



City of Albuquerque

April 4, 2000

J. Graeme Means, PE
Jeff Mortensen & Associates, Inc
6010-B Midway Park Blvd. NE
Albuquerque, NM 87109

**Re: Rinconada Mesa Subdivision
Conceptual Grading and Drainage Plan
Engineer's Stamp dated 3-14-00 (F10/D10)**

Dear Mr. Means,

Based upon the information provided in your submittal dated 3-15-00, the above referenced conceptual grading and drainage plan is acceptable and is approved for Site Development Plan for Subdivision and Site Development Plan for Building Permit.

Please be advised that a more comprehensive grading and drainage submittal to Hydrology will be necessary prior to DRB approval of the Preliminary Plat. Please include a copy of the Infrastructure List with that submittal.

If you have any questions, you can contact me at 924-3986.

Sincerely,

Bradley L. Bingham
Bradley L. Bingham, PE
Hydrology Review Engineer

C: file



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

August 25, 2000

J. Graeme Means, P. E.
Jeff Mortensen & Associates, Inc.
6010-B Midway Park Blvd. NE
Albuquerque, New Mexico 87109

RE: *Grading and Drainage Plan for Rinconada Mesa Subdivision (F10/D10) Submitted for Preliminary and Final Plat Approval, Rough Grading and Work Order Approval, Engineer's Stamp Dated 8/2/00.*

Dear Mr. Means:

Based on the information provided, the above referenced Grading and Drainage Plan for Rinconada Mesa Subdivision dated August 2, 2000 is approved for Preliminary Plat action.

The above referenced plan is also approved for Rough Grading. As you are aware, a topsoil disturbance permit must be obtained before any grading may occur on this site.

Prior to Final Plat sign-off, the Subdivision Improvements Agreement (SIA) must be in place. The Grading and Drainage Certification is required prior to the release of the SIA, or financial guarantees, for this subdivision.

If you have any questions, or if I may be of further assistance to you, please call me at 924-3982.

Sincerely,

Susan M. Calongne, P.E.
City/County Floodplain Administrator

c: Arlan Collatz, Collatz, Inc.
DRB 1000352
File



City of Albuquerque

August 29, 2000

J. Graeme Means, P. E.
Jeff Mortensen & Associates, Inc.
6010-B Midway Park Blvd. NE
Albuquerque, New Mexico 87109

RE: *Grading and Drainage Plan for Rinconada Mesa Subdivision (F10/D10) Submitted for Preliminary and Final Plat Approval, Rough Grading and Work Order Approval, Engineer's Stamp Dated 8/2/00, Revised on 8/29/00.*

Dear Mr. Means:

Based on the information provided, the above referenced Grading and Drainage Plan for Rinconada Mesa Subdivision dated August 2, 2000 is approved for Preliminary Plat action.

The above referenced plan is also approved for Rough Grading. As you are aware, a topsoil disturbance permit must be obtained before any grading may occur on this site.

Prior to Final Plat sign-off, the Subdivision Improvements Agreement (SIA) must be in place. The Grading and Drainage Certification is required prior to the release of the SIA, or financial guarantees, for this subdivision.

If you have any questions, or if I may be of further assistance to you, please call me at 924-3982.

Sincerely,

Susan M. Calongne, P.E.
City/County Floodplain Administrator

c: DRB 1000352
File



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

April 15, 2002

J. Graeme Means, P.E.
Jeff Mortensen & Assoc..
6010-B Midway Park Blvd NE
Albuquerque, New Mexico 87109

RE: RINCONADA MESA SUBDIVISION (F-10/D10)
Engineers Certification For Release of Financial Guaranty
Engineers Stamp dated 8/2/2000 Rev. 8/29/2000
Engineers Certification dated 4/9/2002

Dear Mr. Means:

Based upon the information provided in your Engineers Certification dated 4/9/2002, the above referenced plan is adequate to satisfy the Grading and Drainage Certification for Release of Financial Guaranty for the above referenced project.

If you have any questions, please call me at 924-3981.

