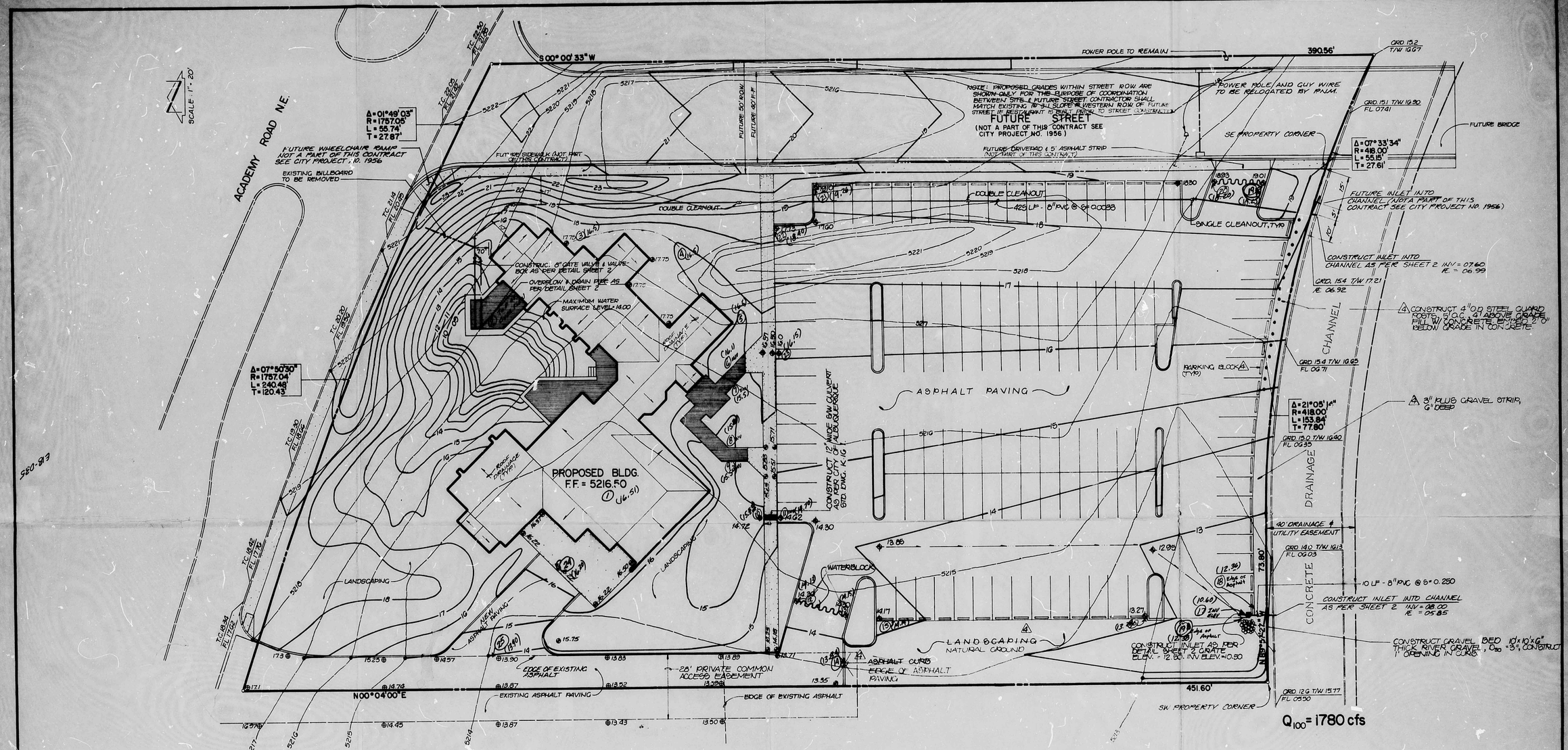
HYDROLOGY E18/D35 SEAGULL STREET RESTAURANT

E18-D35

SEAGULL STREET

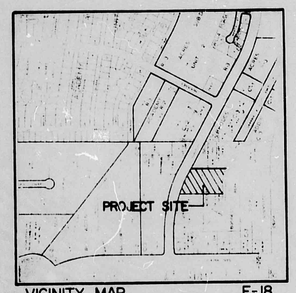
RESTAURANT

2 PLANS



LEGEND

- EXISTING SPOT ELEVATION
- EXISTING CONTOUR
- PROPOSED SPOT ELEVATION
- PROPOSED CONTOUR
- PROPOSED CONCRETE
- PROPERTY LINE
- SWALE
- FUTURE CONSTRUCTION



VICINITY MAP
SCALE: 1" = 800'

E-18

PROJECT BENCHMARK = T.B.M.

