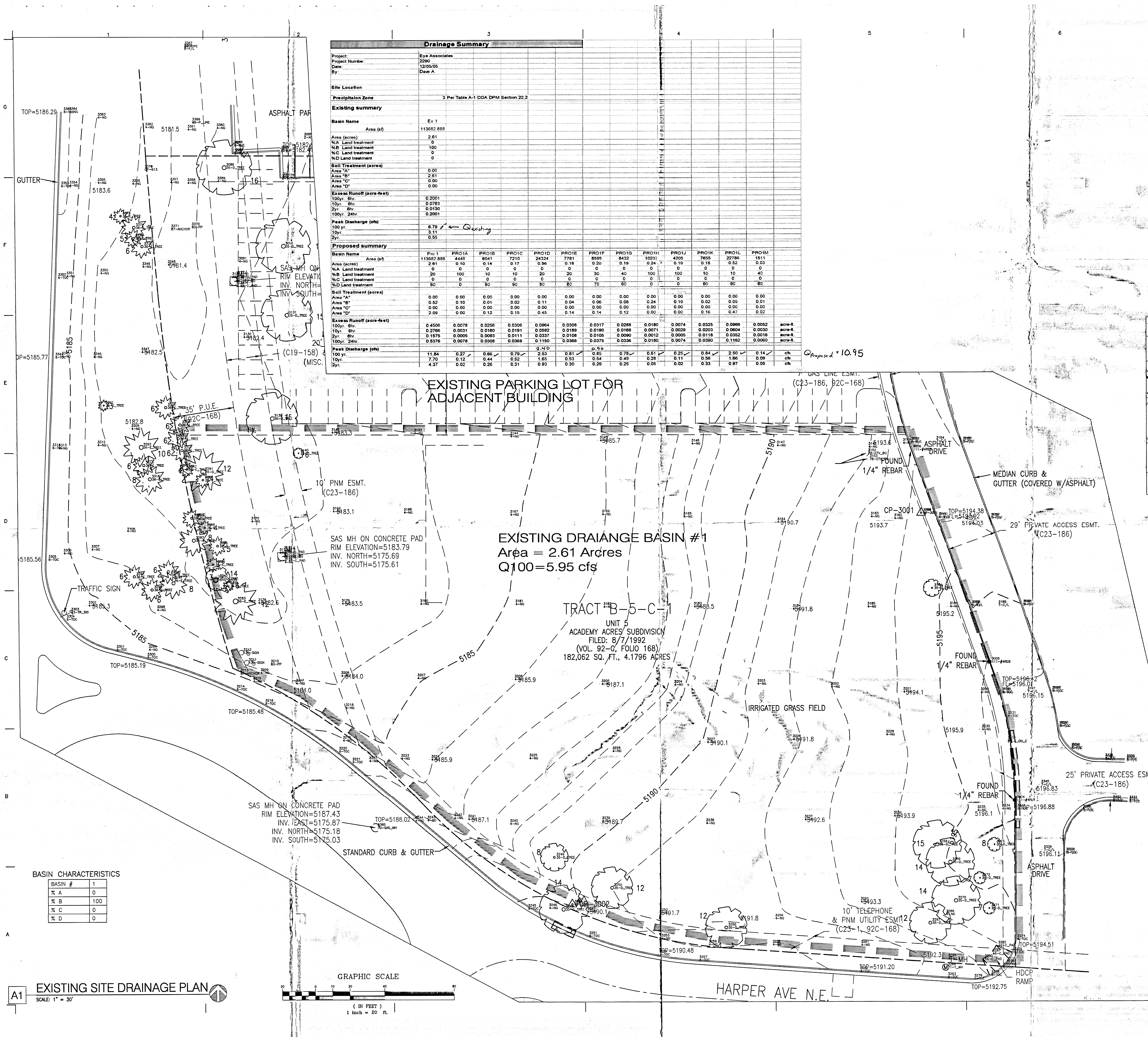
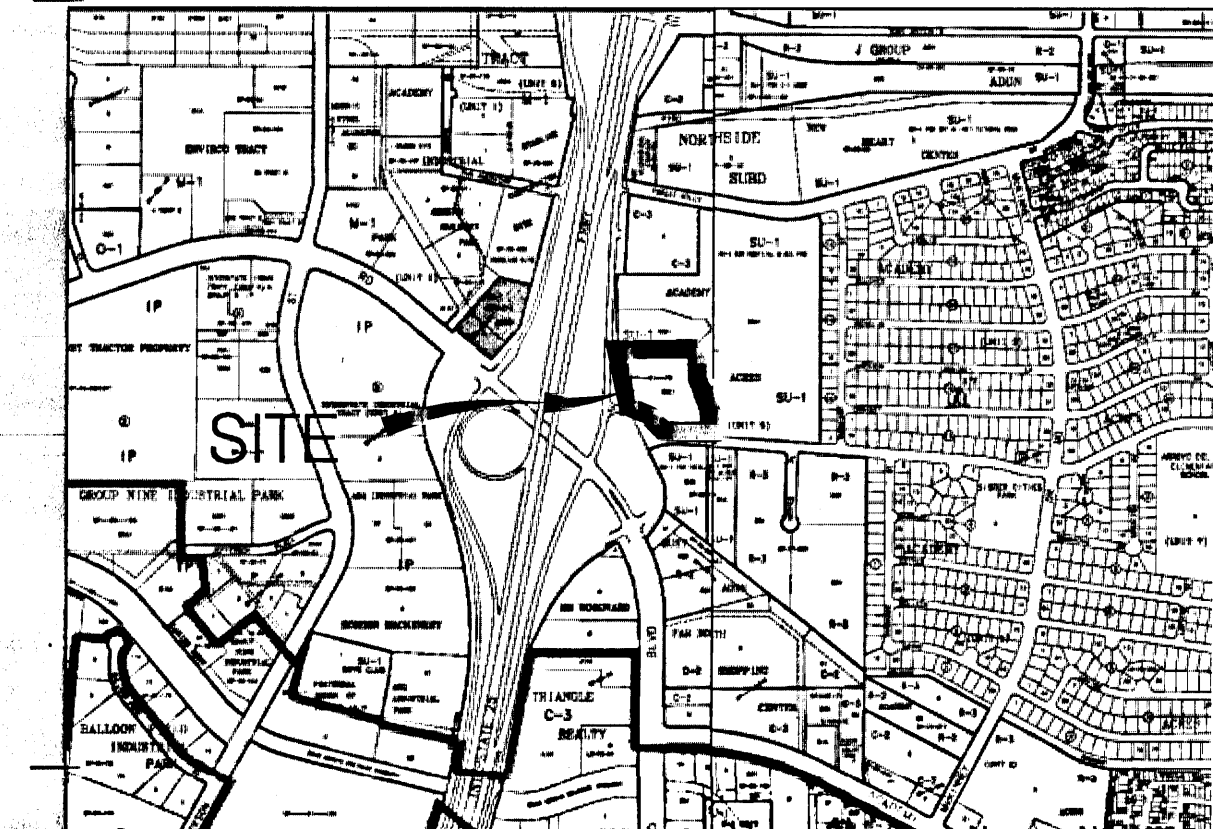




