CITY OF ALBUQUERQUE

Planning Department
Alan Varela, Director

Mayor Timothy M. Keller

August 25, 2022

David Soule, P.E.
Rio Grande Engineering
P.O. Box 93924
Albuquerque, NM 87199

RE: 13420 Osage Orange Rd NE - Addition
Grading and Drainage Plan
Engineer's Stamp Date: 08/10/22
Hydrology File: E23D031

Dear Mr. Soule:

Based upon the information provided in your submittal received 08/12/2022, the Grading and Drainage Plan is approved for Building Permit. Since this is an addition, a pad certification is not needed for this project. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter.

PRIOR TO CERTIFICATE OF OCCUPANCY:

1. Engineer's Certification, per the DPM Part 6-14 (F): Engineer's Certification Checklist For Non-Subdivision is required.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov .

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: 13420 OSAGE ORANGE Building Permit #: _____ Hydrology File #: _____

DRB#: _____ EPC#: _____ Work Order#: _____

Legal Description: LOT 53 UNIT 2 OVERLOOK

City Address: 13420 OSAGE ORANGE

Applicant: _____ Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

Other Contact: RIO GRANDE ENGINEERING Contact: DAVID SOULE

Address: PO BOX 93924 ALB NM 87199

Phone#: 505.321.9099 Fax#: 505.872.0999 E-mail: david@riograndeengineering.com

TYPE OF DEVELOPMENT: ___ PLAT X RESIDENCE ___ DRB SITE ___ ADMIN SITE

Check all that Apply:

DEPARTMENT:

X HYDROLOGY/ DRAINAGE

___ TRAFFIC/ TRANSPORTATION

TYPE OF SUBMITTAL:

___ ENGINEER/ARCHITECT CERTIFICATION

___ PAD CERTIFICATION

___ CONCEPTUAL G & D PLAN

X GRADING PLAN

___ DRAINAGE REPORT

___ DRAINAGE MASTER PLAN

___ FLOODPLAIN DEVELOPMENT PERMIT APPLIC

___ ELEVATION CERTIFICATE

___ CLOMR/LOMR

___ TRAFFIC CIRCULATION LAYOUT (TCL)

___ TRAFFIC IMPACT STUDY (TIS)

___ STREET LIGHT LAYOUT

___ OTHER (SPECIFY) _____

___ PRE-DESIGN MEETING?

IS THIS A RESUBMITTAL?: X Yes ___ No

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

X BUILDING PERMIT APPROVAL

___ CERTIFICATE OF OCCUPANCY

___ PRELIMINARY PLAT APPROVAL

___ SITE PLAN FOR SUB'D APPROVAL

___ SITE PLAN FOR BLDG. PERMIT APPROVAL

___ FINAL PLAT APPROVAL

___ SIA/ RELEASE OF FINANCIAL GUARANTEE

___ FOUNDATION PERMIT APPROVAL

___ GRADING PERMIT APPROVAL

___ SO-19 APPROVAL

___ PAVING PERMIT APPROVAL

___ GRADING/ PAD CERTIFICATION

___ WORK ORDER APPROVAL

___ CLOMR/LOMR

___ FLOODPLAIN DEVELOPMENT PERMIT

___ OTHER (SPECIFY) _____

DATE SUBMITTED: _____ By: _____

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

SITE HYDROLOGY

Weighted E Method

per city of Albuquerque Development Process Manual- chapter 22

Existing Developed Basins

Basin	Area (sf)	Area (acres)	Treatment A %	Treatment A (acres)	Treatment B %	Treatment B (acres)	Treatment C %	Treatment C (acres)	Treatment D %	Treatment D (acres)	100-Year, 6-hr. Weighted E (ac-ft)	100-Year, 6-hr. Volume (ac-ft)	100-Year, 6-hr. Flow cfs	10-day Volume (ac-ft)
BASIN A(proposed)	4303.00	0.099	28%	0.02765932	26%	0.026	20%	0.01976	26%	0.026	1.568	0.013	0.32	0.016
BASIN B(proposed)	5942.00	0.136	10%	0.01364096	20.0%	0.027	17.0%	0.02319	53%	0.072	2.240	0.025	0.53	0.035
BASIN C(proposed)	6878.00	0.158	66%	0.10421212	26.0%	0.041	0.0%	0	8%	0.013	1.016	0.013	0.39	0.015
NATIVE	17123.00	0.393	75%	0.29481749	25.0%	0.098	0.0%	0	0%	0.000	0.808	0.026	0.88	0.026
COMPARISON													0.35	INCREASE

