

CITY OF ALBUQUERQUE

Planning Department
Brennon Williams, Director



Mayor Timothy M. Keller

September 5, 2019

David Soule, P.E.
Rio Grande Engineering
P.O. Box 93924
Albuquerque, NM 87199

**RE: 13648 Apache Plume Place NE
Grading and Drainage Plan
Engineer's Stamp Date: 08/25/19
Hydrology File: E23D034**

Dear Mr. Soule:

PO Box 1293

Based upon the information provided in your submittal received 08/26/19, the Grading and Drainage Plan is approved for Building Permit.

Albuquerque

Once the grading is complete, a pad certification will be required prior to release of Building Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter and the pad certification approval letter.

NM 87103

Prior to approval in support of Permanent Release of Occupancy by Hydrology, Engineer Certification per the DPM checklist will be required.

www.cabq.gov

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department



City of Albuquerque

Planning Department
Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: 13648 APACHE PLUME Building Permit #: _____ Hydrology File #: _____

DRB#: _____ EPC#: _____ Work Order#: _____

Legal Description: LOT 67 UNIT 2 THE HIGHLANDS AT HIGH DESERT

City Address: 13648 APACHE PLUME

Applicant: _____ Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

Other Contact: RIO GRANDE ENGINEERING Contact: DAVID SOULE

Address: PO BOX 93924 ALB NM 87199

Phone#: 505.321.9099 Fax#: 505.872.0999 E-mail: david@riograndeengineering.com

TYPE OF DEVELOPMENT: PLAT RESIDENCE DRB SITE ADMIN SITE

Check all that Apply:

DEPARTMENT:

- HYDROLOGY/ DRAINAGE
- TRAFFIC/ TRANSPORTATION

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

- BUILDING PERMIT APPROVAL
- CERTIFICATE OF OCCUPANCY
- PRELIMINARY PLAT APPROVAL
- SITE PLAN FOR SUB'D APPROVAL
- SITE PLAN FOR BLDG. PERMIT APPROVAL
- FINAL PLAT APPROVAL

TYPE OF SUBMITTAL:

- ENGINEER/ARCHITECT CERTIFICATION
- PAD CERTIFICATION
- CONCEPTUAL G & D PLAN
- GRADING PLAN
- DRAINAGE REPORT
- DRAINAGE MASTER PLAN
- FLOODPLAIN DEVELOPMENT PERMIT APPLIC
- ELEVATION CERTIFICATE
- CLOMR/LOMR
- TRAFFIC CIRCULATION LAYOUT (TCL)
- TRAFFIC IMPACT STUDY (TIS)
- STREET LIGHT LAYOUT
- OTHER (SPECIFY) _____
- PRE-DESIGN MEETING?

- SIA/ RELEASE OF FINANCIAL GUARANTEE
- FOUNDATION PERMIT APPROVAL
- GRADING PERMIT APPROVAL
- SO-19 APPROVAL
- PAVING PERMIT APPROVAL
- GRADING/ PAD CERTIFICATION
- WORK ORDER APPROVAL
- CLOMR/LOMR
- FLOODPLAIN DEVELOPMENT PERMIT
- OTHER (SPECIFY) _____

IS THIS A RESUBMITTAL?: Yes No

DATE SUBMITTED: _____ By: _____

COA STAFF: _____

ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

DRAINAGE REPORT

For

**13648 Apache Plume PL NE
Lot 67 Highlands at High Desert unit2**

Albuquerque, New Mexico

Prepared by

Rio Grande Engineering
PO Box 93924
Albuquerque, New Mexico 87199

AUGUST 2019



8/25/19

David Soule P.E. No. 14522

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Map

Site Grading and Drainage Plan

PURPOSE

The purpose of this report is to provide the Drainage Management Plan for the subdivision of a 1.01 acre lot located in phase 2 of The Highlands. The plan shall conform to the Sustainability Guidelines for Estate lots in addition to the High Desert Master Drainage Plan and specifically the Highlands at High Desert grading plan. All historic drainage patterns shall be maintained. The developed flow shall not exceed existing flow rates for the 100-year, 6- hour event. The report shall demonstrate the project will not adversely affect the surrounding properties, nor the upstream or downstream facilities.

INTRODUCTION

The subject of this report, as shown on the Exhibit A, is a 0.77 -acre parcel of land located on the southside of Apache Plume east of Elena Gallegos. The legal description of this site is lot 67 The Highlands at high desert phase 2. As shown on FIRM map35001C0161G, the entire site is located within Flood Zone X. The site is bound on the north by roadways. The site is impacted by upland flow. The site is an undeveloped site.

EXISTING CONDITIONS

The site is currently undeveloped. The site is impacted by 2.85 acres of upland flows. An upland flow of 7.28 cfs enters the site at the North East corner. This flow combines with the existing site flow and discharges at the southwest corner at a peak rate of 9.11 cfs.

Exhibit A- Vicinity Map

PROPOSED CONDITIONS

The proposed improvements consist of new residential structure. The proposed site development will contain 2 basins. Basin A contains the portion of the lot that is un developed or does not rains to the proposed detention pond. The basin generates 0.65 cfs which continues to drain to the west. Basin B contains the entire portion of the lot that drain to the proposed detention pond. This basin generates 1.84cfs. The peak flow is attenuated by the construction of a detention pond that captures 205 cubic feet for water quality and discharges at a routed peak rate of 1.12 cfs

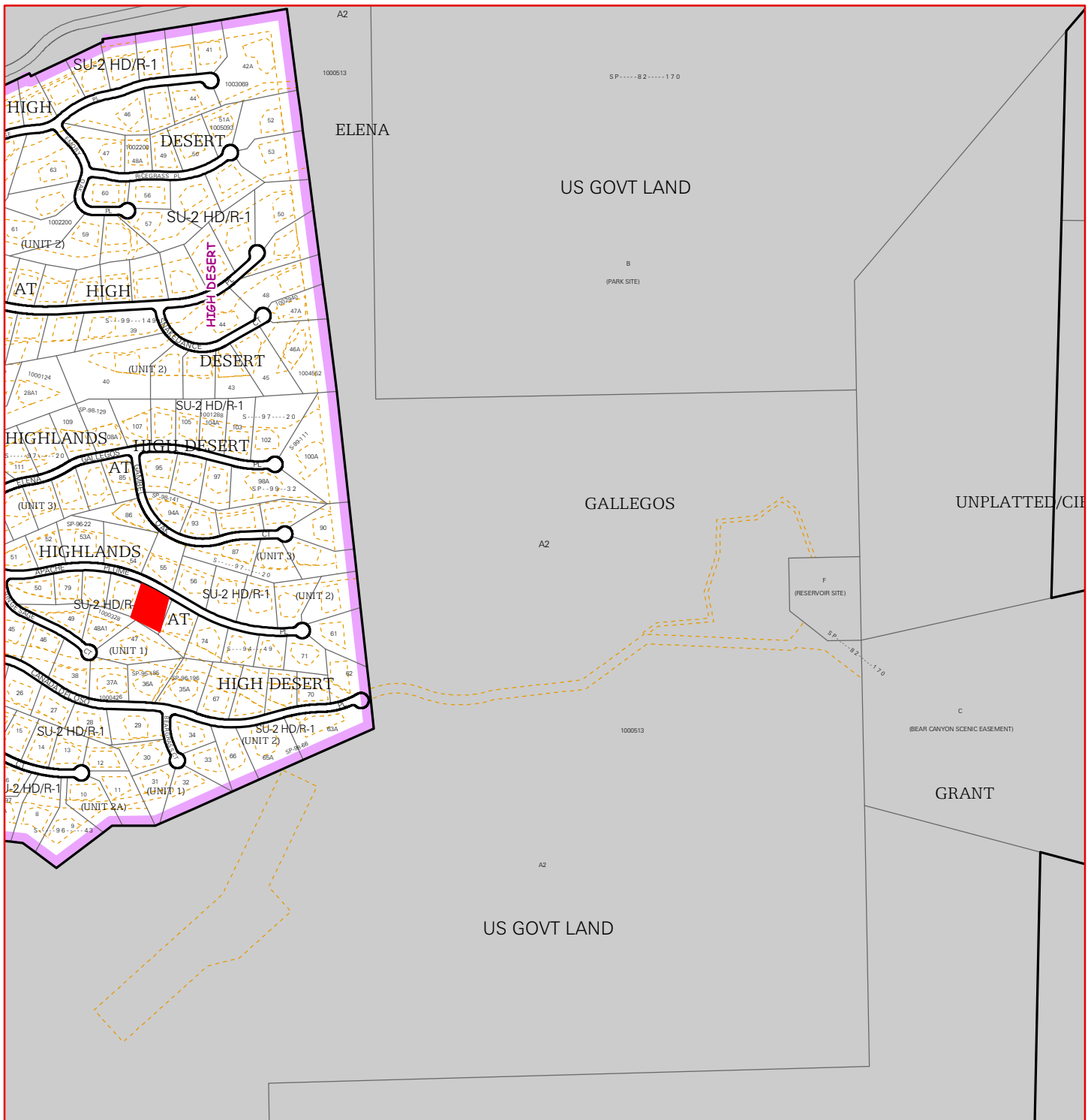
The site has been modeled using AHYMO. The peak flow leaving the site shall be 8.84 cfs which is less than the 9.11 cfs leaving the site currently.

