109</u>
	CONTCACC
ARCHITECT:	CONTACT:
ADDRESS:	PHONE: ZIP CODE:
CITY, STATE:	ZIF CODE.
SURVEYING FIRM:	LICENSED SURVEYOR:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
PROFESSIONAL LICENSED SURVEYOR SIGNATU	<u> </u>
CONTRACTOR: Klinger Constructors, LLC	CONTACT: Joel Loes
ADDRESS: 8701 Washington, N.E.	PHONE: (505) 856-9990
CITY, STATE: Albuquerque, N.M.	ZIP CODE: 87113
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANTEE RELEASE
DRAINAGE PLAN 1 st SUBMITTAL	PRELIMINARY PLAT APPROVAL
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D APPROVAL S. DEV. FOR BLDG. PERMIT APPROVAL
CDADING DE AN	SECTOR PLAN APPROVAL
GRADING PLAN EROSION CONTROL PLAN	FINAL PLAT APPROVAL
X ENGINEER'S CERT (HYDROLOGY)	FOUNDATION PERMIT APPROVAL
CLOMR/LOMR	BUILDING PERMIT APPROVAL
TRAFFIC CIRCULATION LAYOUT	X CERTIFICATE OF OCCUPANCY
ENGINEER'S CERT (TCL)	GRADING PERMIT APPROVAL
ENGINEER'S CERT (DRB SITE PLAN)	PAVING PERMIT APPROVAL
OTHER-	WORK ORDER APPROVAES TO TO TO THE MENT OF THE PROPERTY OF THE
	OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED	
YES	
X NO	
X_NO COPY PROVIDED	BY: J Arthur Blessen LAND DEVELOPMENT SECTION

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

CITY OF ALBUQUERQUE

March 11, 2015



RE: Array Technologies Paving
3901 Midway PL NE
Grading and Drainage Plan
Engineers Stamp Date 2/23/2015 (E17D034C)

Dear Mr. Blessen,

Based upon the information provided in your submittal received 2/25/2015, the above referenced Grading and Drainage Plan cannot be approved for Grading Permit or Paving Permit until the following comments are addressed.

• Mention of the first flush per EPA standards is required and placed on the plans. The amount required for detention is .34" times the new impervious area. Depress the landscape islands/areas as part of the first flush requirements.

PO Box 1293

If you have any questions please contact me at 924-3986 or Rudy Rael at 924-3977.

Albuquerque

RR/CC

C: File

New Mexico 87103

www.cabq.gov

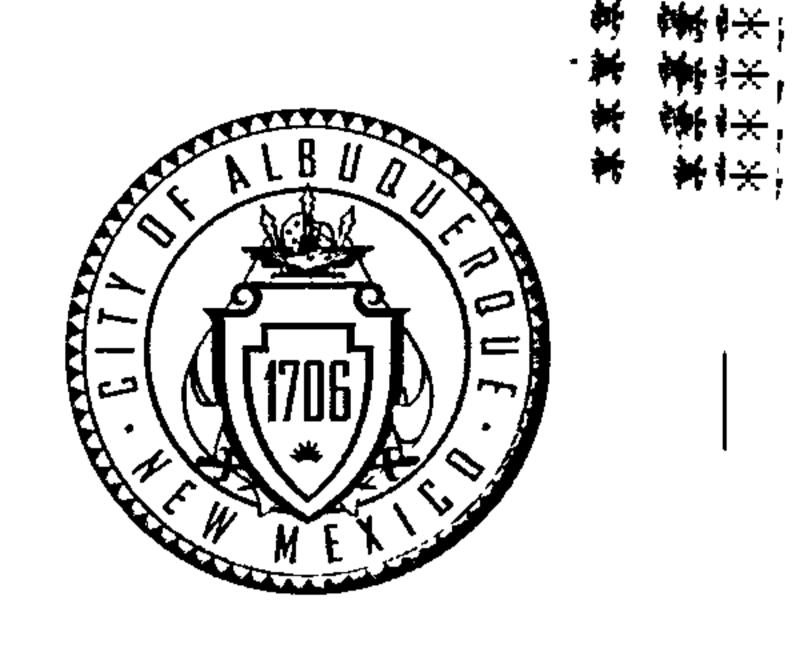
Sincerely,

Curtis Cherne, P.E.

Principal Engineer, Hydrology

Planning Department

CITY OF ALBUQUERQUE



Planning Department Transportation Development Services Section

February 27, 2014

Claudio Vigil, R.A. Claudio Vigil Architects 1801 Rio Grande Blvd. NW Albuquerque, NM 87104

Re: Array Technologies, 3901 Midway Place NE

Certificate of Occupancy – Transportation Development

Architect's Stamp dated 2-21-14 (E17-D034)

Dear Mr. Vigil,

Based upon the information provided in your submittal received 02-26-14, Transportation Development has no objection to the issuance of a <u>Permanent Certificate of Occupancy</u>. This letter serves as a "green tag" from Transportation Development for a <u>Permanent Certificate of Occupancy</u> to be issued by the Building and Safety Division.

PO Box 1293

If you have any questions, please contact me at (505)924-3630.

Albuquerque

Sincerely,

New Mexico 87103

Racquel M. Michel, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

www.cabq.gov

c: File

CO Clerk

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 01/06 - KDM)

E170043C

LEGAL DESCRIPTION: Lot B1B1A1, Albuquerque Industrial Park	ZONE MAP: <u>E-17-Z</u>
LEGAL DESCRIPTION: Lot B1B1A1, Albuquerque Industrial Park	K ORDER#:
	k-
CITY ADDRESS: 3901 Midway Place, N.E.	
CIT I ADDRESS. <u>3901 Mildway Flace, 14.E.</u>	
ENGINEERING FIRM: J. Arthur Blessen Engineering	CONTACT: Art Blessen
ADDRESS: 2625 Pennsylvania Boulevard, N.E., Suite 600	
CITY, STATE: Albuquerque, NM	ZIP CODE: <u>87110</u>
OWNER: Array Technologies	CONTACT: David Mead
ADDRESS: 3901 Midway Place, N.E.	PHONE: <u>(505) 881-7567</u>
CITY, STATE: <u>Albuquerque, NM</u>	ZIP CODE: <u>87109</u>
ARCHITECT: Claudio Vigil Architects	CONTACT: Sandra M. Fairchild
ADDRESS: 1801 Rio Grande Boulevard, N.W.	PHONE: (505) 84 2 -1113
CITY, STATE: Albuquerque, N M	ZIP CODE: <u>87104</u>
SURVEYING FIRM: LICEN	NSED SURVEYOR:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
PROFESSIONAL LICENSED SURVEYOR SIGNATURE CONTRACTOR: Klinger Constructors, LLC	LICENSE NO. DATE CONTACT: Alan White
CONTRACTOR: Klinger Constructors, LLC ADDRESS: 8701 Washington, N.E.	PHONE: (505) 856-9990
CITY, STATE: Albuquerque, N.M.	ZIP CODE: 87113
	OF APPROVAL SOUGHT:
	INANCIAL GUARANTEE RELEASE
	IMINARY PLAT APPROVAL
DD AINIACE DI ANI DECLIDATETAT C. DEL	V. PLAN FOR SUB'D APPROVAL
DRAINAGE PLAN RESUBMITTAL S. DEV	TO DOD DI DO DEDITE ADDROSTAT
	V. FOR BLDG. PERMIT APPROVAL
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CONCEPTUAL G & D PLAN GRADING PLAN EROSION CONTROL PLAN ENGINEER'S CERT (HYDROLOGY) CLOMR/LOMR BUILL X TRAFFIC CIRCULATION LAYOUT X CERT ENGINEER'S CERT (TCL) SHORT CONTROL PLAN OTHER- WORK OTHER YES X NO COPY PROVIDED	OR PLAN APPROVAL L PLAT APPROVAL IDATION PERMIT APPROVAL DING PERMIT APPROVAL DING PERMIT APPROVAL DING PERMIT APPROVAL NG PERMIT APPROVAL K ORDER APPROVAL ER (SPECIFY)

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more

HAS FILE. 13 2-26-14



13

February 21, 2014

Traffic Engineer
Development and Building Services
600 Second St. N.W
Albuquerque, New Mexico

RE: TRAFFIC CERTIFICATION - Submittal

Array Technologies 3901 Midway Place, N.E.

To whom it may concern:

I, Claudio Vigil, NM Registered Architect 1236, of the firm Claudio Vigil Architects, hereby certify that this project is in substantial compliance with and in accordance with the design intent of the site development plan approved through administrative amendment and permit set. The record information documented and edited onto the approved TCL Site Plan has been obtained by Sandra Fairchild of the firm Claudio Vigil Architects. I further certify that I have personally visited the project site on February 21, 2014 and have determined by visual inspection that the survey data provided is representative of actual site conditions and is true and correct to the best of my knowledge and belief and as shown on the attached site plans. This certification is submitted in support of a request for Certificate of Occupancy.

All work necessary to support the facility has been completed, and is in substantial compliance with the approved site development plan.

The record information presented hereon is not necessarily complete and intended only to verify substantial compliance of the traffic aspects of this project. Those relying on the record document are advised to obtain independent verification of its accuracy before using it for any

Signature of Architect

other purpose.