Drainage Summary															
Project:	Eye Associates														
Project Number:	2290														
Date:	12/05/05														
By:	Dave A.														
Site Location															
Precipitation Zone	3 Per Table A-1 COA DPM Section 22.2														
Existing summary															
Basin Name	Ex 1														
Area (a)	115682.888														
Area (acres)	2.61														
%A Land treatment	0														
%B Land treatment	0														
%C Land treatment	0														
%D Land treatment	0														
Soil Treatment (acres)	0.00														
Area "A"	0.00														
Area "B"	0.00														
Area "C"	0.00														
Area "D"	0.00														
Excess Runoff (acre-feet)	0.2001														
100yr. 8hr.	0.02001														
10yr. 8hr.	0.00783														
2yr. 8hr.	0.0150														
100yr. 24hr.	0.2001														
Peak Discharge (cfs)	6.79														
100yr.	3.11														
2yr.	0.55														
Proposed summary															
Basin Name	Area (a)	Pro 1	PRO1A	PRO1B	PRO1C	PRO1D	PRO1E	PRO1F	PRO1G	PRO1H	PRO1I	PRO1J	PRO1K	PRO1L	PRO1M
Area (acres)	115682.888	4448	6041	7210	24334	7781	8595	8432	10037	4205	7855	22785	1511		
%A Land treatment	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
%B Land treatment	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
%C Land treatment	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
%D Land treatment	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Soil Treatment (acres)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Area "A"	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Area "B"	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Area "C"	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Area "D"	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Excess Runoff (acre-feet)	0.4506	0.0078	0.0256	0.0306	0.0864	0.0308	0.0317	0.0288	0.0180	0.0074	0.0325	0.0866	0.0052	0.0052	acre-1
100yr. 8hr.	0.2766	0.0031	0.0180	0.0181	0.0582	0.0189	0.0190	0.0188	0.0071	0.0029	0.0033	0.0504	0.0030	0.0030	acre-1
10yr. 8hr.	0.1878	0.0005	0.0093	0.0111	0.0337	0.0108	0.0105	0.0090	0.0012	0.0005	0.0118	0.0332	0.0016	0.0016	acre-1
100yr. 24hr.	0.5378	0.0078	0.0308	0.0368	0.1150	0.0368	0.0375	0.0336	0.0180	0.0074	0.0330	0.1162	0.0050	0.0050	acre-1
Peak Discharge (cfs)	11.84	0.27	0.86	0.78	2.53	0.81	0.85	0.78	0.61	0.25	0.84	2.50	0.14	0.14	ch
100yr.	7.70	0.12	0.44	0.52	1.65	0.53	0.54	0.49	0.28	0.11	0.56	1.86	0.09	0.09	ch
2yr.	4.37	0.02	0.26	0.31	0.93	0.30	0.29	0.25	0.05	0.02	0.33	0.97	0.05	0.05	ch