SUMMARY AND RECOMMENDATIONS


This project is located within a fully designed basin of The Highlands drainage plan. The site has been designed to allow existing drainage patterns to remain. The site utilizes detention ponds to provide a peak flow rate of 8.84 CFS and captures 205 cubic feet for water quality. The site has been designed to conform to the Sustainability Guidelines for Estate lots in addition to the High Desert Master Drainage Plan. Since the lot area does exceed 1 acre, erosion and sediment Control Plan shall be required prior to any construction activity.

APPENDIX A

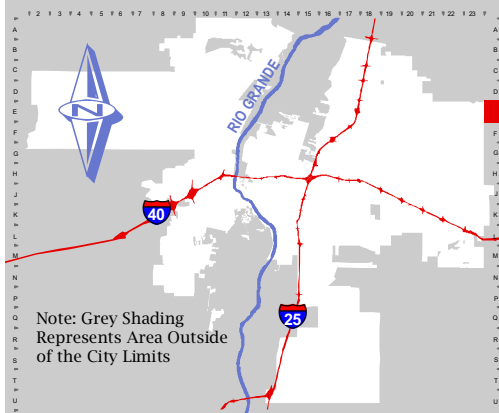
HYDRAULIC MODELING AND CALCULATIONS



For more current information and more details visit: <http://www.cabq.gov/gis>



Map amended through: 2/4/2010

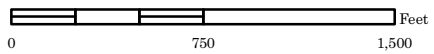


Note: Grey Shading Represents Area Outside of the City Limits

Zone Atlas Page:
E-24-Z

Selected Symbols

SECTOR PLANS	Escarpment
Design Overlay Zones	2 Mile Airport Zone
City Historic Zones	Airport Noise Contours
H-1 Buffer Zone	Wall Overlay Zone
Petroglyph Mon.	



PROPOSED CONDITIONS

The proposed improvements consist of new residential structure. The proposed site development will contain 2 basins. Basin A contains the portion of the lot that is un developed or does not rains to the proposed detention pond. The basin generates 0.65 cfs which continues to drain to the west. Basin B contains the entire portion of the lot that drain to the proposed detention pond. This basin generates 1.84cfs. The peak flow is attenuated by the construction of a detention pond that captures 205 cubic feet for water quality and discharges at a routed peak rate of 1.12 cfs

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APPENDIX A

HYDRAULIC MODELING AND CALCULATIONS

Print Export

Point Query Filter Find an active survey monument

Find Data

Point

Draw

Polygon

Feet (ft) Acres (ac)

Enable Snapping Select Snapping Layers

Advanced Measurement

Edit Export Drawings

Edit Drawings

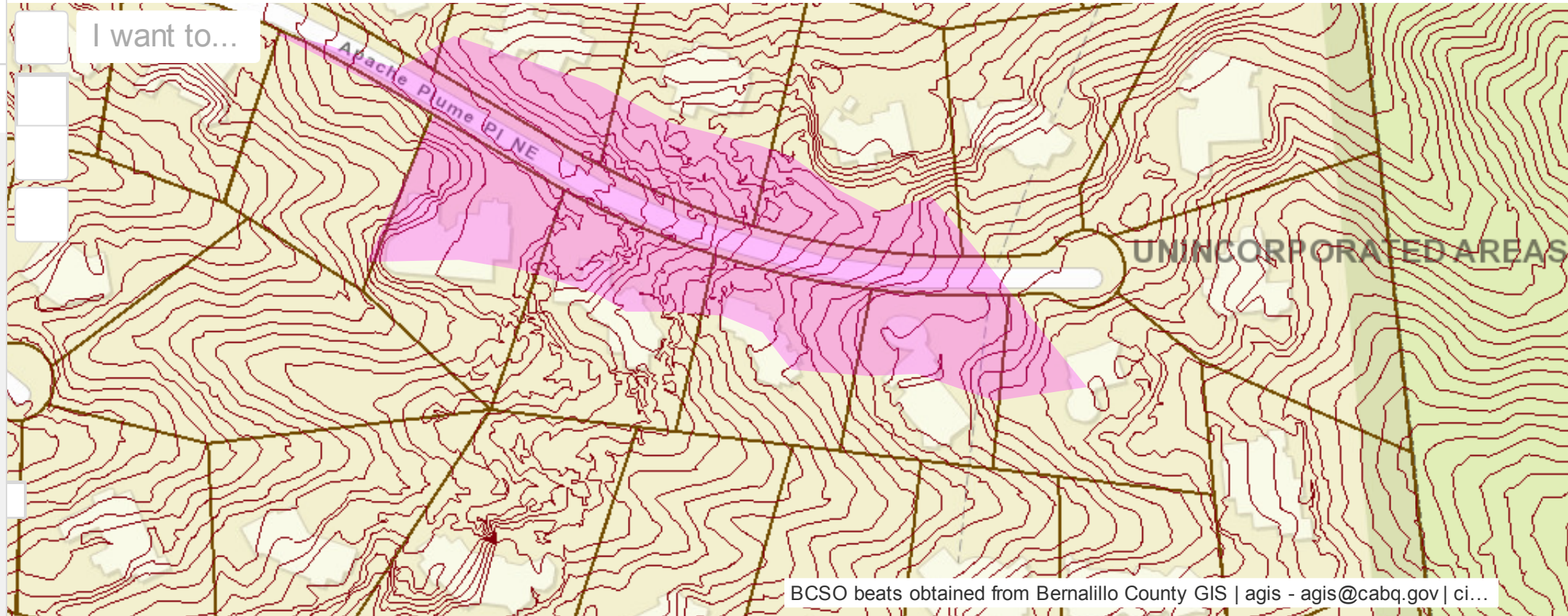
Layers

All Available Layers

Filter Layers... Filter

- Sites
- Environment
- Public Schools
- Traffic Barricades
- Traffic Engineering
- 2ft Contours
- Municipal Limits
- Base Maps**
 - 2018 Aerial
 - 2016 Aerial
 - 2014 Aerial
 - 2012 Aerial
 - 1959 Aerial
 - World Topo Map

Click or tap to draw a multi-sided shape and find area. Double-click/tap to finish.



VOLUME CALCULATIONS

pond b

OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
57.00	0.00	62.00	0	0	0.000	0.00
58.00	0.00	189.00	125.50	125.5	0.001	0.00
58.35	0.00	268.00	79.98	205.475	0.005	0.00
59.00	0.65	545.00	264.23	469.7	0.011	0.76
59.85	1.50	689.00	524.45	994.15	0.023	1.16

Orifice Equation

$$Q = CA \text{ SQRT}(2gH)$$

C = 0.6
 Diameter (in) 6
 Area (ft²)= 0.196349541
 g = 32.2
 H (Ft) = Depth of water above center of orifice
 Q (CFS)= Flow

AHYMO. OUT

AHYMO PROGRAM (AHYMO-S4) - Versi on: S4.01a - Rel: 01a
RUN DATE (MON/DAY/YR) = 08/26/2019
START TIME (HR: MI N: SEC) = 13: 40: 57 USER NO. =
Ri oGrandeSi ngl eA41963517
INPUT FILE = ments and Sett ings\Owner\Desktop\2019 JOBS\19161-APACHE
PLUME\pondrout082519.txt

