CITY OF ALBUQUERQUE

Planning Department
Alan Varela, Director

Mayor Timothy M. Keller

March 3, 2026

David Soule, P.E.
Rio Grande Engineering
PO BOX 93924
Albuquerque, NM 87199

RE: 6105 La Cholla Ct NE
Grading & Drainage Plan
Engineer's Stamp Date: 2/24/26
Hydrology File: E23D003O3
Case # HYDR-2025-00410

Dear Mr. Soule:

Based upon the information provided in your submittal received 2/26/2026, the Grading and Drainage Plans are approved for Building Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter.

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

PRIOR TO CERTIFICATE OF OCCUPANCY:

1. Engineer's Certification, per the DPM Part 6-14 (F): Engineer's Certification Checklist For Non-Subdivision is required.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 505-924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 505-924-3314 or amontoya@cabq.gov.

Sincerely,

Anthony Montoya, Jr., P.E., CFM
Senior Engineer, Hydrology
Planning Department, Development Review Services

BASIN DATA

Basin	Area (sf)	Area (acres)	Treatment A %	Treatment A (acres)	Treatment B %	Treatment B (acres)	Treatment C %	Treatment C (acres)	Treatment D %	Treatment D (acres)	Q100 GENERATED	Q100 DISCHARGED
TOTAL SITE EXISTING	17117	0.393	100%	0.0740	0.0%	0.0847	0.0%	0.1783	0%	0.1163	0.75	0.75
BASIN A TO POND	13333	0.306	10%	0.0306	22.0%	0.0673	30.0%	0.0918	38%	0.1163	1.05	0.52
REMAINING SITE	3784	0.087	50%	0.0434	20.0%	0.0174	30.0%	0.0261	0%	0.0000	0.22	0.22
TOTAL SITE PROPOSE												0.69 *

* SITE DISCHARGE CALCULATED UTILIZING AHYMO. THE DETENTION PONDS HAVE DIFFERENT Tp THEREFORE THE CUMULATIVE DISCHARGE IS SLIGHTLY LESS THAT THE SUMMATION OF EACH BASIN.

WATER QUALITY VOLUME 177.4 CF

City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
APPROVED
DATE: 3/3/2026
BY: [signature]
HydroTrans # E23D003O3

THE APPROVAL OF THESE PLANS/REPORTS SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY OF ALBUQUERQUE FROM REQUIRING CORRECTIONS FOR ERRORS OR DIMENSIONS IN PLANS, SPECIFICATIONS, OR CONSTRUCTION DOCUMENTS. SUCH APPROVED PLANS/REPORTS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.

THE APPROVAL OF THESE PLANS/REPORTS SHALL EXPIRE TWO (2) YEARS AFTER THE APPROVAL DATE IF NO BUILDING PERMIT HAS BEEN PULLED ON THE DEVELOPMENT.

EROSION CONTROL NOTES:

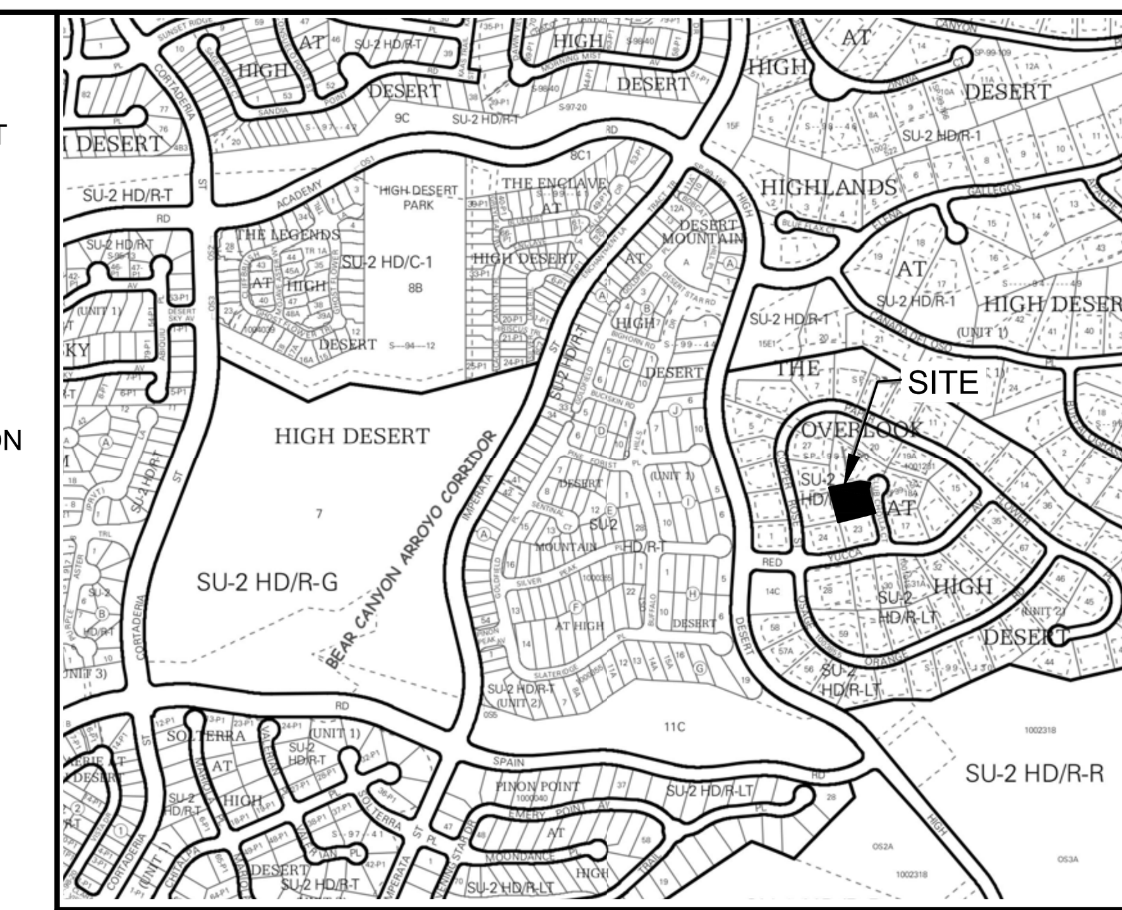
1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOP SOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.