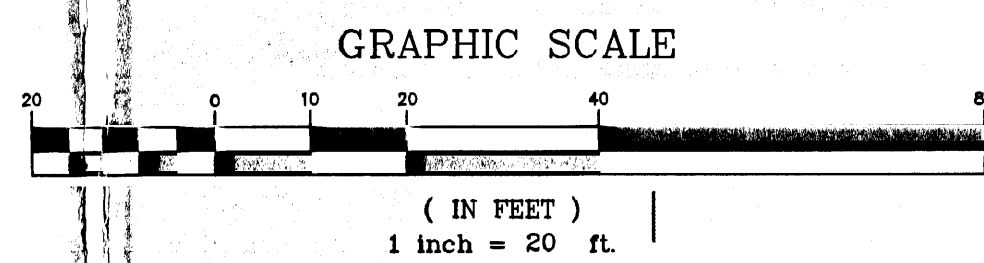


VICINITY MAP
SCALE: NOT TO SCALE
E-17-Z E-18-Z



FLOOD ZONE MAP
SCALE: NOT TO SCALE
E-17-Z E-18-Z

BASIN CHARACTERISTICS	
BASIN #	1
% A	0
% B	100
% C	0
% D	0



EXISTING SITE DRAINAGE PLAN
SCALE: 1" = 30'

2-23-06

THE DESIGN GROUP
ARCHITECT - PLANNING - INTERIOR DESIGN
303 CENTRAL AVENUE, SUITE 200
ALBUQUERQUE, NEW MEXICO 87109
PHONE: 505.242.6165 FAX: 505.242.6551

