NOT TO SCALE Amafea North Diversion Channel ZONE A-1% ANNUAL CHANCE FLOOD DISCHARGE ONTAINED IN CHANNEL

FIRM NO. 35001C0138C

LEGEND

NOT TO SCALE

FLOODPLAIN MAP

NEW CONCRETE SIDEWALK

PROPOSED ELEV. CONTOUR

---4992---- EXISTING ELEV. CONTOUR EXISTING BLDG FINISH FLOOR ELEV.

PROPOSED FLOW DIRECTION

PROPOSED TOP OF CURB SPOT ELEVATION

PROPOSED FLOWLINE ELEVATION

TBM: 5/8" REBAR W/ ALUMINUM CAP STAMPED "CP 100" 36' S 66'53'04" W OF NW PROPERTY CORNER. TBM ELEV = 5119.57 FEET (NAVD 1988 VERTICAL DATUM)

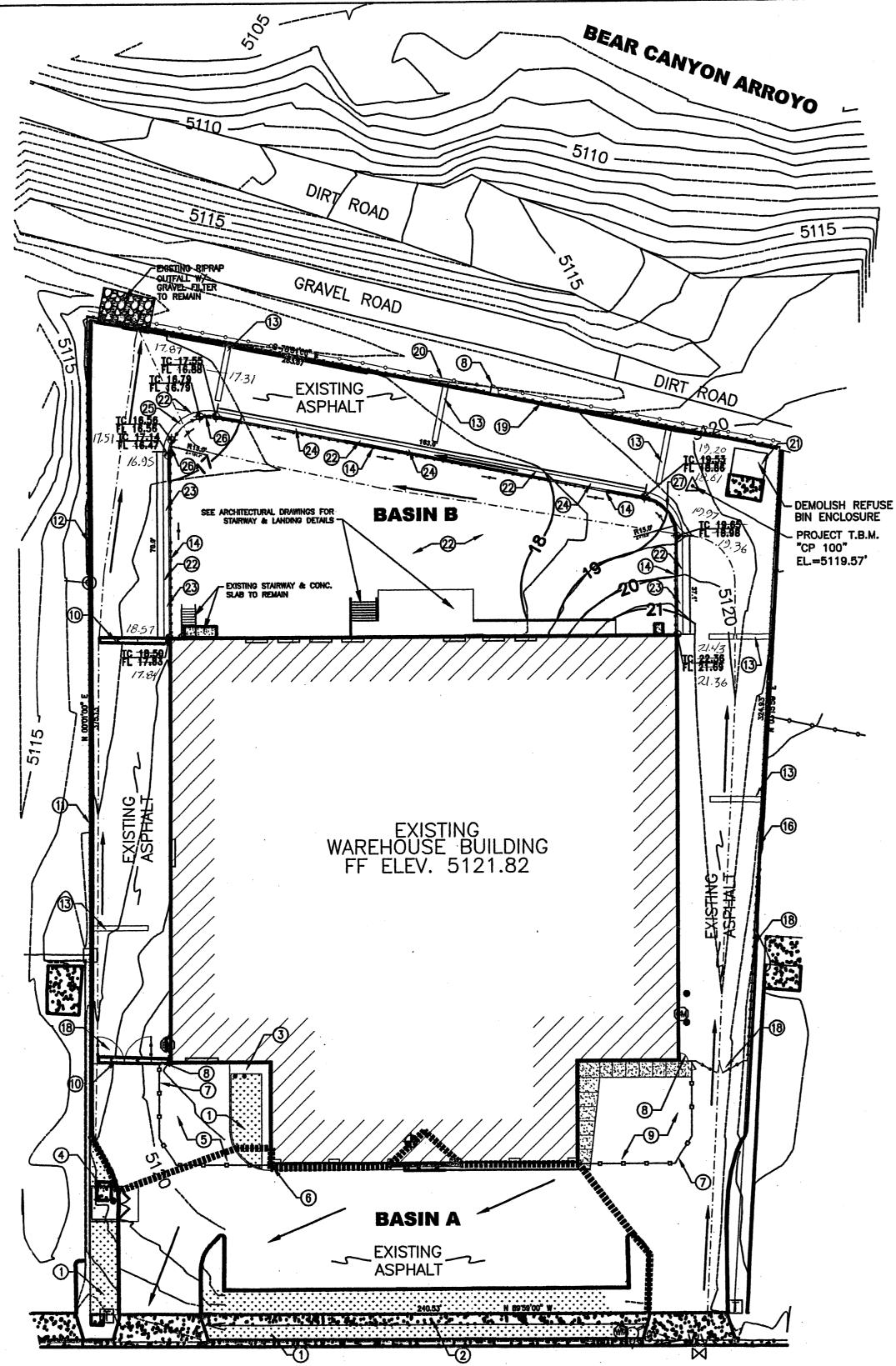
BM: BRASS DISC "SC 27 26 34 35 1965" 264.00' NORTH OF SINGER BLVD. & MIDWAY PARK BLVD. INTERSECTION, THEN WEST 179.00'. BM ELEV = 5113.12 FT (NAVD 1988 VERT. DATUM).

