

Esperia Subdivision

(Sevilla Ave. / Coors Blvd.)

Traffic Impact Study

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FINAL

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Presented to:

**Transportation Development Division
City of Albuquerque
&
New Mexico Department of Transportation
District No. 3**

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Esperia Subdivision (Sevilla Ave. / Coors Blvd.) TRAFFIC IMPACT STUDY

STUDY PURPOSE

The study is being conducted in conjunction with a request for approval of a site development plan proposing a new commercial / residential development as shown conceptually in the Appendix (Page A-2) of this report. The purpose of this study is to identify the impact of the Development on the adjacent transportation system, and to make recommendations to mitigate any significant adverse impact on the adjacent transportation system resulting from the implementation of the site development plan. This study is being prepared to meet the requirements of the City of Albuquerque Transportation Development Section and the New Mexico Department of Transportation, District 3.

STUDY PROCEDURES

A scoping meeting was held on Wednesday, February 08, 2006 with City of Albuquerque staff (Tony Loyd and Steele Nowak) and with the New Mexico Department of Transportation (Tony Abbo) prior to beginning the study to discuss scope and methodology to be utilized within the report. Tony Loyd summarized the meeting and defined the requirements and procedures for the study his in letter dated February XX, 2006 (See Appendix Page A-xx thru A-xx). Specific items included format, intersections to be studied, intersection analysis procedures, existing traffic counts, trip distribution methodology, and implementation year definition. A horizon year analysis was not required for this study.

The basic procedure followed is described as follows:

- 1) Calculate the generated trips for the proposed development consisting of the following described lane uses:
 - *An approximately 67-unit residential subdivision consisting of single family detached homes.*
 - *Approximately 89 residential / townhomes*
 - *Approximately 39,200 S.F. of retail commercial building floor space.*
 - *Approximately 16,800 S.F. of general office space.*
- 2) Calculate trip distribution for the newly generated trips by this development. The new commercial trips will be distributed based on year 2009 population within a two (2) mile radius boundary of the proposed site as shown on Appendix Page 11 of this report. The new Residential trips will be distributed based on year 2009 employment citywide inversely proportional to the distance of the employment subarea from the proposed project location. The new Office trips will be distributed based on year 2009 population citywide inversely proportional to the distance of the population subarea from the proposed project location
- 3) Determine Trip Assignments for the newly generated trips based on the results of the Trip Distribution Analysis and logical routing to and from the site.
- 4) Acquire recent traffic counts for the intersections of Western Trail / Coors Blvd., Dellyne Ave. / Coors Blvd., and Montano Rd. / Coors Blvd.

- 5) Acquire forecast traffic count data for the intersection of Sevilla Ave. / Coors Blvd. from the Andalucia South Traffic Impact Study by BHI.
- 6) Calculate historic growth rates based on 2000 – 2004 Traffic Flow Data from the Mid-Region Council of Governments (MRCOG).
- 7) Consider trips generated from the following recently approved developments that have no been fully implemented at this time
 - a) Andalucia, Phase 1
 - b) Andalucia, Tract 6
 - c) La Luz del Oeste Subdivision.
- 8) Determine 2009 NO BUILD Volumes by growing the existing turning movement counts to the year 2009 utilizing the appropriate annual historic growth rate for the area, and then adding in generated traffic volumes from the other three approved projects.
- 9) Add in data from Trip Assignments Maps and Tables to the 2009 NO BUILD Volumes to obtain 2009 BUILD Volumes for this project. The 2009 BUILD Volumes will include 100% of the traffic generated by the proposed Esperia Development (residential, commercial, and office components).
- 10) Provide signalized and / or unsignalized intersection analyses for the following intersections:

INTERSECTION	TYPE CONTROL	NO BUILD	BUILD
1) Western Trail / Coors Blvd.	Traffic Signal	2009	2009
2) Dellyne Ave. / Coors Blvd.	Traffic Signal	2009	2009
3) Montano Rd. / Coors Blvd.	Traffic Signal	2009	2009
4) Sevilla Ave. / Coors Blvd.*	Traffic Signal	2009	2009
5) Maduri Ave. / Vidal Dr.	Stop Sign	2009	2009

* New Signal installed with Andalucia, Phase 1

PREVIOUS RELATED TRAFFIC IMPACT STUDIES

Base data for this Traffic Impact Study were obtained from the previous Traffic Impact Study:

- 1) *Andalucia (South) Traffic Impact Study* by BHI.
- 2) *Andalucia, Tract 6 Traffic Impact Study (DRAFT)* by Terry O. Brown, P.E. dated October 31, 2004.
- 3) *La Luz del Oeste (no Traffic Study available – this study will determine trip distribution and trip assignments.)*.