PROJECT NAME:
EYE ASSOCIATES
5175 HARPER DRIVE NE
ALBUQUERQUE, NEW MEXICO 87109

REVISIONS

NO.	DATE	DESCRIPTION
02.06.05	ADDITION	#1 - COMPLETE UPDATE

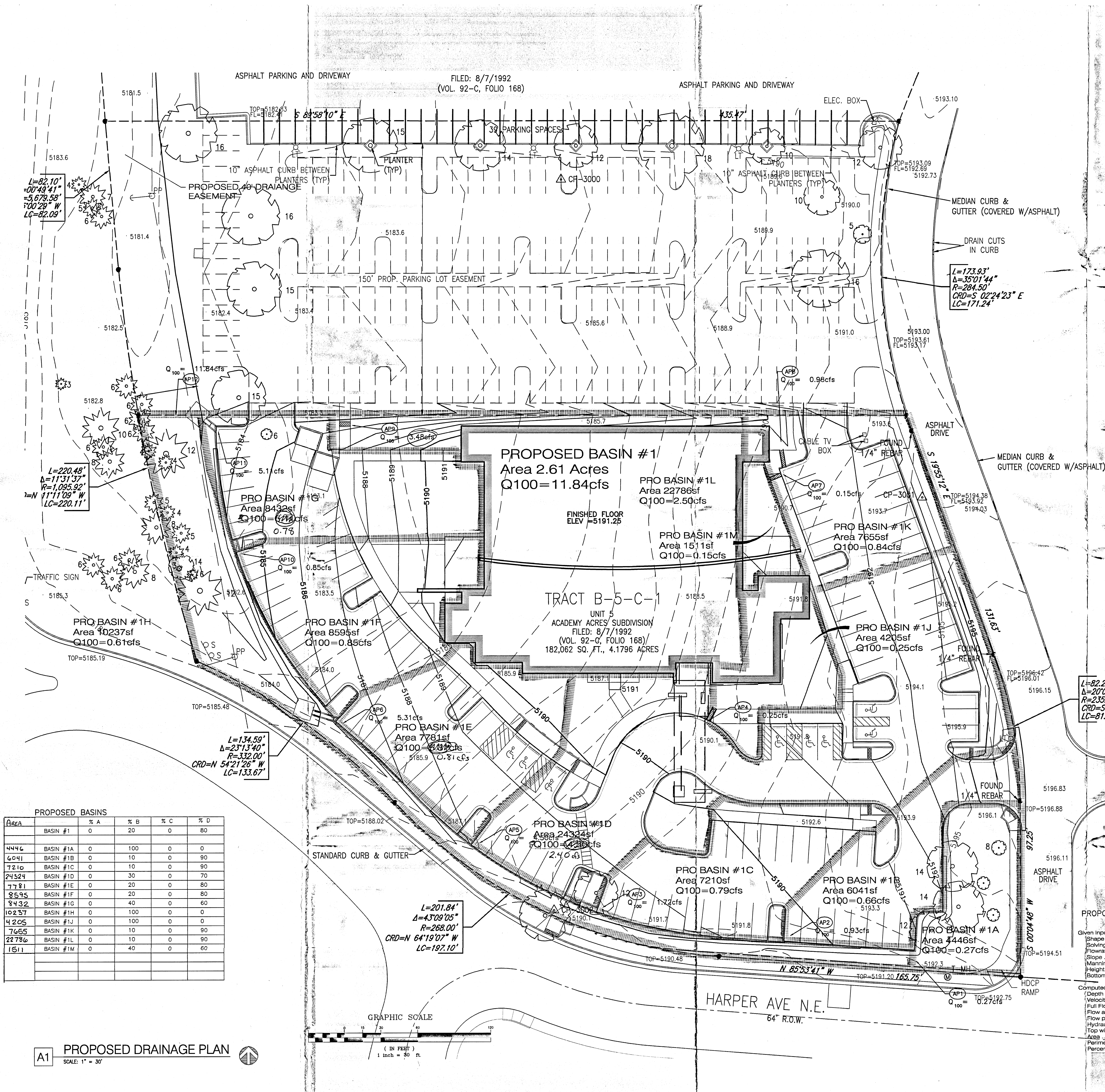
SHEET TITLE:
EXISTING SITE DRAINAGE PLAN

DESIGNED BY	CHECKED BY	DATE	SCALE	VARIES
DAV	DAV	02.06.05	AS SHOWN	VARIES

DATE: 02.06.05 COMP. FILE: 2290-CD1

CD1

HYDROLOGY SECTION



PROPOSED BASINS				
AREA	BASIN #	% A	% B	% C
4444	BASIN #1A	0	100	0
6041	BASIN #1B	0	10	90
7210	BASIN #1C	0	10	90
24324	BASIN #1D	0	30	70
7781	BASIN #1E	0	20	80
9595	BASIN #1F	0	20	80
9432	BASIN #1G	0	40	60
10237	BASIN #1H	0	100	0
4205	BASIN #1I	0	100	0
7665	BASIN #1J	0	10	90
22786	BASIN #1K	0	10	90
1511	BASIN #1L	0	40	60