2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.

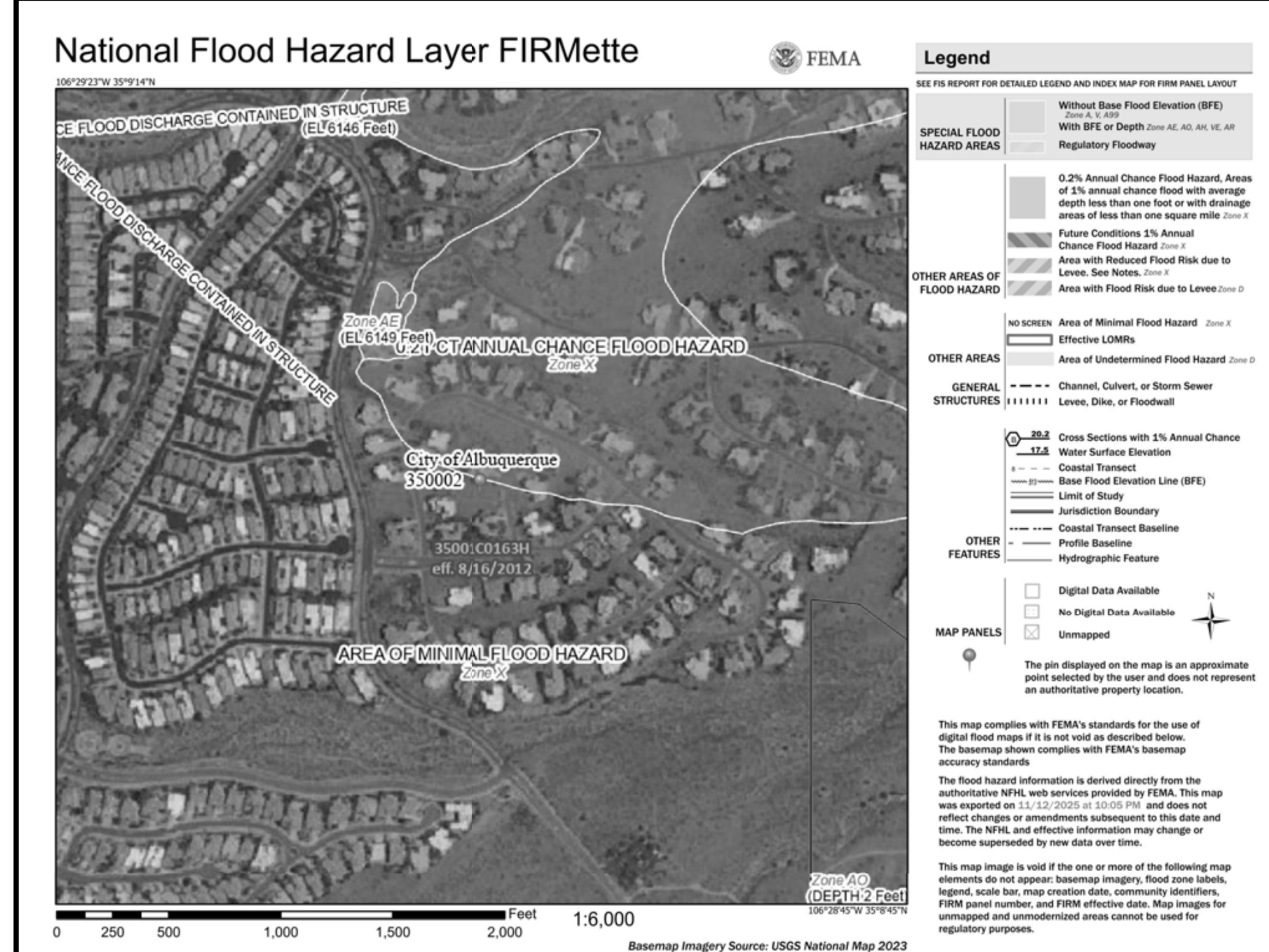
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.

4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.

