

# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

January 30, 2003

Ron Bohannan, PE Tierra West, LLC 8509 Jefferson NE Albuquerque, NM 87113

Re: Santa Fe Village Subdivision Drainage Report

Engineer's Stamp dated 12-17-02 (F10/D6A)

Dear Mr. Bohannan,

Based upon the information provided in your submittal dated 12-19-02, the above referenced report is approved for Preliminary Plat action by the DRB.

If you have any questions, you can contact me at 924-3986.

Sincerely,

Bradley L. Bingham, PE

Sr. Engineer, Planning Dept

Development and Building Services

C: file

# DRAINAGE REPORT

for

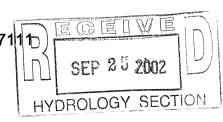
# Santa Fe Village

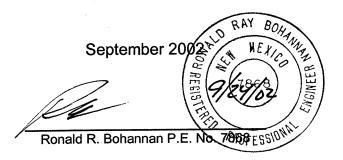
Prepared by

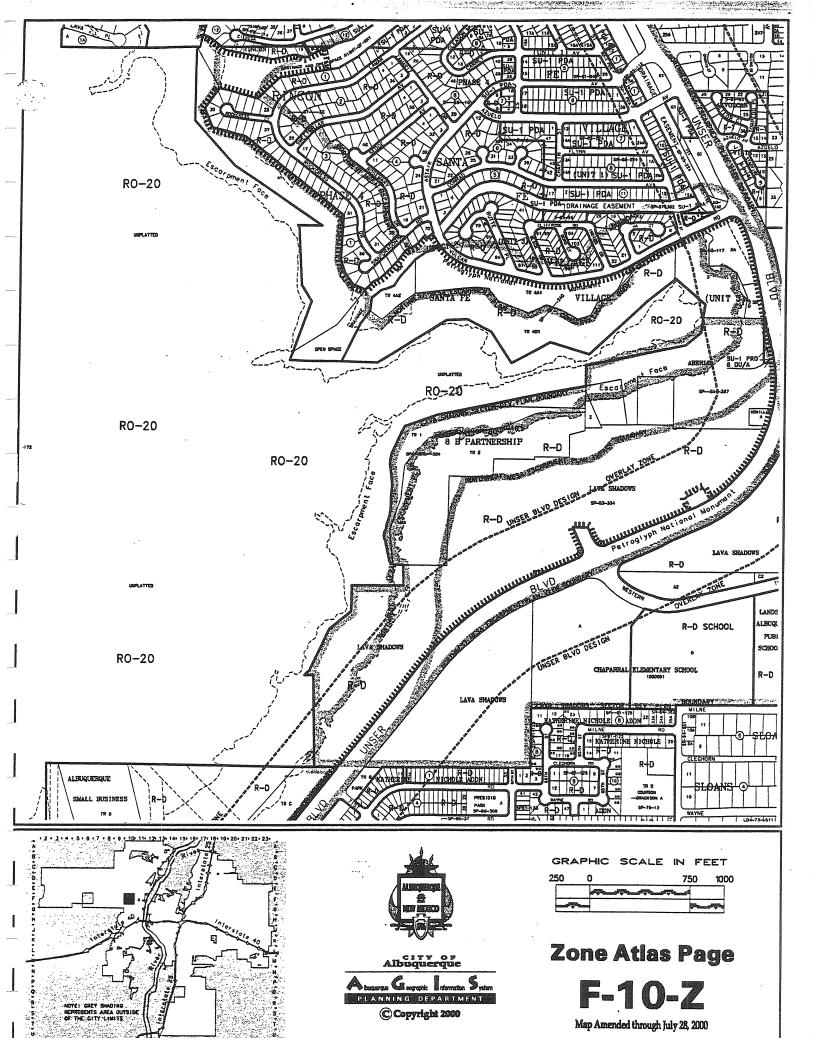
Tierra West, LLC 8509 Jefferson NE Albuquerque, New Mexico 87113

Prepared for

Grant Gist 8509 Canyon Run NE Albuquerque, New Mexico 871







### Location

The proposed residential project is on the west side of the City of Albuquerque located north of Vulcan Boulevard and east of Bogart Street and contains 0.51 acres. The site is shown and highlighted on the attached Zone Atlas Map F-10 and is identified as Tract 6A and 6B of Santa Fe Village. The purpose of this report is to provide the drainage analysis and management plan for this site.

# **Existing Drainage Conditions**

This site is currently undeveloped. There is one existing basin on the site with an existing runoff flow of 1.04 cfs. Currently this basin sheet flows north and east to the San Antonio Diversion Channel. There are no off-site flows entering the site. All upland flows are diverted by the nearby streets into the drainage channels. The channel has a concrete rundown on the west and an earth swale with basaltic or riprap sides.