*S AHYMO - DETENTI ON-APACHE PLUME

*S POND ROUTING

START TIME=0.0 PUNCH CODE=0

RAI NFALL TYPE=2
QUARTER=0.0 ONE= 1.85 IN
SIX=2.44 IN DAY= 2.90 IN DT = 0.05 HR

24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE
AREAS (NM & AZ) - D1

DT = 0.050000 HOURS		END TIME = 24.000002 HOURS					
0.0000	0.0039	0.0080	0.0123	0.0170	0.0218	0.0273	
0.0350	0.0473	0.0605	0.0742	0.0891	0.1041	0.1196	
0.1355	0.1517	0.1696	0.1883	0.2083	0.2329	0.2596	
0.2955	0.3361	0.3863	0.4532	0.5285	0.6585	0.8606	
1.2070	1.4505	1.6425	1.7389	1.8235	1.8843	1.9326	
1.9747	2.0056	2.0339	2.0573	2.0777	2.0966	2.1136	
2.1297	2.1439	2.1574	2.1706	2.1833	2.1938	2.1997	
2.2054	2.2110	2.2163	2.2216	2.2267	2.2317	2.2367	
2.2414	2.2460	2.2506	2.2551	2.2595	2.2637	2.2679	
2.2720	2.2761	2.2801	2.2839	2.2877	2.2915	2.2952	
2.2988	2.3024	2.3060	2.3095	2.3130	2.3164	2.3198	
2.3232	2.3265	2.3298	2.3330	2.3362	2.3394	2.3425	
2.3456	2.3487	2.3518	2.3548	2.3577	2.3607	2.3636	
2.3665	2.3694	2.3722	2.3750	2.3778	2.3805	2.3833	
2.3860	2.3886	2.3913	2.3939	2.3965	2.3991	2.4017	
2.4042	2.4068	2.4093	2.4117	2.4142	2.4166	2.4191	
2.4215	2.4238	2.4262	2.4285	2.4309	2.4332	2.4355	
2.4377	2.4400	2.4423	2.4445	2.4468	2.4490	2.4512	
2.4535	2.4557	2.4579	2.4601	2.4623	2.4645	2.4667	
2.4689	2.4711	2.4733	2.4755	2.4776	2.4798	2.4820	
2.4841	2.4863	2.4884	2.4906	2.4927	2.4948	2.4969	
2.4991	2.5012	2.5033	2.5054	2.5075	2.5096	2.5117	
2.5137	2.5158	2.5179	2.5199	2.5220	2.5241	2.5261	
2.5281	2.5302	2.5322	2.5342	2.5363	2.5383	2.5403	
2.5423	2.5443	2.5463	2.5483	2.5503	2.5522	2.5542	
2.5562	2.5581	2.5601	2.5621	2.5640	2.5659	2.5679	
2.5698	2.5717	2.5736	2.5756	2.5775	2.5794	2.5813	
2.5832	2.5850	2.5869	2.5888	2.5907	2.5925	2.5944	
2.5962	2.5981	2.5999	2.6018	2.6036	2.6054	2.6073	
2.6091	2.6109	2.6127	2.6145	2.6163	2.6181	2.6198	
2.6216	2.6234	2.6252	2.6269	2.6287	2.6304	2.6322	
2.6339	2.6356	2.6374	2.6391	2.6408	2.6425	2.6442	
2.6459	2.6476	2.6493	2.6510	2.6527	2.6543	2.6560	
2.6577	2.6593	2.6610	2.6626	2.6643	2.6659	2.6675	
2.6692	2.6708	2.6724	2.6740	2.6756	2.6772	2.6788	
2.6804	2.6820	2.6835	2.6851	2.6867	2.6882	2.6898	
2.6913	2.6929	2.6944	2.6959	2.6975	2.6990	2.7005	
2.7020	2.7035	2.7050	2.7065	2.7080	2.7095	2.7109	
2.7124	2.7139	2.7153	2.7168	2.7182	2.7197	2.7211	
2.7225	2.7240	2.7254	2.7268	2.7282	2.7296	2.7310	

AHYMO. OUT

2. 7324	2. 7338	2. 7352	2. 7366	2. 7379	2. 7393	2. 7407
2. 7420	2. 7434	2. 7447	2. 7461	2. 7474	2. 7487	2. 7500
2. 7513	2. 7527	2. 7540	2. 7553	2. 7566	2. 7578	2. 7591
2. 7604	2. 7617	2. 7629	2. 7642	2. 7655	2. 7667	2. 7680
2. 7692	2. 7704	2. 7717	2. 7729	2. 7741	2. 7753	2. 7765
2. 7777	2. 7789	2. 7801	2. 7813	2. 7825	2. 7836	2. 7848
2. 7860	2. 7871	2. 7883	2. 7894	2. 7906	2. 7917	2. 7928
2. 7939	2. 7951	2. 7962	2. 7973	2. 7984	2. 7995	2. 8006
2. 8017	2. 8027	2. 8038	2. 8049	2. 8059	2. 8070	2. 8081
2. 8091	2. 8101	2. 8112	2. 8122	2. 8132	2. 8142	2. 8153
2. 8163	2. 8173	2. 8183	2. 8193	2. 8202	2. 8212	2. 8222
2. 8232	2. 8241	2. 8251	2. 8260	2. 8270	2. 8279	2. 8289
2. 8298	2. 8307	2. 8316	2. 8325	2. 8334	2. 8344	2. 8352
2. 8361	2. 8370	2. 8379	2. 8388	2. 8396	2. 8405	2. 8414
2. 8422	2. 8431	2. 8439	2. 8447	2. 8456	2. 8464	2. 8472
2. 8480	2. 8488	2. 8496	2. 8504	2. 8512	2. 8520	2. 8528
2. 8536	2. 8543	2. 8551	2. 8559	2. 8566	2. 8574	2. 8581
2. 8588	2. 8596	2. 8603	2. 8610	2. 8617	2. 8624	2. 8631
2. 8638	2. 8645	2. 8652	2. 8659	2. 8666	2. 8672	2. 8679
2. 8686	2. 8692	2. 8699	2. 8705	2. 8711	2. 8718	2. 8724
2. 8730	2. 8736	2. 8742	2. 8748	2. 8754	2. 8760	2. 8766
2. 8772	2. 8778	2. 8783	2. 8789	2. 8795	2. 8800	2. 8806
2. 8811	2. 8816	2. 8822	2. 8827	2. 8832	2. 8837	2. 8842
2. 8847	2. 8852	2. 8857	2. 8862	2. 8867	2. 8872	2. 8876
2. 8881	2. 8886	2. 8890	2. 8895	2. 8899	2. 8903	2. 8908
2. 8912	2. 8916	2. 8920	2. 8924	2. 8928	2. 8932	2. 8936
2. 8940	2. 8944	2. 8948	2. 8951	2. 8955	2. 8958	2. 8962
2. 8965	2. 8969	2. 8972	2. 8976	2. 8979	2. 8982	2. 8985
2. 8988	2. 8991	2. 8994	2. 8997	2. 9000		

*Basi n ONSITE FREE DI S CHARGE
 COMPUTE NM HYD ID=1 HYD NO=101 DA= .0004404 SQ MI
 PER A=40 PER B=48 PER C=8 PER D=4
 TP=-. 138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.67180E-01CFS UNIT VOLUME = 0.8928 B = 526.28
 P60 = 1.8500
 AREA = 0.000018 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTI ON/INFILTRATI ON NUMBER METHOD - DT =
 0.050000

K = 0.147925HR TP = 0.138000HR K/TP RATIO = 1.071921 SHAPE
 CONSTANT, N = 3.294865
 UNIT PEAK = 0.93456 CFS UNIT VOLUME = 0.9851 B = 305.05
 P60 = 1.8500
 AREA = 0.000423 SQ MI IA = 0.55000 INCHES INF = 1.39000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTI ON/INFILTRATI ON NUMBER METHOD - DT =
 0.050000

PRINT HYD ID=1 CODE=3

PARTIAL HYDROGRAPH 101.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
HRS	FLOW	CFS	HRS	FLOW	HRS	CFS
	HRS		HRS	CFS		CFS
2.250	0.000	0.0	3.000	0.750	1.500	0.6
	0.0			0.0		

			AHYMO. OUT			
2. 400	0. 150	0. 0	0. 900	0. 0	1. 650	0. 4
	0. 0					
2. 550	0. 300	0. 0	1. 050	0. 0	1. 800	0. 2
	0. 0					
2. 700	0. 450	0. 0	1. 200	0. 0	1. 950	0. 1
	0. 0					
2. 850	0. 600	0. 0	1. 350	0. 1	2. 100	0. 1
	0. 0					

RUNOFF VOLUME = 0. 87240 INCHES = 0. 0205 ACRE-FEET
 PEAK DISCHARGE RATE = 0. 65 CFS AT 1. 550 HOURS BASIN AREA =
 0. 0004 SQ. MI.

*Basin ONSITE ROUTED
 COMPUTE NM HYD ID=2 HYD NO=102 DA= .00081479 SQ MI
 PER A=9 PER B=20 PER C=23 PER D=48
 TP=-. 138 MASSRAIN=-1

K = 0. 075210HR TP = 0. 138000HR K/TP RATIO = 0. 545000 SHAPE
 CONSTANT, N = 7. 106428
 UNIT PEAK = 1. 4915 CFS UNIT VOLUME = 0. 9912 B = 526. 28
 P60 = 1. 8500
 AREA = 0. 000391 SQ MI IA = 0. 10000 INCHES INF = 0. 04000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0. 050000

K = 0. 130091HR TP = 0. 138000HR K/TP RATIO = 0. 942685 SHAPE
 CONSTANT, N = 3. 750422
 UNIT PEAK = 1. 0383 CFS UNIT VOLUME = 0. 9888 B = 338. 18
 P60 = 1. 8500
 AREA = 0. 000424 SQ MI IA = 0. 45962 INCHES INF = 1. 13692
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0. 050000