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

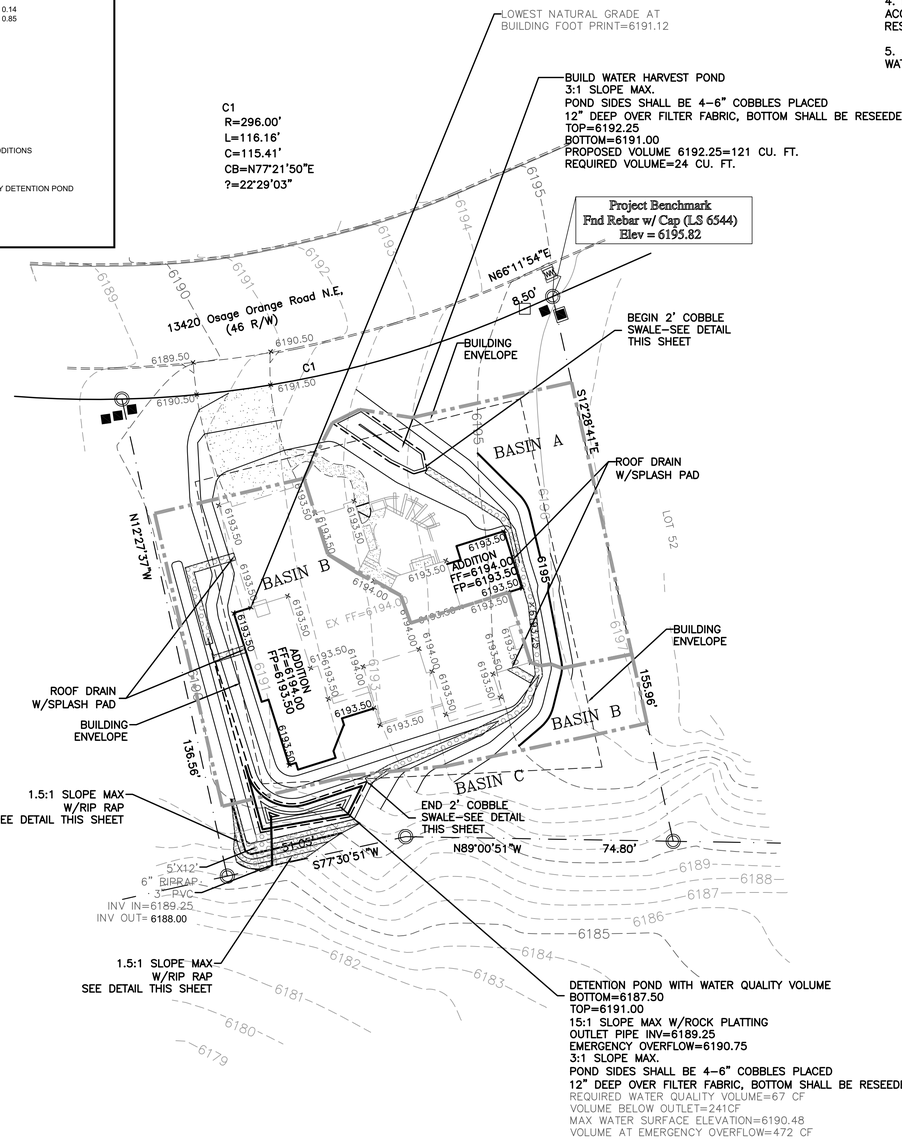
Where for 100-year, 6-hour storm

Ea= 0.76 Qa= 2.09
Eb= 0.95 Qb= 2.73
Ec= 1.2 Qc= 3.41
Ed= 3.34 Qd= 4.78

* routed basin b 0.14
0.85

DRAINAGE NARRATIVE

THIS SITE IS GOVERNED BY THE HIGH DESERT MASTER DRAINAGE PLAN. THE DOWN STREAM IMPROVEMENTS ARE IN PLACE. THE IMPROVEMENT INCLUDE TWO ADDITIONS DUE TO THIS SITE BEING GOVERNED BY THE GUIDELINES FOR SUSTAINABILITY, THE SITE MUST MATCH THE HISTORICAL100-YEAR, 6-HOUR PEAK DISCHARGE RATE
THE SITE HISTORICALY DISCHARGES .88 CFS. THE DEVELOPED SITE WILL CONTAIN THREE DRAINAGE BASINS,
BASIN A CONTAINS THE FRONT OF HOUSE AND LOT. THIS BASIN DISCHARGES .32 CFS THAT PASSES THROUGH A WATER QUALITY POND.
BASIN B CONTAINS THE MAJORITY OF THE HOUSE AND A PORTION AROUND THE HOUSE. THIS BASIN GENERATES .53 CFS THAT IS CAPTURED BY A WATER QUALITY DETENTION POND THE PEAK DISCHARGE IS REDUCED TO .14 CFS LEAVING THE POND AND A WATER QUALITY VOLUME OF 16 CF IS CAPTURED
BASIN C IS THE REMAINING PORTION OF THE LOT AND WILL DISCHARGE .42 CFS AS SHEET FLOW.