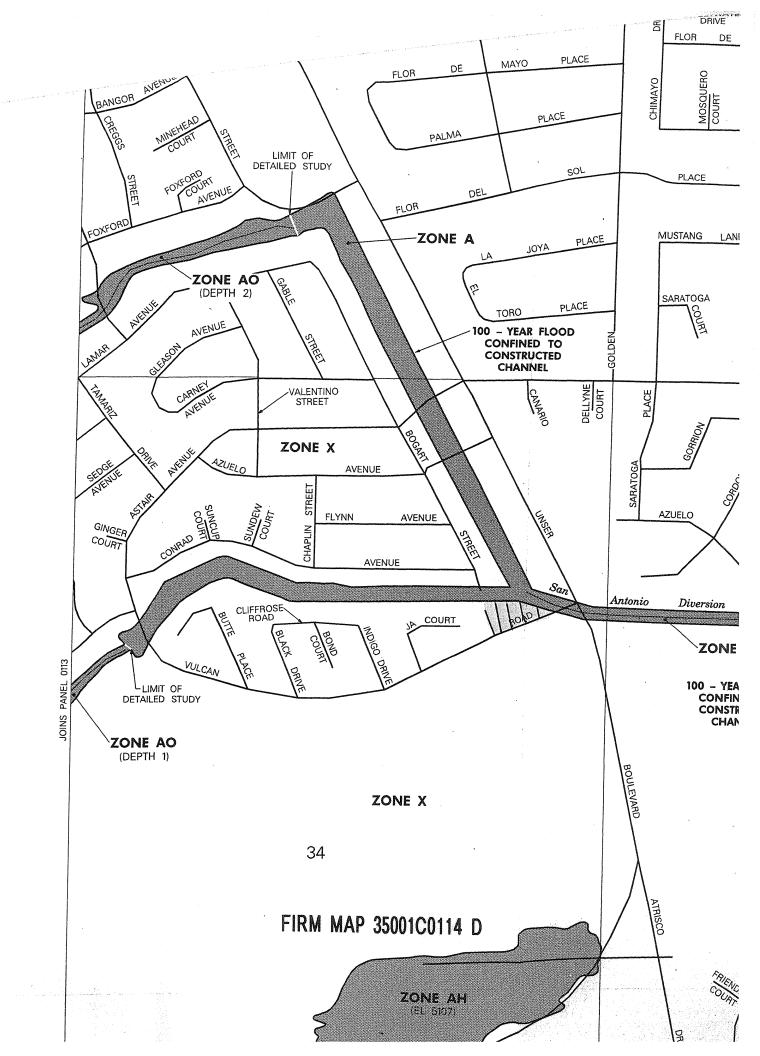
# **FEMA Map and Soil Conditions**

The site is located on FIRM Map 35001C0114 D as shown on the attached excerpt. The map shows that the site does not lie within any 100-year flood plains.

The site contains two different soil types from the Soil Conservation Service Survey of Bernalillo County. One soil is a Bluepoint-Kokan association; it has slow runoff and the hazard of water erosion is moderate or severe. The other nearby soil is a Kokan-Rock outcrop association. This soil occurs at the edge of the basalt mesa breaks on the West Mesa. It has a slight hazard of water erosion and rapid runoff.

# On-Site Drainage Management Plan

There are six proposed basins on the site. Basin 1 has a runoff flow of 0.77 cfs and consists the roof drainage and driveways for the lots fronting Bogart Street Basin 1 will drain to



Bogart Street. The runoff in Bogart Street will drain north to an existing drop inlet located in the street. Basin 2 drains to Vulcan Street with a developed flow of 0.88 cfs and consists of the lots fronting on that street. The runoff in Vulcan Road will drain east down the street to an existing drop inlet located at the corner of Vulcan and Unser. Basin 3 and 4 consist of the rear yards for two of the condos. Basin 4 has a flow of 0.03 cfs and drains north to blocks turned in the wall. The flow will enter Basin 3, which has a flow rate of 0.06 cfs, and the combined flows will drain north through the perimeter wall to the concrete channel adjacent to the site. This is a combined flow of 0.09 cfs. Basin 5, with a flow rate of 0.05 cfs, will also drain north to the adjacent concrete channel. Basin 6, with a flow rate of 0.01 cfs, will continue its natural drainage pattern and drain north of the San Antonio Diversion Channel.

The total developed flow from the site is 1.83 cfs, while the historic flow from the site is 1.04 cfs. The proposed flow is approximately 0.79 cfs greater than the historic flow. The flow in the Diversion Channel is being increased by such a small degree as will not be injurious to the downstream capacity. Therefore, we are recommending free discharge from the site.

# **Summary**

There are six proposed basins and no upland flows entering the site. Basins 1 and 2 will free discharge to Vulcan Boulevard and Bogart Street, and Basins 3, 4, 5, and 6 will drain to the adjacent channels.

# Weighted E Method

Developed On-Site Basins

Basin         Area         Area         Treatment A         Treatment B         Treatment C         Treatment C         Treatment D         Meighted E         Volume         Flow         Weighted E         Volume           1         (sf)         (acres)         %         <	20101	among our or podotog						-					•				
Area         Area         Treatment A         Treatment B         Treatment C         Treatment C         Treatment D         Treatment D         Weighted E         Volume (ac-ft)         Flow (acres)         Weighted E         Volume (ac-ft)         Flow (ac-ft)         Weighted E         Volume (ac-ft)         Meighted E         Volume (ac-ft)         Flow (ac-ft)         Weighted E         Volume (ac-ft)         Meighted E         Volume (ac-ft)         Ac-ft (ac-ft												100	-Year, 6-Hr		10	10-Year, 6-Hr	
(sf)         (acres)         %	Basin	Area	Area	Treatn	nent A	Treat	8	Treatr	nent C	Treat	nent D	Weighted E	Volume	Flow	Weighted E	Volume	Flow
8.592         0.20         0.6         0.04         0.00         80%         0.16         1.710         0.028         0.77         1.036           9,873         0.23         0.8         0.05         0.06         0.00         80%         0.18         1.710         0.032         0.88         1.036           1,308         0.03         0.04         0.05         0.00         0%         0.00         0%         0.00		(st)	(acres)	%	(acres)	%	(acres)	%	(acres)	%	(acres)		(ac-ft)	cfs		(ac-ft)	cts
8,592         0.20         0%         0.00         80%         0.16         1.710         0.028         0.77         1.036           9,873         0.23         0%         0.05         0%         0.00         80%         0.18         1.710         0.032         0.88         1.036           1,308         0.03         0%         0.04         0.00         0%         0.00         0%         0.00																	
9,873         0.23         0%         0.05         0.00         80%         0.18         1.710         0.032         0.88         1.036           1,308         0.03         0%         0.00         0%         0.00         0%         0.00 </td <td>-</td> <td>8,592</td> <td>0.20</td> <td>%0</td> <td>0</td> <td>70%</td> <td>0.04</td> <td>%0</td> <td>00.00</td> <td>%08</td> <td>0.16</td> <td>1.710</td> <td>0.028</td> <td>0.77</td> <td>1.036</td> <td>0.017</td> <td>0.49</td>	-	8,592	0.20	%0	0	70%	0.04	%0	00.00	%08	0.16	1.710	0.028	0.77	1.036	0.017	0.49
1,308         0.03         0,003         0.00         <	2	9,873	0.23	%	0	20%	0.05	%0		80%	0.18	1.710	0.032	0.88	1.036	0.020	0.56
537         0.01         0%         0.00         0%         0.00	3	1,308		%	0	100%	0.03	%0		%0	0.00	0.670	0.002	90.0	0.220	0.001	0.02
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857         0.02         0%         0.00         0%         0.00         0.00         0.001         0.04         0.220           22,324         0.51         0	2	1,158		%	0	100%	0.03	%0		%0	0.00	0.670	0.001	0.05	0.220	000.0	0.02
22,324 0.51 0 0.17 0.00 0.34 0.065 1.83\text{0.00}	9	857	0.02	%0	0	100%	0.02	%0		%0	0.00	0.670	0.001	0.04	0.220	000.0	0.01
	Total	22,324	0.51		0		0.17		00:00		0.34		0.065	1.83		0.038	1.11