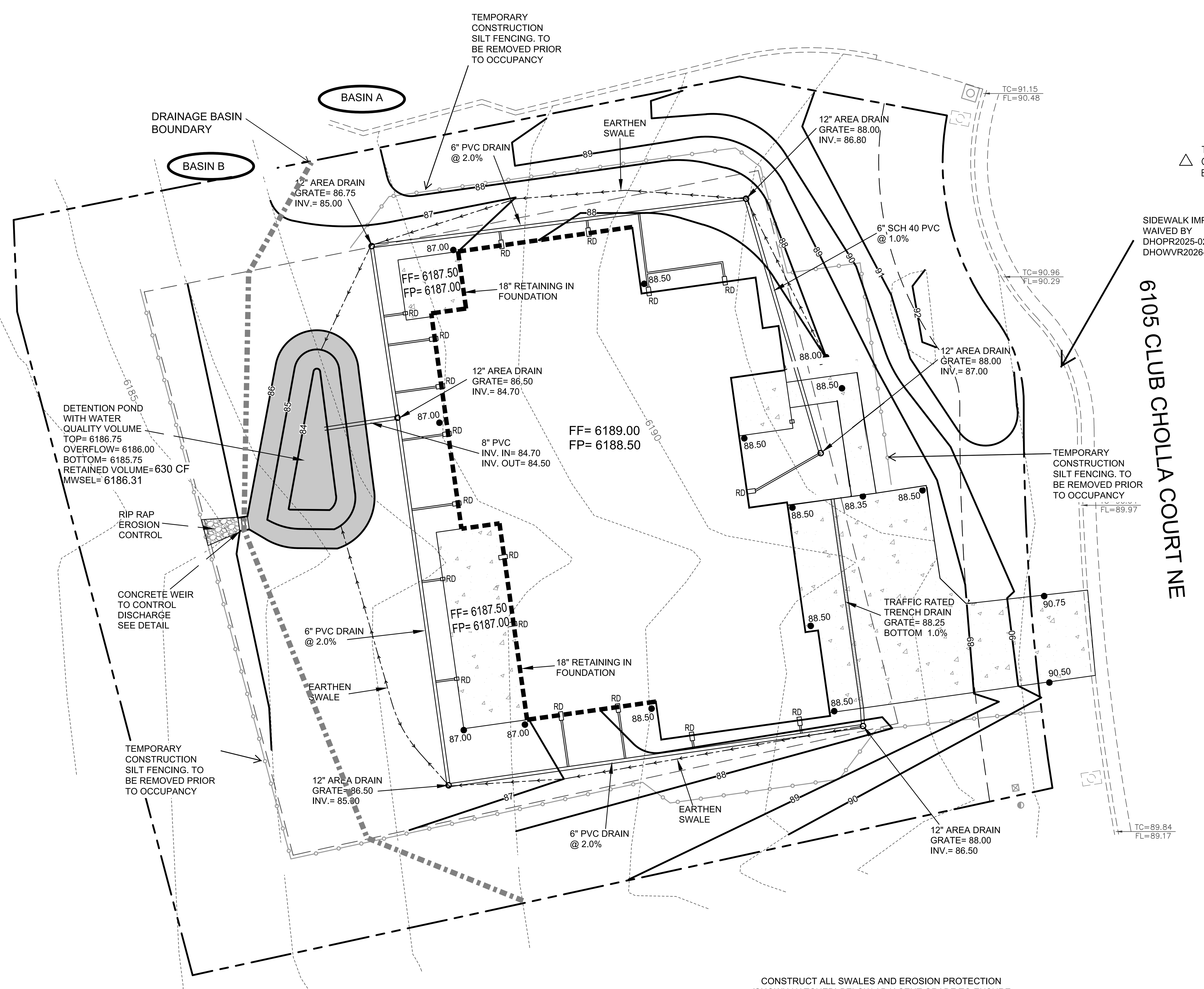
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT.



VICINITY MAP: E-23-Z



FIRM MAP:



LEGAL DESCRIPTION:

LOT 22 UNIT 1 THE OVERLOOK AT HIGH DESERT
CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO

NOTES:

1. ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.

2. ALL SLOPES SHALL BE 3:1 MAX. AND GRAVEL OR NATIVE SEEDING PRIOR TO CO.

3. ANY PERIMETER WALLS MUST BE PERMITED SEPARATELY ALL RETAINING WALL DESIGN SHALL BE BY OTHERS.

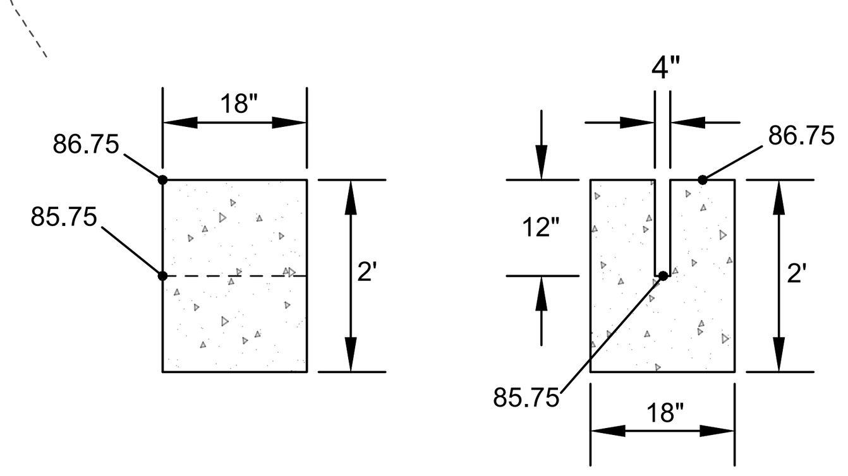
4. SURVEY INFORMATION PROVIDED BY COMMUNITY SCIENCES CORPORATION USING NAVD DATUM 1988.