Sincerely,

Teresa A. Martin
Hydrology Plan Checker
Public Works Department
SAB

C: Arlene Portillo, PWD - #663981

File

CALCULATIONS

I. SITE CHARACTERISTICS

- A. PRECIPITATION ZONE = 1
- B. $P_{6,100} = P_{360} = 2.20$ IN.
- C. TOTAL AREA (A_T) = 743,360 SF (17.07 AC)
- D. EXISTING LAND TREATMENT
- | TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|-----|
| A | 743,360/17.07 | 100 |
- E. DEVELOPED LAND TREATMENT
- | TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|----|
| B | 149,755/3.44 | 20 |
| C | 149,755/3.44 | 20 |
| D | 443,850/10.10 | 60 |

II. EXISTING CONDITION

- A. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = 0.44 \text{ IN.}$$
- $$V_{100} = (E_W / 12) A_T$$
- $$V_{100} = (0.44 / 12) (743,360) = 27,260 \text{ CF}$$
- B. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = (1.29)(17.07) = 22.0 \text{ CFS}$$

III. DEVELOPED CONDITION

- A. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = [(0.67)(3.44) + (0.99)(3.44) + (1.97)(10.19)] / (17.07) = 1.51 \text{ IN.}$$
- $$V_{100} = (E_W / 12) A_T$$
- $$V_{100} = (1.51 / 12) (743,360) = 93,540 \text{ CF}$$
- B. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = (2.03)(3.44) + (2.87)(3.44) + (4.37)(10.19) = 61.4 \text{ CFS}$$

C. SITE BASIN CALCULATIONS

$$Q_{TOTAL} = 61.4 \text{ CFS}$$

$$A_{TOTAL} = 17.1 \text{ AC}$$

BASIN	AREA	%	Q_{100}
A	2.9 AC	17	10.4
B	2.1 AC	12	7.4
C	1.5 AC	09	5.5
D	2.5 AC	15	9.2
E	1.2 AC	07	4.3
F	2.4 AC	14	8.6
G	2.6 AC	15	9.2
H	1.7 AC	10	6.1
I	0.2 AC	01	0.7
TOTAL	17.1 AC	100	61.4

D. ANALYSIS POINT SUMMARY

ANALYSIS POINT	CONTRIBUTING BASINS	CONTRIBUTING AREA	Q_{100}
AP-1	L-1	0.2 AC	0.4 CFS
AP-2	L-1, F	2.6 AC	9.0 CFS
AP-3	L-1, F,G	5.2 AC	18.2 CFS
AP-4	D	2.5 AC	9.2 CFS
AP-5	D, E	3.7 AC	13.5 CFS
AP-6	L-1, D,E,F,G,H	10.6 AC	37.8 CFS
AP-7	L-2	1.5 AC	3.7 CFS
AP-8	L-2, B	3.6 AC	11.1 CFS
AP-9	L-3	0.1 AC	0.3 CFS
AP-10	L-3, A	3.0 AC	10.7 CFS
AP-11	L-2,L-3,A,B,C	8.1 AC	27.3 CFS

$$\text{TOTAL FLOW TO STORM DRAIN} = \text{AP 6} + \text{AP 11} = 37.8 + 27.3 = 65.1 \text{ CFS}$$

IV. OFFSITE FLOWS

- A. FUTURE DEVELOPMENT ON TRACT A-1
- $A = 14.76 \text{ AC}$
- ASSUME MULTIFAMILY ATTACHED (70 % D, 15 % C, 15 % B)
- $$Q_{100} = (2.03)(2.20) + (2.87)(2.20) + (4.37)(10.27) = 55.7 \text{ CFS}$$
- B. BASIN L-1
- $$A_T = 8,000 \text{ SF}/0.18 \text{ AC}; A_B = 7,000 \text{ SF}/0.16 \text{ AC};$$
- $$A_D = 1,000 \text{ SF}/0.02 \text{ AC}$$
- $$Q_{100} = (2.03)(0.16) + (4.37)(0.02) = 0.4 \text{ CFS}$$
- C. BASIN L-2
- $$A_T = 63,500 \text{ SF}/1.46 \text{ AC}; A_B = 49,500 \text{ SF}/1.14 \text{ AC};$$
- $$A_D = 14,000 \text{ SF}/0.32 \text{ AC}$$
- $$Q_{100} = (2.03)(1.14) + (4.37)(0.32) = 3.7 \text{ CFS}$$
- D. BASIN L-3
- $$A_T = 6,100 \text{ SF}/0.14 \text{ AC}; A_B = 5,300 \text{ SF}/0.12 \text{ AC};$$
- $$A_D = 800 \text{ SF}/0.02 \text{ AC}$$
- $$Q_{100} = (2.03)(0.12) + (4.37)(0.02) = 0.3 \text{ CFS}$$