The Implementation Year Trips Generated Volumes from those reports were added into the 2009 Background Subtotal Volumes in this report to obtain the 2009NO BUILD Volumes.

GENERAL AREA CHARACTERISTICS

The proposed requested site development plan are for an approximately 29.5 acre tract of land located along the west side of Coors Blvd. at the new Sevilla Ave. which runs east from Coors Blvd. through the new Andalucia South Residential Development. Properties surrounding this site are mostly residential uses. More detailed zoning

information may be obtained upon inspection of the Vicinity Maps on Page A-1 in the Appendix.

AREA STREET NETWORK

Coors Boulevard is classified as a Principal Arterial roadway south of Alameda Blvd. on the Long Range Roadway System for the Albuquerque Urban Area. It is currently a six lane paved urban roadway with no curbs and gutters on either side of the roadway and raised medians in the center. There is a paved shoulder on each side of Coors Blvd.

Montano Road is classified as a Limited Access Principal Arterial Roadway on the Long Range Roadway System for the Albuquerque Urban Area. It is a four lane paved urban section roadway with curbs and gutters on both side of the street and a raised median. The posted speed limit on Montano Rd. from Taylor Ranch Rd. to Coors Blvd. is 40 MPH.

Dellyne Ave. is classified as a Collector Street on the Long Range Roadway System for the Albuquerque Metropolitan Area. It is currently a two lane paved facility west of Coors Blvd.

Western Trail west of Coors Blvd. is classified as a Minor Arterial roadway on the Long Range Roadway Plan for the Albuquerque Urban Area. It is currently a four lane paved facility to the west of Coors Blvd. The extension of Western Trail east of Coors is called Namaste Rd. Namaste Rd. is not classified on the Long Range Roadway Plan for the Albuquerque Urban Area.

EXISTING TRAFFIC VOLUMES

2004 Average Weekday Traffic Volumes (AWDT) for major streets in the site plan area are shown on Page A-4 of the Appendix.

Current turning movement volumes obtained during the AM and PM Peak Hours for this project were acquired from recent field counts conducted by the Mid-Region Council of Governments (M.R.C.O.G.).

Existing AM and PM Peak Hour turning movement counts were provided by the City of Albuquerque for the following intersections:

Western Trail / Coors Blvd. (2003)
Dellyne Ave. / Coors Blvd. (2003)
Montano Rd. / Coors Blvd. (2003)

Additionally, AM and PM Peak Hour turning movement count forecasts were obtained from the Andalucia (South) Residential Development Traffic Impact Study (by BHI) for the intersection of Sevilla Ave. / Coors Blvd.

The counts are included after the City of Albuquerque Transportation Development Division Scoping Letter in Appendix Pages A-xx thru A-xx.

EXISTING (2006) LEVELS OF SERVICE

The Highway Capacity Manual defines Level of Service (LOS) for signalized intersections in terms of average controlled delay per vehicle as follows:

LOS A	10.0" or less	Most Vehicles do not stop
LOS B	10.1 to 20.0"	Some Vehicles stop
LOS C	20.1 to 35.0"	Significant number of vehicles stop
LOS D	35.1 to 55.0"	Many vehicles stop.
LOS E	55.1 to 80.0"	Limit of acceptable delay.
LOS F	> 80.0"	Unacceptable delay.

Level of Service D is generally considered acceptable in urban areas and is the desirable base condition for analysis in a traffic study. In addition to consideration of the overall level-of-service of the signalized intersection, the levels-of-service of each individual movement should be considered.

Existing levels-of-service associated with this development were not analyzed. Only the 2009 AM and PM Peak Hour NO BUILD and BUILD Conditions were analyzed and reported.

PROPOSED DEVELOPMENT

There proposed development of this property is projected to incorporate the following land uses:

- a) *Approximately 67 single family dwelling units*
- b) *Approximately 89 residential condominiums / townhouses*
- c) *Approximately 39,200 S.F. of retail commercial floor space*
- d) *Approximately 16,800 S.F. of general office space.*

See the conceptual site development plan on Page A-2 in the Appendix of this report to acquire more detailed information about the proposed development of Esperia Subdivision.