PRINT HYD ID=2 CODE=3

PARTIAL HYDROGRAPH 102. 00

TIME	TIME	FLOW	TIME	TIME	FLOW	TIME	FLOW
HRS	FLOW	CFS	HRS	FLOW	CFS	HRS	CFS
HRS	HRS	CFS	HRS	HRS	CFS	HRS	CFS
14. 850	0. 000	0. 0	19. 800	4. 950	0. 0	9. 900	0. 0
	0. 150	0. 0		5. 100	0. 0	10. 050	0. 0
15. 000	0. 0	0. 0	19. 950	5. 250	0. 0	10. 200	0. 0
	0. 300	0. 0		5. 400	0. 0	10. 350	0. 0
15. 150	0. 0	0. 0	20. 100	5. 550	0. 0	10. 500	0. 0
	0. 450	0. 0		5. 700	0. 0	10. 650	0. 0
15. 300	0. 0	0. 0	20. 250	5. 850	0. 0	10. 800	0. 0
	0. 600	0. 0		6. 000	0. 0	10. 950	0. 0
15. 450	0. 0	0. 0	20. 400				
	0. 750	0. 0					
15. 600	0. 0	0. 1	20. 550				
	0. 900	0. 1					
15. 750	0. 0	0. 1	20. 700				
	1. 050	0. 1					

AHYMO. OUT							
15.900	0.0		20.850	0.0			
	1.200	0.2		6.150	0.0	11.100	0.0
16.050	0.0		21.000	0.0			
	1.350	0.5		6.300	0.0	11.250	0.0
16.200	0.0		21.150	0.0			
	1.500	1.8		6.450	0.0	11.400	0.0
16.350	0.0		21.300	0.0			
	1.650	1.1		6.600	0.0	11.550	0.0
16.500	0.0		21.450	0.0			
	1.800	0.5		6.750	0.0	11.700	0.0
16.650	0.0		21.600	0.0			
	1.950	0.3		6.900	0.0	11.850	0.0
16.800	0.0		21.750	0.0			
	2.100	0.2		7.050	0.0	12.000	0.0
16.950	0.0		21.900	0.0			
	2.250	0.1		7.200	0.0	12.150	0.0
17.100	0.0		22.050	0.0			
	2.400	0.1		7.350	0.0	12.300	0.0
17.250	0.0		22.200	0.0			
	2.550	0.0		7.500	0.0	12.450	0.0
17.400	0.0		22.350	0.0			
	2.700	0.0		7.650	0.0	12.600	0.0
17.550	0.0		22.500	0.0			
	2.850	0.0		7.800	0.0	12.750	0.0
17.700	0.0		22.650	0.0			
	3.000	0.0		7.950	0.0	12.900	0.0
17.850	0.0		22.800	0.0			
	3.150	0.0		8.100	0.0	13.050	0.0
18.000	0.0		22.950	0.0			
	3.300	0.0		8.250	0.0	13.200	0.0
18.150	0.0		23.100	0.0			
	3.450	0.0		8.400	0.0	13.350	0.0
18.300	0.0		23.250	0.0			
	3.600	0.0		8.550	0.0	13.500	0.0
18.450	0.0		23.400	0.0			
	3.750	0.0		8.700	0.0	13.650	0.0
18.600	0.0		23.550	0.0			
	3.900	0.0		8.850	0.0	13.800	0.0
18.750	0.0		23.700	0.0			
	4.050	0.0		9.000	0.0	13.950	0.0
18.900	0.0		23.850	0.0			
	4.200	0.0		9.150	0.0	14.100	0.0
19.050	0.0		24.000	0.0			
	4.350	0.0		9.300	0.0	14.250	0.0
19.200	0.0						
	4.500	0.0		9.450	0.0	14.400	0.0
19.350	0.0						
	4.650	0.0		9.600	0.0	14.550	0.0
19.500	0.0						
	4.800	0.0		9.750	0.0	14.700	0.0
19.650	0.0						

RUNOFF VOLUME = 1.74744 INCHES = 0.0759 ACRE-FEET
 PEAK DISCHARGE RATE = 1.80 CFS AT 1.500 HOURS BASIN AREA =
 0.0008 SQ. MI.

*Basin ONSITE EXISTING

COMPUTE NM HYD ID=3 HYD NO=103 DA= .0012552 SQ MI
 PER A=42 PER B=40 PER C=14 PER D=4
 TP=-.138 MASSRAIN=-1

AHYMO. OUT

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.19147 CFS UNIT VOLUME = 0.9442 B = 526.28
 P60 = 1.8500
 AREA = 0.000050 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

K = 0.146970HR TP = 0.138000HR K/TP RATIO = 1.065003 SHAPE
 CONSTANT, N = 3.315776
 UNIT PEAK = 2.6775 CFS UNIT VOLUME = 0.9955 B = 306.64
 P60 = 1.8500
 AREA = 0.001205 SQ MI IA = 0.54375 INCHES INF = 1.37250
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

PRINT HYD ID=3 CODE=3

PARTIAL HYDROGRAPH 103.00

TIME	TIME	FLOW	TIME	TIME	FLOW	TIME	FLOW
HRS	FLOW	CFS	HRS	FLOW	CFS	HRS	CFS
	HRS			HRS			
8.100	0.000	0.0	10.800	2.700	0.0	5.400	0.0
8.250	0.150	0.0	10.950	2.850	0.0	5.550	0.0
8.400	0.300	0.0	11.100	3.000	0.0	5.700	0.0
8.550	0.450	0.0	11.250	3.150	0.0	5.850	0.0
8.700	0.600	0.0	11.400	3.300	0.0	6.000	0.0
8.850	0.750	0.0	11.550	3.450	0.0	6.150	0.0
9.000	0.900	0.0	11.700	3.600	0.0	6.300	0.0
9.150	1.050	0.0	11.850	3.750	0.0	6.450	0.0
9.300	1.200	0.0	12.000	3.900	0.0	6.600	0.0
9.450	1.350	0.3	12.150	4.050	0.0	6.750	0.0
9.600	1.500	1.8	12.300	4.200	0.0	6.900	0.0
9.750	1.650	1.2	12.450	4.350	0.0	7.050	0.0
9.900	1.800	0.5	12.600	4.500	0.0	7.200	0.0
10.050	1.950	0.2	12.750	4.650	0.0	7.350	0.0
10.200	2.100	0.1		4.800	0.0	7.500	0.0
10.350	2.250	0.1		4.950	0.0	7.650	0.0
10.500	2.400	0.1		5.100	0.0	7.800	0.0
10.650	2.550	0.1		5.250	0.0	7.950	0.0

RUNOFF VOLUME = 0.88097 INCHES = 0.0590 ACRE-FEET
 PEAK DISCHARGE RATE = 1.84 CFS AT 1.550 HOURS BASIN AREA =

AHYMO. OUT

0.0013 SQ. MI.

* ONSITE ROUTED THRU POND-

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR

ROUTE RESERVOIR	ID=4	HYD NO=104	INFLOW=3	CODE=3
	OUTFLOW(CFS)		STORAGE(AC-FT)	ELEV(FT)
	0.00		0.005	58.35
	0.76		0.011	59.00
	1.16		0.023	59.85

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	58.35	0.005	0.00
0.15	0.00	58.35	0.005	0.00
0.30	0.00	58.35	0.005	0.00
0.45	0.00	58.35	0.005	0.00
0.60	0.00	58.35	0.005	0.00
0.75	0.01	58.35	0.005	0.00
0.90	0.01	58.36	0.005	0.01
1.05	0.01	58.36	0.005	0.01
1.20	0.02	58.37	0.005	0.02
1.35	0.33	58.45	0.006	0.11
1.50	1.84	59.21	0.014	0.86
1.65	1.23	59.77	0.022	1.12
1.80	0.48	59.52	0.018	1.01
1.95	0.22	59.03	0.011	0.77
2.10	0.14	58.60	0.007	0.30
2.25	0.10	58.48	0.006	0.15
2.40	0.07	58.43	0.006	0.10
2.55	0.05	58.41	0.006	0.07
2.70	0.04	58.39	0.005	0.05
2.85	0.03	58.38	0.005	0.03
3.00	0.02	58.37	0.005	0.02
3.15	0.01	58.36	0.005	0.02
3.30	0.01	58.36	0.005	0.01
3.45	0.00	58.36	0.005	0.01
3.60	0.00	58.35	0.005	0.00
PEAK DISCHARGE =	1.120 CFS	- PEAK OCCURS AT HOUR	1.65	
MAXIMUM WATER SURFACE ELEVATION =	59.765			
MAXIMUM STORAGE =	0.0218 AC-FT	INCREMENTAL TIME=	0.050000HRS	

*Basin DEVELOPED UPLAND

COMPUTE NM HYD ID=5 HYD NO=105 DA= .004453 SQ MI
 PER A=60 PER B=9 PER C=4 PER D=27
 TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 4.5851 CFS UNIT VOLUME = 0.9962 B = 526.28
 P60 = 1.8500
 AREA = 0.001202 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

AHYMO. OUT

K = 0.162464HR TP = 0.138000HR K/TP RATIO = 1.177274 SHAPE
 CONSTANT, N = 3.013732
 UNIT PEAK = 6.6650 CFS UNIT VOLUME = 0.9969 B = 282.95
 P60 = 1.8500
 AREA = 0.003251 SQ MI IA = 0.61507 INCHES INF = 1.57219
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