V. COMPARISON

$$\Delta V_{100} = 93,540 - 27,250 = 66,290 \text{ CF (INCREASE)}$$

$$\Delta Q_{100} = 61.4 - 22.0 = 39.4 \text{ CFS (INCREASE)}$$

VI. STREET CAPACITY CALCULATIONS

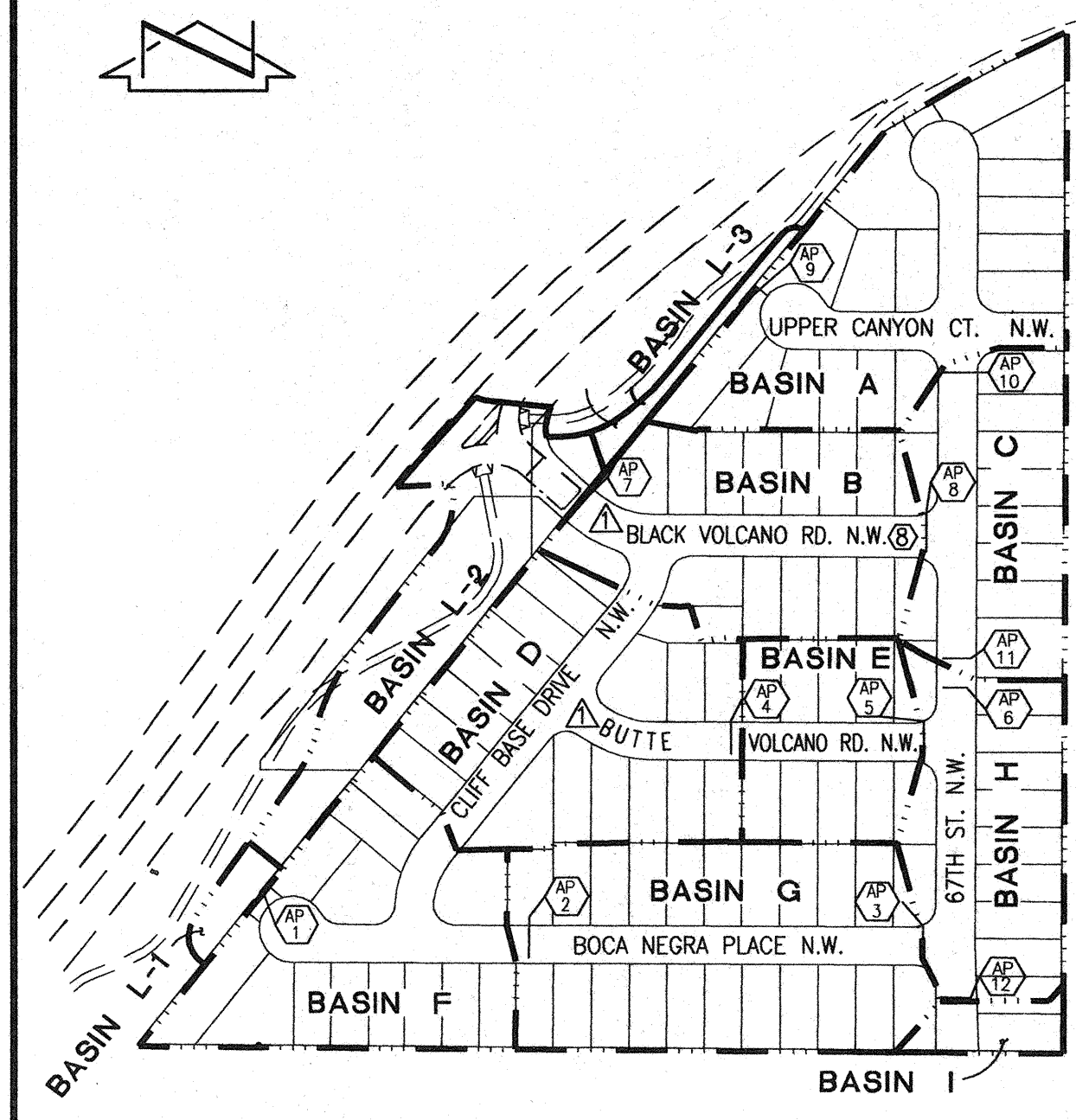
- (USING HAESTAD FLOWMASTER 6.0, MANNINGS EQUATION: $n = 0.017$)
- A. AP-2: $Q_{100} = 9.0 \text{ CFS}; s = 0.0050$;
- SECTION C-C: 28' F-F W/MOUNTABLE CURB AND GUTTER (ROLL TYPE);
DEPTH = 0.33 FT (CURB HEIGHT)
F = 0.80 (SUBCRITICAL: NO HYDRAULIC JUMP POTENTIAL)
OKAY TO USE MOUNTABLE CURB AND GUTTER (ROLL TYPE) UPSTREAM FROM AP-2;
USE STANDARD CURB AND GUTTER DOWNSTREAM FROM AP-2
- B. AP-4: $Q_{100} = 9.2 \text{ CFS}; s = 0.0060$;
- SECTION C-C: 28' F-F W/MOUNTABLE CURB AND GUTTER (ROLL TYPE);
DEPTH = 0.33 FT (CURB HEIGHT)
F = 0.88 (SUBCRITICAL: NO HYDRAULIC JUMP POTENTIAL)
OKAY TO USE MOUNTABLE CURB AND GUTTER (ROLL TYPE) UPSTREAM FROM AP-4;
USE STANDARD CURB AND GUTTER DOWNSTREAM FROM AP-4
- C. AP-8: $Q_{100} = 11.1 \text{ CFS}; s = 0.0100$;
- SECTION A-A: 32' F-F W/MOUNTABLE CURB AND GUTTER (ROLL TYPE);
DEPTH = 0.32 FT (LESS THAN CURB HEIGHT)
F = 1.11 (SUPERCRITICAL: SPECIFIC ENERGY = 0.41 FT < R.O.W. HEIGHT)
OKAY TO USE MOUNTABLE CURB AND GUTTER (ROLL TYPE) UPSTREAM FROM AP-8;
USE STANDARD CURB AND GUTTER DOWNSTREAM FROM AP-8
- D. AP-6: $Q_{100} = 37.8 \text{ CFS}; s = 0.0050$;
- SECTION B-B: 28' F-F W/STANDARD CURB AND GUTTER;
DEPTH = 0.62 FT (LESS THAN CURB HEIGHT);
F = 0.92 (SUBCRITICAL: NO HYDRAULIC JUMP POTENTIAL)
- E. AP-11: $Q_{100} = 27.3 \text{ CFS}; s = 0.0050$;
- SECTION B-B: 28' F-F W/STANDARD CURB AND GUTTER;
DEPTH = 0.55 FT (LESS THAN CURB HEIGHT);
F = 0.89 (SUBCRITICAL: NO HYDRAULIC JUMP POTENTIAL)