AN A.C.S. BRASS CAP STAMPED "1-E18A 1978" SET IN THE TOP OF A CONCRETE POST FLUSH WITH THE GROUND LOCATED IN THE CENTER OF THE MEDIAN IN ACADEMY ROAD N.E. NEAR THE EAST NOSE, 0.1 MILE EAST OF SAN MATEO BLVD. N.E, ELEVATION: 5222.426 FEET (MSLD).

LEGAL DESCRIPTION: TRACT B OF A REPLAT OF NORTH ACADEMY BUSINESS PARK

Requested Survey 8-14-84
Hydrology Book A-2 Page 25-26
(14.13) = Survey Shots.
(13) = Requested Location of Shot.

CONSTRUCTION NOTES:

1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES.
2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.

EROSION CONTROL

DURING CONSTRUCTION, EROSION CONTROL SHALL BE ACCOMPLISHED BY THE SEDIMENTATION BASIN CREATED BY THE PONDING AREA IN THE VICINITY OF THE PROPOSED INLET AT THE SOUTHWEST CORNER OF THE SITE. ALL SURFACE DRAINAGE SHALL BE DIRECTED TOWARDS THIS BASIN.

NOTE:

This plan was revised on July 20, 1984. Revisions include the replacement of header curb with parking blocks along the perimeter of the parking lot. This revision does not affect the drainage concepts. Should the drop inlet become clogged, ponding will occur to elevation 5213.00, then spill to adjacent property to the west. Surface drainage easements have been granted over and across adjacent property as per the covenants and restrictions for North Academy Business Park, Book MISC. 997, Page 33, Document No. 83-14636.

REGISTERED PROFESSIONAL ENGINEER — STATE OF NEW MEXICO — THOMAS J. MARTIN — 3782

11/7/84

Martin/Martin Associates Inc.
811 DALLAS N.E. · ALBUQUERQUE · NEW MEXICO · 87110
ENGINEERS

NO.	DATE	BY	REVISIONS
1	10/28/83	SKS	GENERAL REVISIONS
2	10/24/83	SKS	REVISE INLET INTO CHANNEL
3	3/20/84	JMC	General - Site plan revisions.
4	7/20/84	SKS	General - Site plan revisions

DESIGNED BY: S.K.S.
DRAWN BY: C.G.C.
APPROVED: T.J.M.

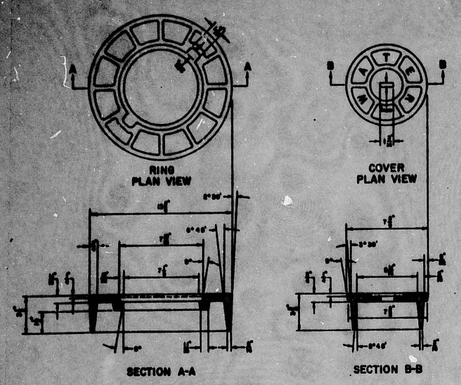
JOB NO. 3-0683
DATE 9/83

GRADING & DRAINAGE PLAN
LAUGHING SEAGULL RESTAURANT

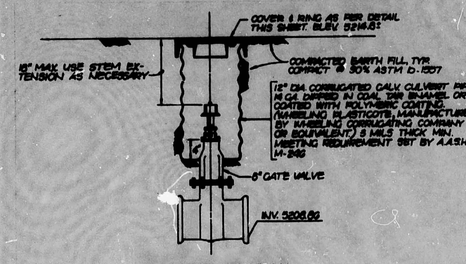
RECEIVED AUG 08 1984

FILE NO.