Undeveloped On-Site Basins

		2			-											
											100	100-Year, 6-Hr		-01	10-Year, 6-Hr	
Basin	Basin Area Area Treatment A Treatme	Area	Treatr	reatment A	Treatme	tment B	Treatr	reatment C	Treat	Treatment D	Weighted E	Volume	Flow	Weighted E	Volume	Flow
	(sf)	(acres)	%	%   (acres)   %   (	%	(acres)	%	(acres)	%	(acres)		(ac-ft)	cts		(ac-ft)	cts
-	22,324	0.51	%0	0	100%	0.51	%0	00.0	%0	0.00	0.670	0.029	1.04	0.220	0.009	0.39

# Equations for Weighted E Method:

Weighted E = Ea\*Aa + Eb\*Ab + Ec\*Ac + Ed\*Ad / (Total Area)

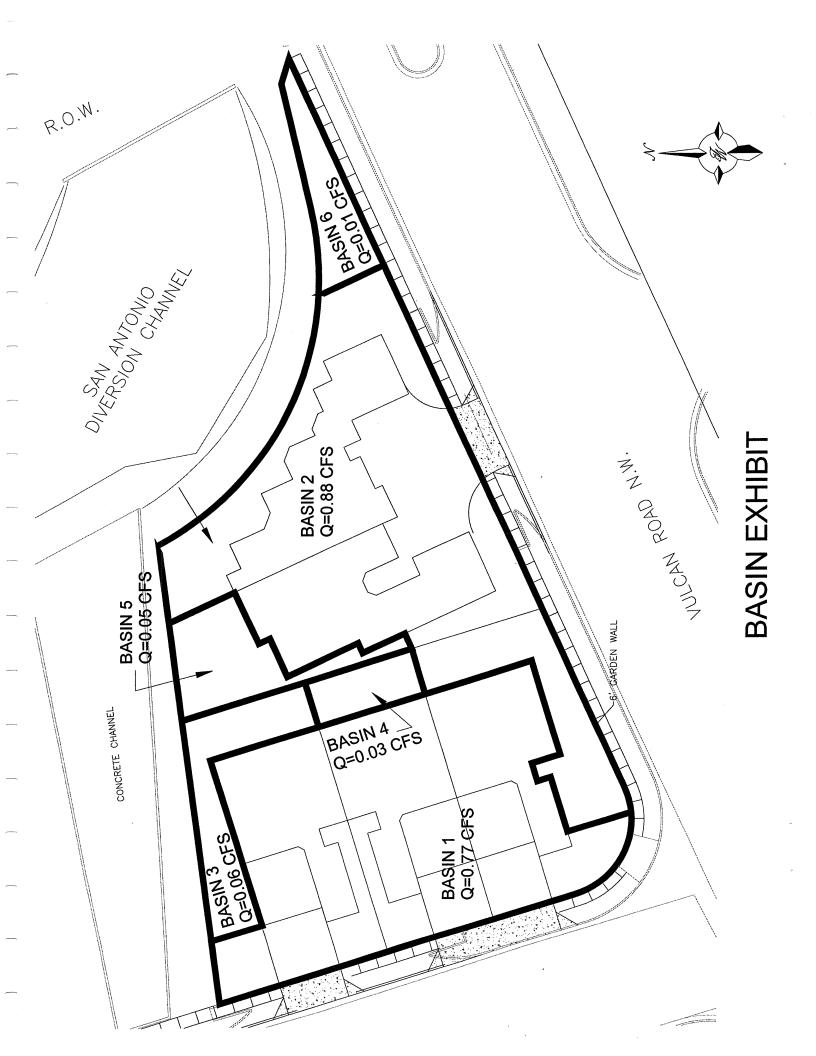
Volume = Weighted D \* Total Area

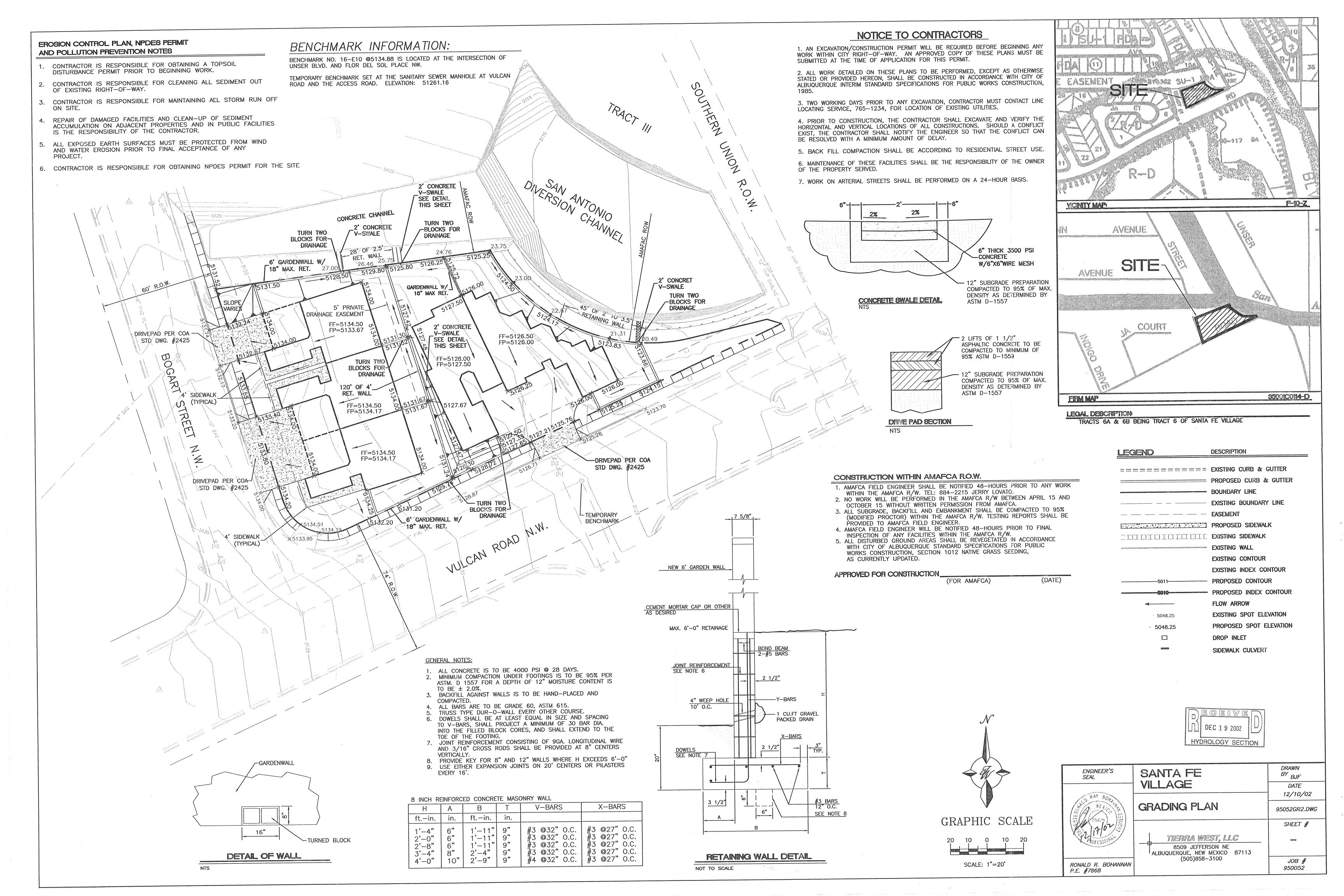
Flow = Qa \* Aa + Qb \* Ab + Qc \* Ac + Qd \* Ad