GENERAL NOTES:

1, ADD 5100.00 FEET TO ALL SPOT

ELEVATIONS. 2. FOR TOP OF CONCRETE SLABS AND TOP OF FINISH GRADE SEE: FLOOR PLAN, FOUNDATION PLAN AND DETAILS, AND EXTERIOR WALL SECTION INCLUDED IN BUILDING PERMIT SET OF DRAWINGS.

3. THIS IS NOT A BOUNDARY SURVEY. BEARINGS AND DISTANCES SHOWN HEREON ARE FOR REFERENCE ONLY.



MIDWAY PLACE, N.E.

LEGAL DESCRIPTION:
Lot 13-A-1, Midway Business Park, City of Albuquerque New Mexico.

3831 Midway Place, NE, City of Albuquerque New Mexico.

FLOOD HAZARD ZONE: Tract A-1 is located in Flood Hazard Zone X (i.e., Areas determined to be outside 500—year floodplain) designated on the Federal Emergency Management Agency's (FEMA's) Flood Insurance Rate Map Panel No. 35001C138E (November 19, 2003).

EXISTING SITE CONDITIONS:

Lot 13—A—1 is fully developed. Existing site improvements include an existing 34,783 sf warehouse building, loading docks, an asphalt-paved parking lot, and lanscape planters along the Midway Place frontage.

PROPOSED IMPROVEMENTS:

Proposed site improvements include a change of use for the existing 34,783 sf warehouse building (i.e., occupancy by the Montessori Elementary School) and removal and replacement of asphalt pavement on the north side of the building and at the southeast corner of the building to accommodate a student drop off traffic lane, a children's playgound and additional landscaping, respectively.

DRAINAGE ANALYSIS:
REFERENCE: City of Albuquerque, Development Process Manual -Vol. 2, Section 22.2 - Hydrology, January, 1993.

Principal Design Storm: 100-year 6-hour event

Precipitation Zone 2 (Table A-1)

Excess Precipitation (Table A-8): $E_4 = 0.53$ in (Land Treatment 'A'), $E_2 = 0.78$ in (Land Treatment 'B') $E_3 = 1.13$ in (Land Treatment 'C'), & $E_4 = 2.12$ in (Land Treatment 'D')

Peak Discharge (Table A-9): $Q_{P1} = 1.56 \text{ ft}^3/\text{sec-acre}$ (Land Treatment 'A') $Q_{P2} = 2.28 \text{ ft}^3/\text{sec-acre}$ (Land Treatment 'B') $Q_{P3} = 3.14 \text{ ft}^3/\text{sec-acre}$ (Land Treatment 'C') $Q_{P4} = 4.70 \text{ ft}^3/\text{sec-acre}$ (Land Treatment 'D')

Total Area (Lot 13-A-1) = 87584.0 ft²x 1 acre/43,560 ft² = 2.0 acres

On-Site 'Existing' Condition (Lot 13-A-1, Basin A): Basin A Area = 11,418 ft^2x 1 acre/43,560 ft^2 = 0.26 acres 22.06% Land Treatment 'B': 77.94% Land Treatment 'D'

Weighted E = $((E_2 \times 0.06 \text{ acres}) + (E_4 \times 0.20 \text{ acres}))/0.26 \text{ acres}$ = 1.68 in

 $V_{360} = (1.68 \text{ in } \times 0.26 \text{ acres}) \times \frac{1}{1} \text{ ft/12 in}$ = 0.037 acre-ft x 43,560 ft²/acre