VII. SUMP CONDITION

- A. INLET CALCULATIONS
- METHODOLOGY FROM U.S.D.O.T., FEDERAL HIGHWAY ADMINISTRATION URBAN DRAINAGE MANUAL (HEC-22)
- $$Q_{100} = 65.1 \text{ CFS}$$
- MAXIMUM DEPTH = CURB HEIGHT = 0.90 FT (MEASURED FROM TC TO GRATE)
- 6 INLETS TOTAL (4 SINGLE A, 2 DOUBLE C)
- ASSUME GRATES 100 % CLOGGED (WORST CASE)
- ALL FLOWS WILL ENTER CURB OPENINGS
- $$Q_i = 0.67 h L (2g d_o)^{0.5} \text{ (EQ. 4-35)}$$
- h_i = CURB OPENING HEIGHT = 0.5 FT
 L = LENGTH OF OPENING = 7 FT (PER INLET)
- $$g = 32.2 \text{ FT/S}^2$$
- d_o = EFFECTIVE DEPTH AT CENTER OF OPENING = 0.65 FT
- $$Q_i = 15.2 \text{ CFS PER INLET}$$
- 15.2 CFS/INLET x 6 INLETS = 91.2 CFS > Q_{100}
- B. EMERGENCY OVERFLOW CALCULATIONS
- BECAUSE INLETS ARE LOCATED AT A SUMP CONDITION, AN EMERGENCY OVERFLOW IS PROVIDED WITHIN 67TH STREET. OVERFLOW SPILLWAY CALCULATIONS WERE PERFORMED USING HAESTAD FLOWMASTER 6.0 ASSUMING A BROAD CRESTED WEIR AS FOLLOWS:
- MAX HEADWATER ELEVATION = 5115.35 (MAX W.S.L.)
CREST ELEVATION = 5114.50 (FL @ OVERFLOW)
TAILWATER = 5114.82 (FL + 1.00' @ SOUTH BOUNDARY)
CREST SURFACE: PAVED
CREST LENGTH: 30' (STREET WIDTH @ OVERFLOW)
DISCHARGE = 71.3 CFS (> Q_{100})