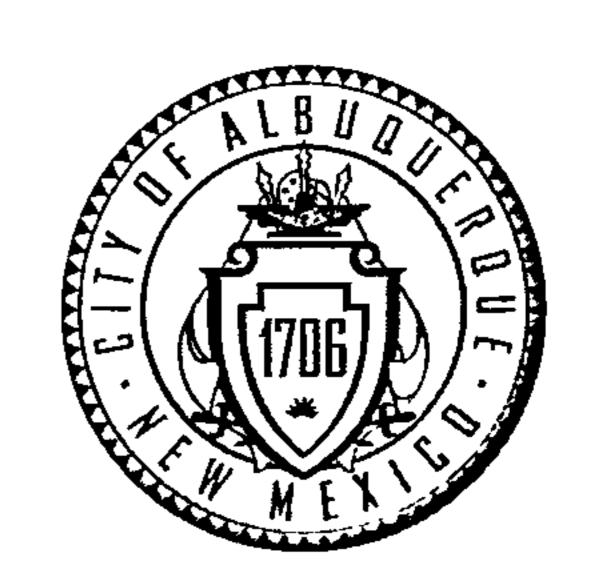
ARCHITECT'S STAMP

Date





CITY OF ALBUQUERQUE



February 27, 2014

John Arthur Blessen, P.E.
J. Arthur Blessen Engineering
2429 Zena Lona NE
Albuquerque, NM 87112

Re: Array Technologies 3901 MIDWAY Pl NE

Request for Permanent C.O. - Accepted

Engineer's Stamp dated: 07-08-13 (E17/D034C)

Certification dated: 2-26-14

Dear Mr. Blessen,

Based on the Certification received 2/26/2014, the site is acceptable for release of Permanent Certificate of Occupancy by the Hydrology.

PO Box 1293

Hydrology is asking for an electronic copy, in .pdf format, of this certification for our records once the permanent CO is accepted. This certification can be e-mailed to: rrael@cabq.gov.

Albuquerque

If you have any questions, you can contact me at 924-3986 or Rudy Rael at 924-3977.

New Mexico 87103

www.cabq.gov

Sincerely,

Lux a Chin

Curtis Cherne, P.E.

Principal Engineer, Planning Dept. Development and Review Services

RR/CC

C: CO Clerk—Katrina Sigala

email

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 01/06 - KDM)

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	ZONE MAP: <u>E17/D034C</u>
DRB#: <u>13DRB-70553</u> EPC#: <u>1001360</u>	WORK ORDER#:
T DO AT INDOORDING TO THE COMMENT OF	T 1 . • 1 T 1
LEGAL DESCRIPTION: Lot B1B1A1, Albuquero	ue Industrial Park
CITY ADDRESS: 3901 Midway Place, N.E.	· · · · · · · · · · · · · · · · · · ·
ENGINEERING FIRM: J. Arthur Blessen Enginee	ering CONTACT: Art Blessen
ADDRESS: 2429 Zena Lona	PHONE: (505) 293-1477
CITY, STATE: Albuquerque, NM	ZIP CODE: 87112
OWNER: Array Technologies	CONTACT: David Mead
ADDRESS: 3901 Midway Place, N.E.	PHONE: (505) 881-7567
CITY, STATE: <u>Albuquerque, NM</u>	ZIP CODE: <u>87109</u>
ARCHITECT: Claudio Vigil Architects	CONTACT: Sandra M. Fairchild
ADDRESS: 1801 Rio Grande Boulevard, N	
CITY, STATE: Albuquerque, N M	ZIP CODE: <u>87104</u>
CHIDMEVINIC TIDM.	I ICENICED CHIDMENAD.
SURVEYING FIRM:ADDRESS:	LICENSED SURVEYOR: PHONE:
CITY, STATE:	ZIP CODE:
PROFESSIONAL LICENSED SURVEYOR SIGNATU	RE LICENSE NO. DATE
CONTRACTOR: Klinger Constructors, LLC	CONTACT: Alan White
ADDRESS: 8701 Washington, N.E.	PHONE: (505) 856-9990
ADDRESS: 8701 Washington, N.E. CITY, STATE: Albuquerque, N.M.	PHONE: (505) 856-9990 ZIP CODE: 87113
ADDRESS: 8701 Washington, N.E. CITY, STATE: Albuquerque, N.M. TYPE OF SUBMITTAL:	PHONE: (505) 856-9990 ZIP CODE: 87113 CHECK TYPE OF APPROVAL SOUGHT:
ADDRESS: 8701 Washington, N.E. CITY, STATE: Albuquerque, N.M. TYPE OF SUBMITTAL: DRAINAGE REPORT	PHONE: (505) 856-9990 ZIP CODE: 87113 CHECK TYPE OF APPROVAL SOUGHT: SIA/FINANCIAL GUARANTEE RELEASE
ADDRESS: 8701 Washington, N.E. CITY, STATE: Albuquerque, N.M. TYPE OF SUBMITTAL: DRAINAGE REPORT DRAINAGE PLAN 1st SUBMITTAL	PHONE: (505) 856-9990 ZIP CODE: 87113 CHECK TYPE OF APPROVAL SOUGHT: SIA/FINANCIAL GUARANTEE RELEASE PRELIMINARY PLAT APPROVAL
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

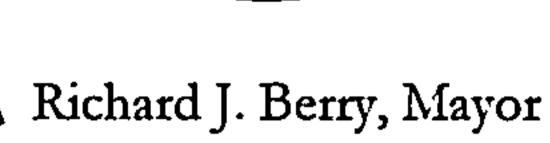
CITY OF ALBUQUERQUE

PLANNING DEPARTMENT - Development & Building Services

July 25, 2013

John Arthur Blessen, P.E. J Arthur Blessen Engineering 2429 Zena Lona NE Albuquerque, NM 87112





RE:

Array Tech Addition – 3901 Midway Place NE Drainage Report and Plan for Building Permit

File **E17-D034C**Stamp: 7/8/13

Dear Mr. Blessen,

Based upon the information provided in your submittal received July 10, 2013, the above referenced Drainage Report and Grading Plan is approved for Building Permit. As discussed by telephone, please correct the "Existing Spillway" elevation, at the upper left from 5129.0 to **5126.0**, and initial the correction.

PO Box 1293

Attach a copy of this approved plan to the Building Permit, construction sets prior to sign-off by Hydrology.

Albuquerque

Prior to Certificate of Occupancy release, an Engineer's Certification of the As-Built grading and drainage, per the DPM checklist will be required. The Certification for C.O. must include Certification of the volume and elevations on the Existing Detention Pond A, at the NW corner of the site.

New Mexico 87103

If you have questions, please contact me by email at grolson@cabq.gov, or telephone 505-924-3994.

www.cabq.gov

Sincerely,

Gregory R. Olson, P.E.

Senior Engineer

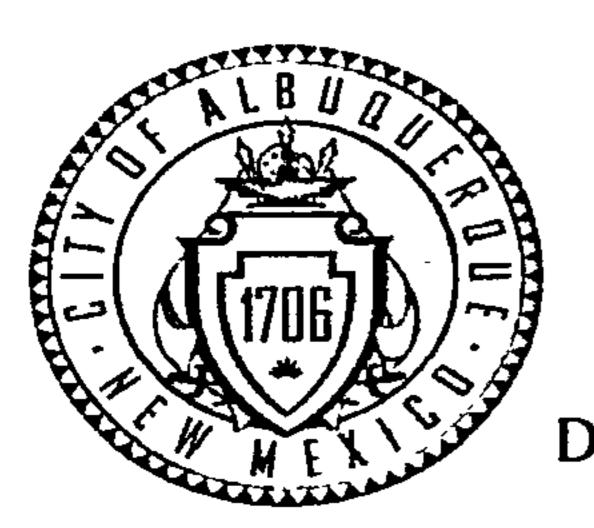
Orig:

Drainage file E17-D034C

c.pdf

Addressee via Email: JAB-Enginering@hotmail.com





City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

Project Title: Array Tech Addition	Building Permit #:		City Drainage #: E17/D034C
DRB#:		Work Order#:	
Legal Description: Tract B-1-B-1A-1A-1 Albuquerque Industrial Pa	rk	•	
City Address: 3901 Midway Place NE			<u>-</u> -
Engineering Firm: J Arthur Blessen Engineering		Contact: Arthu	ır Blessen
Address: 2429 Zena Lona Albuquerque, NM *7112			
Phone#: 505-293-1477 Fax#: 505-	237-8314	E-mail: jab-en	gineering@hotmail.com
Owner:	· · · · · · · · · · · · · · · · · · ·	Contact:	
Address:			· · · · · · · · · · · · · · · · · · ·
Phone#: Fax#:		E-mail:	· · · · · · · · · · · · · · · · · · ·
Architect: Claudiao Vigil Architects		Contact:	
Address:			······································
Phone#: Fax#:		E-mail:	
			3 4
Surveyor:	· · · · · · · · · · · · · · · · · · ·	Contact:	4 ³ 3°5,
Address:		T 11.	
Phone#: Fax#:		E-mail:	· · · · · · · · · · · · · · · · · · ·
Contractor:		Contact:	4
Address:			
Phone#: Fax#:		E-mail:	
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROV	AL/ACCEPTA	NCE SOUGHT:
× DRAINAGE REPORT	SIA/FINANCIAL GUARAN		
× DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APP		•
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D		•
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERM		1 ~
× GRADING PLAN	SECTOR PLAN APPROVAL		\$ 50 QQ
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL		
ENGINEER'S CERT (HYDROLOGY)	CERTIFICATE OF OCCUPA	ANCY (PERM)	
CLOMR/LOMR	CERTIFICATE OF OCCUPA	•	MP)
TRAFFIC CIRCULATION LAYOUT (TCL)	FOUNDATION PERMIT AF		-· <i>y</i>
ENGINEER'S CERT (TCL)	* BUILDING PERMIT APPRO		
ENGINEER'S CERT (TCL) ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPRO		SO-19 APPROVAL
	PAVING PERMIT APPROV	 	ESC PERMIT APPROVAL
ENGINEER'S CERT (ESC)	WORK ORDER APPROVAL		ESC CERT. ACCEPTANCE
SO-19			
OTHER (SPECIFY)	GRADING CERTIFICATIO		OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED:	Yes × No C	opy Provided	•
DATE SUBMITTED: 7/10/13	By: It medical		
Requests for approvals of Site Development Plans and/or Subdivis scope to the proposed development defines the degree of drainage 1. Conceptual Grading and Drainage Plan: Required for ap 2. Drainage Plans: Required for building permits, grading per 3. Drainage Report: Required for subdivision containing mo 4. Erosion and Sediment Control Plan: Required for any ne project less than 1-acre than are part of a larger common p	detail. One or more of the following levels oproval of Site Development Plans greater to mits, paving permits and site plans less that the ten (10) lots or constituting five (5) we development and redevelopment site with	of submittal may han five (5) acres in five (5) acres acres or more the 1-acre or more	be required based on the following and Sector Plans