Volume (10-day) =  $V_{360}$  + Ad \* ( $P_{10days}$  -  $P_{360}$ )/ 12 in/ft

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-	eak Disch	Peak Discharge (cfs/acre)	re)
Zone 1	100-Year	10 - Year	2 - Year
ő ő	1.29	0.24	0
රී	2.03	0.76	0.03
ő	2.87	1.49	0.47
ď	4.37	2.89	1.69







Martin J. Chávez, Mayor

September 12, 1996

Ronald R. Bohannan, P.E. Tierra West Development 4421 McLeod Rd. NE, Suite D Albuquerque, New Mexico 87109

RE: GRADING AND DRAINAGE PLAN FOR TRACT 6 OF SANTA FE VILLAGE (F10/D6A)

SUBMITTED FOR SITE DEVELOPMENT FOR SUBDIVISION APPROVAL AND GRADING

PERMIT APPROVAL, ENGINEER'S STAMP DATED 8/15/96.

Dear Mr. Bohannan:

Based on the information provided in the submittal of August 20, 1996, the above referenced plan is approved for Site Development for Subdivision and for Grading Permit release.

Please be advised that the Engineer's Certification for this site is required prior to release of the Certificate of Occupancy.

If you should have any questions, or if I may be of further assistance to you, please call me at 768-2666.

Sincerely,

Susan M. Calongne, P.E.

City/County Floodplain Administrator

c: Andrew Garcia, City Hydrology File





Martin J. Chávez, Mayor

June 4, 1996

Ronald R. Bohannan, P.E. Tierra West Development 4600 Montgomery Blvd. NE, Suite 3 Albuquerque, New Mexico 87109

RE:

GRADING AND DRAINAGE TLAN FOR TRACT 6 OF SANTA FE VILLAGE (F10/D6A) SUBMITTED FOR SITE DEVELOPMENT FOR SUBDIVISION APPROVAL AND GRADING PERMIT APPROVAL, ENGINEER'S STAMP DATED 5/14/96.

# Dear Mr. Bohannan:

Based on the information provided in the submittal of May 15, 1996, the above referenced plan is approved for Site Development for Subdivision and for Grading Permit release.

Please be advised that the concrete rundown channel into the San Antonio arroyo must be constructed under Work Order. Work order plans must be approved prior to Building Permit release. This drainage structure must be certified with the grading and drainage prior to release of the Certificate of Occupancy.

If you should have any questions, or if I may be of further assistance to you, please call me at 768-2666.

Sincerely,

Susan M. Calongne, P.E.

City/County Floodplain Administrator

c: Andrew Garcia, City Hydrology File



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AHYMO Input and Summary Output

# DRAINAGE REPORT

for

# Santa Fe Village

# Prepared by

Tierra West Development Management Sevices 4421 McLeod Road NE, Suite D Albuquerque, New Mexico 87109

Prepared for

Grant E. Gist and Tom Muchmore Santa Fe Village Inc. 9943 E. Island Circle Scottsdale, Arizona 85258

May 1996

Ronald R. Bohannan P.E. Voy 78687 11

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# **On-Site Drainage Management Plan**