VIII. OFFSITE DOWNSTREAM STORM DRAIN CALCULATIONS (MANNING'S EQUATION)

- A. 42" RCP @ $S = 0.0037$
- $n = 0.013$
- $$Q_{100} = 65.1 \text{ CFS}$$
- $$Q_{CAP} = 65.8 \text{ CFS} > Q_{100}$$
- B. 54" RCP @ $S = 0.0035$
- $n = 0.013$
- $$Q_{100} = 65.1 + 55.7 = 120.8 \text{ CFS}$$
- $$Q_{CAP} = 125.1 \text{ CFS} > Q_{100}$$



BASIN MAP WITH ANALYSIS POINTS

SCALE : 1" = 200'

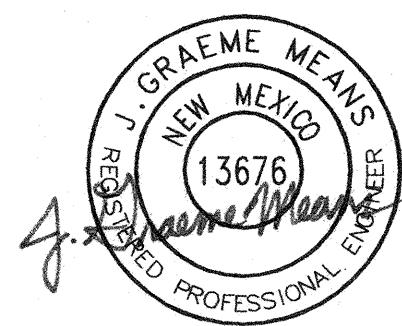
SITE BASIN CALCULATIONS

BASIN	AREA	%	Q_{100}
A	2.9 AC	17	10.4
B	2.1 AC	12	7.4
C	1.5 AC	09	5.5
D	2.5 AC	15	9.2
E	1.2 AC	07	4.3
F	2.4 AC	14	8.6
G	2.6 AC	15	9.2
H	1.7 AC	10	6.1
I	0.2 AC	01	0.7
TOTAL	17.1 AC	100	61.4

ANALYSIS POINT SUMMARY

ANALYSIS POINT	CONTRIBUTING BASINS	CONTRIBUTING AREA	Q_{100}
AP-1	L-1	0.2 AC	0.4 CFS
AP-2	L-1, F	2.6 AC	9.0 CFS
AP-3	L-1, F,G	5.2 AC	18.2 CFS
AP-4	D	2.5 AC	9.2 CFS
AP-5	D, E	3.7 AC	13.5 CFS
AP-6	L-1, D,E,F,G,H	10.6 AC	37.8 CFS
AP-7	L-2	1.5 AC	3.7 CFS
AP-8	L-2, B	3.6 AC	11.1 CFS
AP-9	L-3	0.1 AC	0.3 CFS
AP-10	L-3, A	3.0 AC	10.7 CFS
AP-11	L-2,L-3,A,B,C	8.1 AC	27.3 CFS

$$\text{TOTAL FLOW TO STORM DRAIN} = \text{AP 6} + \text{AP 11} = 37.8 + 27.3 = 65.1 \text{ CFS}$$



JEFF MORTENSEN & ASSOCIATES, INC.
680-B MIDWAY PARK BLVD. NE
ALBUQUERQUE, NEW MEXICO 87109
ENGINEERS SURVEYORS (505) 345-4250

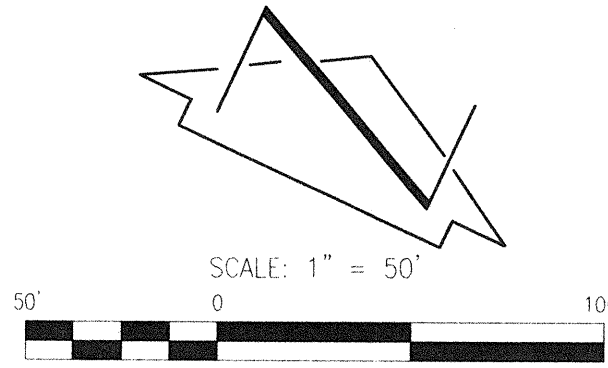
CALCULATIONS AND BASIN MAP RINCONADA MESA

DESIGNED BY J.M.
DRAWN BY J.Y.R.
APPROVED BY J.G.M.