- EYE ASSOCIATES OF NEW MEXICO**
- I. PURPOSE AND SCOPE**
The purpose of this drainage plan is to present the existing and proposed drainage management plans for a new Eye Associates facility located on north side of Harper Blvd. NE and on the East of 125 Frontage Road.
- II. SITE DESCRIPTION AND HISTORY**
The existing site is currently vacant. The land was subdivided as part of the Presbyterian North Campus. The site formerly contained a 165' wide parking lot easement and is in the process of being replatted to transfer the parking lot easement into the parcel B-5-B-1 that is serves.
- III. COMPUTATIONAL PROCEDURES**
Hydrologic analysis was performed utilizing the design criteria found in the COA-DPM Section 22.2 released in June 1997.
- IV. PRECIPITATION**
The 100-yr 6-hr duration storm was used as the design storm for this analysis. This site is within Zone III as identified in the DPM Section 22.2. Tables within the section were used to establish the 6-hr precipitation, excess precipitation and peak discharge.
- V. EXISTING DRAINAGE CONDITIONS OVERVIEW**
The 2.6098 acre site, Tract B-5-C-1, located within Zone Attic E-17-7 and E-18-7 is currently vacant. The site generally slopes from south-east to the north-west. Harper Avenue to the south has standard curb and gutter and conveys offsite flows toward the west and into the 125 North bound frontage road. The private drive to the east of the site also has curb and gutter and collects offsite water into a grass lined detention pond on the east side of the private drive.
Drainage from the parking lot to the north is generally sheet flow and collects in the north-west corner and is conveyed through culverts and a grass lined channel to the headwall that conveys all flow from the Presbyterian North Campus under the highway and to the concrete drainage channel on the west side of the highway.
The site being developed currently is covered in turf and gently slope toward the north-west. The excess runoff from this site combines with the flow from the parking lot on the property to the north and is conveyed as described above.
The peak runoff from existing site has a peak runoff of 6.79 cfs.
- VI. DRAINAGE MANAGEMENT PLAN**
The site improvements include construction of a new medical office building, and associated parking areas. The proposed improvements also include approximately 20% landscaped area.
The proposed site will generally drain from south east to the north west. The runoff from the parking lot areas will discharge through curb openings under concrete rundowns into a rip rap or grass lined channel that will convey the runoff to the existing outlet in the north west corner.
The proposed drainage will be increased to a peak discharge of 11.84 cfs. There is a large stand of trees and shrubs on the west side of the site (near the 125 North bound frontage road). Drainage from the site will be directed to the south and to the west to flow through and past the dense vegetation. This will help to harvest the smaller rainfall events and to reduce travel velocities.
There will be a newly plattd drainage easement to allow for runoff from this site to travel, combined with water on the neighbors site, to the existing storm drainage culverts that discharge under the highway.
The site has been divided into 12 sub basins. The sub basins are shown on this plan and the analysis point define the combined peak flow rate. The concrete rundowns were designed for the 5.31 cfs which is the larger of the two rundowns and the concrete channels between sections of the parking lot are also sized for that same peak 5.31 cfs. The sidewalk culverts are all 2' wide clear and were designed for the worst case which occurs at the northwest of the building and has a peak flowrate of 6.48 cfs.
- VII. CONCLUSIONS**
The will be an increase in the developed flow rates from those originally computed for the fully developed condition, 6.79 cfs to 11.84 cfs. After combining this increase with the developed flowrates for the entire Presbyterian Campus, there should be a minimal effects to downstream storm drainage systems.

PROPOSED CONCRETE RUNDOWNS & CULVERTS

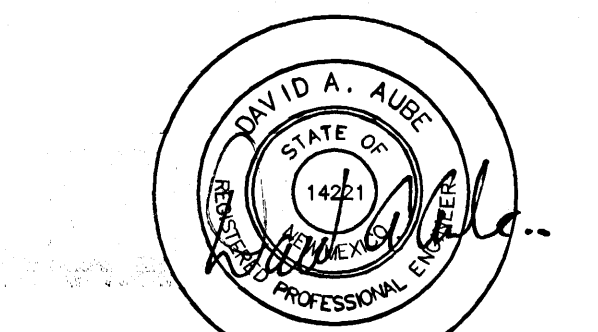
Channel Calculator

Given Input Data:
Shape Rectangular
Solving for Depth of Flow
Flowrate 5.3100 cfs
Slope 0.0300 ft/ft
Manning's n 0.0130
Height 12.0000 in
Bottom width 24.0000 in

Computed Results:
Depth 4.0366 in
Velocity 7.8929 fps
Full Flowrate 24.9448 cfs
Flow area 0.6726 ft²
Flow perimeter 32.0731 in
Hydraulic radius 3.0205 in
Top width 24.0000 in
Perimeter 2.0000 ft
Area 48.0000 in
Percent full 33.6379 %

Orifice Calculator
Given Input Data:
Solving for Flowrate
Coefficient 1.0000 ft²
Area 1.0000 ft²
Headwater 0.5000 ft
Tailwater 0.0000 ft

Computed Results:
Flowrate 5.5598 cfs
Velocity 6.5688 fps



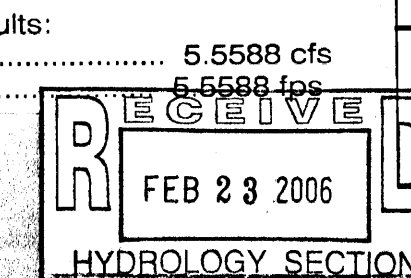
2-25-06

THE DESIGN GROUP
ARCHITECTS, PLANNERS, INTERIOR DESIGN
301 CENTRAL AVENUE SE SUITE 200
ALBUQUERQUE, NEW MEXICO 87102
PHONE: 505.242.0000 FAX: 505.242.0001