5. LONG TERM MAINTAINANCE OF ALL PONDS, SWALES AND OVERFLOWS IS REQUIRED

6. A PAD ELEVATION CERTIFICATION SHALL BE REQUIRED PRIOR TO RELEASE OF BUILDING PERMIT.

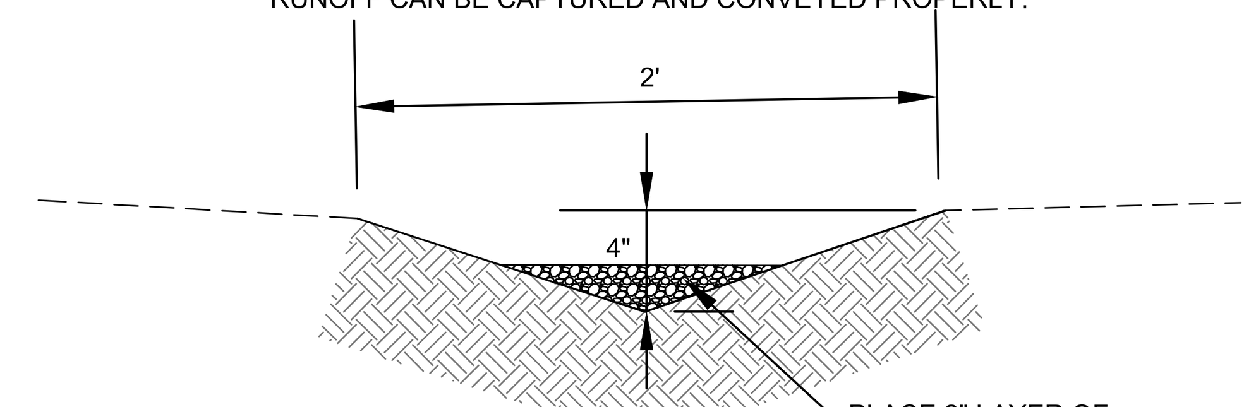
LEGEND

- EXISTING CONTOUR
- EXISTING INDEX CONTOUR
- PROPOSED CONTOUR
- PROPOSED INDEX CONTOUR
- EXISTING SPOT ELEVATION
- PROPOSED SPOT ELEVATION
- BOUNDARY
- ADJACENT BOUNDARY
- EXISTING CURB AND GUTTER
- PROPOSED EARTHEN SWALE
- PROPOSED RETAINING WALL
- PROPOSED ROOF DRAIN
- PROPOSED CONCRETE
- PROPOSED PONDING

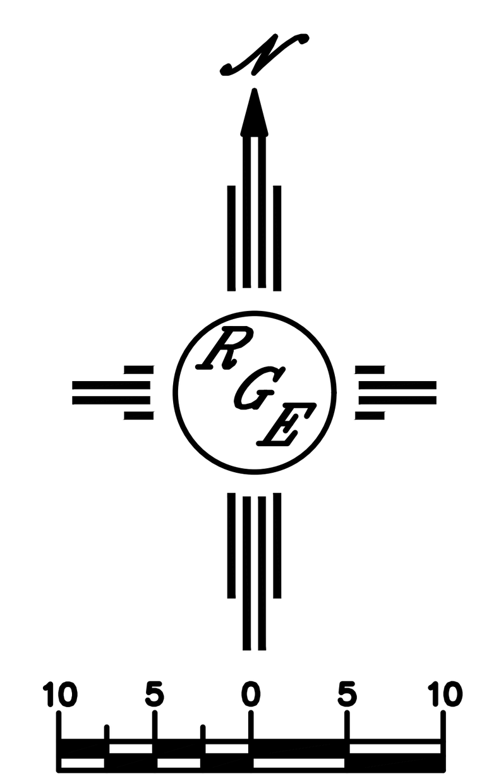


CONCRETE OUTLET CONTROL WEIR
NTS

CONSTRUCT ALL SWALES AND EROSION PROTECTION (SHOWN HATCHED) BELOW ADJACENT GRADE TO ENSURE RUNOFF CAN BE CAPTURED AND CONVEYED PROPERLY.



EARTHEN SWALE
NTS



SCALE: 1"=10'

CAUTION:

EXISTING UTILITIES ARE NOT SHOWN.
IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO ANY EXCAVATION TO DETERMINE THE ACTUAL LOCATION OF UTILITIES & OTHER IMPROVEMENTS.

ENGINEER'S SEAL	LOT 22 U 1 THE OVERLOOK AT HIGH DESERT 6105 CLUB CHOLLA COURT NE	DRAWN BY DEM
DAVID SOULE NEW MEXICO 14522 REGISTERED PROFESSIONAL ENGINEER 2/24/26 11/17/25	GRADING AND DRAINAGE PLAN	DATE 11/13/25
		6105 Club Cholla Court NE.dwg
DAVID SOULE P.E. #14522	Rio Grande Engineering PO BOX 93924 ALBUQUERQUE, NM 87199 (505) 321-9099	SHEET # C1
		JOB #

BASIN DATA

Basin	Area	Area	Treatment A		Treatment B		Treatment C		Treatment D		Q100	Q100
	(sf)	(acres)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	GENERATED	DISCHARGED
TOTAL SITE EXISTING	17117	0.393	100%	0.0740	0.0%	0.0847	0.0%	0.1783	0%	0.1163	0.75	0.75
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WATER QUALITY VOLUME 177.4 CF

VOLUME CALCULATIONS

south pond

	ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVI	VOLUME AC-FT	Q (CFS)
	84.00	0.00	87.00	0	0	0.000	0.00
OUTLET	85.75	0.00	634.00	630.88	630.875	0.014	0.00
	86.75	1.00	1592.00	1113.00	1743.875	0.040	0.97