NO.	DATE	BY	REVISIONS
1	08/00	G.M.	REVISE STREET NAMES

JOB NO.	2000.016.2
DATE	08-2000
SHEET	2 OF 6

File Path: E:\JMM\A\2000\06\A
Plot Date: 08-29-2000
File Name: 01626RT.DWG
Plot Time: 07:57 am



NOTE:
THIS IS NOT A BOUNDARY SURVEY. APPARENT PROPERTY CORNERS ARE SHOWN FOR ORIENTATION ONLY. BOUNDARY AND TOPOGRAPHIC INFORMATION SHOWN IS BASED UPON THE BOUNDARY AND TOPOGRAPHIC SURVEY PERFORMED BY JEFF MORTENSEN AND ASSOC. ON AUGUST 26, 1999.

ROUGH GRADING APPROVAL
CITY HYDROLOGY _____ DATE _____

- LEGEND**
- INC IN CONCRETE
 - CMU CEMENT MASONRY UNIT
 - BW BARBED WIRE
 - CLF CHAINLINK FENCE
 - EC ELECTRIC CABINET
 - EM ELECTRIC METER
 - EPB ELECTRIC PULL BOX
 - FH FIRE HYDRANT
 - MLP METAL LIGHT POLE
 - MSG METAL SIGN, GENERAL
 - SCF SMALL CONIFEROUS TREE
 - SDT SMALL DECIDUOUS TREE
 - SOP STEEL GUARD POST
 - SCV SPRINKLER CONTROL VALVE
 - NG NATURAL GRADE
 - TC TOP OF CURB
 - TA TOP OF ASPHALT
 - EA EDGE OF ASPHALT
 - FL FLOWLINE
 - TCO TOP OF CONCRETE
 - TW TOP OF WALL
 - TR TELEPHONE RISER
 - TS TRAFFIC SIGN
 - WCR WHEELCHAIR RAMP
 - WV WATER VALVE
 - WHB WATER HOT BOX
 - WM WATER METER
 - WMH WATER MANHOLE
 - WPP WOOD POWER POLE
 - WVA WATER VAULT
 - PATT CONC. PATTERNED CONCRETE
 - SASM SANITARY SEWER MANHOLE
 - SS SANITARY SEWER
 - SD STORM DRAIN
 - + 16.20 EXISTING SPOT ELEVATION
 - EXISTING CONTOUR
 - EXISTING DECIDUOUS TREE
 - EXISTING CONIFEROUS TREE
 - EXISTING TREE, SHRUB LINE
 - 58.45 PROPOSED SPOT ELEVATION
 - 57 PROPOSED CONTOUR
 - PROPOSED RETAINING WALL
 - PROPOSED CMU WALL (CAN RETAIN 18" MAX)
 - PROPOSED SLOPE
 - HIGH POINT
 - ROOF DRAINAGE
 - FLOWLINE
 - STANDARD (8") CURB AND GUTTER
 - MOUNTABLE CURB AND GUTTER (ROLL TYPE)
 - DRAINAGE BASIN BOUNDARY
 - PROPOSED CONCRETE RETAINING CURB (CAN RETAIN UP TO 18")
 - TOP OF GARDEN WALL ELEV
 - TOP OF RETAINING WALL ELEV
 - ANALYSIS POINT
 - MAX LIMITS OF EMERGENCY OVERFLOW WATER SURFACE LEVEL

NOTE:
CMU WALL MAY BE CONSTRUCTED IN LIEU OF RETAINING CURB.



DRB PROJECT
1000352



JEFF MORTENSEN & ASSOCIATES, INC.
6010-B MIDWAY PARK BLVD., N.E.
ALBUQUERQUE, NEW MEXICO 87109
ENGINEERS & SURVEYORS (505) 345-4250

**GRADING PLAN
RINCONADA MESA**

DESIGNED BY	NO.	DATE	BY	REVISIONS	JOB NO.
G.M.	1	08/00	G.M.	REVISE STREET NAMES, EXTEND SIDEWALK CULVERT, ADD EMERGENCY WATER LIMITS SHADING, ADD "MIN" TO RETAINING WALL ELEVATIONS, SHOW FUTURE DRIVEPAD LOCATIONS	2000.016.2
DRAWN BY			J.Y.R.		DATE
APPROVED BY			J.G.M.		08-2000
					SHEET 3 OF 6