PRINT HYD

ID=5 CODE=3

PARTIAL HYDROGRAPH 105.00

TIME	TIME FLOW	FLOW	TIME	TIME FLOW	FLOW	TIME	FLOW
HRS	HRS CFS	CFS	HRS	HRS CFS	CFS	HRS	CFS
	0.000	0.0	4.950	0.0	0.0	9.900	0.0
14.850	0.0	0.0	19.800	0.0	0.0	10.050	0.0
15.000	0.0	0.0	19.950	0.0	0.0	10.200	0.0
15.150	0.0	0.0	20.100	0.0	0.0	10.350	0.0
15.300	0.0	0.0	20.250	0.0	0.0	10.500	0.0
15.450	0.0	0.0	20.400	0.0	0.0	10.650	0.0
15.600	0.0	0.1	20.550	0.0	0.0	10.800	0.0
15.750	0.0	0.2	20.700	0.0	0.0	10.950	0.0
15.900	0.0	0.3	20.850	0.0	0.0	11.100	0.0
16.050	0.0	0.5	21.000	0.0	0.0	11.250	0.0
16.200	0.0	1.7	21.150	0.0	0.0	11.400	0.0
16.350	0.0	7.3	21.300	0.0	0.0	11.550	0.0
16.500	0.0	4.8	21.450	0.0	0.0	11.700	0.0
16.650	0.0	2.2	21.600	0.0	0.0	11.850	0.0
16.800	0.0	1.1	21.750	0.0	0.0	12.000	0.0
16.950	0.0	0.7	21.900	0.0	0.0	12.150	0.0
17.100	0.0	0.5	22.050	0.0	0.0	12.300	0.0
17.250	0.0	0.4	22.200	0.0	0.0	12.450	0.0
17.400	0.0	0.2	22.350	0.0	0.0	12.600	0.0
17.550	0.0	0.2	22.500	0.0	0.0	12.750	0.0
17.700	0.0	0.1	22.650	0.0	0.0	12.900	0.0
17.850	0.0	0.1	22.800	0.0	0.0	13.050	0.0
18.000	0.0	0.1	22.950	0.0	0.0	13.200	0.0
18.150	0.0	0.1	23.100	0.0	0.0	13.350	0.0
18.300	0.0	0.1	23.250	0.0	0.0		

				AHYMO. OUT			
18.450	3.600	0.0	23.400	8.550	0.0	13.500	0.0
	0.0			0.0			
18.600	3.750	0.0	23.550	8.700	0.0	13.650	0.0
	0.0			0.0			
18.750	3.900	0.0	23.700	8.850	0.0	13.800	0.0
	0.0			0.0			
18.900	4.050	0.0	23.850	9.000	0.0	13.950	0.0
	0.0			0.0			
19.050	4.200	0.0	24.000	9.150	0.0	14.100	0.0
	0.0			0.0			
19.200	4.350	0.0	24.150	9.300	0.0	14.250	0.0
	0.0			0.0			
19.350	4.500	0.0		9.450	0.0	14.400	0.0
	0.0						
19.500	4.650	0.0		9.600	0.0	14.550	0.0
	0.0						
19.650	4.800	0.0		9.750	0.0	14.700	0.0
	0.0						

RUNOFF VOLUME = 1.23312 INCHES = 0.2929 ACRE-FEET
 PEAK DISCHARGE RATE = 7.28 CFS AT 1.500 HOURS BASIN AREA =
 0.0045 SQ. MI.

*TOTAL EXISTING

ADD HYD ID=6 HYD NO=106 ID I=3 ID II=5
 PRINT HYD ID=6 CODE=3

PARTIAL HYDROGRAPH 106.00

TIME	TIME	FLOW	TIME	TIME	FLOW	TIME	FLOW
HRS	FLOW	CFS	HRS	FLOW	CFS	HRS	CFS
HRS	HRS		HRS	HRS		HRS	
14.850	0.000	0.0	19.800	4.950	0.0	9.900	0.0
	0.0			0.0			
15.000	0.150	0.0	19.950	5.100	0.0	10.050	0.0
	0.0			0.0			
15.150	0.300	0.0	20.100	5.250	0.0	10.200	0.0
	0.0			0.0			
15.300	0.450	0.0	20.250	5.400	0.0	10.350	0.0
	0.0			0.0			
15.450	0.600	0.0	20.400	5.550	0.0	10.500	0.0
	0.0			0.0			
15.600	0.750	0.1	20.550	5.700	0.0	10.650	0.0
	0.0			0.0			
15.750	0.900	0.2	20.700	5.850	0.0	10.800	0.0
	0.0			0.0			
15.900	1.050	0.3	20.850	6.000	0.0	10.950	0.0
	0.0			0.0			
16.050	1.200	0.6	21.000	6.150	0.0	11.100	0.0
	0.0			0.0			
16.200	1.350	2.0	21.150	6.300	0.0	11.250	0.0
	0.0			0.0			
16.350	1.500	9.1	21.300	6.450	0.0	11.400	0.0
	0.0			0.0			
16.500	1.650	6.0	21.450	6.600	0.0	11.550	0.0
	0.0			0.0			
16.650	1.800	2.6	21.600	6.750	0.0	11.700	0.0
	0.0			0.0			
16.800	1.950	1.3	21.750	6.900	0.0	11.850	0.0
	0.0			0.0			

AHYMO. OUT						
16.950	2.100	0.9	7.050	0.0	12.000	0.0
	0.0		0.0			
17.100	2.250	0.6	7.200	0.0	12.150	0.0
	0.0		0.0			
17.250	2.400	0.5	7.350	0.0	12.300	0.0
	0.0		0.0			
17.400	2.550	0.3	7.500	0.0	12.450	0.0
	0.0		0.0			
17.550	2.700	0.2	7.650	0.0	12.600	0.0
	0.0		0.0			
17.700	2.850	0.2	7.800	0.0	12.750	0.0
	0.0		0.0			
17.850	3.000	0.1	7.950	0.0	12.900	0.0
	0.0		0.0			
18.000	3.150	0.1	8.100	0.0	13.050	0.0
	0.0		0.0			
18.150	3.300	0.1	8.250	0.0	13.200	0.0
	0.0		0.0			
18.300	3.450	0.1	8.400	0.0	13.350	0.0
	0.0		0.0			
18.450	3.600	0.0	8.550	0.0	13.500	0.0
	0.0		0.0			
18.600	3.750	0.0	8.700	0.0	13.650	0.0
	0.0		0.0			
18.750	3.900	0.0	8.850	0.0	13.800	0.0
	0.0		0.0			
18.900	4.050	0.0	9.000	0.0	13.950	0.0
	0.0		0.0			
19.050	4.200	0.0	9.150	0.0	14.100	0.0
	0.0		0.0			
19.200	4.350	0.0	9.300	0.0	14.250	0.0
	0.0		0.0			
19.350	4.500	0.0	9.450	0.0	14.400	0.0
	0.0		0.0			
19.500	4.650	0.0	9.600	0.0	14.550	0.0
	0.0		0.0			
19.650	4.800	0.0	9.750	0.0	14.700	0.0
	0.0		0.0			

RUNOFF VOLUME = 1.15380 INCHES = 0.3513 ACRE-FEET
 PEAK DISCHARGE RATE = 9.11 CFS AT 1.500 HOURS BASIN AREA =
 0.0057 SQ. MI.

*TOTAL PROPOSED

ADD HYD ID=7 HYD NO=107 ID I=1 ID II=4
 ADD HYD ID=8 HYD NO=108 ID I=5 ID II=7
 PRINT HYD ID=8 CODE=3

PARTIAL HYDROGRAPH 108.00

TIME	TIME	FLOW	TIME	TIME	FLOW	TIME	FLOW
HRS	FLOW	CFS	HRS	FLOW	CFS	HRS	CFS
	HRS			HRS			
	0.000	0.0	4.950	0.0	9.900	0.0	
14.850	0.150	0.0	5.100	0.0	10.050	0.0	
15.000	0.300	0.0	5.250	0.0	10.200	0.0	
15.150	0.0		20.100	0.0			