Orifice Equation

Q=CLH^1.5

C = 2.95

Width 0.33

Area (ft^2)= 0.000593957

g = 32.2

H (Ft) = Depth of water above center of INV

Q (CFS)= Flow


```
AHYMO PROGRAM (AHYMO-S4)                    - Version: S4.01a - Rel: 01a
     RUN DATE (MON/DAY/YR) = 11/17/2025
     START TIME (HR:MIN:SEC) = 12:10:11      USER NO.= RioGrandeSingleA41963517
     INPUT FILE = cuments and Settings\Owner\Desktop\2025jobs\25242-club cholla\pondrout111725.txt

*S     AHYMO -  DETENTION-CHOLLA CACTUS
*S              POND ROUTING

START              TIME=0.0  PUNCH CODE=0

RAINFALL           TYPE=2
                   QUARTER=0.0    ONE= 1.91 IN
                   SIX=2.60 IN   DAY= 3.34 IN   DT = 0.05 HR

     24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE AREAS (NM & AZ) - D1
     DT =    0.050000 HOURS          END TIME =    24.000002 HOURS
      0.0000  0.0046  0.0093  0.0144  0.0198  0.0254  0.0318
      0.0409  0.0554  0.0709  0.0870  0.1044  0.1220  0.1401
      0.1588  0.1777  0.1985  0.2202  0.2435  0.2697  0.2973
      0.3343  0.3762  0.4280  0.4971  0.5749  0.7091  0.9178
      1.2754  1.5267  1.7250  1.8246  1.9119  1.9746  2.0245
      2.0680  2.0999  2.1291  2.1532  2.1759  2.1979  2.2177
      2.2365  2.2532  2.2691  2.2847  2.2997  2.3119  2.3187
      2.3254  2.3320  2.3381  2.3442  2.3502  2.3561  2.3618
      2.3673  2.3727  2.3780  2.3833  2.3885  2.3934  2.3983
      2.4031  2.4078  2.4125  2.4170  2.4214  2.4258  2.4301
      2.4344  2.4386  2.4428  2.4469  2.4510  2.4550  2.4590
      2.4629  2.4668  2.4706  2.4744  2.4782  2.4819  2.4856
      2.4892  2.4928  2.4964  2.4999  2.5034  2.5068  2.5103
      2.5136  2.5170  2.5203  2.5236  2.5269  2.5301  2.5333
      2.5365  2.5396  2.5428  2.5458  2.5489  2.5519  2.5550
      2.5580  2.5609  2.5638  2.5668  2.5696  2.5725  2.5754
      2.5782  2.5810  2.5838  2.5865  2.5893  2.5920  2.5947
      2.5973  2.6000  2.6027  2.6053  2.6080  2.6106  2.6133
      2.6159  2.6185  2.6212  2.6238  2.6264  2.6291  2.6317
      2.6343  2.6369  2.6395  2.6421  2.6447  2.6474  2.6499
      2.6525  2.6551  2.6577  2.6603  2.6629  2.6655  2.6681
      2.6706  2.6732  2.6758  2.6783  2.6809  2.6834  2.6860
      2.6885  2.6911  2.6936  2.6962  2.6987  2.7013  2.7038
      2.7063  2.7088  2.7114  2.7139  2.7164  2.7189  2.7214
      2.7239  2.7264  2.7289  2.7314  2.7339  2.7364  2.7389
      2.7413  2.7438  2.7463  2.7488  2.7512  2.7537  2.7561
      2.7586  2.7611  2.7635  2.7660  2.7684  2.7708  2.7733
      2.7757  2.7781  2.7806  2.7830  2.7854  2.7878  2.7902
      2.7927  2.7951  2.7975  2.7999  2.8023  2.8047  2.8070
      2.8094  2.8118  2.8142  2.8166  2.8189  2.8213  2.8237
      2.8260  2.8284  2.8308  2.8331  2.8355  2.8378  2.8401
      2.8425  2.8448  2.8472  2.8495  2.8518  2.8541  2.8565
      2.8588  2.8611  2.8634  2.8657  2.8680  2.8703  2.8726
      2.8749  2.8772  2.8795  2.8817  2.8840  2.8863  2.8886
      2.8908  2.8931  2.8954  2.8976  2.8999  2.9021  2.9044
      2.9066  2.9089  2.9111  2.9133  2.9156  2.9178  2.9200
      2.9223  2.9245  2.9267  2.9289  2.9311  2.9333  2.9355
      2.9377  2.9399  2.9421  2.9443  2.9465  2.9486  2.9508
      2.9530  2.9552  2.9573  2.9595  2.9617  2.9638  2.9660
      2.9681  2.9703  2.9724  2.9746  2.9767  2.9788  2.9810
      2.9831  2.9852  2.9873  2.9894  2.9916  2.9937  2.9958
      2.9979  3.0000  3.0021  3.0042  3.0063  3.0083  3.0104
      3.0125  3.0146  3.0167  3.0187  3.0208  3.0229  3.0249
      3.0270  3.0290  3.0311  3.0331  3.0352  3.0372  3.0392
      3.0413  3.0433  3.0453  3.0473  3.0494  3.0514  3.0534
      3.0554  3.0574  3.0594  3.0614  3.0634  3.0654  3.0674
      3.0693  3.0713  3.0733  3.0753  3.0773  3.0792  3.0812
      3.0831  3.0851  3.0871  3.0890  3.0910  3.0929  3.0948
      3.0968  3.0987  3.1006  3.1026  3.1045  3.1064  3.1083
      3.1102  3.1121  3.1140  3.1159  3.1178  3.1197  3.1216
      3.1235  3.1254  3.1273  3.1292  3.1310  3.1329  3.1348
      3.1367  3.1385  3.1404  3.1422  3.1441  3.1459  3.1478
      3.1496  3.1515  3.1533  3.1551  3.1569  3.1588  3.1606
      3.1624  3.1642  3.1660  3.1678  3.1696  3.1714  3.1732
      3.1750  3.1768  3.1786  3.1804  3.1822  3.1839  3.1857
      3.1875  3.1893  3.1910  3.1928  3.1945  3.1963  3.1980
      3.1998  3.2015  3.2033  3.2050  3.2067  3.2085  3.2102
      3.2119  3.2136  3.2153  3.2170  3.2188  3.2205  3.2222
      3.2239  3.2256  3.2272  3.2289  3.2306  3.2323  3.2340
      3.2357  3.2373  3.2390  3.2407  3.2423  3.2440  3.2456
      3.2473  3.2489  3.2506  3.2522  3.2538  3.2555  3.2571
      3.2587  3.2603  3.2620  3.2636  3.2652  3.2668  3.2684
      3.2700  3.2716  3.2732  3.2748  3.2764  3.2780  3.2796
      3.2811  3.2827  3.2843  3.2858  3.2874  3.2890  3.2905
      3.2921  3.2936  3.2952  3.2967  3.2983  3.2998  3.3013
      3.3029  3.3044  3.3059  3.3074  3.3089  3.3105  3.3120
      3.3135  3.3150  3.3165  3.3180  3.3195  3.3210  3.3224
      3.3239  3.3254  3.3269  3.3283  3.3298  3.3313  3.3327
      3.3342  3.3357  3.3371  3.3386  3.3400

*Basin DEVELOPED EXISTING
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```
COMPUTE NM HYD      ID=1  HYD NO=101  DA= .000613988 SQ MI
                    PER A=100  PER B=0 PER C=0  PER D=0
                    TP=-.138 MASSRAIN=-1

     K =  0.168411HR    TP =  0.138000HR    K/TP RATIO =  1.220371     SHAPE CONSTANT, N =  2.915919
     UNIT PEAK =    1.2231    CFS   UNIT VOLUME =   0.9874        B =   274.90      P60 = 1.9100
     AREA =     0.000614 SQ MI    IA =   0.65000 INCHES    INF =    1.67000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.050000

PRINT HYD           ID=1  CODE=3
```