PROJECT NAME:
EYE ASSOCIATES
570 HARPER DRIVE NE
ALBUQUERQUE, NEW MEXICO 87109

REVISIONS		
NO.	DATE	DESCRIPTION
02.06.08	ADDENDUM #1 - COMPLETE UPDATE	

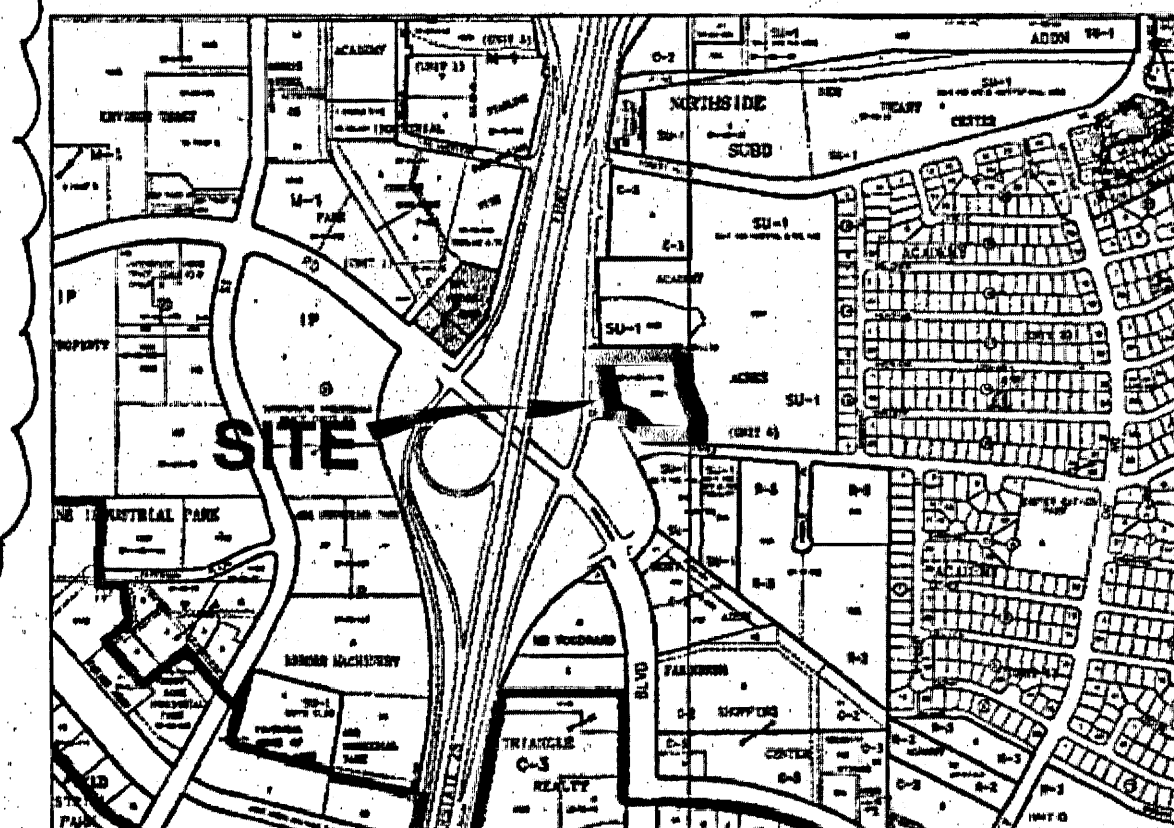
SHEET TITLE:
PROPOSED DRAINAGE PLAN
DESIGNED: PMD SCALE: VARIOUS
CHECKED: DAA JOB NO.: 2290
DATE: 02.06.08 DRAWN: PMD



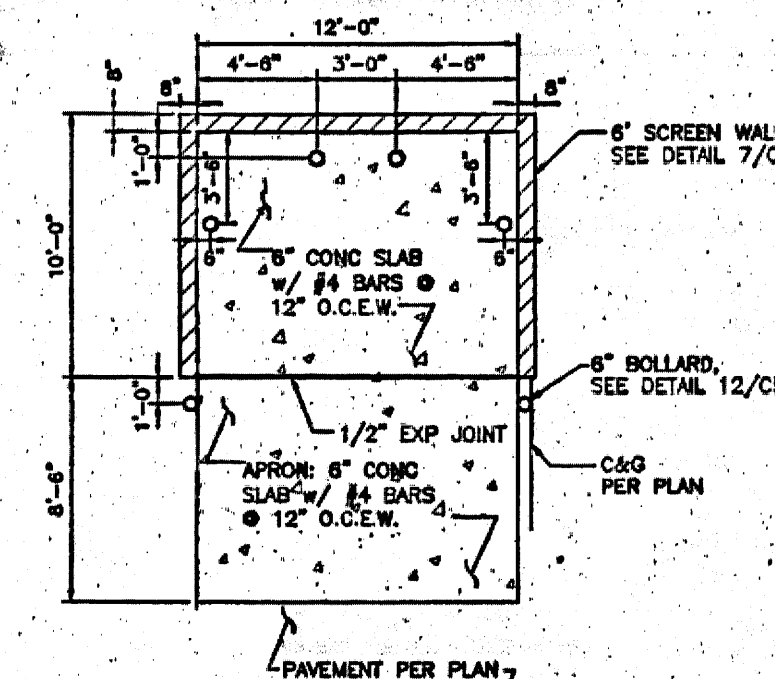
CD.2

- A. Lighting for on-site parking to comply with section 14.16.3.9.
- B. No billboards are permitted on site.
- C. Reciprocal parking and vehicular access is permitted between Tracts B-5-B1A and B-5-C-1A.