Total $Q_P = (Q_{P2} \times 0.06 \text{ acres}) + (Q_{P4} \times 0.20 \text{ acres})$ = 1.01 ft³/sec

On-Site 'Existing' Condition (Lot 13-A-1. Basin B): Basin B Area = 76,166 ft 2 x 1 acre/43,560 ft 2 = 1.75 acres 0.78% Land Treatment 'B'; 99.22% Land Treatment 'D'

Weighted E = $((E_2 \times 0.01 \text{ acres}) + (E_4 \times 1.73 \text{ acres}))/1.75 \text{ acres}$

 $V_{360} = (1.96 \text{ in } \times 1.75 \text{ acres}) \times \frac{1}{1} \text{ ft/12 in}$ = 0.286 acre-ft x 43,560 $ft^2/acre$ $= 12,439 \text{ ft}^3$

Total $Q_P = (Q_{P2} \times 0.01 \text{ acres}) + (Q_{P4} \times 1.73 \text{ acres})$ = 7.61 ft³/sec

On-Site 'Post Development' Condition (Lot 13-A-1. Basin A): Basin A Area = $11,418 \text{ ft}^2 \text{x} 1 \text{ acre}/43,560 \text{ ft}^2 = 0.26 \text{ acres}$ 20.85% Land Treatment 'B'; 79.15% Land Treatment 'D'

Weighted E = $((E_2 \times 0.05 \text{ acres}) + (E_4 \times 0.21 \text{ acres}))/0.26 \text{ acres}$ = 1.70 in

 $V_{360} = (1.70 \text{ in } \times 0.26 \text{ acres}) \times \frac{1}{1} \text{ ft/12 in}$ = 0.037 acre-ft x 43,560 ft²/acre

Total $Q_P = (Q_{P2} \times 0.05 \text{ acres}) + (Q_{P4} \times 0.21 \text{ acres})$ = 1.02 ft³/sec

On-Site 'Post Development' Condition (Lot 13-A-1, Basin B) Basin B Area = $76,166 \text{ ft}^2 \times 1 \text{ acre}/43,560 \text{ ft}^2 = 1.75 \text{ acres}$ 17.53% Land Treatment 'C'; 82.47% Land Treatment 'D'

Weighted E = $((E_3 \times 0.31 \text{ acres}) + (E_4 \times 1.44 \text{ acres}))/1.75 \text{ acres}$ = 1.80 in

 $V_{360} = (1.80 \text{ in } \times 1.75 \text{ acres}) \times \frac{1}{2} \text{ ft/12 in}$ = 0.262 acre-ft x 43,560 ft 2 /acre $= 11,413 \text{ ft}^3$

Total $Q_p = (Q_{p3} \times 0.31 \text{ acres}) + (Q_{p4} \times 1.44 \text{ acres})$ = 7.18 ft³/sec

Runoff from offsite drainage basins does not enter onto Lot 13-A-1. All runoff from Lot 13-A-1 is discharged to the AMAFCA Bear Canyon Arroyo right-of-way and, subsequently, enters the AMAFCA North Diversion Channel. The 'Post Development' peak discharge from Lot 13—A—1 will have a reduced impact on the AMAFCA Bear Canyon Arroyo right—of—way. Free discharge from Lot 13-A-1 is recommended for the following

- 1. The 'Post Development' runoff from Lot 13-A-1 is a small percentage of the total runoff from the entire offsite drainage basin.
- 2. The Bear Canyon Arroyo has sufficent capacity to convey
- the 'Post Development' runoff from Lot 13-A-1. 3. There are no storm water ponds on parcels adjacent to Lot 13-A-1.

ENGINEER'S CERTIFICATION:

THE ENGINEER HAS PERSONALLY VISITED AND INSPECTED THE SITE. NO GRADING, FILLING, OR EXCAVATIONS HAVE OCCURRED AFTER THE "AS BUILT" GRADING AND PLAN WAS SUBMITTED IN MARCH 1994.

OKeved Notes

I. EXISTING LANDSCAPE, TYPICAL. EXISTING 4'-0" WIDE SIDEWALK. CONCRETE SIDEWALK.

NEW REFUSE BIN ENCLOSURE.

ART PATIO & BREAK, EXISTING ASPHALT PAVING.

6'-0" HIGH WROUGHT IRON FENCE.

PRESCHOOL PATIO & PLAY AREA. 10. EXISTING SPEED BUMP.