PARTIAL HYDROGRAPH 101.00

TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS
0.000	0.0	0.750	0.0	1.500	0.7	2.250	0.0	3.000	0.0
0.150	0.0	0.900	0.0	1.650	0.5	2.400	0.0	3.150	0.0
0.300	0.0	1.050	0.0	1.800	0.2	2.550	0.0	3.300	0.0
0.450	0.0	1.200	0.0	1.950	0.1	2.700	0.0		
0.600	0.0	1.350	0.1	2.100	0.1	2.850	0.0		

```
     RUNOFF VOLUME =     0.72413 INCHES     =       0.0237 ACRE-FEET
     PEAK DISCHARGE RATE =        0.75 CFS  AT   1.550 HOURS   BASIN AREA =  0.0006 SQ. MI.

* ONSITE Basin A PROPOSED
COMPUTE NM HYD      ID=2  HYD NO=102  DA= .000478256 SQ MI
                    PER A=10  PER B=22 PER C=30  PER D=38
                    TP=-.138  MASSRAIN=-1

     K =  0.075210HR    TP =  0.138000HR    K/TP RATIO =  0.545000     SHAPE CONSTANT, N =  7.106428
     UNIT PEAK =  0.69307    CFS   UNIT VOLUME =   0.9842        B =   526.28      P60 = 1.9100
     AREA =     0.000182 SQ MI    IA =   0.10000 INCHES    INF =    0.04000 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.050000

     K =  0.128500HR    TP =  0.138000HR    K/TP RATIO =  0.931162     SHAPE CONSTANT, N =  3.798854
     UNIT PEAK =  0.73385    CFS   UNIT VOLUME =   0.9837        B =   341.53      P60 = 1.9100
     AREA =     0.000297 SQ MI    IA =   0.45161 INCHES    INF =    1.11452 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.050000

PRINT HYD           ID=2  CODE=3
```

PARTIAL HYDROGRAPH 102.00

TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS
0.000	0.0	4.950	0.0	9.900	0.0	14.850	0.0	19.800	0.0
0.150	0.0	5.100	0.0	10.050	0.0	15.000	0.0	19.950	0.0
0.300	0.0	5.250	0.0	10.200	0.0	15.150	0.0	20.100	0.0
0.450	0.0	5.400	0.0	10.350	0.0	15.300	0.0	20.250	0.0
0.600	0.0	5.550	0.0	10.500	0.0	15.450	0.0	20.400	0.0
0.750	0.0	5.700	0.0	10.650	0.0	15.600	0.0	20.550	0.0
0.900	0.0	5.850	0.0	10.800	0.0	15.750	0.0	20.700	0.0
1.050	0.1	6.000	0.0	10.950	0.0	15.900	0.0	20.850	0.0
1.200	0.1	6.150	0.0	11.100	0.0	16.050	0.0	21.000	0.0
1.350	0.3	6.300	0.0	11.250	0.0	16.200	0.0	21.150	0.0
1.500	1.1	6.450	0.0	11.400	0.0	16.350	0.0	21.300	0.0
1.650	0.7	6.600	0.0	11.550	0.0	16.500	0.0	21.450	0.0
1.800	0.3	6.750	0.0	11.700	0.0	16.650	0.0	21.600	0.0
1.950	0.2	6.900	0.0	11.850	0.0	16.800	0.0	21.750	0.0
2.100	0.1	7.050	0.0	12.000	0.0	16.950	0.0	21.900	0.0
2.250	0.1	7.200	0.0	12.150	0.0	17.100	0.0	22.050	0.0

2.400	0.0	7.350	0.0	12.300	0.0	17.250	0.0	22.200	0.0
2.550	0.0	7.500	0.0	12.450	0.0	17.400	0.0	22.350	0.0
2.700	0.0	7.650	0.0	12.600	0.0	17.550	0.0	22.500	0.0
2.850	0.0	7.800	0.0	12.750	0.0	17.700	0.0	22.650	0.0
3.000	0.0	7.950	0.0	12.900	0.0	17.850	0.0	22.800	0.0
3.150	0.0	8.100	0.0	13.050	0.0	18.000	0.0	22.950	0.0
3.300	0.0	8.250	0.0	13.200	0.0	18.150	0.0	23.100	0.0
3.450	0.0	8.400	0.0	13.350	0.0	18.300	0.0	23.250	0.0
3.600	0.0	8.550	0.0	13.500	0.0	18.450	0.0	23.400	0.0
3.750	0.0	8.700	0.0	13.650	0.0	18.600	0.0	23.550	0.0
3.900	0.0	8.850	0.0	13.800	0.0	18.750	0.0	23.700	0.0
4.050	0.0	9.000	0.0	13.950	0.0	18.900	0.0	23.850	0.0
4.200	0.0	9.150	0.0	14.100	0.0	19.050	0.0	24.000	0.0
4.350	0.0	9.300	0.0	14.250	0.0	19.200	0.0	24.150	0.0
4.500	0.0	9.450	0.0	14.400	0.0	19.350	0.0		
4.650	0.0	9.600	0.0	14.550	0.0	19.500	0.0		
4.800	0.0	9.750	0.0	14.700	0.0	19.650	0.0		

```
     RUNOFF VOLUME =     1.78253 INCHES     =       0.0455 ACRE-FEET
     PEAK DISCHARGE RATE =        1.05 CFS  AT   1.500 HOURS   BASIN AREA =  0.0005 SQ. MI.

*ONSITE BASIN B PROPOSED-REMAINING SITE
COMPUTE NM HYD      ID=3  HYD NO=103  DA= .000135732 SQ MI
                    PER A=50 PER B=20 PER C=30  PER D=0
                    TP=-.138 MASSRAIN=-1

     K =  0.144325HR    TP =  0.138000HR    K/TP RATIO =  1.045832     SHAPE CONSTANT, N =  3.375539
     UNIT PEAK =  0.30602    CFS   UNIT VOLUME =   0.9560        B =   311.14      P60 = 1.9100
     AREA =     0.000136 SQ MI    IA =   0.53000 INCHES    INF =    1.33400 INCHES PER HOUR
     RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  0.050000

PRINT HYD           ID=3  CODE=3
```