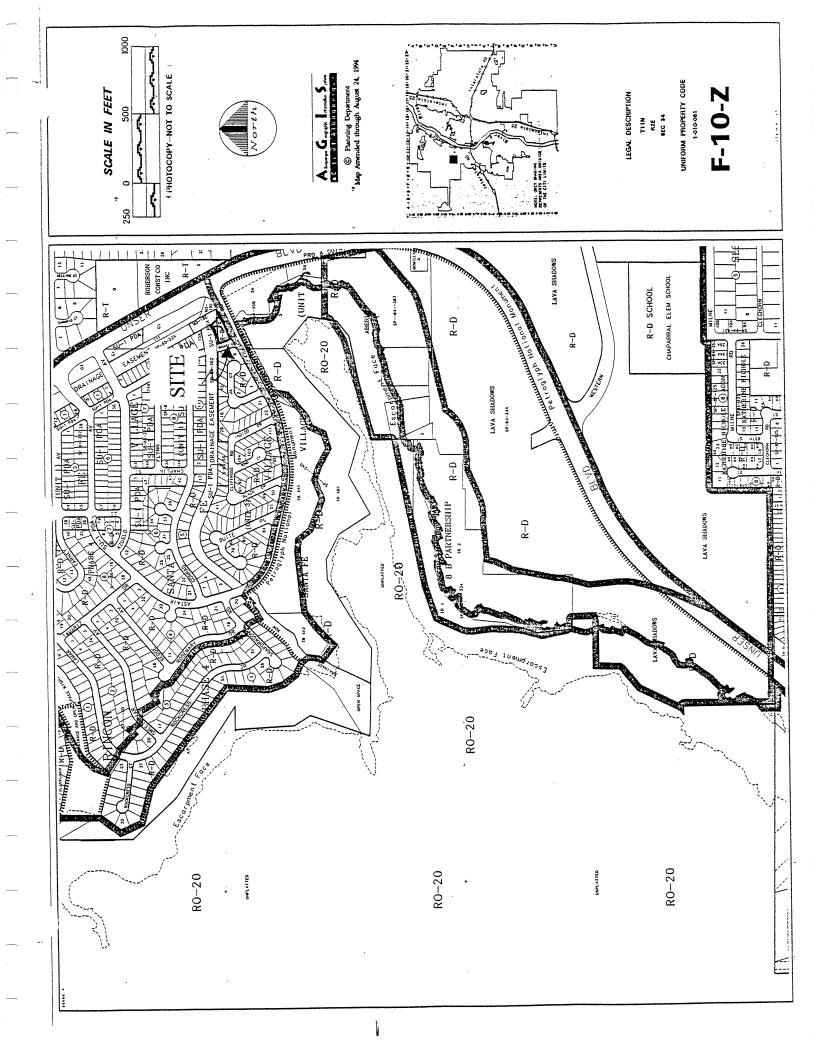
There are three proposed basins on the site. The roof drainage and the bulk of the flows drain to a paved parking lot designated as Basin 1. Basin 1 has a runoff flow of 1.52 cfs. This basin will drain to the northeast corner of the site into a proposed concrete channel that will route the flow into the existing San Antonio Diversion Channel. Basin 2 has a runoff flow of 0.18 cfs. It consists of a courtyard in the front of each condominium that will drain to Vulcan Street. In Basin 3 a small side yard and common landscape area will continue to flow north with a runoff flow of 0.15 cfs to the natural channel.

The combined flow from all three basins is 1.85 cfs. This flow is approximately 0.81 cfs greater than the historic flow. The flow in the Diversion Channel is being increased by such a small degree as will not be injurious to the downstream capacity. Therefore, we are recommending free discharge from the three points.

## **Summary**

There are three proposed basins and no upland flows entering the site. Basin 1 and Basin 3 will free discharge into the existing San Antonio Diversion Channel. Basin 2 will drain into Vulcan Road. — for Unsur?





# **Location and Location Map**

The proposed project is on the west side of the City of Albuquerque located north of Vulcan Road and east of Bogart Street and contains 0.5125 acres. The proposed project consists of seven condominiums totaling approximately 6680 SF of single family houses. The site is shown and highlighted on the attached Zone Atlas Map F-10 and is identified as Tract 6A and 6B of Santa Fe Village. The purpose of this report is to provide the drainage analysis and management plan for this site.

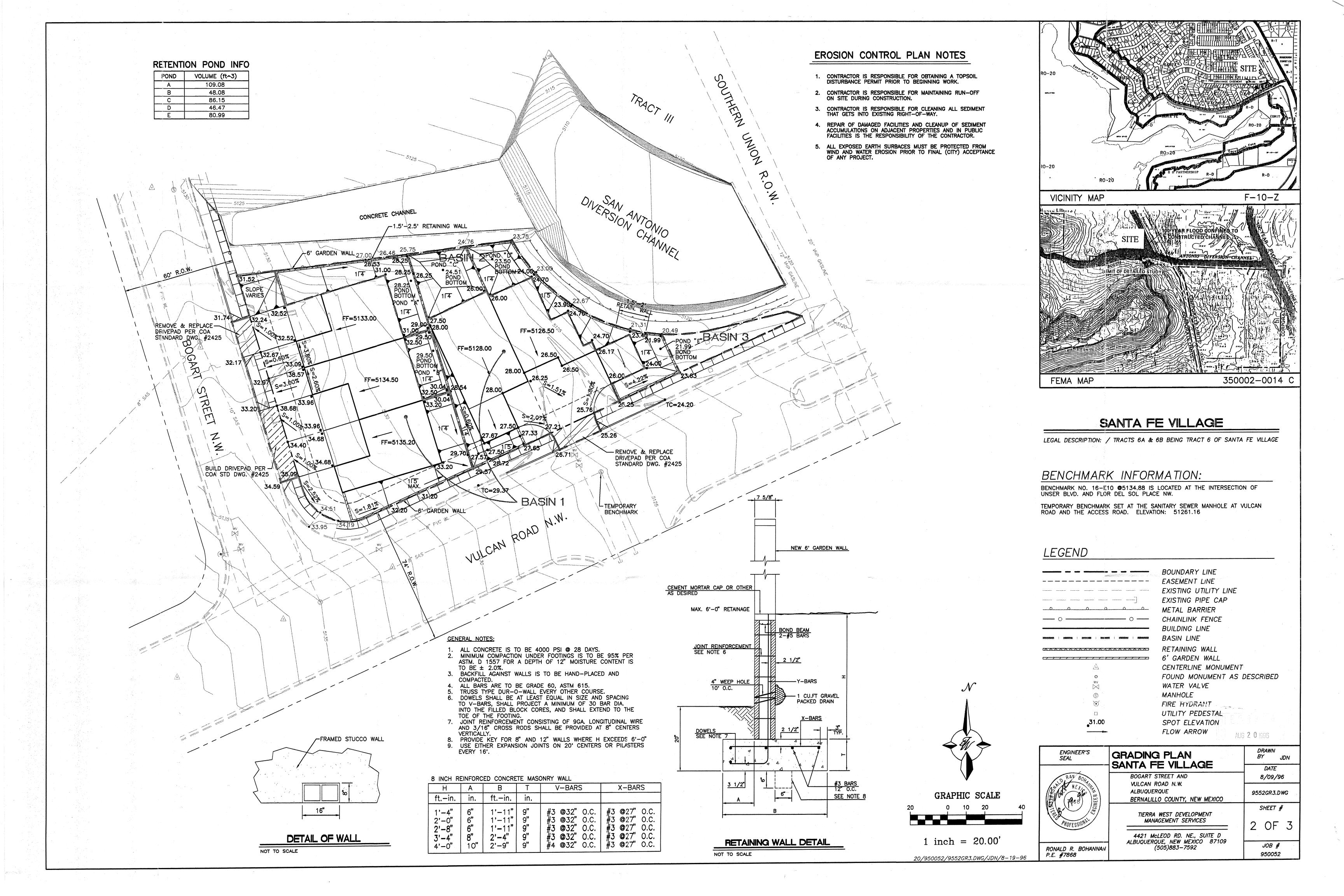
# **Existing Drainage Conditions**

This site is currently undeveloped. There is one existing basin on the site with an existing runoff flow of 1.04 cfs. This basin sheet flows east into the adjacent San Antonio Diversion Channel. There are no off-site flows entering the site. All upland flows are diverted by the nearby streets into the drainage channel. The channel has a concrete rundown on the west and an earth swale with basaltic or riprap sides. Currently this site sheet flows north and east to the natural channel.