ACCESS GATE.

11. EXISTING 8'-0" HIGH CHAIN LINK FENCE (±276' LONG).

12. 8' HIGH NEW CHAIN LINK FENCE (±99' LONG - V.I.F.). 13. SPEED BUMP.

14. 4'-0" HIGH NEW CHAIN LINK FENCE WITH 4'-0" ACCESS POINTS @ 40 O.C TYP. 15. PLAY AREA.

16. EXISTING 8'-0" HIGH CHAIN LINK FENCE (±159' LONG). 17. 8'-0" HIGH NEW CHAIN LINK FENCE (±166' LONG - V.I.F.).

18. 8'-0" HIGH NEW CHAIN LINK GATE - 18' WIDE - (2) 9'-0" SECTIONS.

19. 8'-0" HIGH NEW CHAIN LINK FENCE (±264' LONG - V.I.F.)

21. REPAIR EXISTING CONCRETE AFTER REMOVAL OF REFUSE BIN ENCLOSURE.
22 SAW CUT AND REMOVE ASPHALT PAVEMENT, FINE GRADE AND INSTALL NON-WOVEN

GEOTEXTILE (MIRAFI 140NL OR ENGINEER APPROVED EQUAL) W/ 2 FT OVERLAP & NO STAPLES, AND BACKFILL W/ "PLAYGROUND GRADE" SAND. FINISH GRADE TO 5 IN DEPTH OVER ENTIRE PAVEMENT REMOVAL AREA.

23 CONSTRUCT DEPRESSED GUTTER (OVER EXISTING SUBGRADE COMPACTED TO 95%)
PER COA STD DWG #2415A. MATCH GUTTER TO TOP OF EXISTING ASPHALT

24 CONSTRUCT STANDARD CURB AND GUTTER (OVER EXISTING SUBGRADE COMPACTED 25 CONSTRUCT 2'-7 1/2 " X 6" RIBBON CURB (OVER EXISTING SUBGRADE COMPACTED

TO 95%) BETWEEN CURB RETURNS. MATCH GUTTER TO TOP OF EXISTING ASPHALT 26 CONSTRUCT FILLET CURB TRANSITION FROM DEPRESSED GUTTER OR STANDARD

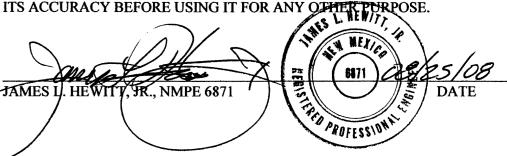
BEYOND CURB RETURN).
27 TRANSITION FROM DEPRESSED GUTTER TO STANDARD CURB AND GUTTER BETWEEN

Legend 8'-0" NEW CHAINLINK FENCE EXISTING CHAINLINK FENCE ' wrought iron fence

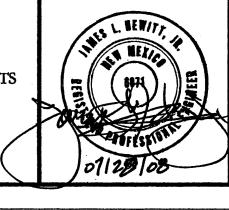
CERTIFICATION WITH VERIFICATION BY ENGINEER OF RECORD

I, JAMES L. HEWITT, JR., NMPE 6871, OF THE FIRM HEWITT ENGINEERING & BEEN GRADED AND WILL DRAIN IN SUBSTANTIAL COMPLIANCE AND IN THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY ME, OR UNDER MY DIRECT SUPERVISION, AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY



Hewitt Engineering & Environmental Consultants 5615 CREGGS STREET, NW ALBUQUERQUE, NEW MEXICO 87120 PHONE: 505.899.3195 FAX: 505.899.3195





624 Tijeras Avenue NW Albuquerque, NM 87102 tel: 505.243.3499 fax: 505.243.3583 info@integrateddesignarch.com www.integrateddesignarch.com