THE COMBINED PROPOSED DISCHARGE WILL BE .85 CFS WHICH IS LESS THAN NATIVE CONDITIONS. THE PONDS CAPTURE 137 CF FOR WATER QUALITY



EROSION CONTROL NOTES:

1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.

2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN–OFF ON SITE DURING CONSTRUCTION.

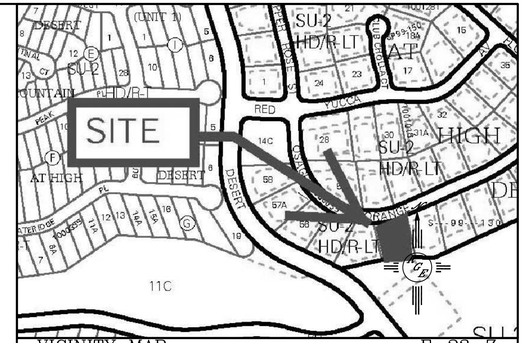
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT–OF–WAY.

4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.

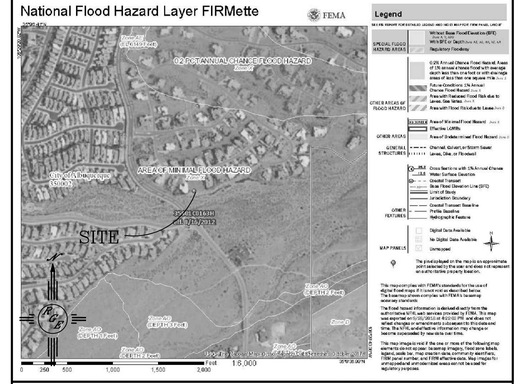
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT.

CAUTION:

EXISTING UTILITIES ARE NOT SHOWN. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO ANY EXCAVATION TO DETERMINE THE ACTUAL LOCATION OF UTILITIES & OTHER IMPROVEMENTS.



VICINITY MAP: E–23–Z



FIRM MAP: FM35001C0163H

LEGAL DESCRIPTION:

Lot 53, Overlook at High Desert, Unit 2

NOTES:

1. ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.

2. ALL ROOF DRAINS SHALL HAVE A SLASH PAD AND 2' COBBLE SWALE THAT SHALL TIE TO MAIN COBBLES SWALES AROUND THE HOUSE PER DETAIL THIS SHEET..

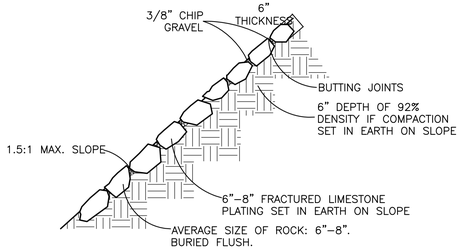
3. ALL DISTURBED AREAS SHALL BE RESEEDED WITH APPROVED HIGH DESERT MIX WITHIN 30 DAYS AFTER THE END OF DISTURBANCE.