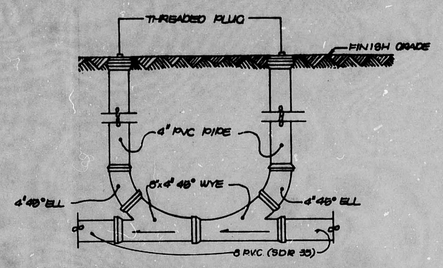
SHEET 1 OF 2



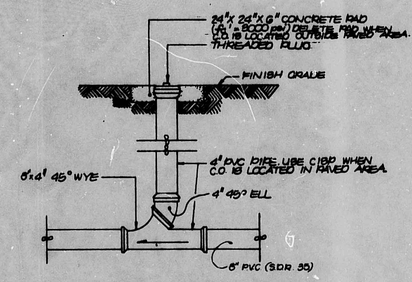
VALVEBOX COVER DETAILS
NO SCALE



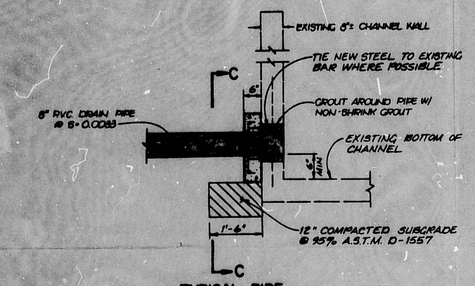
VALVEBOX SECTION
NO SCALE



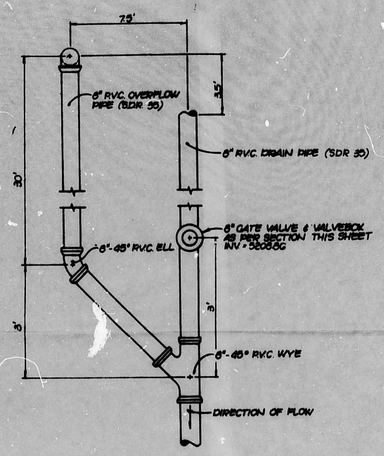
TYPICAL DOUBLE CLEANOUT SECTION
NO SCALE:



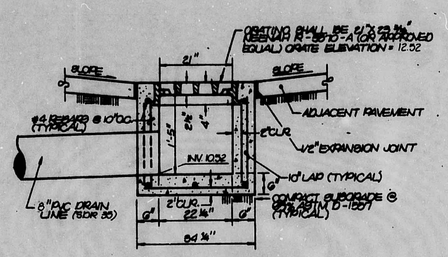
TYPICAL SINGLE CLEANOUT SECTION
NO SCALE:



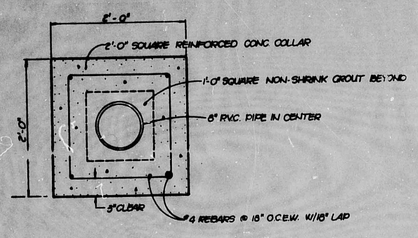
TYPICAL PIPE INLET INTO CHANNEL
SCALE: 1" = 2'



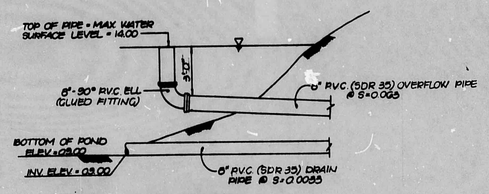
POND OUTLET DETAIL
NO SCALE



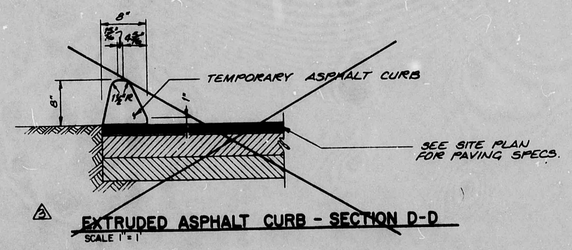
INLET SECTION
NO SCALE:



SECTION C-C
SCALE: 1" = 1'



POND OUTLET SECTION
NO SCALE



EXTRUDED ASPHALT CURB - SECTION D-D
SCALE 1" = 1'

Drainage Plan

The following items concerning the Laughing Seagull Restaurant Drainage Plan are contained hereon:

1. Vicinity Map
2. Grading Plan
3. Calculations

The proposed improvements as shown on the Vicinity Map are located within Tract 2-A-2-B, North Academy Business Park. Proposed improvements consist of the construction of a one-story building, asphalt paved parking and associated landscaped areas, including a pond.