RECEIVED AUG 26 2008 HYDROLOGY

MONTESSORI SCHOOL **TENANT IMPROVEMENTS** Albuquerque, New Mexico

PROJECT ARCHITECT: BOB HALL, AIA

Project #: IDA-07-21-P JULY 28, 2008

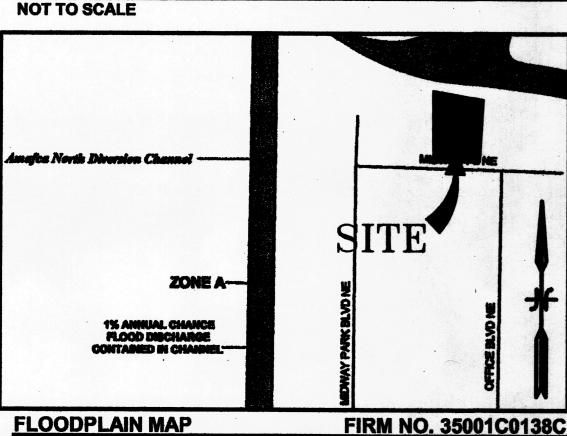
REVISED GRADING & DRAINAGE PLAN

ACAD-GradingDrainagePlan_REV1.dwg

Sheet of

GRADING & DRAINAGE PLAN

VICINITY MAP NOT TO SCALE



LEGEND

NOT TO SCALE

NEW CONCRETE SIDEWALK

PROPOSED ELEV. CONTOUR

FIN. FLR 94.00 EXISTING BLDG FINISH FLOOR ELEV.

PROPOSED FLOW DIRECTION

TC 12.67 PROPOSED TOP OF CURB SPOT ELEVATION

FL 12.67 PROPOSED FLOWLINE ELEVATION

TBM: 5/8" REBAR W/ ALUMINUM CAP STAMPED "CP 100" 36' S 66'53'04" W OF NW PROPERTY CORNER. TBM ELEV = 5119.57 FEET (NAVD 1988 VERTICAL DATUM)

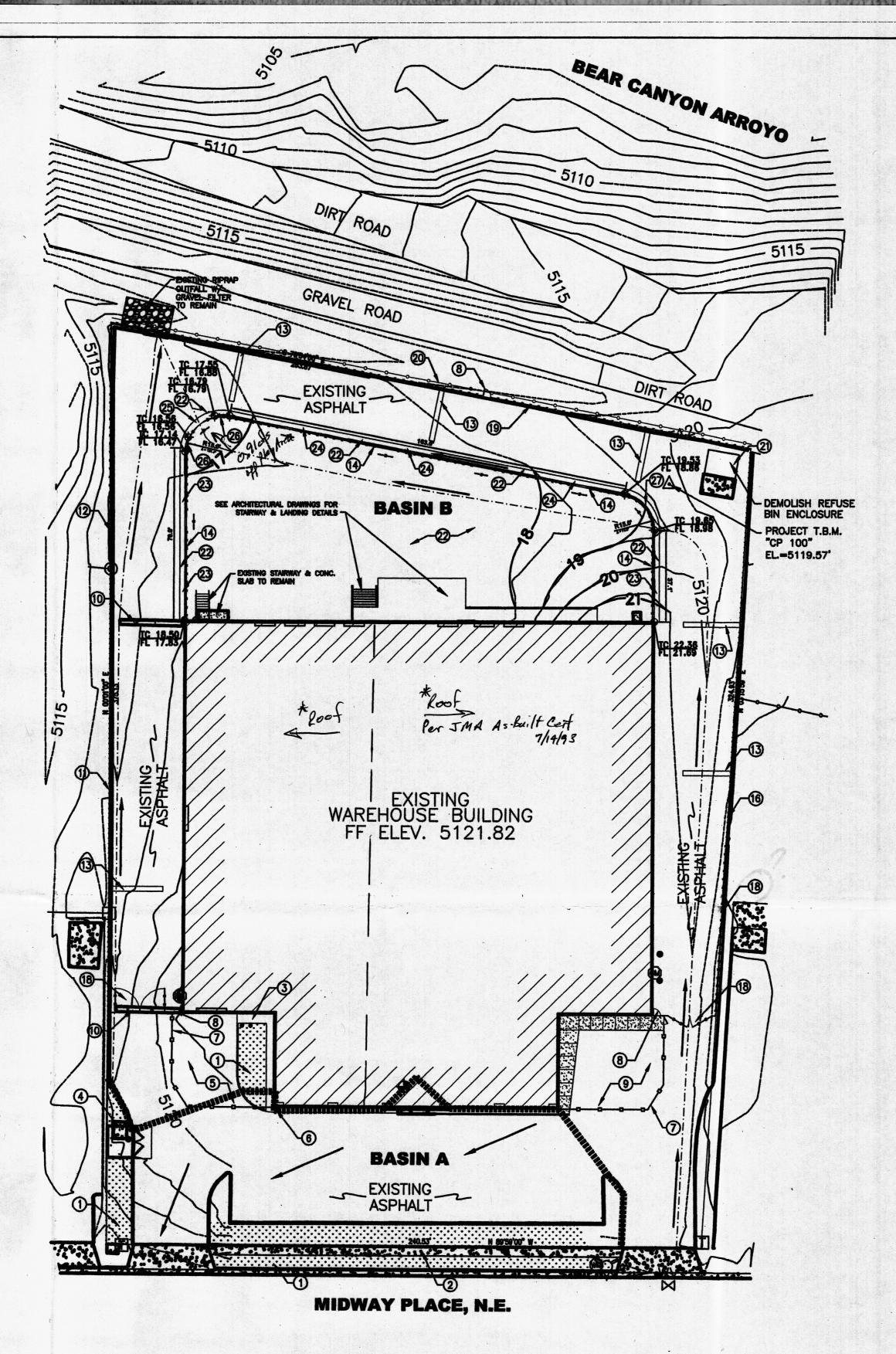
BM: BRASS DISC "SC 27 26 34 35 1965" 264.00' NORTH OF SINGER BLVD. & MIDWAY PARK BLVD. INTERSECTION, THEN WEST 179.00'. BM ELEV = 5113.12 FT (NAVD 1988 VERT. DATUM).