4. ALL SLOPES SHALL BE 3:1 MAX. AND GRAVEL OR NATIVE SEEDING PRIOR TO CO.

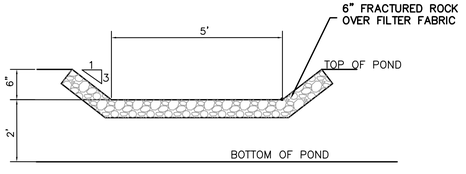
5. SURVEY PROVIDED BY COMMUNITY SCIENCES CORPORATION, BASED UPON NAVD 1988 DATUM.

LEGEND

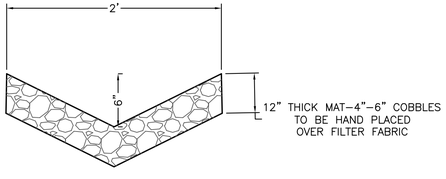
- EXISTING CONTOUR
- EXISTING INDEX CONTOUR
- PROPOSED CONTOUR
- PROPOSED INDEX CONTOUR
- SLOPE TIE
- EXISTING SPOT ELEVATION
- PROPOSED SPOT ELEVATION
- BOUNDARY
- CENTERLINE
- RIGHT–OF–WAY
- PROPOSED CURB AND GUTTER
- EXISTING EDGE OF ASPHALT
- PROPOSED 1'–2 LANDSCAPE WALL
- 2' COBBLE SWALE–SEE DETAIL THIS SHEET
- BASIN LIMITS



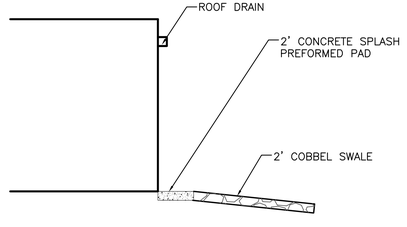
ROCK PLATING DETAIL
NTS



EMERGENCY OVERFLOW DETAIL
NTS



COBBLE SWALE DETAIL
NTS



ROOF DRAIN SPLASH PAD DETAIL
NTS

SCALE: 1"=20'

City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
APPROVED
DATE: 08/25/22
BY: Randy C. Brissette
HydroTrans # E23D031
THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY FROM REQUIRING THE CORRECTION OF ERROR OR OMISSIONS IN PLANS, SPECIFICATIONS, OR CONSTRUCTIONS. SUCH APPROVED PLANS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.

ENGINEER'S SEAL DAVID SOULE NEW MEXICO 14522 REGISTERED PROFESSIONAL ENGINEER 8/10/22 DAVID SOULE P.E. #14522	13420 OSAGE ORANGE GRADING AND DRAINAGE PLAN Rio Grande Engineering 1606 CENTRAL AVENUE SE SUITE 201 ALBUQUERQUE, NM 87106 (505) 872–0999	DRAWN BY WCWJ DATE 8–09–22 218145–LAYOUT–10–02–19 SHEET # 1 OF 2 JOB # 21740

STAGE/STORAGE VOLUME CALCULATIONS

pond C

OUTFALL

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVI	VOLUME AC-FT	Q (CFS)
87.50	0.00	8.00	0	0	0.000	0.00
88.00	0.00	59.00	16.75	16.75	0.0004	0.00
89.00	0.75	246.00	127.00	143.75	0.003	0.09
90.00	1.75	596.00	327.50	471.25	0.011	0.14
90.75	2.50	89.00	125.63	596.88	0.014	0.17

Orifice Equation

Q = CA SQRT(2gH)