PARTIAL HYDROGRAPH 103.00

TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS
0.000	0.0	0.600	0.0	1.200	0.0	1.800	0.1	2.400	0.0
0.150	0.0	0.750	0.0	1.350	0.0	1.950	0.0		
0.300	0.0	0.900	0.0	1.500	0.2	2.100	0.0		
0.450	0.0	1.050	0.0	1.650	0.1	2.250	0.0		

```
     RUNOFF VOLUME =     0.87651 INCHES     =       0.0063 ACRE-FEET
     PEAK DISCHARGE RATE =        0.22 CFS  AT   1.550 HOURS   BASIN AREA =  0.0001 SQ. MI.

*  ROUTED BASIN A THRU POND-PROPOSED

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
ROUTE RESERVOIR     ID=4   HYD NO=104   INFLOW=2   CODE=3
                    OUTFLOW(CFS)    STORAGE(AC-FT)    ELEV(FT)
                       0.00             0.014          85.75
                       0.97             0.040          86.75

  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *
```

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	85.75	0.014	0.00
0.15	0.00	85.75	0.014	0.00
0.30	0.00	85.75	0.014	0.00
0.45	0.00	85.75	0.014	0.00
0.60	0.00	85.75	0.014	0.00
0.75	0.03	85.76	0.014	0.01

0.90	0.04	85.77	0.014	0.02
1.05	0.05	85.78	0.015	0.03
1.20	0.09	85.79	0.015	0.04
1.35	0.30	85.84	0.016	0.09
1.50	1.05	86.08	0.023	0.32
1.65	0.67	86.31	0.028	0.54
1.80	0.29	86.27	0.028	0.50
1.95	0.15	86.16	0.025	0.39
2.10	0.10	86.05	0.022	0.29
2.25	0.07	85.97	0.020	0.22
2.40	0.05	85.91	0.018	0.16
2.55	0.03	85.87	0.017	0.11
2.70	0.01	85.83	0.016	0.08
2.85	0.01	85.81	0.015	0.05
3.00	0.01	85.79	0.015	0.04
3.15	0.01	85.78	0.015	0.03
3.30	0.01	85.77	0.015	0.02
3.45	0.01	85.76	0.014	0.01
3.60	0.01	85.76	0.014	0.01
3.75	0.01	85.76	0.014	0.01
3.90	0.01	85.76	0.014	0.01
4.05	0.01	85.76	0.014	0.01
4.20	0.01	85.76	0.014	0.01
4.35	0.01	85.76	0.014	0.01
4.50	0.01	85.76	0.014	0.01
4.65	0.01	85.76	0.014	0.01
4.80	0.01	85.76	0.014	0.01
4.95	0.01	85.76	0.014	0.01
5.10	0.01	85.76	0.014	0.01
5.25	0.01	85.76	0.014	0.01
5.40	0.01	85.76	0.014	0.01
5.55	0.01	85.76	0.014	0.01
5.70	0.01	85.76	0.014	0.01
5.85	0.01	85.76	0.014	0.01
6.00	0.01	85.76	0.014	0.01
6.15	0.01	85.76	0.014	0.01
6.30	0.01	85.76	0.014	0.01
6.45	0.01	85.76	0.014	0.01
6.60	0.01	85.76	0.014	0.01
6.75	0.01	85.76	0.014	0.01
6.90	0.01	85.76	0.014	0.01
7.05	0.01	85.76	0.014	0.01
7.20	0.01	85.76	0.014	0.01
7.35	0.01	85.76	0.014	0.01
7.50	0.01	85.76	0.014	0.01
7.65	0.01	85.76	0.014	0.01
7.80	0.01	85.76	0.014	0.01
7.95	0.01	85.76	0.014	0.01
8.10	0.01	85.76	0.014	0.01
8.25	0.01	85.76	0.014	0.01

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
8.40	0.01	85.76	0.014	0.01
8.55	0.01	85.76	0.014	0.01
8.70	0.01	85.76	0.014	0.01
8.85	0.01	85.76	0.014	0.01
9.00	0.01	85.76	0.014	0.01
9.15	0.01	85.76	0.014	0.01
9.30	0.01	85.76	0.014	0.01
9.45	0.01	85.76	0.014	0.01
9.60	0.01	85.76	0.014	0.01
9.75	0.01	85.76	0.014	0.01
9.90	0.01	85.76	0.014	0.01
10.05	0.01	85.76	0.014	0.01
10.20	0.01	85.76	0.014	0.01
10.35	0.01	85.76	0.014	0.01
10.50	0.01	85.76	0.014	0.01
10.65	0.01	85.76	0.014	0.01
10.80	0.01	85.76	0.014	0.01
10.95	0.01	85.76	0.014	0.01
11.10	0.01	85.76	0.014	0.01
11.25	0.01	85.76	0.014	0.01
11.40	0.01	85.76	0.014	0.01
11.55	0.01	85.76	0.014	0.01
11.70	0.01	85.76	0.014	0.01
11.85	0.01	85.76	0.014	0.01
12.00	0.01	85.76	0.014	0.01
12.15	0.01	85.76	0.014	0.01
12.30	0.01	85.76	0.014	0.01
12.45	0.01	85.76	0.014	0.01
12.60	0.01	85.76	0.014	0.01
12.75	0.01	85.76	0.014	0.01
12.90	0.01	85.76	0.014	0.01
13.05	0.01	85.76	0.014	0.01
13.20	0.01	85.76	0.014	0.01
13.35	0.01	85.76	0.014	0.01
13.50	0.01	85.76	0.014	0.01
13.65	0.01	85.76	0.014	0.01
13.80	0.01	85.76	0.014	0.01
13.95	0.01	85.76	0.014	0.01
14.10	0.00	85.76	0.014	0.01