GENERAL NOTES:

1. ADD 5100.00 FEET TO ALL SPOT ELEVATIONS.

2. FOR TOP OF CONCRETE SLABS AND TOP OF FINISH GRADE SEE: FLOOR PLAN, FOUNDATION PLAN AND DETAILS, AND EXTERIOR WALL SECTION INCLUDED IN BUILDING PERMIT SET OF DRAWINGS.

3. THIS IS NOT A BOUNDARY SURVEY. BEARINGS AND DISTANCES SHOWN HEREON ARE FOR REFERENCE ONLY.



Legal Description:
Lot 13-A-1, Midway Business Park, City of Albuquerque New Mexico.

STREET ADDRESS:

3831 Midway Place, NE, City of Albuquerque New Mexico.

Tract A-1 is located in Flood Hazard Zone X (i.e., Areas determined to be outside 500—year floodplain) designated on the Federal Emergency Management Agency's (FEMA's) Flood Insurance Rate Map Panel No. 35001C138E (November 19, 2003).

EXISTING SITE CONDITIONS:

Lot 13—A—1 is fully developed. Existing site improvements include an existing 34,783 sf warehouse building, loading docks, an asphalt—paved parking lot, and lanscape planters along the Midway Place frontage.

PROPOSED IMPROVEMENTS:

Proposed site improvements include a change of use for the existing 34,783 sf warehouse building (i.e., occupancy by the Montessori Elementary School) and removal and replacement of asphalt pavement on the north side of the building and at the southeast corner of the building to accommodate a student drop off traffic lane, a children's playgound and additional landscaping, respectively.

DRAINAGE ANALYSIS: REFERENCE: City of Albuquerque, Development Process Manual -Vol. 2, Section 22.2 - Hydrology, January, 1993.

Principal Design Storm: 100-year 6-hour event

Precipitation Zone 2 (Table A-1)

Excess Precipitation (Table A-8): $E_1 = 0.53$ in (Land Treatment 'A'), $E_2 = 0.78$ in (Land Treatment 'B') $E_3 = 1.13$ in (Land Treatment 'C'), & $E_4 = 2.12$ in (Land Treatment 'D')

Peak Discharge (Table A-9): $Q_{P1} = 1.56 \text{ ft}^3/\text{sec-acre}$ (Land Treatment 'A') $Q_{P2} = 2.28 \text{ ft}^3/\text{sec-acre} \text{ (Land Treatment 'B')}$ $Q_{P3} = 3.14 \text{ ft}^3/\text{sec-acre} \text{ (Land Treatment 'C')}$ $Q_{P4} = 4.70 \text{ ft}^3/\text{sec-acre} \text{ (Land Treatment 'D')}$

Total Area (Lot 13-A-1) = 87584.0 ft²x 1 acre/43,560 ft² = 2.0 acres

On-Site 'Existing' Condition (Lot 13-A-1, Basin A): Basin A Area = 11,418 ft^2x 1 acre/43,560 ft^2 = 0.26 acres 22.06% Land Treatment 'B'; 77.94% Land Treatment 'D'