C =	0.6
Diameter (in)	2
Area (ft^2)=	0.021816616
g =	32.2
H (Ft) =	Depth of water above center of orifice
Q (CFS)=	Flow


```
      AHYMO_PROGRAM (AHYMO-S4)                    - Version: S4.01a - Rel: 01a
           RUN DATE (MON/DAY/YR) = 08/09/2022
           START TIME (HR:MIN:SEC) = 13:12:35     USER NO.=
RioGrandeSingle4a41963517
           INPUT FILE = nts and Settings\Owner\Desktop\2022 JOBS\2022176-osage
orange\pondrout080922.txt

    *S     AHYMO -  DETENTION-ORANGE OSAGE BASIN B POND
    *S              POND ROUTING

    START              TIME=0.0  PUNCH CODE=0

    RAINFALL           TYPE=2
                       QUARTER=0.0    ONE= 1.96 IN
                       SIX=2.64 IN    DAY= 3.60 IN   DT = 0.05 HR



                  24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE
AREAS (NM & AZ) - D1
                  DT =   0.050000 HOURS       END TIME =    24.000002 HOURS
                     0.0000  0.0045  0.0092  0.0142  0.0195  0.0251  0.0314
                     0.0403  0.0546  0.0699  0.0857  0.1028  0.1202  0.1380
                     0.1564  0.1750  0.1956  0.2170  0.2400  0.2666  0.2949
                     0.3329  0.3759  0.4291  0.5000  0.5798  0.7175  0.9317
                     1.2986  1.5566  1.7600  1.8622  1.9518  2.0162  2.0674
                     2.1120  2.1447  2.1747  2.1994  2.2222  2.2439  2.2635
                     2.2820  2.2984  2.3141  2.3293  2.3441  2.3561  2.3629
                     2.3695  2.3760  2.3820  2.3880  2.3939  2.3997  2.4054
                     2.4108  2.4162  2.4214  2.4266  2.4317  2.4365  2.4413
                     2.4461  2.4507  2.4554  2.4598  2.4642  2.4685  2.4727
                     2.4770  2.4811  2.4852  2.4893  2.4933  2.4973  2.5012
                     2.5050  2.5089  2.5126  2.5164  2.5201  2.5238  2.5274
                     2.5309  2.5345  2.5380  2.5415  2.5449  2.5483  2.5517
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                     2.5986  2.6015  2.6044  2.6073  2.6101  2.6130  2.6158
                     2.6185  2.6213  2.6240  2.6267  2.6294  2.6321  2.6348
                     2.6374  2.6400  2.6427  2.6453  2.6480  2.6507  2.6533
                     2.6560  2.6587  2.6613  2.6640  2.6667  2.6693  2.6720
                     2.6747  2.6773  2.6800  2.6827  2.6853  2.6880  2.6907
                     2.6933  2.6960  2.6987  2.7013  2.7040  2.7067  2.7093
                     2.7120  2.7147  2.7173  2.7200  2.7227  2.7253  2.7280
                     2.7307  2.7333  2.7360  2.7387  2.7413  2.7440  2.7467
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                     2.8053  2.8080  2.8107  2.8133  2.8160  2.8187  2.8213
                     2.8240  2.8267  2.8293  2.8320  2.8347  2.8373  2.8400
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                     2.9173  2.9200  2.9227  2.9253  2.9280  2.9307  2.9333
                     2.9360  2.9387  2.9413  2.9440  2.9467  2.9493  2.9520
                     2.9547  2.9573  2.9600  2.9627  2.9653  2.9680  2.9707
                     2.9733  2.9760  2.9787  2.9813  2.9840  2.9867  2.9893
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                     3.1973  3.2000  3.2027  3.2053  3.2080  3.2107  3.2133
                     3.2160  3.2187  3.2213  3.2240  3.2267  3.2293  3.2320
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                     3.3653  3.3680  3.3707  3.3733  3.3760  3.3787  3.3813
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                     3.4027  3.4053  3.4080  3.4107  3.4133  3.4160  3.4187
                     3.4213  3.4240  3.4267  3.4293  3.4320  3.4347  3.4373
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                     3.4587  3.4613  3.4640  3.4667  3.4693  3.4720  3.4747
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                     3.4960  3.4987  3.5013  3.5040  3.5067  3.5093  3.5120
                     3.5147  3.5173  3.5200  3.5227  3.5253  3.5280  3.5307
                     3.5333  3.5360  3.5387  3.5413  3.5440  3.5467  3.5493
                     3.5520  3.5547  3.5573  3.5600  3.5627  3.5653  3.5680
                     3.5706  3.5733  3.5760  3.5786  3.5813  3.5840  3.5866