LAND DEVELOPMENT SECTION



Revised Drainage Report

for

Array Tech Addition

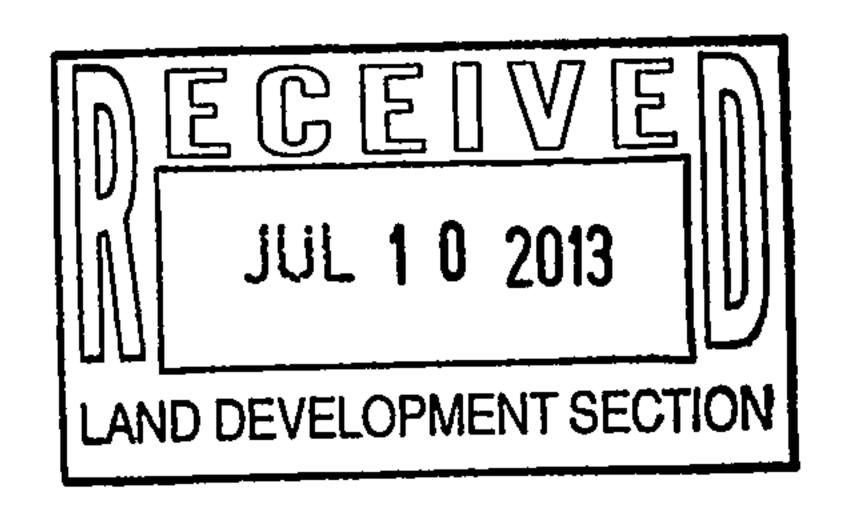
3901 Midway Place NE Albuquerque, New Mexico

July 8, 2013

File: E17/D034C

by

J Arthur Blessen Engineering





Location:

Tract B-1-B-1A-1A-1 Albuquerque Industrial Park, Bernalillo County New Mexico 3901 Midway Place NE.

Zone map E-17-Z

Located east of Office Blvd. at the east end of Midway Place, south of the Bear Arroyo North Diversion Channel.

Site Area: 6.16 Acres

Precipitation Zone 2

Flood Zone:

The proposed building locations on the site does not lie within a flood hazard zone (Map 35001C0138H panel 138 of 825). The site slopes from east to west. The Bear Arroyo is located to the north of the site. The lands to the west and south are lower than the site. The existing drainage channel along the east property line block flow from that direction. Therefore offsite flows are considered negligible.

Existing Conditions:

The existing 6.16 acre site is developed. The previous developments included the a warehouse, and asphalt parking lots and access road, landscaping, and detention ponds. The previously approved drainage plan (COA File #E17/D034C) directs the runoff from the site to two existing detention ponds, and to Midway Place.

The existing master drainage plan "Overall Drainage Plan" for the Nabisco Inc Warehouse (1990) established a discharge rate from the site of 2.1 cfs /acre (2.1cfs/acre x 6.16 acre = 12.9 cfs). The previously approved drainage plan for the site was prepared by Bohannan-Houston Inc (1-6-95) and divided the site into six sub-basins:

Basin A-1

A 0.582 ac portion of the site located at the southwest corner of the site. Runoff from the basin discharges to existing detention pond B. The previously approved drainage plan established the a discharged rate from pond B of 0.3 csf. The volume of the pond is 1077 cf at an elevation of 5128.6 ft. The proposed development will not effect basin A-1.

Basin A-2

A 0.446 Ac portion of the site located north of basin A-1. Runoff from the basin discharges to Midway Place. The previously approved drainage plan established free discharge from the basin at a rate of 1.7 cfs. The proposed development will not effect basin A-2.

Basin B-1

LAND DEVELOPMENT SECTION

The basin includes the existing warehouse and parking lot located to the west of the facility. Runoff from the basin flows to the existing catch basin located in the parking lot, and is routed to the existing detention pond located at the northwest corner of the site via a 21" dia culvert at a rate of 8 cfs. The proposed development will continue to use the existing drainage route. The proposed

2,14 p

development will increase the size of the basin to include a portion of the proposed warehouse,

Basin B-2

The basin includes the existing service drive and landscaping area located at the southeast corner of the site. Runoff from the basin flows to an existing catch basin located east corner of the existing warehouse, and is routed to the existing detention pond located at the northwest corner of the site vis a 15" dia culvert at a rate of 3.3 cfs. The proposed development includes a small parking area which will increase the discharge rate to the basin.

Basin B-3

The basin includes the gravel storage yard and asphalt service drive located at the northeast corner of the site. Runoff from the basin flows to an existing catch basin located on the east side of the existing loading dock, and is routed to the existing detention pond located at the northwest corner of the site via a 15" dia culvert at a rate of 2.8 cfs, when combined with basin B-2 the flow through the 15" culvert is 6.1 cfs. The proposed development will not effect basin B-3.

Basin C-1 (previously unnamed)

The basin includes the existing parking lot on the north side of the facility and the north section of the site. Runoff form the site discharges to the existing detention pond (pond A) located within the site. The proposed development will increase the rate of runoff to the basin.

The combined runoff from basins B-1, B-2, B-3, and A is discharged to existing pond A at a rate of 23.5 cfs. The previously approved drainage plan established a discharge rate from the pond at a rate of 10.6 cfs via a 16.9 orifice to the existing asphalt swale within the drainage easement running parallel to the Bear Arroyo. The existing pond volume was established as 16,000 cf at an elevation of 5126.0.)

The combined existing discharge from the site is (0.3cfs + 1.7cfs + 10.6cfs = 12.6cfs).

Note: The elevations have been adjusted to correspond to the current bench mark reference.

Developed Conditions:

The proposed development include a 20,595 sf office / warehouse addition, small parking lot, and associated landscaping. The proposed additions will be confined to basins B-1, B-2 and C-1. The remainder of the site will remain unchanged. The runoff from the proposed addition will be directed to the existing detention pond A.

The combined area of basins B-1, B-2, B-3, and C-1 is 5.132 ac. and produces a combined runoff of 20.4 cfs. The existing orifice in the outlet pipe for pond A limits the discharge to 10.6 cfs. The required pond volume for the combined basis in 10,528 cfs. The existing pond A has a capacity of 10,578 cf at a elevation of 5125.52 ft, therefore no modification to the pond is required.

JUL 1 0 2013

LAND DEVELOPMENT SECTION

Proposed & calcs de sinches la servición de la character de la



Summary:

As shown by the attached calculations the existing runoff for the site will be decreased form the existing due to the additional landscaping. The existing detention pond with a controlled discharge will control the addition runoff from the site.

	.			
	storic cfs cfs	Existing 1.5 cfs 1.7 cfs	Proposed no change no change	to existing pond B discharge 0.3 cfs
B-2 1.1 B-3 1.6 C-1 2.8	cfs cfs csf cfs	8.0 cfs 3.3 cfs 2.8 cfs 9.4 cfs	7.0 cfs 3.0 cfs 3.9 cfs 6.9 cfs	
Total to po	ond (23.5 cfs	20.8 cfs	to existing pond A discharge 10.6 cfs

To follow are the calculations analyze the historic and proposed conditions for the 6-HOUR, 100 year event. The analysis is in. accordance with the City of Albuquerque Development Process Manual Volume II.