Weighted E = $((E_2 \times 0.06 \text{ acres}) + (E_4 \times 0.20 \text{ acres}))/0.26 \text{ acres}$ = 1.68 in

 $V_{360} = (1.68 \text{ in } \times 0.26 \text{ acres}) \times 1 \text{ ft/12 in}$ = 0.037 acre-ft x 43,560 ft²/acre = 1,602 ft³

Total $Q_P = (Q_{P2} \times 0.06 \text{ acres}) + (Q_{P4} \times 0.20 \text{ acres})$ = 1.01 ft³/sec

On-Site 'Existing' Condition (Lot 13-A-1, Basin B): Basin B Area = $76,166 \text{ ft}^2 \times 1 \text{ acre}/43,560 \text{ ft}^2 = 1.75 \text{ acres}$ 0.78% Land Treatment 'B'; 99.22% Land Treatment 'D'

Weighted E = $((E_2 \times 0.01 \text{ acres}) + (E_4 \times 1.73 \text{ acres}))/1.75 \text{ acres}$ = 1.96 in

 $V_{360} = (1.96 \text{ in } \times 1.75 \text{ acres}) \times 1 \text{ ft/}12 \text{ in}$ = 0.286 acre-ft × 43,560 ft²/acre = 12,439 ft³

Total $Q_P = (Q_{P2} \times 0.01 \text{ acres}) + (Q_{P4} \times 1.73 \text{ acres})$ = 7.61 ft³/sec

On-Site 'Post Development' Condition (Lot 13-A-1, Basin A): Basin A Area = $11,418 \text{ ft}^2 \times 1 \text{ acre}/43,560 \text{ ft}^2 = 0.26 \text{ acres}$ 20.85% Land Treatment 'B': 79.15% Land Treatment 'D'

Weighted E = $((E_2 \times 0.05 \text{ acres}) + (E_4 \times 0.21 \text{ acres}))/0.26 \text{ acres}$ = 1.70 in

 $V_{360} = (1.70 \text{ in } \times 0.26 \text{ acres}) \times 1 \text{ ft/12 in}$ = 0.037 acre-ft × 43,560 ft²/acre = 1,617 ft³

Total $Q_P = (Q_{P2} \times 0.05 \text{ acres}) + (Q_{P4} \times 0.21 \text{ acres})$ = 1.02 ft³/sec

On-Site 'Post Development' Condition (Lot 13-A-1, Basin B): Basin B Area = $76,166 \text{ ft}^2 \times 1 \text{ acre}/43,560 \text{ ft}^2 = 1.75 \text{ acres}$ 17.53% Land Treatment 'C': 82.47% Land Treatment 'D'

Weighted E = $((E_3 \times 0.31 \text{ acres}) + (E_4 \times 1.44 \text{ acres}))/1.75 \text{ acres}$ = 1.80 in

 $V_{360} = (1.80 \text{ in } \times 1.75 \text{ acres}) \times 1 \text{ ft/12 in}$ = 0.262 acre—ft × 43,560 ft²/acre = 11,413 ft³

Total $Q_p = (Q_{p3} \times 0.31 \text{ acres}) + (Q_{p4} \times 1.44 \text{ acres})$ = 7.18 ft³/sec

Runoff from offsite drainage basins does not enter onto Lot 13—A—1.

All runoff from Lot 13—A—1 is discharged to the AMAFCA Bear Canyon Arroyo right—of—way and, subsequently, enters the AMAFCA North Diversion Channel. The 'Post Development' peak discharge from Lot 13—A—1 will have a reduced impact on the AMAFCA Bear Canyon Arroyo right—of—way. Free discharge from Lot 13—A—1 is recommended for the following

1. The 'Post Development' runoff from Lot 13—A—1 is a small percentage of the total runoff from the entire offsite drainage basin.

2. The Bear Canyon Arroyo has sufficent capacity to convey the 'Post Development' runoff from Lot 13-A-1.

3. There are no storm water ponds on parcels adjacent to Lot 13-A-1.

ENGINEER'S CERTIFICATION:

THE ENGINEER HAS PERSONALLY VISITED AND INSPECTED THE SITE. NO GRADING, FILLING, OR EXCAVATIONS HAVE OCCURRED AFTER THE "AS BUILT" GRADING AND PLAN WAS SUBMITTED IN MARCH 1994.