                     3.5893  3.5920  3.5946  3.5973  3.6000

    COMPUTE NM HYD       ID=1  HYD NO=101  DA= .000213 SQ MI
                         PER A=10  PER B=20 PER C=17  PER D=53
                         TP=-.160  MASSRAIN=-1



              K =  0.087200HR    TP =  0.160000HR    K/TP RATIO =  0.545000      SHAPE
CONSTANT, N =   7.106428
              UNIT PEAK =  0.37132    CFS   UNIT VOLUME =   0.9703       B =   526.28
 P60 = 1.9600
              AREA =      0.000113 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
              RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


              K =  0.154783HR    TP =  0.160000HR    K/TP RATIO =  0.967396      SHAPE
CONSTANT, N =  3.651430
              UNIT PEAK =  0.20724    CFS   UNIT VOLUME =   0.9356       B =   331.23
 P60 = 1.9600
              AREA =      0.000100 SQ MI    IA =   0.47766 INCHES    INF =   1.18745
INCHES PER HOUR
              RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD            ID=1  CODE=3

                                                PARTIAL HYDROGRAPH   101.00

          TIME      FLOW          TIME      FLOW          TIME      FLOW
TIME       FLOW             TIME      FLOW
          HRS       CFS           HRS       CFS           HRS       CFS
HRS        CFS       HRS
          0.000       0.0       4.950       0.0       9.900       0.0
14.850       0.0      19.800       0.0
          0.150       0.0       5.100       0.0      10.050       0.0
15.000       0.0      19.950       0.0
          0.300       0.0       5.250       0.0      10.200       0.0
15.150       0.0      20.100       0.0
          0.450       0.0       5.400       0.0      10.350       0.0
15.300       0.0      20.250       0.0
          0.600       0.0       5.550       0.0      10.500       0.0
15.450       0.0      20.400       0.0
          0.750       0.0       5.700       0.0      10.650       0.0
15.600       0.0      20.550       0.0
          0.900       0.0       5.850       0.0      10.800       0.0
15.750       0.0      20.700       0.0
          1.050       0.0       6.000       0.0      10.950       0.0
15.900       0.0      20.850       0.0
          1.200       0.1       6.150       0.0      11.100       0.0
```



```
16.050       0.0      21.000       0.0
          1.350       0.1       6.300       0.0      11.250       0.0
16.200       0.0      21.150       0.0
          1.500       0.5       6.450       0.0      11.400       0.0
16.350       0.0      21.300       0.0
          1.650       0.4       6.600       0.0      11.550       0.0
16.500       0.0      21.450       0.0
          1.800       0.2       6.750       0.0      11.700       0.0
16.650       0.0      21.600       0.0
          1.950       0.1       6.900       0.0      11.850       0.0
16.800       0.0      21.750       0.0
          2.100       0.1       7.050       0.0      12.000       0.0
16.950       0.0      21.900       0.0
          2.250       0.0       7.200       0.0      12.150       0.0
17.100       0.0      22.050       0.0
          2.400       0.0       7.350       0.0      12.300       0.0
17.250       0.0      22.200       0.0
          2.550       0.0       7.500       0.0      12.450       0.0
17.400       0.0      22.350       0.0
          2.700       0.0       7.650       0.0      12.600       0.0
17.550       0.0      22.500       0.0
          2.850       0.0       7.800       0.0      12.750       0.0
17.700       0.0      22.650       0.0
          3.000       0.0       7.950       0.0      12.900       0.0
17.850       0.0      22.800       0.0
          3.150       0.0       8.100       0.0      13.050       0.0
18.000       0.0      22.950       0.0
          3.300       0.0       8.250       0.0      13.200       0.0
18.150       0.0      23.100       0.0
          3.450       0.0       8.400       0.0      13.350       0.0
18.300       0.0      23.250       0.0
          3.600       0.0       8.550       0.0      13.500       0.0
18.450       0.0      23.400       0.0
          3.750       0.0       8.700       0.0      13.650       0.0
18.600       0.0      23.550       0.0