# **FEMA Map and Soil Conditions**

The site is located on FEMA Map section 350002 panel 14 as shown on the attached excerpt. The map shows that the site does not lie within any 100 year flood plains.

The site contains two different soil types from the Soil Conservation Service Survey of Bernalillo County. One soil is a Bluepoint-Kokan association; it has slow runoff and the hazard of water erosion is moderate or severe. The other nearby soil is a Kokan-Rock outcrop association. This soil occurs at the edge of the basalt mesa breaks on the West Mesa. It has a slight hazard of water erosion and rapid runoff.





# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

Ken Schultz Mayor

UTILITY DEVELOPMENT DIVISION HYDROLOGY SECTION (505) 768-2650

August 20, 1987

Elvidio Diniz, P.E. Resource Technology, Inc. 2129 Osuna Road, NE Suite 2A Albuquerque, New Mexico 87113

RE: REVISED DRAINAGE PLAN OF SANTA FE VILLAGE UNIT III - COMMERCIAL AREA; RECEIVED AUGUST 6, 1987 FOR SITE DEVELOPMENT PLAN APPROVAL (F-10/D6A)

Dear Elvidio:

The above referenced submittal dated August 4, 1987, is approved for Site Development Plan sign-off by City Engineer.

Prior to Building Permit request, provide the following additional items:

- a breakdown of on-site area 4 land treatment to justify using a "C" value of 0.77 and RCN of 77.
- 2. detailed large scale drainage and grading plans for each building site.

If you have any questions, please call me at 768-2650.

Cordially,

Roger A. Green, P.E. C.E./Hydrology Section

cc: Coda Roberson

Walter Nickerson, P.E., City Engineer

RAG/bsj

**PUBLIC WORKS DEPARTMENT** 

**ENGINEERING GROUP** 

= AN EQUAL OPPORTUNITY EMPLOYER

Telephone (505) 768-2500

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# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

### DESIGN HYDROLOGY SECTION 123 Central NW, Albuquerque, NM 87102 (505) 766-7644

March 25, 1986

Elvidio Diniz, P.E. Resource Technology, Inc. 2620 San Mateo Blvd., NE Suite B Albuquerque, New Mexico 87110

RE: CONCEPTUAL GRADING & DRAINAGE PLAN SUBMITTAL OF SANTA FE VILLAGE, UNIT III COMMERCIAL AREA, RECEIVED MARCH 13, 1986 FOR SITE DEVELOPMENT PLAN APPROVAL (F-10/D6A)

Dear Elvidio:

The above referenced submittal, dated August 7, 1986 with revised drawing dated March 13, 1986, is approved for Site Development Plan.

A detailed Grading & Drainage plan will be required at time of Rough Grading and Building Permit approval, and when Platting actions or a work order require an approved infrastructure listing.

If you have any questions, call me at 766-7644.

Cordially,

Roger A. Green, P.E. C.E./Design Hydrology

cc: Coda Roberson

RAG/bsj

### **MUNICIPAL DEVELOPMENT DEPARTMENT**

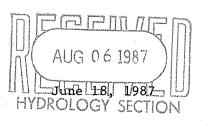
# DRAINAGE PLAN (REVISED) FOR SANTA FE VILLAGE UNIT III - COMMERCIAL AREA ALBUQUERQUE, NEW MEXICO

# prepared for

Roberson Construction Company 6001 Atrisco Road, NW Albuquerque, New Mexico 87120

# prepared by

Resource Technology, Incorporated 2129 Osuna Road NE, Suite 2A
Albuquerque, New Mexico 87113



# DRAINAGE PLAN FOR

# SANTA FE VILLAGE UNIT III - COMMERCIAL AREA

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## INTRODUCTION

This report discusses the grading and drainage plan for the proposed commercial area within Santa Fe Village Unit III subdivision, located in the City of Albuquerque, New Mexico. A previous drainage report titled "Drainage Plan for Santa Fe Village Unit III - Commercial Area Albuquerque, New Mexico (August 7, 1985)" was submitted to and approved by the City Design Hydrology Section. However, the site plan has changed since that report and therefore this report is submitted to reflect the changes to the drainage plan, and serve as the currently effective drainage report.

## SITE LOCATION AND DESCRIPTION

Figure 1 shows the location of the commercial area within Santa Fe Village Unit III subdivision which is located on Zone Atlas page F-10. The site is composed of two locations divided by Vulcan Road. The area located south of Vulcan Road is bounded on the east by Unser Blvd. NW (Atrisco Road NW) and on the west by the volcanic escarpment. The area located north of Vulcan Road is bounded on the north by a concrete drainage channel within a drainage right-of-way which varies from 54 ft. to 63 ft. wide. This channel conveys flows from the South Branch of San Antonio Arroyo into an AMAFCA gabion grade control structure located on the east side of the area.

Figure 2 shows the on- and off-site soil types for the commercial area. The Bluepoint-Kokan association covers the on-site area, and the Kokan-Rock outcrop association covers all of the off-site area. A general description of each soil type follows:

Bluepoint-Kokan association - About 50% Bluepoint loamy fine sand and 40% Kokan gravely sand. Hydrologic Soil Group A.

Kokan-Rock outcrop association - About 75% Kokan gravelly sand with 25% to 45% slopes with about 40% of the surface covered with large basalt boulders. This soil is located off-site on the volcanic escarpment. Hydrologic Soil Group A.

### Land Use

Boca Negra Park is located on top of the escarpment, as shown on the Grading and Drainage Plan Figure 6 of 6 (submitted as Figure 6 of 6 of the Site Development Plan to Environmental Planning Commission), and the escarpment area between the park and the proposed development area may become City open space. Consequently, no development is expected on the off-site areas contributing drainage to the study area.