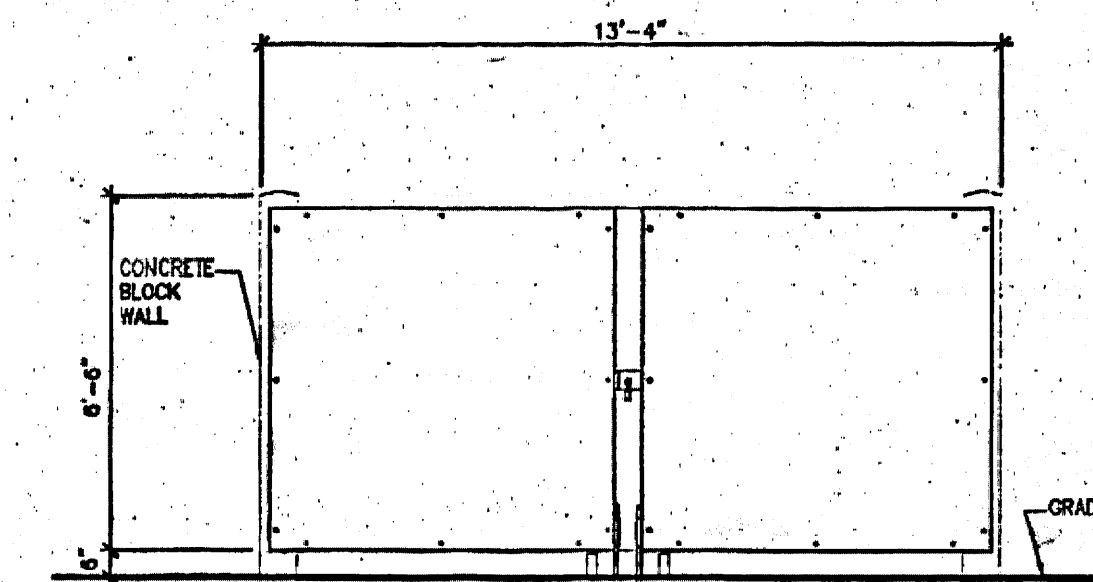
TABULATED PARKING	
PROPOSED:	
REGULAR	106
SMALL	0
HC	7
VAN HC	1
TOTAL	114
3 Motorcycle spaces	
7 Bike Rack	
REQUIRED @ 5 SPACES / DOCTOR	
22 DOCTORS	
TOTAL	110



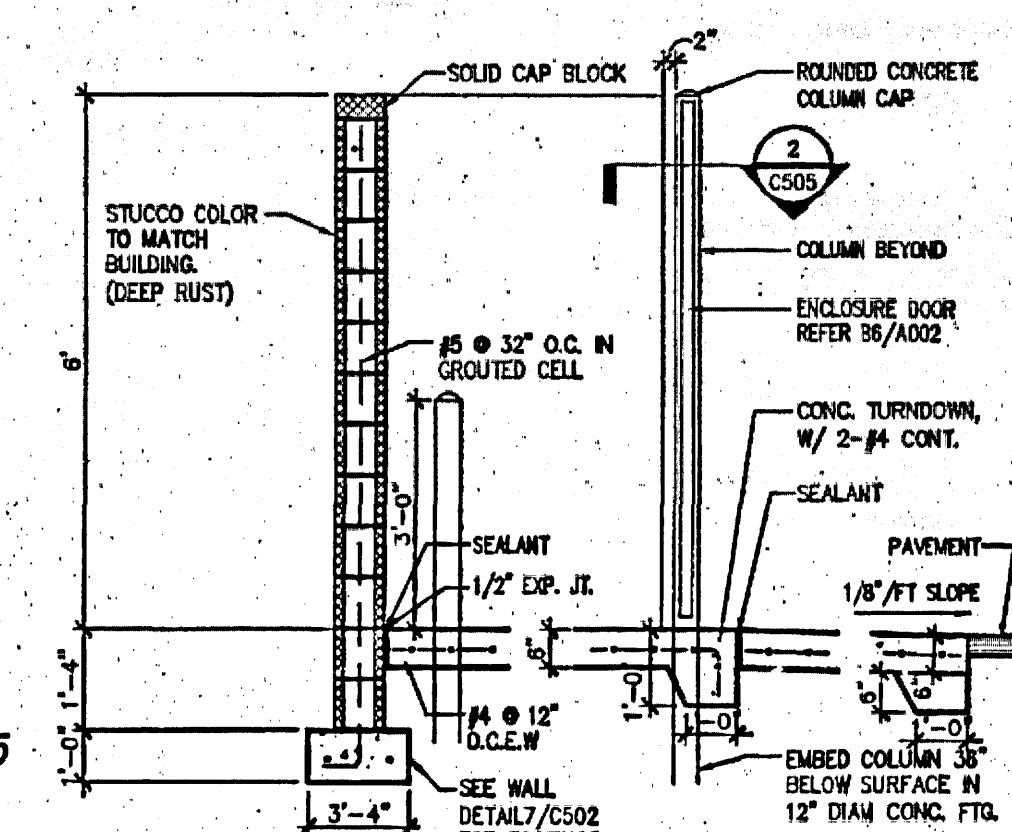
E6 VICINITY MAP
SCALE: NOT TO SCALE
E-17-Z E-18-Z