01/28/08

Legend 3'-0" NEW CHAINLINK FENCE EXISTING CHAINLINK FENCE

WROUGHT IRON FENCE

OKeyed Notes

CONCRETE SIDEWALK. NEW REFUSE BIN ENCLOSURE.

ACCESS GATE.

13. SPEED BUMP.

15. PLAY AREA.

10. EXISTING SPEED BUMP.

20. EXISTING GUARDRAIL.

. EXISTING LANDSCAPE, TYPICAL.

EXISTING 4'-0" WIDE SIDEWALK.

7. 6'-0" HIGH WROUGHT IRON FENCE.

. PRESCHOOL PATIO & PLAY AREA.

. ART PATIO & BREAK, EXISTING ASPHALT PAVING.

11. EXISTING 8'-0" HIGH CHAIN LINK FENCE (±276' LONG). 12. 8' HIGH NEW CHAIN LINK FENCE (±99' LONG - V.I.F.).

16. EXISTING 8'-0" HIGH CHAIN LINK FENCE (±159' LONG).

17. 8'-0" HIGH NEW CHAIN LINK FENCE (±166' LONG - V.I.F.).

19. 8'-0" HIGH NEW CHAIN LINK FENCE (±264' LONG - V.I.F.).

IN DEPTH OVER ENTIRE PAVEMENT REMOVAL AREA.

18. 8'-0" HIGH NEW CHAIN LINK GATE - 18' WIDE - (2) 9'-0" SECTIONS.

21. REPAIR EXISTING CONCRETE AFTER REMOVAL OF REFUSE BIN ENCLOSURE.

22 SAW CUT AND REMOVE ASPHALT PAVEMENT, FINE GRADE AND INSTALL NON-WOVEN

23 CONSTRUCT DEPRESSED GUTTER (OVER EXISTING SUBGRADE COMPACTED TO 95%)

PER COA STD DWG #2415A. MATCH GUTTER TO TOP OF EXISTING ASPHALT

24 CONSTRUCT STANDARD CURB AND GUTTER (OVER EXISTING SUBGRADE COMPACTED TO 95%) PER COA STD DWG #2415A. MATCH GUTTER TO TOP OF EXISTING ASPHALT

25 CONSTRUCT 2'-7 1/2 " X 6" RIBBON CURB (OVER EXISTING SUBGRADE COMPACTED TO 95%) BETWEEN CURB RETURNS. MATCH GUTTER TO TOP OF EXISTING ASPHALT

CURB AND GUTTER TO RIBBON CURB (TO EXTEND FROM CURB RETURN TO 6 FT

27 TRANSITION FROM DEPRESSED GUTTER TO STANDARD CURB AND GUTTER BETWEEN

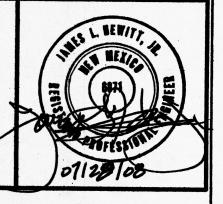
26 CONSTRUCT FILLET CURB TRANSITION FROM DEPRESSED GUTTER OR STANDARD

GEOTEXTILE (MIRAFI 140NL OR ENGINEER APPROVED EQUAL) W/ 2 FT OVERLAP &

NO STAPLES, AND BACKFILL W/ "PLAYGROUND GRADE" SAND. FINISH GRADE TO 5

14. 4'-0" HIGH NEW CHAIN LINK FENCE WITH 4'-0" ACCESS POINTS @ 40 O.C TYP.

HEWITT ENGINEERING & ENVIRONMENTAL CONSULTANTS 5615 CREGGS STREET, NW ALBUQUERQUE, NEW MEXICO 87120 PHONE: 505.899.3195 FAX: 505.899.3195



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JUL 2 9 2008

HYDROLOGY

SECTION

IDA-07-21-P

JULY 28, 2008

Project #:

624 Tijeras Avenue NW Albuquerque, NM 87102 tel: 505.243.3499 fax: 505.243.3583 info@integrateddesignarch.com www.integrateddesignarch.com

MONTESSORI SCHOOL TENANT IMPROVEMENTS

Albuquerque, New Mexico PROJECT ARCHITECT:

BOB HALL, AIA

REVISED GRADING & DRAINAGE PLAN

ACAD-GradingDrainagePlan_REV1.dwg C-2.0