The proposed development area is presently undeveloped with no structures; however, an AMAFCA gabion grade control structure is located in San Antonio Arroyo as shown on Figure 6 of 6, and Vulcan Road has been constructed with concrete curb and gutter and temporary paving. Also the present intersection with Unser is temporary.

The proposed commercial area is divided by Vulcan Road which is the main access into Santa Fe Village Unit III. A restaurant and a retail

building with offices on the second floor will be located on the south side of Vulcan Road; a convenience store may be built on the north side of Vulcan Road. The entire area (also contributing off-site area), including the restaurant, retail/office building and convenience store will drain into the AMAFCA gabion grade control structure.

The parking lot and drive area located between the retail/office building and Unser Blvd. NW (Atrisco Road NW) will be paved.

A hike/bike path is also planned along the base of the volcanic escarpment, and is shown as an open space trail on the drainage plan.

According to the Federal Emergency Management Agency flood hazard maps, the commercial area south of Vulcan Road is located in Flood Hazard Zone C and the commercial area north of Vulcan Road is located in Flood Hazard Zone AO. However, due to the existing concrete channel (discussed above) the 100-yr. flood will be confined within the channel and therefore the commercial site is not within the 100-yr. flood plain.

### OFF-SITE FLOWS

Three off-site drainage areas are located on the volcanic escarpment west of the proposed restaurant and retail/office buildings, and are named off-site drainage areas A, B, and C. Off-site Area D is a part of Vulcan Road located west of the trench drain (discussed with On-Site Flows) as shown on Figure 6 of 6. Off-site Area E contains the escarpment and half of Vulcan Road up to the south entrance to the

commercial area. Flow from Off-site Area E is considered in this drainage plan because its flow will be conveyed to the AMAFCA gabion grade control structure by means of a trench drain across Vulcan Road. This feature is discussed below with on-site flows.

Table 1 lists the areas, runoff coefficients and runoff curve numbers for all off-site areas. The time of concentration for the largest off-site area, Off-site Area E, is only 2.6 minutes; therefore, the other off-site areas which are much smaller than Off-site Area E, can be assumed to have a time of concentration less than 10 minutes for all off-site areas and therefore the Tc is assumed to be 10 minutes based on DPM criteria. Table 2 lists the peak discharges (using the rational method) and runoff volumes (using the SCS method) from each off-site area. No development can occur in Boca Negra Park or on the escarpment, and therefore future condition runoff will remain the same as for existing conditions.

The flow from off-site areas A, B, C and E will enter the project site as overland flow, except for some flow concentration from Off-site Area B. The edge of asphalt on the west side of the development and these off-site areas will be set to meet existing ground.

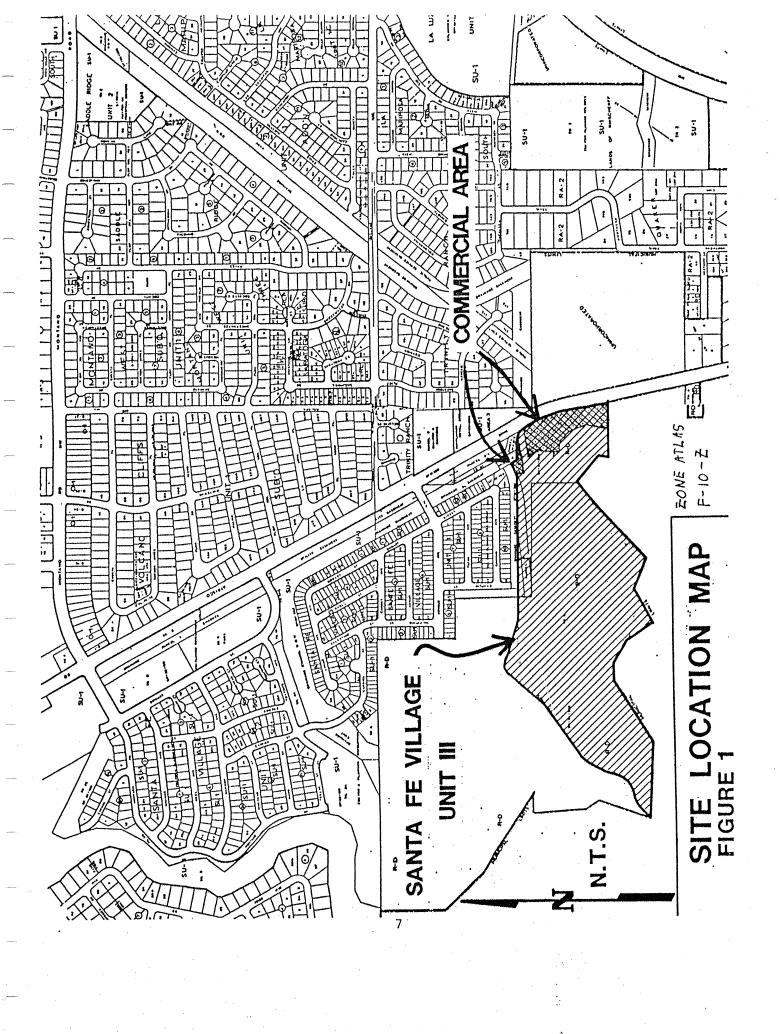
## ON-SITE FLOWS

The on-site area was divided into 6 drainage areas to accommodate the grading plan. These areas are designated Areas 1 through 6, and are shown on Figure 6 of 6, the Grading and Drainage Plan. Table 3 shows

the areas, runoff coefficients and runoff curve numbers for each area. The time of concentration for the longest flow path (areas 1,3 and 4), from the south end of the parking area near the restaurant to the northern retail/office building parking lot is 2.8 minutes. All other flow paths are shorter; therefore, the times of concentration for all other on-site areas were assumed to be less than 10 minutes. Based on DPM criteria, the time of concentration for all on-site areas is assumed to be 10 minutes. Table 4 lists the peak discharge and runoff volume for each on-site area.

The peak discharges and runoff volumes from the on- and off-site areas were analyzed at the analysis points (labled 10-18) shown on the drainage plan. Table 5 lists the peak discharge and runoff volumes at each analysis point based on the contributing areas. The flow data from contributing areas at each analysis point were added because the time of concentration for each contributing area is less than 10 minutes and therefore discharges from all areas are assumed to peak at the same time.