          3.900       0.0       8.850       0.0      13.800       0.0
18.750       0.0      23.700       0.0
          4.050       0.0       9.000       0.0      13.950       0.0
18.900       0.0      23.850       0.0
          4.200       0.0       9.150       0.0      14.100       0.0
19.050       0.0      24.000       0.0
          4.350       0.0       9.300       0.0      14.250       0.0
19.200       0.0      24.150       0.0
          4.500       0.0       9.450       0.0      14.400       0.0
19.350       0.0
          4.650       0.0       9.600       0.0      14.550       0.0
19.500       0.0
          4.800       0.0       9.750       0.0      14.700       0.0
19.650       0.0

        RUNOFF VOLUME =      2.23566 INCHES     =       0.0254 ACRE-FEET
        PEAK DISCHARGE RATE =        0.50 CFS  AT   1.550 HOURS   BASIN AREA =
0.0002 SQ. MI.


    * ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
    ROUTE RESERVOIR      ID=2    HYD NO=102   INFLOW=1    CODE=3
                         OUTFLOW(CFS)      STORAGE(AC-FT)    ELEV(FT)
                            0.00              0.0004          88.00
                            0.09              0.003           89.00
                                    0.14                0.011           90.00

                                    0.17                0.014           90.75
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 *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *

  TIME      INFLOW    ELEV      VOLUME    OUTFLOW
  (HRS)     (CFS)     (FEET)    (AC-FT)   (CFS)

    0.00      0.00     88.00      0.000     0.00
    0.15      0.00     88.00      0.000     0.00
    0.30      0.00     88.00      0.000     0.00
    0.45      0.00     88.00      0.000     0.00
    0.60      0.00     88.00      0.000     0.00
    0.75      0.02     88.03      0.000     0.00
    0.90      0.02     88.10      0.001     0.01
    1.05      0.03     88.17      0.001     0.02
    1.20      0.05     88.26      0.001     0.02
    1.35      0.13     88.50      0.002     0.04
    1.50      0.47     89.18      0.004     0.10
    1.65      0.37     89.71      0.009     0.13
    1.80      0.18     89.91      0.010     0.14
    1.95      0.09     89.90      0.010     0.14
    2.10      0.06     89.81      0.009     0.13
    2.25      0.04     89.68      0.008     0.12
    2.40      0.02     89.54      0.007     0.12
    2.55      0.01     89.39      0.006     0.11
    2.70      0.01     89.24      0.005     0.10
    2.85      0.01     89.10      0.004     0.09
    3.00      0.01     88.90      0.003     0.08
    3.15      0.00     88.60      0.002     0.05
    3.30      0.00     88.41      0.001     0.04
    3.45      0.00     88.28      0.001     0.03
    3.60      0.00     88.20      0.001     0.02
    3.75      0.00     88.14      0.001     0.01
    3.90      0.00     88.11      0.001     0.01
    4.05      0.00     88.08      0.001     0.01
    4.20      0.00     88.07      0.001     0.01
    4.35      0.00     88.06      0.001     0.01
    4.50      0.00     88.05      0.001     0.00
 PEAK DISCHARGE =          0.136 CFS - PEAK OCCURS AT HOUR    1.85
 MAXIMUM WATER SURFACE ELEVATION =        89.921
 MAXIMUM STORAGE =          0.0104 AC-FT      INCREMENTAL TIME=     0.050000HRS

FINISH

    NORMAL PROGRAM FINISH          END TIME (HR:MIN:SEC) = 13:12:35
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City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
APPROVED
DATE: 08/25/22
BY: Rosa C. Brunetti
HydroTrans # E23D031

THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO BE A VIOLATION OF THIS OR ANY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY OF ALBUQUERQUE FROM REQUIRING CORRECTION OF ERRORS IN THE PLANS/REPORT, SPECIFICATIONS, OR CONSTRUCTION. SUCH APPROVED PLANS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.

ENGINEER'S SEAL DAVID SOULE NEW MEXICO 14522 REGISTERED PROFESSIONAL ENGINEER 8/10/22 DAVID SOULE P.E. #14522	13420 OSAGE ORANGE CALCULATION SHEET Rio Grande Engineering 1606 CENTRAL AVENUE SE SUITE 201 ALBUQUERQUE, NM 87106 (505) 872-0999	DRAWN BY WCWJ DATE 8-09-22 218145-LAYOUT-10-02-19 SHEET # 2 JOB # 21740