The Academy Business Park Master Plan and Drainage Report was approved by the City of Albuquerque Hydrology Department on November 5, 1981. The master plan was updated on May 12, 1983 reflecting the change in configuration of the Seagull Restaurant and showing Phase I (Gardunos) as existing. The plan shown hereon conforms with the approved master drainage plan which calls for Basin 6 to be discharged to an inlet into the box culvert. The proposed ponding area north of the restaurant will not retain rainfall runoff. The pond water surface level will be held at a constant elevation with overflow being discharged through an 8" pipe to the open channel. Offsite flows do not contribute to the project site. Flows from the east will be contained within the future street to the east of the site which will be built immediately following the proposed improvements shown hereon. Grades have been coordinated with proposed street grades. With the exception of the channel itself, the site does not lie within a flood hazard zone as indicated by Plate 17, Flood Hazard Boundary Maps prepared by the National Flood Insurance Program, August, 1982.

The Grading Plan shows 1) existing contours at 1'0" intervals, 2) proposed grades indicated by spot elevations and contours at 1'0" intervals, 3) continuity between proposed and existing elevations, and 4) limit and character of proposed improvements.

The Calculations of runoff in the existing and developed states for both the 100-year, 6-hour storm and the 10-year, 6-hour storm appear below. The SCS Curve Number Method was used in order to maintain consistency with the approved master drainage report previously prepared by this office.

DRAINAGE BASIN COMPUTATIONS (S.C.S. METHOD)
BASIN:
PORTION OF BASIN #6
EXISTING STATE

LAND-USE DESCRIPTION:

DESCRIPTION	SOIL GROUP	AREA (ac)	CN	% OF TOTAL
NATIVE	B	2.60	80	100 %
COMPOSITE:		2.60	80	100 %

WATERSHED CHARACTERISTICS:

LENGTH: 220 ft.
DROP: 7 ft.
SLOPE: 3.18 %
TIME OF CONCENTRATION: 0.03 hr.

STORM CHARACTERISTICS:

100 yr.; 6hr.		10 yr.; 6hr	
FREQUENCY: 100 yr.; 6hr.		FREQUENCY: 10 yr.; 6hr	
RAINFALL: 2.6 in.		RAINFALL: 1.8 in.	
RUNOFF: 0.94 in.		RUNOFF: 0.44 in.	
DISTRIBUTION CURVE: 75		DISTRIBUTION CURVE: 75	
DISCHARGE: 1.55 cfs/ac/in		DISCHARGE: 1.55 cfs/ac/in	
PEAK DISCHARGE (Q):	3.8 c.f.s.	PEAK DISCHARGE (Q):	1.7 c.f.s.
VOLUME: 9,000 c.f.	0.21 ac.ft.	VOLUME: 4,100 c.f.	0.09 ac.ft.

DRAINAGE BASIN COMPUTATIONS (S.C.S. METHOD)
BASIN:
PORTION OF BASIN #6
DEVELOPED STATE

LAND-USE DESCRIPTION:

DESCRIPTION	SOIL GROUP	AREA (ac)	CN	% OF TOTAL
PERVIOUS AREA	B	0.50	80	24 %
IMPERVIOUS AREA		1.55	95	76 %
COMPOSITE:		2.05	91	100 %

WATERSHED CHARACTERISTICS:

LENGTH: 250 ft.
DROP: 8 ft.
SLOPE: 3.00 %
TIME OF CONCENTRATION: 0.04 hr.

STORM CHARACTERISTICS:

100 yr.; 6hr.		10 yr.; 6hr	
FREQUENCY: 100 yr.; 6hr.		FREQUENCY: 10 yr.; 6hr	
RAINFALL: 2.6 in.		RAINFALL: 1.8 in.	
RUNOFF: 1.73 in.		RUNOFF: 1.01 in.	
DISTRIBUTION CURVE: 75		DISTRIBUTION CURVE: 75	
DISCHARGE: 1.55 cfs/ac/in		DISCHARGE: 1.55 cfs/ac/in	
PEAK DISCHARGE (Q):	5.4 c.f.s.	PEAK DISCHARGE (Q):	3.2 c.f.s.
VOLUME: 12,800 c.f.	0.29 ac.ft.	VOLUME: 7,500 c.f.	0.17 ac.ft.

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ENGINEERS

NO.	DATE	BY	REVISIONS
1	10-24-83	SKB	REVISE INLET INTO CHANNEL
2	3-19-84	GKB	General site plan revisions.

DESIGNED BY: SKB
DRAWN BY: CGC
APPROVED: TTM

JOB NO. 3-0683
DATE 9/83

GRADING & DRAINAGE PLAN
LAUGHING SEAGULL RESTAURANT

FILE NO.
SHEET 2 OF