- Reduced Runa H - Pavit when



City of Albuquerque DPM 1997 edition

Basin B1

Precipitation Zone

Basin Area = 1.570 acres

Historic			Improved Condition	ons	
Treatment			Treatment		
Area of A =	68379 sf	100%	Area of A =	sf	0%
Area of B =	0 sf	. 0%	Area of B =	6822 sf	10%
Area of C =	0 sf	0%	Area of C =	0 sf	0%
Area of D =	0 sf	0%	Area of D =	61557 sf	90% 🗸

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditions				Improved Conditions			
Treatment	% of Area	E	Ξn	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.10 x	0.78 =	0.08
С	0.00 x	1.13 =	0.00	С	0.00 x	1.13 =	0.00
D	0.00 x	2.12 =	0.00	D	0.90 x	2.12 =	1.91
		E =	0.53			E =	1.99
Volume V = E A / 1	2						
Ve =	0.530 x	1.5698 /	12 =	0.069 a	cre ft	3020 cf	

0.260 acre ft

11319 cf

Discharge Rate, Q (cfs / acre) 100 vr storm table A-9

Treatment	% of Area	•	2	Treatment	% of Area		Q
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.10 x	2.28 =	0.23
С	0.00 x	3.14 =	0.00	С	0.00 x	3.14 =	0.00
D	0.00 x	4.7 =	0.00	D	0.90 x	4.7 =	4.23
		q =	1.56			q =	4.46

Peak Rate Qp = q A

 $1.56 \times 1.5698 =$ 2.45 cfs Qp(i) = $4.46 \times 1.5698 =$ 7.00 cfs

1.986 x 1.5698 /

0.191 acre ft Excess Volume = 4.55 cfs Excess Rate =

> 0.2 hr tc = (2.107 *E*At/Qp)-(0.25*Ad/At) =0.714 hr (0.7*tc)+((1.6-(Ad/At))/12) 0.198 hr

7.00 cfs 4.46 cfs/ac Discharge Rate

11825 cf Volume 11825 cf Discharged

0 cf Pond Voulme



City of Albuquerque DPM 1997 edition

Basin B2

Basin Area =

Precipitation Zone

0.723 acres

Historic **Improved Conditions** Treatment

Treatment Area of A =31500 sf 100% Area of A =0% Area of B = Area of B = 0 sf 0% 12% 3652 sf Area of C = 6924 sfArea of C = 0 sf 0% 22% Area of D =0 sf 0% Area of D = 20924 sf 66%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existina Conditions Improved Conditions

Existing Contains	110			improved cond			
Treatment	% of Area	E	Ξn	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.12 x	0.78 =	0.09
С	0.00 x	1.13 = 1	0.00	С	0.22 x	1.13 =	0.25
D	0.00 x	2.12 =	0.00	D	0.66 x	2.12 =	1.41
		E =	0.53			E =	1.75
Volume V = E A	/ 12						
Ve =	0.530 x	0.7231 /	12 =	0.032 ad	cre ft	1391 cf	
Vi =	1.747 x	0.7231 /	12 =	0.105 ad	cre ft	4586 cf	

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9

,	(3.3	J. 212					
Treatment	% of Area	Q		Treatment	% of Area	Q	
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.12 x	2.28 =	0.26
С	0.00 x	3.14 =	0.00	C	0.22 x	3.14 =	0.69
D	0.00 x	4.7 =	0.00	D	0.66 x	4.7 =	3.12
		q =	1.56			q =	4.08

Peak Rate Qp = q A

 $1.56 \times 0.7231 =$ 1.13 cfs $4.08 \times 0.7231 =$ Qp(i) =2.95 cfs

0.073 acre ft Excess Volume = 1.82 cfs Excess Rate =

> 0.2 hr tc =

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) =0.737 hr (0.7*tc)+((1.6-(Ad/At))/12)0.218 hr

2.95 cfs 4.08 cfs/ac Discharge Rate

4791 cf Volume 4791 cf Discharged -

0 cf Pond Voulme



City of Albuquerque DPM 1997 edition

Basin B3

Precipitation Zone 2

Basin Area = 1.040 acres

Historic			Improved Condition	ons	
Treatment			Treatment		
Area of A =	45296 sf	100%	Area of A =	sf	0%
Area of B =	0 sf	0%	Area of B =	sf	0%
Area of C =	0 sf	0%	Area of C =	28362 sf	63%
Area of D =	0 sf	0%	Area of D =	16934 sf	37%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditions (Improved Conditions				
Treatment	% of Area		En	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.00 x	0.78 =	0.00
С	0.00 x	1.13 =	0.00	С	0.63 x	1.13 =	0.71
D	0.00 x	2.12 =	0.00	D	0.37 x	2.12 =	0.79
		E =	0.53			E =	1.50
Volume $V = E A$	\ / 12						
Ve =	0.530 x	1.0399 /	12 =	0.046 ac	re ft	2001 cf	
Vi =	1.500 x	1.0399 /	12 =	0.130 ad	ere ft	5662 cf	

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9

Treatment	% of Area	Q		Treatment % of Area		Q	
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.00 x	2.28 =	0.00
С	0.00 x	3.14 =	0.00	С	0.63 x	3.14 =	1.97
D	0.00 x	4.7 =	0.00	D	0.37 x	4.7 =	1.76
		q =	1.56			q =	3.72

Peak Rate Qp = q A

 $Qp(e) = 1.56 \times 1.0399 = 1.62 \text{ cfs}$ $Qp(i) = 3.72 \times 1.0399 = 3.87 \text{ cfs}$

Excess Volume = 0.084 acre ft
Excess Rate = 2.25 cfs

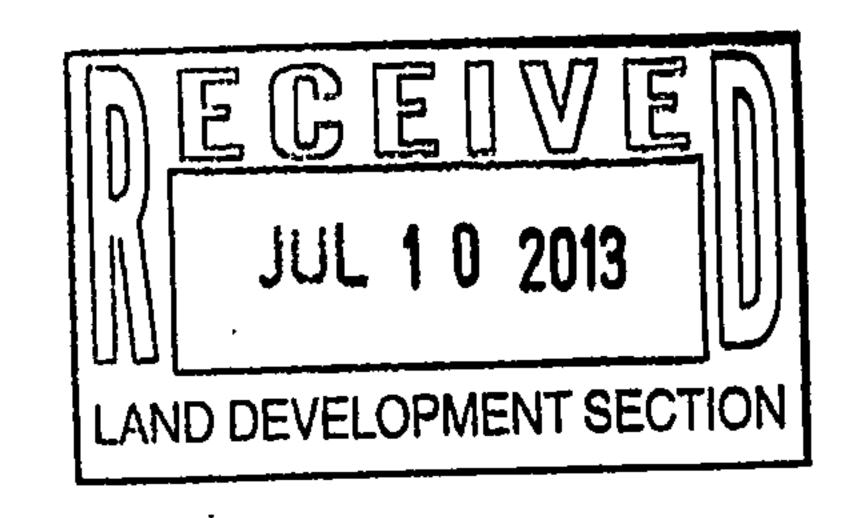
tc = 0.2 hr

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.755 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.242 hr

Discharge Rate 3.87 cfs 3.72 cfs/ac

Volume 5916 cf Discharged - 5916 cf

Pond Voulme 0 cf



'Drainage Calculation

City of Albuquerque DPM 1997 edition

Basin C1

Precipitation Zone

2

Basin Area =

1.798 acres

Historic			Improved Condition	ons	
Treatment			Treatment		•
Area of A =	78336 sf	100%	Area of A =	sf	0% / A
Area of B =	0 sf	0%	Area of B =	24113 sf	31% ! Pard A
Area of C =	0 sf	0%	Area of C =	7012 sf	9%
Area of D =	0 sf	0%	Area of D =	47211 sf	60% ~

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditio	ns			Improved Cond	itions		
Treatment	% of Area	E	Ξn	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.31 x	0.78 =	0.24
C	0.00 x	1.13 =	0.00	С	0.09 x	1.13 =	0.10
D	0.00 x	2.12 =	0.00	D	0.60 x	2.12 =	1.28
		E =	0.53			E =	1.62
Volume $V = E A$	/ 12						
Ve =	0.530 x	1.7983 /	12 =	0.079 ac	re ft	3460 cf	
Vi =	1.619 x	1.7983 /	12 =	0.243 ac	re ft	10568 cf	

Discharge Rate, Q (cfs / acre) 100 yr storm table A-9

Treatment	% of Area		2	Treatment	% of Area		Q
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.31 x	2.28 =	0.70
C	0.00 x	3.14 =	0.00	С	0.09 x	3.14 =	0.28
D	0.00 x	4.7 =	0.00	D	0.60 x	4.7 =	2.83
		q =	1.56			q =	3.82
Peak Pate On - a	Λ	•				•	

Peak Rate Qp = q A

 $Qp(e) = 1.56 \times 1.7983 = 2.81 \text{ cfs}$ $Qp(i) = 3.82 \times 1.7983 = 6.86 \text{ cfs}$

Excess Volume = 0.163 acre ft
Excess Rate = 4.06 cfs

tc = 0.2 hr

tb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.743 hrtp = (0.7*tc)+((1.6-(Ad/At))/12) = 0.223 hr

Discharge Rate 6.86 cfs 3.82 cfs/ac

Volume 11042 cf Discharged - 11042 cf

Pond Voulme 0 cf



Drainage Calculation

City of Albuquerque DPM 1997 edition

Basins B-1, B-2, B-3 and C-1 (to pond A)

Precipitation Zone 2

Basin Area = 5.132 acres

Historic			Improved Conditi	ions	
Treatment			Treatment		
Area of A =	223563 sf	100%	Area of A =	sf	0%
Area of B =	0 sf	0%	Area of B =	39587 sf	18%
Area of C =	0 sf	0%	Area of C =	42298 sf	19%
Area of D =	0 sf	0%	Area of D =	141678 sf	63%

Excess Precipitation, E (inches) 6 hr - 100 yr storm table A-8

Existing Conditions				Improved Cond	ditions		
Treatment 9	% of Area	E	n	Treatment	% of Area		En
Α	1.00 x	0.53 =	0.53	Α	0.00 x	0.53 =	0.00
В	0.00 x	0.78 =	0.00	В	0.18 x	0.78 =	0.14
С	0.00 x	1.13 =	0.00	С	0.19 x	1.13 =	0.21
D	0.00 x	2.12 =	0,00	D	0.63 x	2.12 =	1.34
		E =	0.53			E =	1.70
Volume $V = EA/12$	2						
Ve =	0.530 x	5.1323 /	12 =	0.227 ad	cre ft	9874 cf	

12 =

Discharge Rate. Q (cfs / acre) 100 vr storm table A-9

D 10011013011010,	a (0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,0,	y. Otolili ta.					
Treatment	% of Area	G)	Treatment	% of Area		Q
Α	1.00 x	1.56 =	1.56	Α	0.00 x	1.56 =	0.00
В	0.00 x	2.28 =	0.00	В	0.18 x	2.28 =	0.40
С	0.00 x	3.14 =	0.00	С	0.19 x	3.14 =	0.59
D	0.00 x	4.7 =	0.00	D	0.63 x	4.7 =	2.98
		q =	1.56			q =	3.98