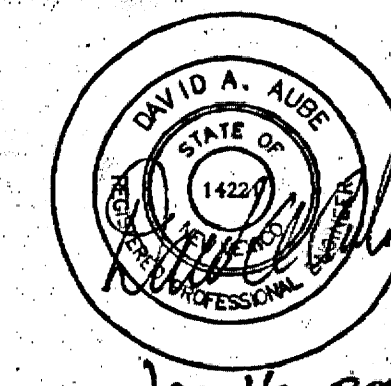
TRASH ENCLOSURE PLAN



TRASH ENCLOSURE ELEVATION
SCALE: 3/8"=1'-0"



TRASH ENCLOSURE SECTION



Jan 16, 2006

THE DESIGN GROUP
ARCHITECTS • PLANNERS • INTERIOR DESIGNERS
212 CENTRAL AVENUE SUITE 20
ALBUQUERQUE, NEW MEXICO 87102
PHONE: 505 262-1800 FAX: 505 262-1801

PROJECT NAME:
EYE ASSOCIATES
HARPER AVENUE N.E.
ALBUQUERQUE, NEW MEXICO 871xx

REVISIONS		
No.	DATE	DESCRIPTION
1	12.19.05	PLANING COMMENTS

SHEET TITLE:
SITE DEVELOPMENT
PLAN FOR
BUILDING PERMIT

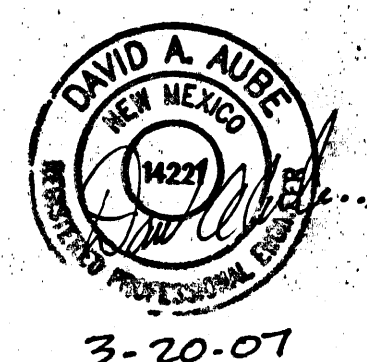
DESIGNED: DAA	SCALE: 1"
CHECKED: DAA	JOB NO: 22
DATE: 11 22 05	COMP. BY: 22

C11

OUT

C1.1

ADMINISTRATIVE AMENDMENT
File # 06-0081 Project # 100871
Reverse shape of optical shop
from elliptical to rectangular
C. Marone 2/3/06
APPROVED BY _____ DATE _____



Drainage Certification

I, David A Aube, NMPE 14221, of the firm The Hartman + Majewski Design Group, Inc, hereby certify that this project is in substantial compliance with and in accordance with the design intent of the Administrative Amendment to the Site Development Plan approved plan dated 1-16-08. The record information edited onto the original design documents when obtained by Gary Bortough of the firm Cerdig Martin Construction further certifies that I have personally visited the project site on March 12, 2007 and have determined by visual inspection that the actual site conditions shown on this plan to be true and correct to the best of my knowledge and belief. This certification is submitted in support of a request for Permanent Certificate of Occupancy.

The record information presented hereon is not necessarily complete and intended only to verify substantial compliance of the drainage aspects of this project. Those relying on the record documents are advised to obtain independent verification of its accuracy before using it for any other purpose.

A1 SITE DEVELOPMENT PLAN FOR BUILDING PERMIT

SCALE: 1"=20'

~~HARPER~~ AVE N.E.

S' EXISTING
BIKE ROUTE

—EXISTING SIDEWALK
GRAPHIC SCALE

(IN FEET)
1 inch = 20 f

EXISTING ACCESS RAMP
TO BE REBUILT.
PER COA STD DWG 244