			AHYMO. OUT			
15.300	0.450	0.0	5.400	0.0	10.350	0.0
	0.0		0.0			
	0.600	0.0	5.550	0.0	10.500	0.0
15.450	0.0		0.0			
	0.750	0.1	5.700	0.0	10.650	0.0
15.600	0.0		0.0			
	0.900	0.2	5.850	0.0	10.800	0.0
15.750	0.0		0.0			
	1.050	0.3	6.000	0.0	10.950	0.0
15.900	0.0		0.0			
	1.200	0.6	6.150	0.0	11.100	0.0
16.050	0.0		0.0			
	1.350	1.9	6.300	0.0	11.250	0.0
16.200	0.0		0.0			
	1.500	8.8	6.450	0.0	11.400	0.0
16.350	0.0		0.0			
	1.650	6.3	6.600	0.0	11.550	0.0
16.500	0.0		0.0			
	1.800	3.3	6.750	0.0	11.700	0.0
16.650	0.0		0.0			
	1.950	2.0	6.900	0.0	11.850	0.0
16.800	0.0		0.0			
	2.100	1.1	7.050	0.0	12.000	0.0
16.950	0.0		0.0			
	2.250	0.7	7.200	0.0	12.150	0.0
17.100	0.0		0.0			
	2.400	0.5	7.350	0.0	12.300	0.0
17.250	0.0		0.0			
	2.550	0.3	7.500	0.0	12.450	0.0
17.400	0.0		0.0			
	2.700	0.2	7.650	0.0	12.600	0.0
17.550	0.0		0.0			
	2.850	0.2	7.800	0.0	12.750	0.0
17.700	0.0		0.0			
	3.000	0.1	7.950	0.0	12.900	0.0
17.850	0.0		0.0			
	3.150	0.1	8.100	0.0	13.050	0.0
18.000	0.0		0.0			
	3.300	0.1	8.250	0.0	13.200	0.0
18.150	0.0		0.0			
	3.450	0.1	8.400	0.0	13.350	0.0
18.300	0.0		0.0			
	3.600	0.0	8.550	0.0	13.500	0.0
18.450	0.0		0.0			
	3.750	0.0	8.700	0.0	13.650	0.0
18.600	0.0		0.0			
	3.900	0.0	8.850	0.0	13.800	0.0
18.750	0.0		0.0			
	4.050	0.0	9.000	0.0	13.950	0.0
18.900	0.0		0.0			
	4.200	0.0	9.150	0.0	14.100	0.0
19.050	0.0		0.0			
	4.350	0.0	9.300	0.0	14.250	0.0
19.200	0.0		0.0			
	4.500	0.0	9.450	0.0	14.400	0.0
19.350	0.0		0.0			
	4.650	0.0	9.600	0.0	14.550	0.0
19.500	0.0		0.0			
	4.800	0.0	9.750	0.0	14.700	0.0
19.650	0.0					

RUNOFF VOLUME = 1.13193 INCHES = 0.3712 ACRE-FEET
 PEAK DISCHARGE RATE = 8.84 CFS AT 1.550 HOURS BASIN AREA =
 0.0061 SQ. MI.

AHYMO. OUT

FINISH

NORMAL PROGRAM FINISH

END TIME (HR: MIN: SEC) = 13: 40: 57

*S AHYMO - DETENTION-APACHE PLUME
 *S POND ROUTING

START TIME=0.0 PUNCH CODE=0

RAINFALL TYPE=2
 QUARTER=0.0 ONE= 1.85 IN
 SIX=2.44 IN DAY= 2.90 IN DT = 0.05 HR

*Basin ONSITE FREE DISCHARGE
 COMPUTE NM HYD ID=1 HYD NO=101 DA= .0004404 SQ MI

PER A=40 PER B=48 PER C=8 PER D=4
 TP=-.138 MASSRAIN=-1

PRINT HYD ID=1 CODE=3

*Basin ONSITE ROUTED
 COMPUTE NM HYD ID=2 HYD NO=102 DA= .00081479 SQ MI

PER A=9 PER B=20 PER C=23 PER D=48
 TP=-.138 MASSRAIN=-1

PRINT HYD ID=2 CODE=3

*Basin ONSITE EXISTING
 COMPUTE NM HYD ID=3 HYD NO=103 DA= .0012552 SQ MI

PER A=42 PER B=40 PER C=14 PER D=4
 TP=-.138 MASSRAIN=-1

PRINT HYD ID=3 CODE=3

* ONSITE ROUTED THRU POND-

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
 ROUTE RESERVOIR ID=4 HYD NO=104 INFLOW=3 CODE=3

OUTFLOW(CFS)	STORAGE(AC-FT)	ELEV(FT)
0.00	0.005	58.35
0.76	0.011	59.00
1.16	0.023	59.85

*Basin DEVELOPED UPLAND
 COMPUTE NM HYD ID=5 HYD NO=105 DA= .004453 SQ MI

PER A=60 PER B=9 PER C=4 PER D=27
 TP=-.138 MASSRAIN=-1

PRINT HYD ID=5 CODE=3 pondrout082519.txt

*TOTAL EXISTING

ADD HYD ID=6 HYD NO=106 ID I=3 ID II=5

PRINT HYD ID=6 CODE=3

*TOTAL PROPOSED

ADD HYD ID=7 HYD NO=107 ID I=1 ID II=4

ADD HYD ID=8 HYD NO=108 ID I=5 ID II=7

PRINT HYD ID=8 CODE=3

FINISH

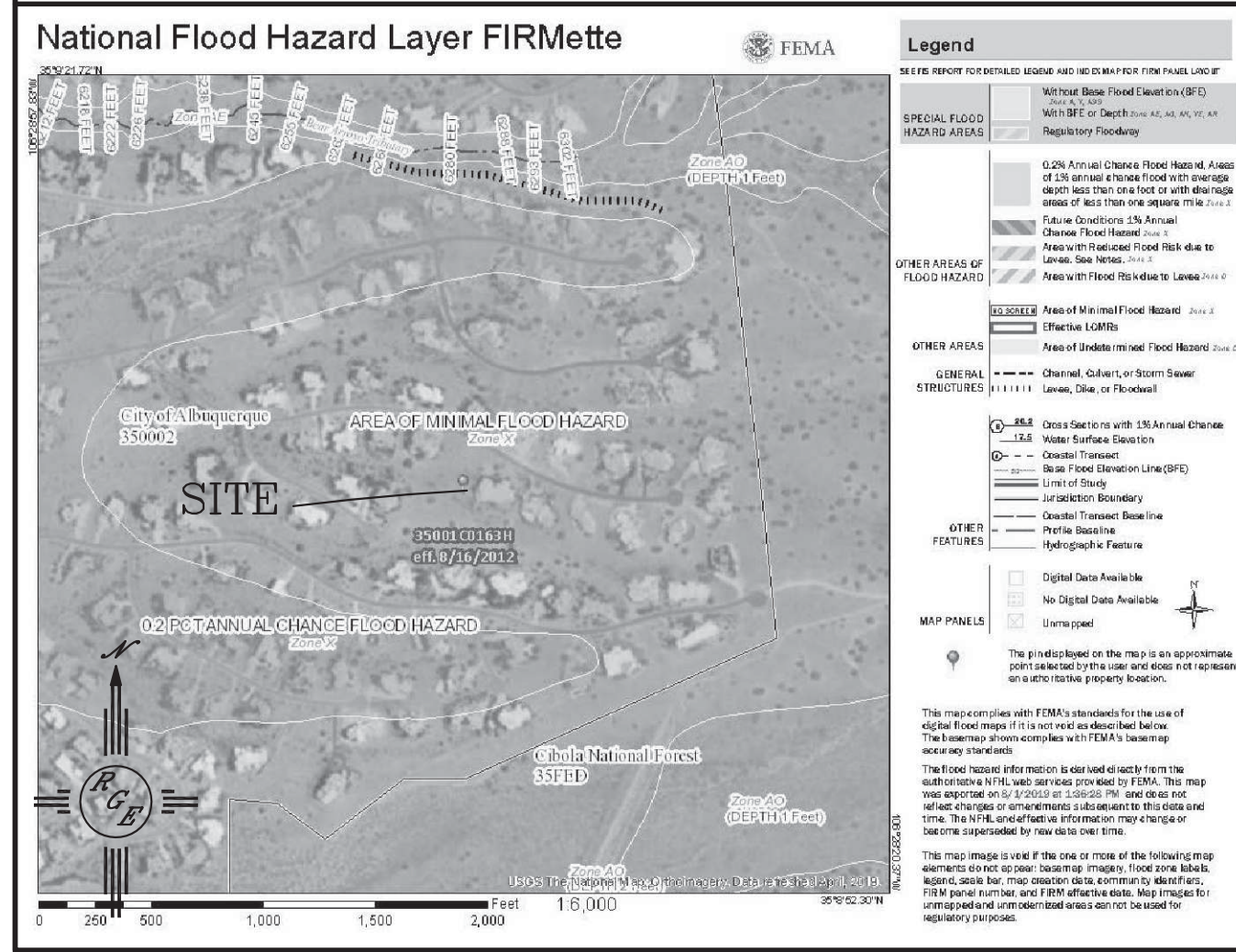
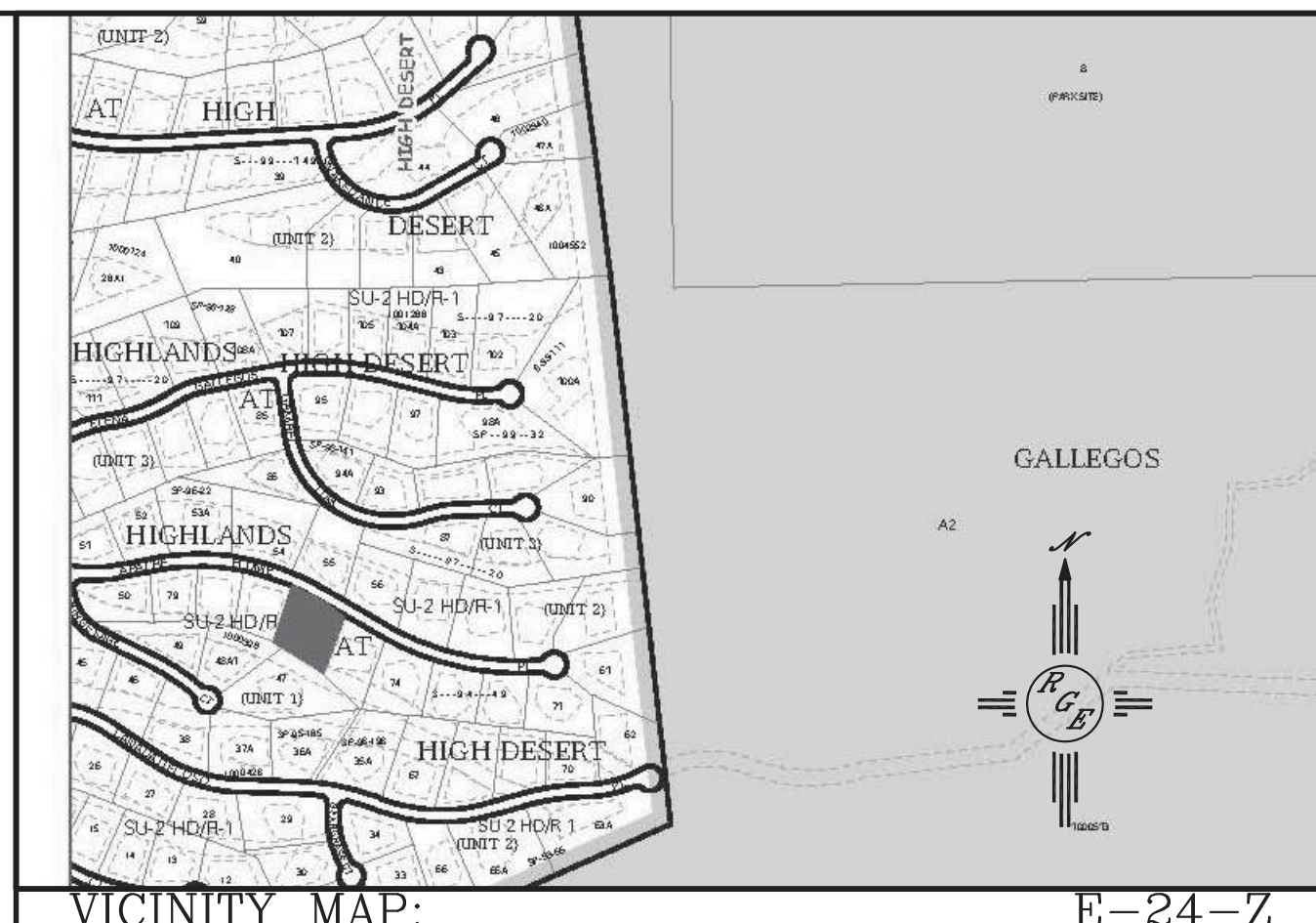
CAUTION:
EXISTING UTILITIES ARE NOT SHOWN.
IT SHALL BE THE SOLE RESPONSIBILITY
OF THE CONTRACTOR TO CONDUCT ALL
NECESSARY FIELD INVESTIGATIONS PRIOR
TO ANY EXCAVATION TO DETERMINE THE
ACTUAL LOCATION OF UTILITIES & OTHER
IMPROVEMENTS.