Peak Rate Qp = q A

 $\forall i =$

 $Qp(e) = 1.56 \times 5.1323 = 8.01 \text{ cfs}$ $Qp(i) = 3.98 \times 5.1323 = 20.41 \text{ cfs}$

(0.7*tc)+((1.6-(Ad/At))/12)

1.695 x 5.1323 /

Excess Volume = 0.498 acre ft Excess Rate = 12.40 cfs

> tc = 0.2 hrtb = (2.107 *E*At/Qp)-(0.25*Ad/At) = 0.740 hr

Discharge Rate 10.60 cfs 2.07 cfs/ac

Volume 33001 cf
Discharged - 22473 cf

Pond Voulme 10528 cf

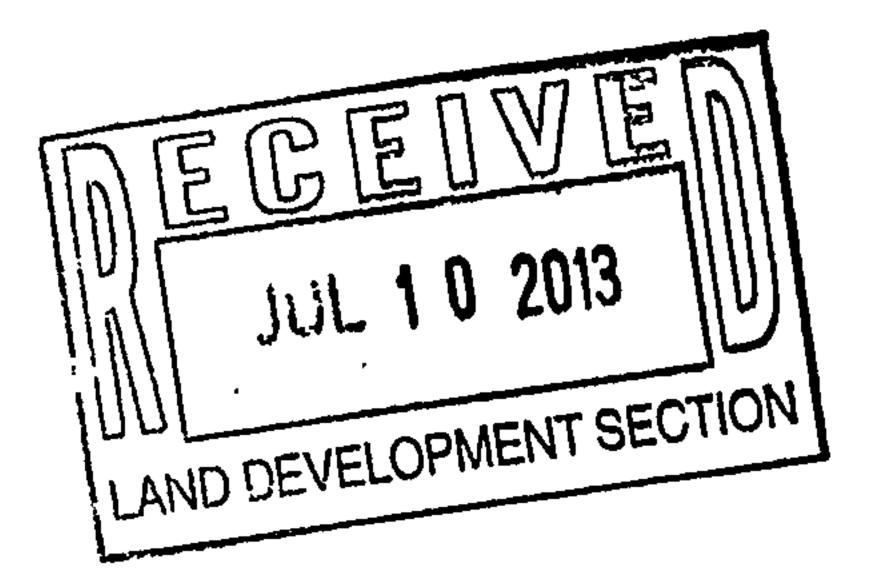


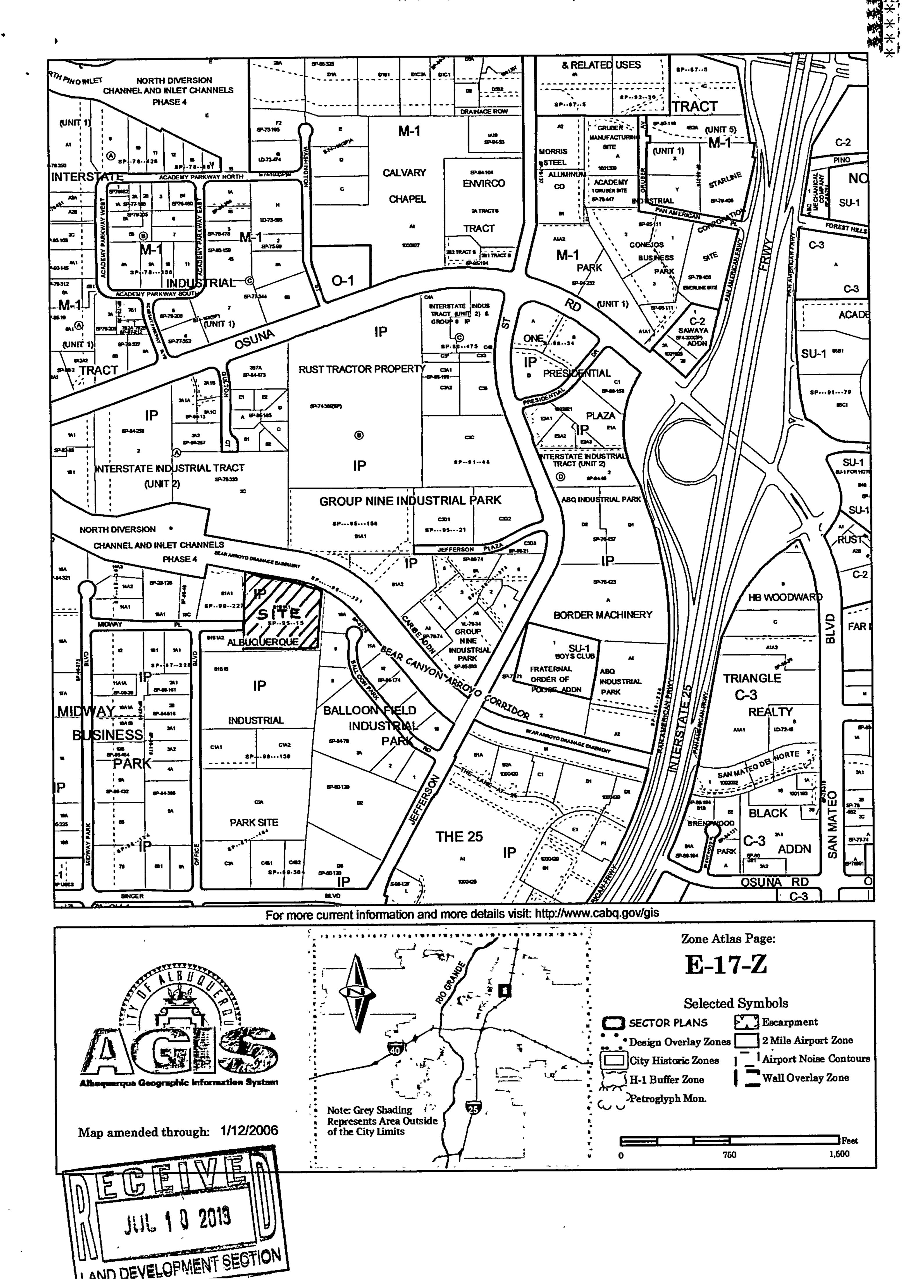
0.221 hr

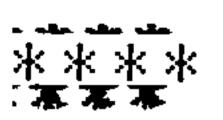
0.725 acre ft

31586 cf

· top of pond	elevation 25.52 25 24	depth ft 0.52 1 1.19		area sf 6304 5582 4290	V	olume cf 3090.36 4936 2552.55
bottom Orifice	22.81			0		10578.91
pond depth = pipe dia =	2.71 ft 16.9 in pond depth -		==	1.56 2.01	_	
Qpipe =	0.6 A (2 * 32.2	2 * H)^.5	=	10.6	cfs	







AND DEVELOPMENT SECTION

AND DEVELOPMENT SECTI

MAP SCALE 1" = 500'

0 500 1000 FEET

PANEL 0138H

FIRM

BERNALILLO COUNTY,
NEW MEXICO
AND INCORPORATED AREAS

PANEL 138 OF 825

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
ALBUQUERQUE, CITY OF	350002	0138	н
BERNALILLO COUNTY UNINCORPORATED AREAS	350001	0138	н

Notice to User. The Map Number shown below should be used when placing map orders, the Community Number shown above should be used on insurance applications for the subject community.



MAP NUMBER 35001C0138H

MAP REVISED AUGUST 16, 2012

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov





City of Albuquerque P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 14, 2001

James R. Topmiller, P.E.
Bohannan-Huston, Inc.
Courtyard 1
7500 Jefferson NE
Albuquerque, NM 87109-4335

RE: FLOWSERVE BUILDING ADDITION (Formerly BWIP International, Inc.) (E17-D34C). Revised DRAINAGE PLAN FOR BUILDING PERMIT APPROVAL. ENGINEER'S STAMP DATED MAY 31, 2001.

Dear Mr. Topmiller:

Based on the information provided on your June 1, 2001 submittal, the above referenced project is approved for Building Permit.

Thank you for the early notification of this modification to the G&D Plan. As often as not, such (limited) modifications are recorded during the Certification Process.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Prior to Certificate of Occupancy approval, an Engineer's Certification per the DPM will be required.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

c: Terri Martin File





City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

August 9, 1994

James R. Topmiller Bohannan-Huston, Inc. 7500 Jefferson NE Albuquerque, NM 87109

RE: BW/IP INTERNATIONAL LCF BUILDING (E17-D34C) ENGINEER'S CERTIFICATION FOR CERTIFICATE OF OCCUPANCY APPROVAL. ENGINEER'S CERTIFICATION DATED 8-4--95.

Dear Mr. Topmiller:

Based on the information provided on your August 4, 1995 submittal, a Certificate of Occupancy is approved for BW/IP International LCF Building.

If I can be of further assistance, please feel free to contact me at 768-3622.

Sincerely,

Lisa Ann Manwill

Engineering Assoc./Hyd.

c: Andrew Garcia
File>



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

February 17, 1995

James Topmiller, P.E.
Bohannan Huston, Inc.
7500 Jefferson NE
Albuquerque, N.M. 87109

RE: DRAINAGE REPORT FOR BW/IP INTERNATIONAL LCF BUILDING (E-17/D34C) RECEIVED FEBRUARY 2, 1995 FOR BUILDING PERMIT APPROVAL

ENGINEER'S STAMP DATED 1-24-95

Dear Mr. Topmiller:

Based on the information included in the submittal referenced above, City Hydrology approves this project for Building Permit.