14.25	0.00	85.76	0.014	0.01
14.40	0.00	85.76	0.014	0.01
14.55	0.00	85.76	0.014	0.00

```
PEAK DISCHARGE =          0.548 CFS - PEAK OCCURS AT HOUR    1.70
MAXIMUM WATER SURFACE ELEVATION =        86.315
MAXIMUM STORAGE =         0.0287 AC-FT       INCREMENTAL TIME=     0.050000HRS

*ADD POND OUTLET TO REMAINING SITE TO CALCULATE PROPOSED FLOW LEAVING SITE

ADD HYD             ID=5  HYD NO=105  ID I=4 ID II=3
PRINT HYD           ID=5  CODE=3
```

PARTIAL HYDROGRAPH 105.00

TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS	TIME HRS	FLOW CFS
0.000	0.0	4.950	0.0	9.900	0.0	14.850	0.0	19.800	0.0
0.150	0.0	5.100	0.0	10.050	0.0	15.000	0.0	19.950	0.0
0.300	0.0	5.250	0.0	10.200	0.0	15.150	0.0	20.100	0.0
0.450	0.0	5.400	0.0	10.350	0.0	15.300	0.0	20.250	0.0
0.600	0.0	5.550	0.0	10.500	0.0	15.450	0.0	20.400	0.0
0.750	0.0	5.700	0.0	10.650	0.0	15.600	0.0	20.550	0.0
0.900	0.0	5.850	0.0	10.800	0.0	15.750	0.0	20.700	0.0
1.050	0.0	6.000	0.0	10.950	0.0	15.900	0.0	20.850	0.0
1.200	0.0	6.150	0.0	11.100	0.0	16.050	0.0	21.000	0.0
1.350	0.1	6.300	0.0	11.250	0.0	16.200	0.0	21.150	0.0
1.500	0.5	6.450	0.0	11.400	0.0	16.350	0.0	21.300	0.0
1.650	0.7	6.600	0.0	11.550	0.0	16.500	0.0	21.450	0.0
1.800	0.6	6.750	0.0	11.700	0.0	16.650	0.0	21.600	0.0
1.950	0.4	6.900	0.0	11.850	0.0	16.800	0.0	21.750	0.0
2.100	0.3	7.050	0.0	12.000	0.0	16.950	0.0	21.900	0.0
2.250	0.2	7.200	0.0	12.150	0.0	17.100	0.0	22.050	0.0
2.400	0.2	7.350	0.0	12.300	0.0	17.250	0.0	22.200	0.0
2.550	0.1	7.500	0.0	12.450	0.0	17.400	0.0	22.350	0.0
2.700	0.1	7.650	0.0	12.600	0.0	17.550	0.0	22.500	0.0
2.850	0.1	7.800	0.0	12.750	0.0	17.700	0.0	22.650	0.0
3.000	0.0	7.950	0.0	12.900	0.0	17.850	0.0	22.800	0.0
3.150	0.0	8.100	0.0	13.050	0.0	18.000	0.0	22.950	0.0
3.300	0.0	8.250	0.0	13.200	0.0	18.150	0.0	23.100	0.0
3.450	0.0	8.400	0.0	13.350	0.0	18.300	0.0	23.250	0.0
3.600	0.0	8.550	0.0	13.500	0.0	18.450	0.0	23.400	0.0
3.750	0.0	8.700	0.0	13.650	0.0	18.600	0.0	23.550	0.0
3.900	0.0	8.850	0.0	13.800	0.0	18.750	0.0	23.700	0.0
4.050	0.0	9.000	0.0	13.950	0.0	18.900	0.0	23.850	0.0
4.200	0.0	9.150	0.0	14.100	0.0	19.050	0.0	24.000	0.0
4.350	0.0	9.300	0.0	14.250	0.0	19.200	0.0	24.150	0.0
4.500	0.0	9.450	0.0	14.400	0.0	19.350	0.0	24.300	0.0
4.650	0.0	9.600	0.0	14.550	0.0	19.500	0.0	24.450	0.0
4.800	0.0	9.750	0.0	14.700	0.0	19.650	0.0		

```
     RUNOFF VOLUME =     1.58091 INCHES     =       0.0518 ACRE-FEET
     PEAK DISCHARGE RATE =        0.69 CFS  AT   1.600 HOURS   BASIN AREA =  0.0006 SQ. MI.

FINISH
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NORMAL PROGRAM FINISH         END TIME (HR:MIN:SEC) = 12:10:11
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ENGINEER'S SEAL
DAVID SOULE NEW MEXICO 14522 REGISTERED PROFESSIONAL ENGINEER
11/17/25
DAVID SOULE
P.E. #14522

LOT 22 U 1 THE OVERLOOK AT HIGH DESERT
6105 CLUB CHOLLA COURT NE

GRADING AND DRAINAGE PLAN -CALCULATIONS

Rio Grande Engineering
PO BOX 93924
ALBUQUERQUE, NM 87199
(505) 321-9099

DRAWN BY DEM

DATE 11/13/25

6105 Club Cholla Court NE.dwg

SHEET # C2

JOB # ______