The site plan is conceptual at this point in time and is subject to some changes as progress takes place in the design process. The plan should, however, provide a reliable basis upon which to analyze the impact of the development on the adjacent transportation system and provide guidelines for mitigating the impact and establishing access criteria. The conceptual site plan as shown in this report proposes one primary access point (or driveway) accessing Coors Blvd.

TRIP GENERATION

Projected trips were calculated from data in the Institute of Transportation Engineers Trip Generation report (7th Edition, 2003). Trips for the development were determined based on land uses defined on the Conceptual Site Development Plan on Page A-2 in the Appendix of this report. The resulting number of trips generated for the proposed development is summarized in the following table:

McNaney Development (Sevilla Ave. / Coors Blvd.) **Trip Generation Data**

USE (ITE CODE)	DESCRIPTION	24 HR VOL	A. M. PEAK HR.			P. M. PEAK HR.	
		GROSS	ENTER	EXIT	ENTER	EXIT	
Summary Sheet		Units					
Single-Family Detached Housing (210)	67.00	717	14	42	48	27	
Residential Condominium / Townhouse (230)	89.00	581	8	39	37	18	
Shopping Center (820)	39.20	3,695	54	35	162	176	
General Office Building (710)	16.80	338	40	5	17	81	
Subtotal		5,331	116	121	264	302	

Pass-by trip credits were not applied to this study.

TRIP DISTRIBUTION

Primary and Diverted Linked Trips:

Trips were distributed as follows:

Commercial Land Use

Primary and diverted linked trips for the credit union land use development were distributed proportionally to the 2007 projected population of Data Analysis Subzones within a two-mile radius of the proposed development. Population data for the years 2005 and 2025 were taken from the 2025 Socioeconomic Forecasts for Data Analysis Subzones for the Mid Regional Council of Governments (MRCOG) (S-03-01). Population data from the years 2005 and 2025 was interpolated linearly to obtain 2007 population data to utilize for this analysis. Population Subzones were grouped based on the most likely major street(s) or route(s) to the subject development. The trip distribution worksheets and associated map of subareas and data analysis subzones is shown in the Appendix.

Residential Land Use

Primary and diverted linked trips for residential development have been distributed proportionally to the 2009 projected employment of Subareas citywide. Employment data for 2005 and 2010 were taken from the 2025 Socioeconomic Forecasts for Data Analysis Subzones for the Mid-Region of New Mexico, S-03-01 (April, 2003), Appendix B, supplied by the Mid-Region Council of Governments (MRCOG). Employment Data was interpolated linearly to obtain 2009 values and adjusted for distance from the proposed

new facility. The trip distribution worksheets and associated map of subareas are shown in the Appendix.

Office Land Use

Primary and diverted linked trips for office development have been distributed proportionally to the 2009 projected population of Subareas citywide. Population data for 2005 and 2010 were taken from the *2025 Socioeconomic Forecasts for Data Analysis Subzones for the Mid-Region of New Mexico*, S-03-01 (April, 2003), Appendix B, supplied by the Mid-Region Council of Governments (MRCOG). Population Data was interpolated linearly to obtain 2009 values and adjusted for distance from the proposed new facility. The trip distribution worksheets and associated map of subareas are shown in the Appendix.

TRIP ASSIGNMENT

Trip assignments are made on a percentage basis derived from data established in the trip distribution determination process and logical routing. Those percentages are then applied to the projected trips to determine individual traffic movements. Percentage trip assignments are shown in the Appendix.

BACKGROUND TRAFFIC GROWTH

Background traffic growth rates were considered for each individual approach to an intersection that was targeted for analysis based on data from the 2000, 2001, 2002, 2003, and 2004 Traffic Flow maps prepared by the Mid-Region Council of Governments. Almost all of the Traffic Flow Data for the years 2000, 2001, 2002, 2003, and 2004 taken from the MRCOG Traffic Flow Maps were Standard Data. The data from those years for each approach was plotted on a graph and a linear "regression trend line" calculated using the equation format $y=mx+b$. The growth rate was determined by calculating the average volume increase per year during the time period considered and dividing that volume into the most recent Average Weekday Traffic (AWDT) used in the analysis from which future volumes will be calculated. The rate of growth of that trend line was utilized as the growth rate for each approach if that calculated rate appeared feasible. However, there were some instances where the rate indicated a negative growth trend. In those cases, an appropriate growth rate from an adjacent segment of the same roadway was considered. Due to the potential for growth in the area, it was believed that a zero percent growth rate was inappropriate for this study in most cases. Additionally, if the R^2 value of the trend line was low, other means of establishing a probable growth rate from the data accumulated was considered. Historical Growth Rate Graphs with linear regression trend lines are shown on Appendix Pages A-28 through A-37. Calculated growth rates for each segment of the streets were adjusted if necessary to provide some level of logical uniformity in the transportation system.