Include a copy of the approved Grading & Drainage Plan, dated 1-24-95, in the set of construction documents that will be submitted to Code Administration for the Building Permit.

Engineer's Certification of grading & drainage, per DPM checklist, must be approved before any Certificate of Occupancy will be released.

If you have any questions about this project, You may contact me at 768-2727.

Sincerely,

John P. Curtin, P.E. Civil Engineer/Hydrology

c: Andrew Garcia

Ken Ishihara; BW/IP International; 2300 East Vernon Ave; Vernon, CA 90058



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

December 16, 1994

James Topmiller, P.E.
Bohannan Huston, Inc.
7500 Jefferson NE
Albuquerque, N.M. 87109

RE: CON GRADING & DRAINAGE PLAN FOR BW/IP LCF BUILDING (E-17/D34C)
RECEIVED NOVEMBER 28, 1994 FOR S DEV PLAN & FINAL PLAT APPROVAL
ENGINEER'S STAMP DATED 11-23-94

Dear Mr. Topmiller:

Based on the information included in the submittal referenced above, City Hydrology accepts the concept for this project and approves the Site Development Plan and Final Plat.

The following comments must be addressed prior to Rough Grading approval:

- 1. Add the proposed retaining walls to the Legend. If the retaining walls are constructed on the property line, permission to work on the adjacent property must be obtained in writing from the property owner.
- 2. Indicate the extent of the ponding on the Plan. It appears that the pond in Basin B will extend on to the dirt road & the parking lot.
- 3. Expound on the Erosion Control Plan. Indicate where the erosion control berm is located. Explain what the purpose of the berm is. Remind the Contractor that he must obtain a "Topsoil Disturbance Permit".
- 4. Indicate roof drain locations. The water blocks indicate that significant areas are draining to the loading docks. It appears that some type of drain is required at the loading docks.

If you have any questions about this project, You may contact me at 768-2727.

Sincerely,

John P. Curtin, P.E.

Civil Engineer/Hydrology

c: Andrew Garcia Fred Aguirre, DRB 94-559



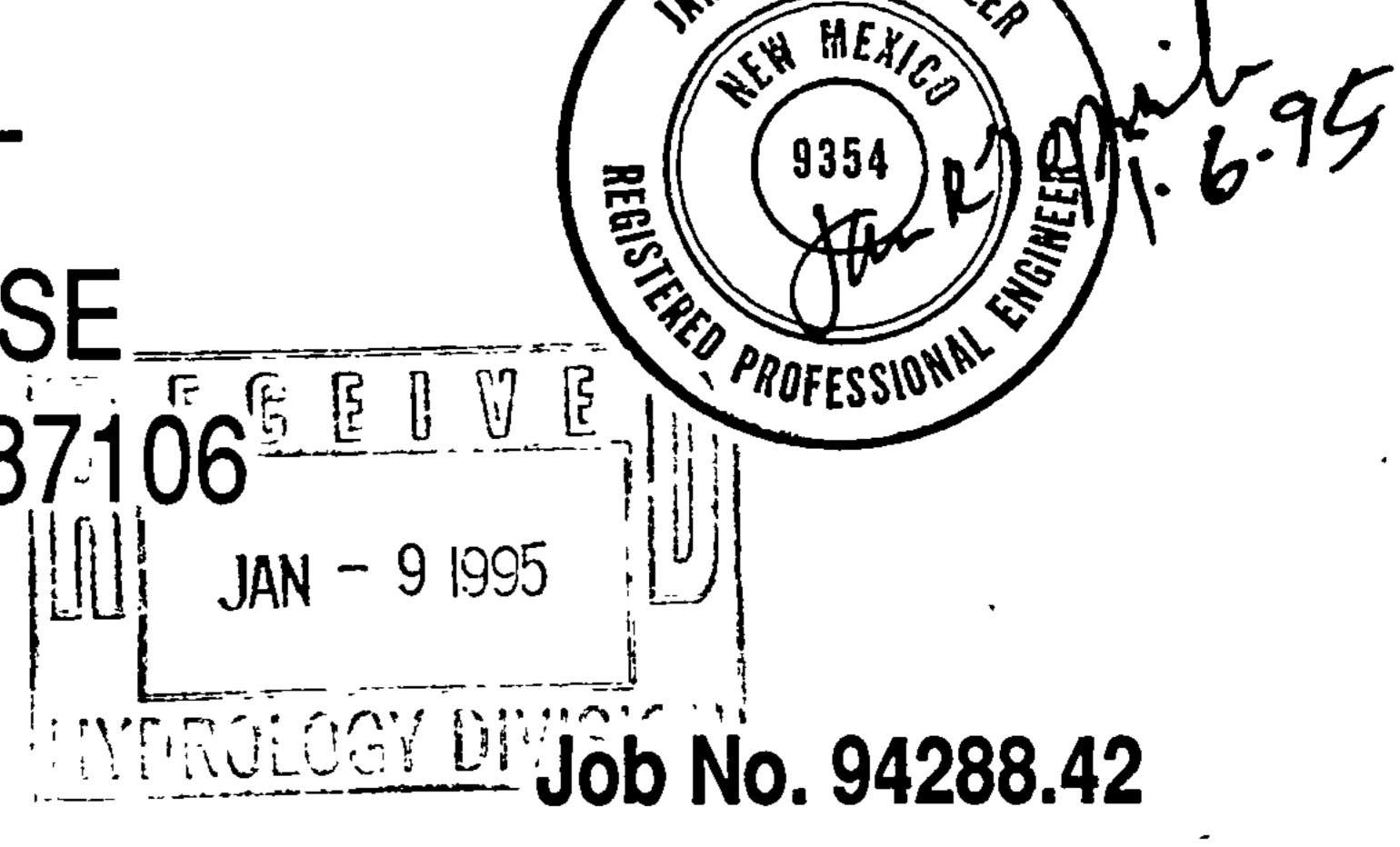
Prepared for:

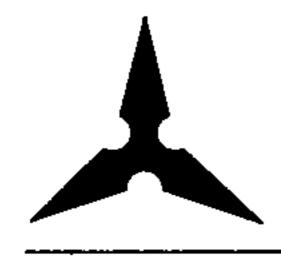
BWIP INTERNATIONAL INC. 2300 East Vernon Avenue

Vernon, CA 90058

SMPC ARCHITECT 115 Amherst Drive SE Albuquerque, NM 87106

Prepared by:





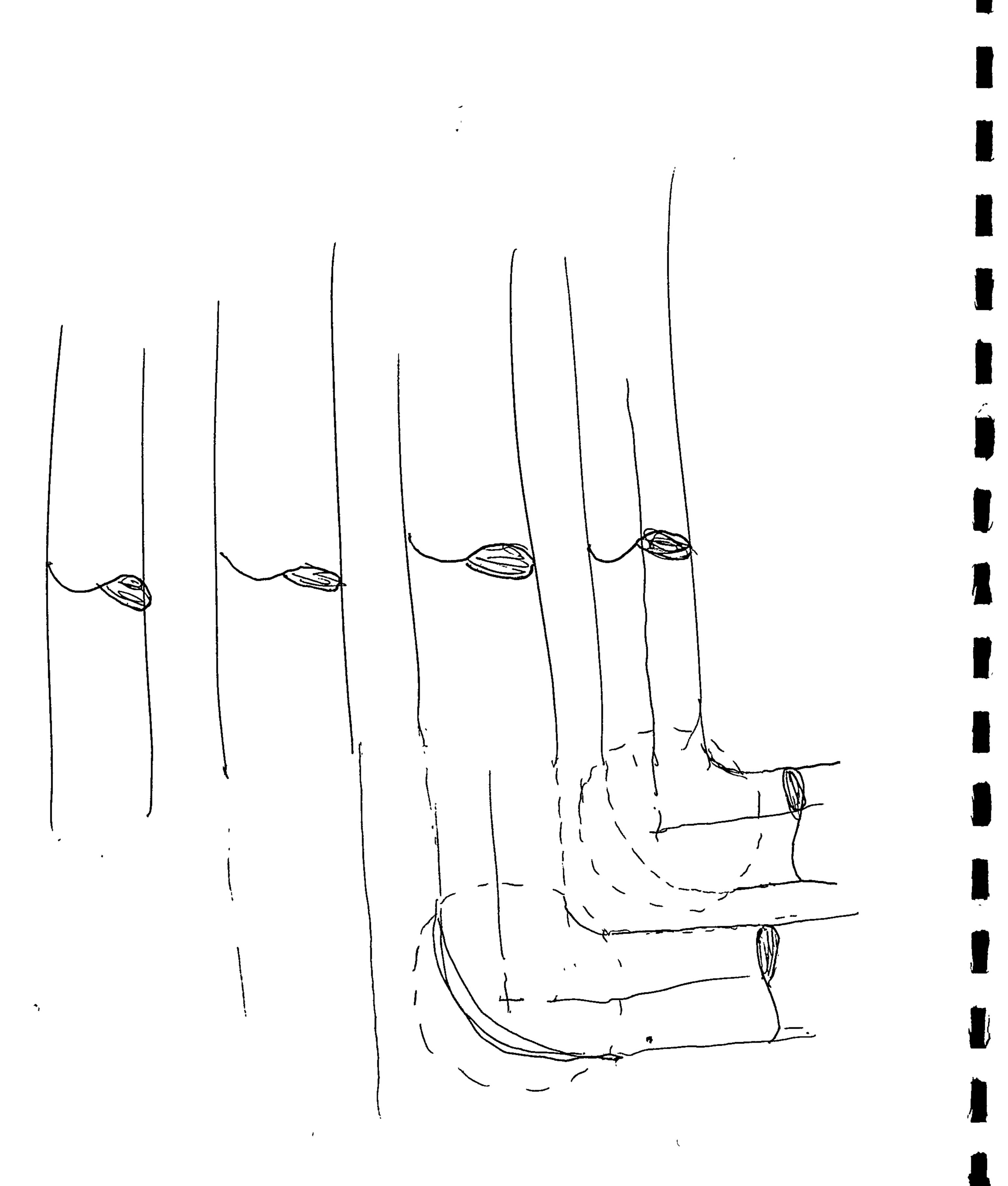
BOHANNAN-HUSTON INC.