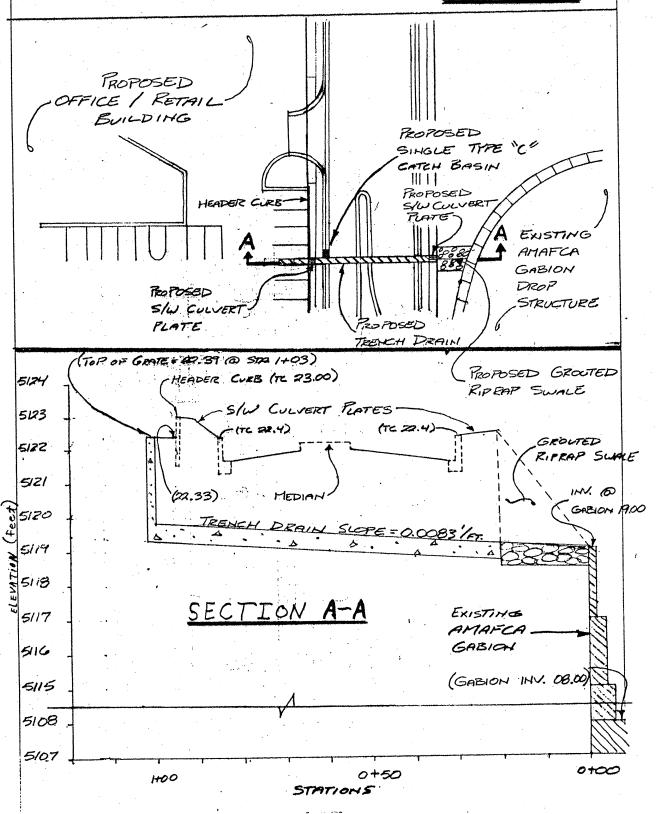
The off-site flows plus the on-site flows will drain down swales located in the drives of the restaurant and retail/office building to a 3-foot wide trench drain as shown on the drainage plan. The flow will enter the trench drain grate in the parking lot and the grate will continue to the back of sidewalk at Vulcan Road. Steel sidewalk culvert plates will cover the trench drain on both sides of Vulcan Road and the trench drain will cross the entire width of Vulcan Road. The trench

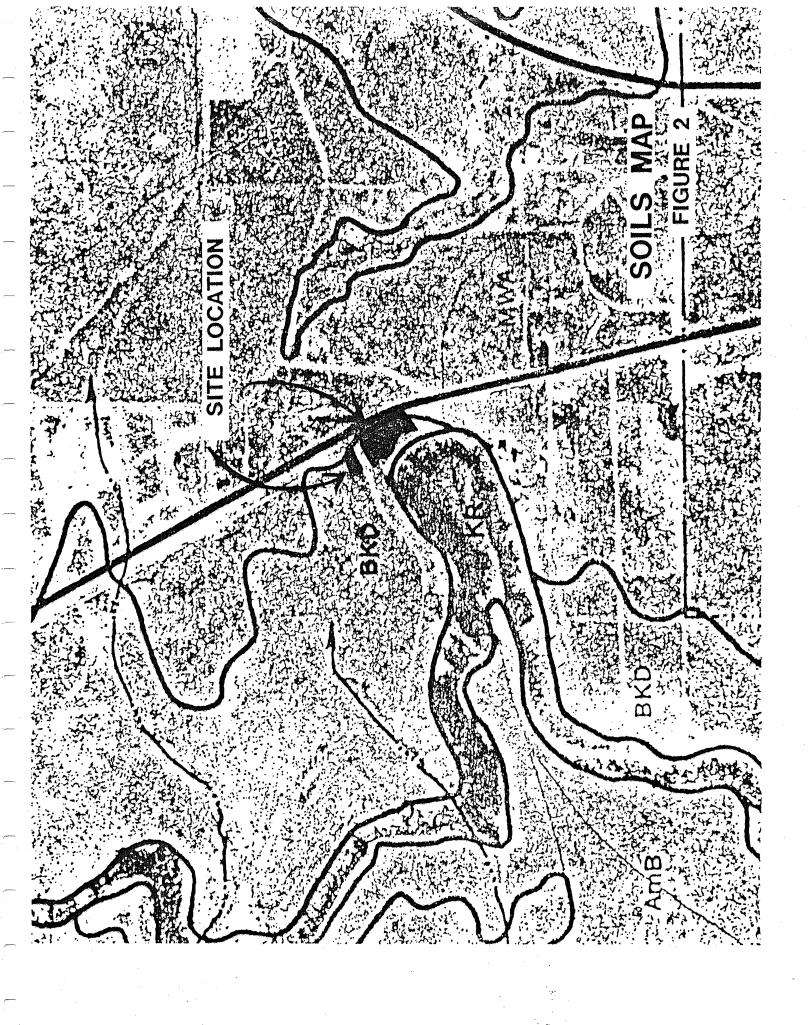


Varional

# TRENCH DRAIN DETAIL

# FIGURE 3





drain will outfall into a grouted rip-rap or concrete swale and discharge into the AMAFCA gabion grade control structure. A single Type "C" catch basin will be located at the south flow line of Vulcan Road adjacent and west of the trench drain and it will drain into the trench drain. This catch basin is necessary to ensure that the flow from Off-site Area E (and a portion of Area D) are intercepted and conveyed into the trench drain. Figure 3 shows a plan and profile sketch of the proposed trench drain.

A landscaped berm will be located between Unser Blvd. NW (Atrisco Road NW) and the parking area for the restaurant - retail/office building. This berm will have a maximum height of 6 feet, with maxmium side slopes of 3H:1V and will be landscaped with native plants.

Due to the steep grade near the escarpment behind the retail stores/offices and the large differences between existing ground and slab elevations, retaining walls will be required at the rear of the restaurant and retail/office buildings and several other locations as shown on Figure 6 of 6.

The flow characteristics at the analysis points shown on Figure 6 of 6 are listed in Table 6.

The invert of the proposed grouted rip-rap outfall will be located on top of the AMAFCA gabion grade control structure. The headwall soffit elevation of the box culverts (under Unser Boulevard) downstream from the gabion structure is approximately five feet below the outfall

elevation at Analysis Point 19. Consequently backwater effects are unlikely at the outfall (from flow in the gabion grade control structure).