EROSION CONTROL NOTES:

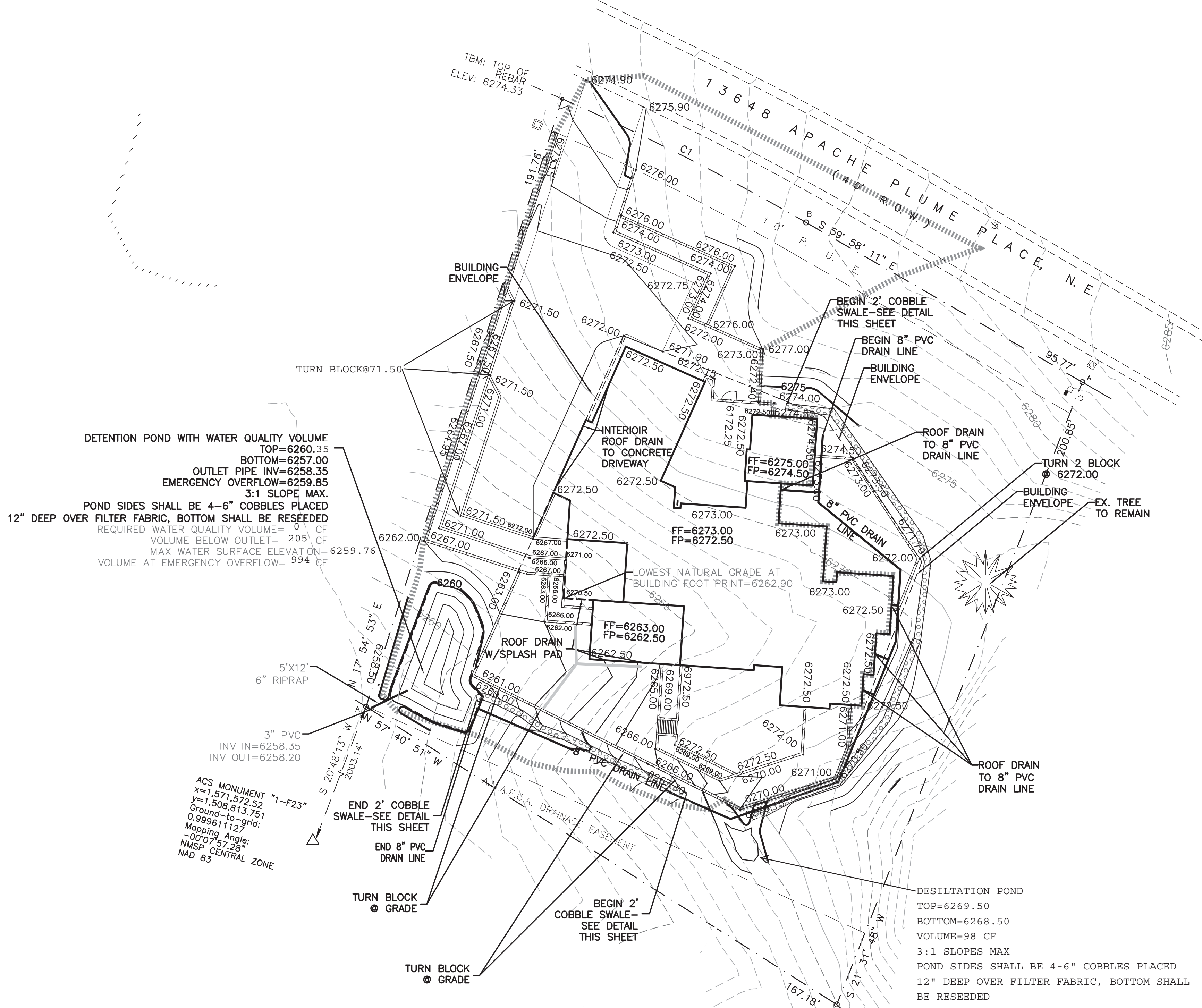
1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT.

NOTES:

1. ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.
2. ALL ROOF DRAINS SHALL HAVE A SLASH PAD AND 2' COBBLE SWALE THAT SHALL TIE TO MAIN COBBLES SWALES AROUND THE HOUSE PER DETAIL THIS SHEET..
3. ALL DISTURBED AREAS SHALL BE RESEDED WITH APPROVED HIGH DESERT MIX WITHIN 30 DAYS AFTER THE END OF DISTURBANCE.
4. ALL SLOPES SHALL BE 3:1 MAX. AND GRAVEL OR NATIVE SEEDING PRIOR TO CO.
5. SURVEY PROVIDED BY COMMUNITY SCIENCES CORPORATION, BASED UPON NAVD 1988 DATUM.
6. ALL ROOF DRAINS SHALL CONNECT TO 8" UNDERGROUND DRAIN. LINE SHALL HAVE AIR GAP AT HOUSE AND DISCHARGE TO POND. INVERT OUT @ 6259