ENGINEERS

ARCHITECTS

PHOTOGRAMMETRISTS

SURVEYORS



•

•

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FINAL GRADING/DRAINAGE PLAN (E17/d34c)

PURPOSE

The purpose of this Final Grading/Drainage Plan is to present existing and proposed hydrological conditions and grading for the proposed development. This plan is prepared and submitted to support rough grading and building permit approval.

EXISTING CONDITIONS

The site is currently a vacant piece of ground sloping downstream in a westerly direction at a slope of approximately 2% to 4%. This approximately 6.1 acre site generates 11cfs in a uncontrolled sheetflow fashion during a 100-year storm event. The site accepts essentially no offsite drainage flows from its perimeter properties.

Except for the property immediately to the west of the proposed site and immediately south of the Midway Place, all adjacent properties are developed. To the north lies the Bear Arroyo Diversion Channel, an AMAFCA facility.

The area of this proposed site is governed by an existing Master Drainage Plan. The plan is called "Overall Drainage Plan" and was first prepared under drainage plans provided for a "Warehouse for Nabisco, Inc." in 1990. This Master Plan identifies that the site has two drainage outfalls. The southern-most portion of the site will drain directly to Midway Place or Office Boulevard. The larger north portion of the site will drain into an existing drainage swale provided with the 1990 Nabisco project. The Drainage Management Plan further requires that developed flows be controlled to a 2.1 cfs per acre rate.

No FEMA flood plains exist on the site.' The nearest floodplain is located within the Bear Arroyo Diversion Channel to the north and does not impact the site in any way.

PROPOSED CONDITIONS

All flow references are 100-yr storm events.

Under developed conditions, the site will drain in a manner to maintain the basins identified on the Overall Drainage Plan (the Master Plan). Accordingly, the southwestern-most corner of the property will discharge directly to Midway Place while the remainder of the site discharges northwesterly to the existing swale on the Lot B-1-B-1. A 20-foot private drainage easement currently exists on this adjacent tract expressly for the purposes of collecting these flows.

The flows generated in Basin A-1 are approximately 1.4 cfs and calculations for it are shown in the Appendix. Using a small detention pond in this basin to detain a portion of the flows allows this basin to restrict flows to 0.3 cfs and helps to comply with the 2.1 cfs per acre maximum discharge requirement of the Overall Drainage Plan. By raising the west berm to an elevation (Elev. 26.4) that is 0.5' higher than the berm elevation (Elev. 25.9) adjacent to the paved cul-de-sac, a spillway is created. Accordingly, any spillage from the pond (in a storm exceeding 100-yr event) will be discharged almost directly (northerly) into the cul-de-sac. Via the landscape plan,

the "spillway" will be graveled. This should be sufficient for the very low flows (1.4 cfs) of this basin and the "spread-out" (very low depth) nature of any spillage over the berm.

Basin A-2 provides no detention and free discharges 1.7 cfs to Midway Place. 20 Accordingly, the actual discharge to Midway Place from both Basins is only 2.1 cfs. Since the two Basins comprise 1 acre, the discharge rate is 2.1-cfs per acre, approximately equal to the 2.0-cfs allowed by the Overall Drainage Plan. 2.0

Basins B-1, B-2, B-3 and the unnamed basin (in the northwest) comprise the remainder of the site at approximately 5.1 acres and generate 23.5 cfs in the 100-year storm event. For this area, the Overall Drainage Plan requires a controlled discharge of only 10.6 cfs. Accordingly, the detention pond in the northwest corner of the site will control discharge to the allowable 10.6cfs rate. Calculations in the Appendix show that a pond of 16,000 cubic feet with a 16.9-inch orifice outlet as shown on the plan will control discharge from these basins to 10.6 cfs. A asphalt-lined emergency spillway is provided from this pond to the downstream drainage swale. The Appendix calculations size the spillway by weir equation for 23.5 cfs (the max. 100-yr storm flow to the pond). At a depth of 1', the spillway weir must be 8' wide. Additional details are provided on the plan.

The three named basins, B-1, B-2 and B-3 are analyzed in order to determine pipe sizing for the storm drain. Basin B-1, 8 cfs, drains to the inlet at the west end of the parking. The Appendix calculations show that a single type "d" inlet collects the flow with only 0.5' headwater above the grate. The inlet then discharges to the pond by 21" pipe.

Basin B-2 discharges 3.3 cfs to a 15" pipe located in the south dock area. At a 0.8% slope, the 15" pipe can carry the 3.3 cfs.

Basin B-3 discharges 6.1 cfs to the east loading dock (which contains a 15" pipe outlet) and the grated-lid manhole, whereupon these then discharge to the 24" storm drain. Appendix calculations show that the 15" pipe outlet alone could carry/discharge the basin's flows should the grated-lid manhole plug. Accordingly, no further calculations are provided for the manhole lid's collection capacity. The 24" storm drain can carry the 11 cfs of Basins B-2 and B-3 at a slope of 0.8%. This 24" storm drain then discharges to the previously discussed detention pond.

Conclusion

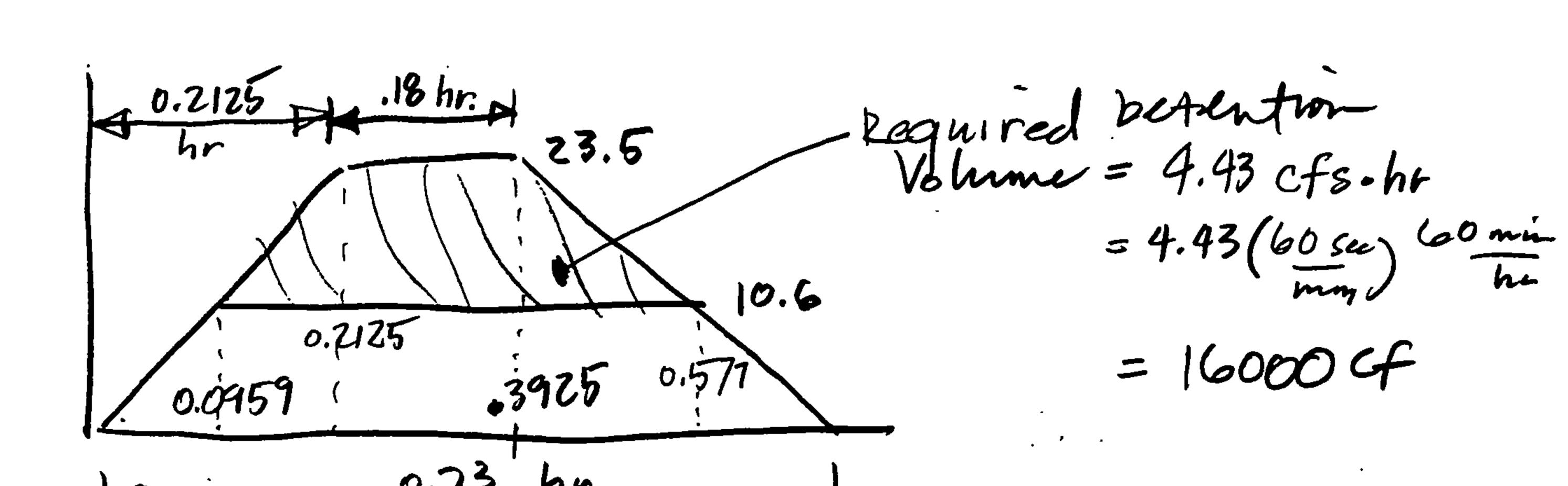
We believe this plan complies in all respects with the Development Process Manual. We respectfully request building permit approval.

Land	= 5,1 20 1 treat	ments	73%	treatme	wh "B"	
Q100	= 0.73 (5			c.+ 0.27 (E		
Mo	walle pe	r Over	M Drai	maqu pla	w = 10,6	off
	Detent In Rue	ion le Aviet	nding	Regune	10.6 cfs	
- ·						
				‡		

POND SIZINGT (Bassing B-1, B-2, B-3)

Ond unsparsed remaining basin in NN corner of site)

$$t_{c} = 0.2 \text{ hours}$$
 $t_{p} = 0.7(0.2) + (1.6-0.73) = 0.2125 \text{ hours}$
 $t_{p} = 0.75(0.73) = 0.18 \text{ hours}$
 $t_{p} = 0.73(5.4) \cdot 2.12 + 0.27(5.1) \cdot 0.78 = 10.2 = 1.75$



 $t_b = \frac{2.107(1.75)5.1}{23.5} - 0.18 = 0.73 hoter$

@ 1.5' depth, 103 x 103 @ 3.0', 5300#, 73'x 73'

A23,3=(160+60)80/2=8800

A = 20.3 = (1/5 + 50) 50/2 = 4/25 SH1.2V = (8800 + 4425) 3/2 = 19,3880

pond Outlet

Q=10.6 cfs (max. Mourble) 18" pipe culvert using orifice... $Q = cay/2gh = 10.6 = 0.6(1.8)\sqrt{2(32.2)}h = 8.7\sqrt{h}$ or $\left(\frac{10.6}{8.7}\right)^2 = h = 1.5$ h=2,25 Q=19,10/s

find orifice size, when h= 2.0, Q100 = 10.6 cfs then we have in 1.0 X

c 129h = 2 = 10.6 cfc 0.6 \square 64.4(2.0)

arva et = 1.55# orifue

(4) = 1.40 = 16.9" üch 12.

			$\stackrel{\circ}{\Rightarrow} \stackrel{\circ}{\downarrow} \stackrel{\bullet}{\Box}$
BA61N B-1	Zone 2, DPM		**************************************
A. Area = 1.7 a $Q_{100} = 1.7 a$	cre6 c. (4.7 cfs/ac.) = 8 cfs	100% imper	vou.
B. Emgly = 4.5 Fact = 4.5	56'59. Ft., depth = 0.	5/ Langth \$35-	+18(
= awalyze as	=3.0(8.8)	= 8,8 0.5 0.5	
i	= 9.3 cts 8.4 orifice Q= CA \ Zg	2015,01	
	= 0.6 (4.50	.) 72(32.2-)0.5-	
C. 21 pipe 6	170 = 8 cfs capacity =	28 cfs, OK B cfs flow to thet, OK	
BASIN B-2			- · ·
→	c. assume 100% treatmen		• • • • • • • •
4100	(4.7 cfs/20) = 3.3 cfs	0.008	
B. Nect 15	untet 6 dock, wring	FHA Culverts C	har
	1 Hw= 0.9, then Hw=		OK
C, 15 proce care	3,2 cfs = 3.3 cfs,000 BOH	ANNAN-HUSTON IN	IC.
PROJECT NAME	SHE	ET 4 OF	•

PROJECT NO.

SUBJECT _____

a sense sense de company de compa	では、 ・
BASIN 15-3	· · · · · · · · · · · · · · · · · · ·
A. Anca = 1.3 acrec, con Q100 = 1.3 (4.7 cf.	servatively, assume 100% Land Treatments.) = 6.1 cfs
B. Strume all flow go (which could happy plugged),	per if the grated MH
Check worning FHA C @ 6.1 cfs, HW = 1.7	West-chapt, 15" culvert, thw=1.7(15")=25" < dock height, 0x
· · · · · · · · · · · · · · · · · · ·	the grated manhale will pick up 1/2 of B-3 flows =
C. 9120 pripe, Quos=	6.1 (B-3) + 3.3 (BSSW) -9.4 cfs
use 2	
	for gravity (low)

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and the control of th

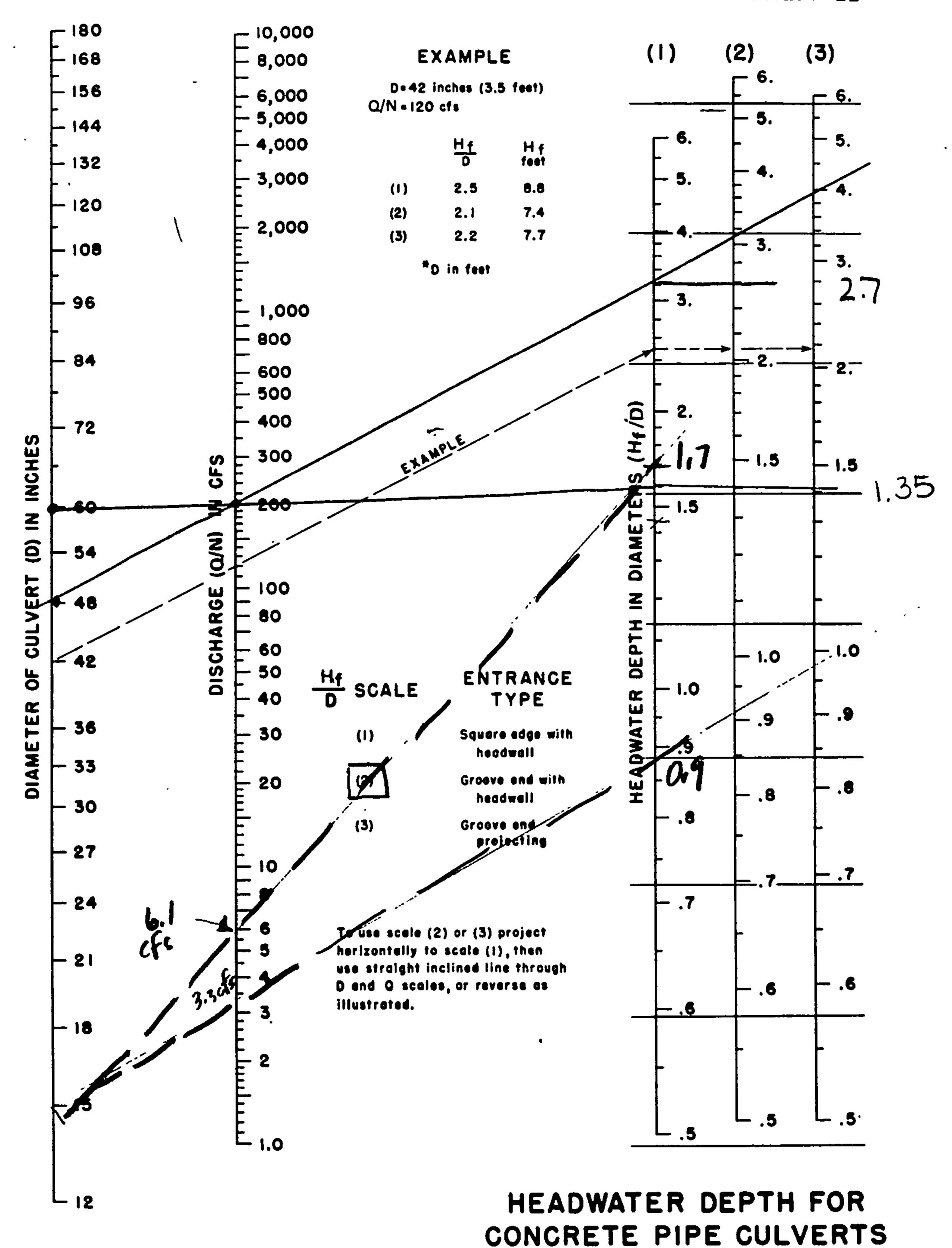
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	•

BOHANNAN-HUSTON INC.

PROJECT NAME	SHEET	OF
PROJECT NO	BY	DATE
SUBJECT	CH'D	DATE



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FEDERAL HIGHWAY ADMINISTRATION
MAY 1973

13-75

91+. 5A

WITH INLET CONTROL

of at = "ar turnsternt 2013 = % mo11M1 26 34.0 = BMY (99.0) 98.0 (92.2) 4.910 (1.4) 99.0 = 001) 18, +004, 0/98 (snes) sndrom 0/91. 1 mol 95.0 = 0014

30 L'1 =82.2 (24.0)04.0 + r.4 (24.0)00.0 = 00/D

27 1.5=(221,5),261 = 2 propher 201/A 2) 11.8 = 1.1 + p.1 = pombin at apprendent letal of

BOHANNAHUSTON INC.

3TAQ_ CH **JIAO** SHEET **JO**

PROJECT NAME

PROJECT NO.

SUBJECT

SHEET PROJECT NAME OE BOHANNAHUSTON INC. M62 - 10 = 0 4.00.0 = H 19.0 = 1 sto84°=0 mitro pa 1 mg 1 mg 对为如何 851 -211-170-2:190 -1 AN AMO Mad Foot norm [La] = = Jun 164 = (09364) 32.0 (-1) "62.0 = 7) "81.0(38.0) 39.0 + 11.1 (31.0) se 33.0 = 3 el-A wese a nortet gevern som worked. Ebs 6,0 at mat zp 24,1 sumbed Colombate Betentien Pand w

3TAQ

DATE

YΒ

SUBJECT

PROJECT NO.

EMERGENCY SPILL WAY DE516N I. Main Pond & NW Corner of Site A. 100-40st Storm Flow = 23.5 Cfs B. use weir equation to size spill way over

A	•	
	BOHANNAN-HUSTON	INC.

PROJECT NAME	SHEET 8	OF
PROJECT NO.	BY Jan	DATE 1/5/95
SUBJECT		DATE

Pupe Extends Out from Spilling	Top of Borm 24.3		For of Asphatt Spith 23.3
41.5'			
New Asthall Drawage Swal	Tov. 24" Pipe 20.15	Spittusy	
Drawage 8m2	See Von	-D. N.T.S.	
	Emergency	Spillway	
•			
* * * * * * * * * * * * * * * * * * *	•	· · · · · · · · · · · · · · · · · · ·	1
	1		
		BOHANNAN-H	USTON INC.
PROJECT NAM PROJECT NO.	\E	SHEETC	OF OATE

SUBJECT _____

_CH'D ______DATE _____

14

_	180-year 8form flows = 1.4 cfs
В,	use weir equation.
•	$L = \frac{Q}{M}, H = 0.5$
	$CH^{1/2}$
	L=1/3
	SIWAW;
•	
C .	WC(810N;
** *	Only graveled spillway is
•	provided, pecsuse;
, 	1. the max. 100-yr storm
- - -	flow is very low, only 1.4 cfs
	2. The mond is boested essentially
• ~ ···································	adjacent to the curb of the
	cut-de-sac, and the west top of
· · · · · · · · · · · · · · · · · · ·	bern elev. is possed 0.5 above
	the berm adjacent to the curb, so that flows are directed
	immediately to the curb.
	3. Londscapet plan provider BOHANNAN-HUSTON INC.