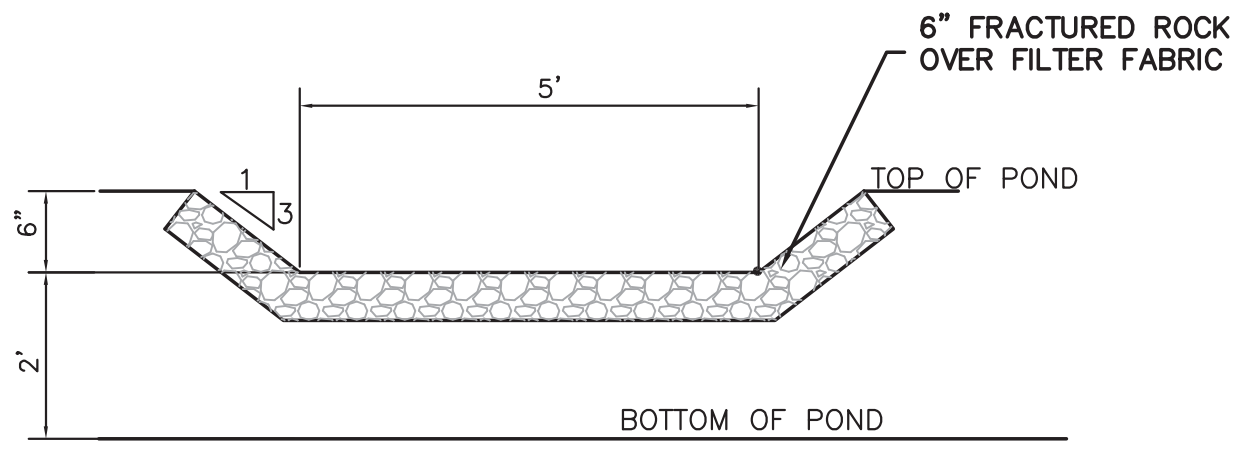


FIRM MAP:
LEGAL DESCRIPTION:
LOT 76, THE HIGHLANDS, AT HIGH DESERT UNIT 2

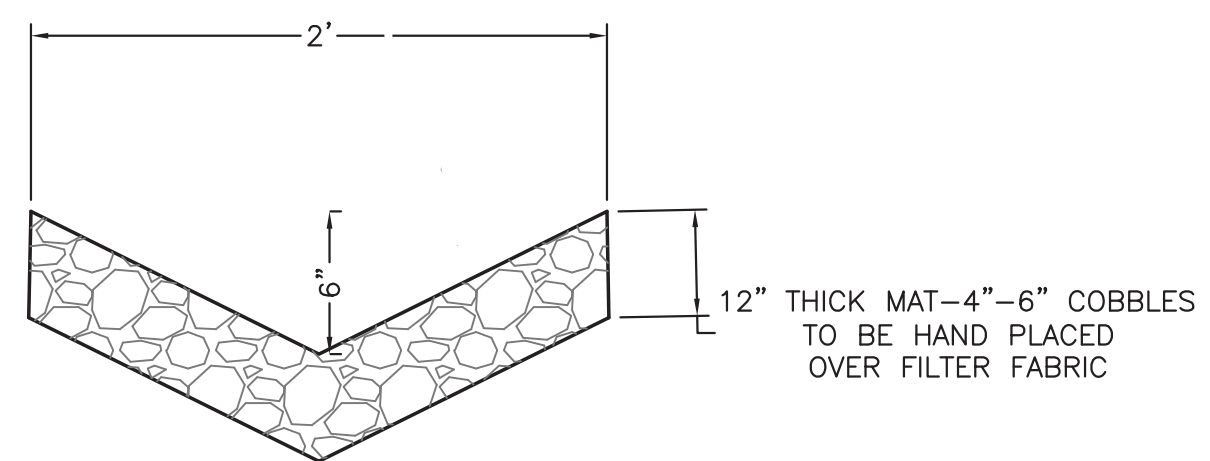


DETENTION POND WITH WATER QUALITY VOLUME
TOP=6260.35
BOTTOM=6257.00
OUTLET PIPE INV=6258.35
EMERGENCY OVERFLOW=6259.85
3:1 SLOPE MAX.
POND SIDES SHALL BE 4-6" COBBLES PLACED
12" DEEP OVER FILTER FABRIC, BOTTOM SHALL BE RESEDED
REQUIRED WATER QUALITY VOLUME= 9 CF
VOLUME BELOW OUTLET= 205 CF
MAX WATER SURFACE ELEVATION= 6259.76
VOLUME AT EMERGENCY OVERFLOW= 994 CF

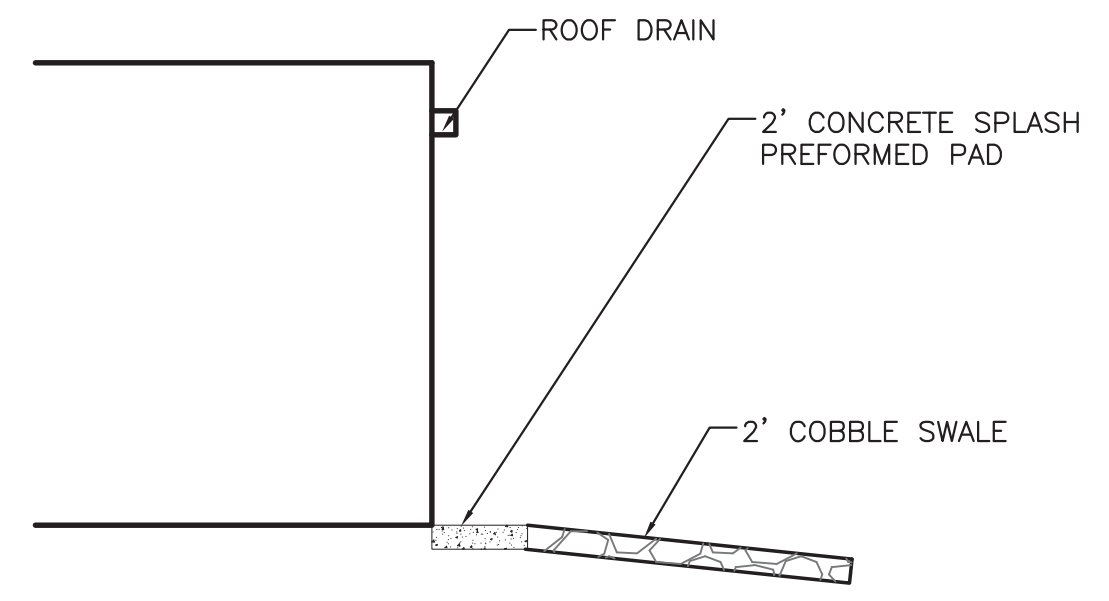
DESILTATION POND
TOP=6269.50
BOTTOM=6268.50
VOLUME=98 CF
3:1 SLOPES MAX
POND SIDES SHALL BE 4-6" COBBLES PLACED
12" DEEP OVER FILTER FABRIC, BOTTOM SHALL
BE RESEDED



EMERGENCY OVERFLOW DETAIL
NTS



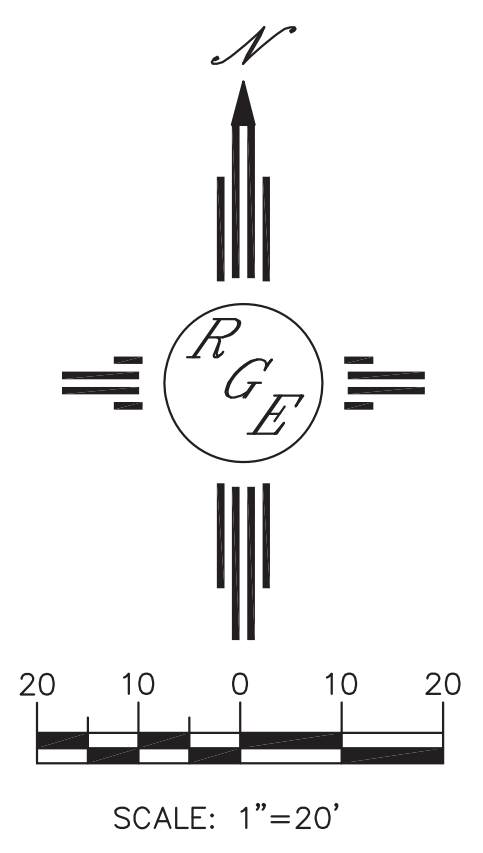
COBBLE SWALE DETAIL
NTS



ROOF DRAIN SPLASH PAD DETAIL
NTS

LEGEND

- XXXXX ----- EXISTING CONTOUR
- XXXXX ----- EXISTING INDEX CONTOUR
- XXXXX ----- PROPOSED CONTOUR
- XXXXX ----- PROPOSED INDEX CONTOUR
- XXXXX ----- SLOPE TIE
- XXXXX EXISTING SPOT ELEVATION
- + XXXXX PROPOSED SPOT ELEVATION
- BOUNDARY
- CENTERLINE
- RIGHT-OF-WAY
- ===== PROPOSED CURB AND GUTTER
- EXISTING EDGE OF ASPHALT
- ===== PROPOSED 1'-2' LANDSCAPE WALL
- ===== 2' COBBLE SWALE-SEE DETAIL THIS SHEET
- ===== BASIN LIMITS



ENGINEER'S SEAL DAVID SOULE P.E. #14522	13648 APACHE PLUME	DRAWN BY WCVJ
	GRADING AND DRAINAGE PLAN	DATE 8-12-19
8/25/19	 Rio Grande Engineering 1606 CENTRAL AVENUE SE SUITE 201 ALBUQUERQUE, NM 87106 (505) 872-0998	2109064-LAYOUT-8-12-19
		SHEET #
		JOB # 2109064