The growth rate utilized for each approach to an intersection is printed at the top of the Turning Movement sheets for each intersection (Appendix Pages A-40 through A-49).

PROJECTED PEAK HOUR TURNING MOVEMENTS FOR 2009 BUILDOUT

The established growth rates were applied to the most recent peak hour traffic counts (furnished by the City of Albuquerque and conducted for this study), and then the trips from the *Andalucia (South) Traffic Impact Study*, *Andalucia, Tract 6 Traffic Impact Study*, and the *La Luz del Oeste development* were added in to establish the 2009 background NO BUILD traffic volumes. To these volumes, the generated trips based on implementation of the proposed Esperia Subdivision were added to obtain 2009 BUILD volumes for the intersection analyses. See Appendix Pages A-38 thru A-52 for further information regarding 2009 turning movement counts.

INTERSECTION CAPACITY ANALYSIS

Intersection capacity analyses were performed in accordance with the procedures for signalized and unsignalized intersections in the Highway Capacity Manual, Special Report 209, Transportation Research Board, 2000, using TEAPAC Signal 2000, Version 2.70 for signalized intersections and HiCAP2000 version 2.0 for unsignalized intersections. For signalized intersections, the operational method of analysis was used for implementation year (2009) conditions (NO BUILD and BUILD).

Capacity analyses were performed for the following traffic conditions.

Implementation Year (2009) - NO BUILD
Implementation Year (2009) – BUILD

The results of the implementation year (2009) capacity analyses are summarized in the following sections - *Results and Discussion of Intersection Capacity Analyses*.

RESULTS OF SIGNALIZED INTERSECTION CAPACITY ANALYSES

IMPLEMENTATION YEARS (2009)

Inter section #1 – Western Trail / Coors Blvd. – Pages A-53 thru A-62

The results of the analysis of the signalized intersection of Western Trail / Coors Blvd. are summarized in the following table:

Western Trail / Coors Blvd.		AM Peak Hour		PM Peak Hour	
	2009	<u>NO BUILD</u>	<u>BUILD</u>	<u>NO BUILD</u>	<u>BUILD</u>
Existing Geometry		C-30.3	C-33.8	<i>D-46.2</i>	<i>D-49.9</i>

D - 40.0 - Bold Italicized LOS denotes that one or more movements in the analysis run operated at Level-of-Service "E" or worse.

Existing Geometry (Western Trail / Coors Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Western Trail	1	0	0	1	0
WB Western Trail	1	0	0	1	0
NB Coors Blvd.	1	0	3	0	1
SB Coors Blvd.	1	0	3	0	1

The 209 analysis indicated that some individual turning movements would operate at LOS "E" for the NO BUILD and the BUILD conditions. The impact of the added traffic from this development is minor and, therefore, no improvements are recommended to the intersection as a result of implementation of this project. The suggested mitigation measure to remedy the NO BUILD capacity shortfall is to construct a fourth northbound thru lane on Coors Blvd.

The Queuing Analysis for this intersection results in the lanes length changes summarized in the following table:

Queueing Analysis Summary Sheet

Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Intersection: Western Trail / Coors Blvd

2009												
Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	42	200				1	0	Cont	0	39	0
AM NO BUILD Queue	2	67	75				1	3	0	0	47	100
AM BUILD Queue	2	72	75				1	3	0	0	47	100
Existing Lane Length	2	27	200				1	0	Cont	0	49	0
PM NO BUILD Queue	2	70	100				1	10	50	0	59	125
PM BUILD Queue	2	81	100				1	10	50	0	59	125
Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	2	320				1	0	Cont	0	1	0
AM NO BUILD Queue	1	93	150				1	8	25	0	59	100
AM BUILD Queue	1	93	150				1	8	25	0	59	100
Existing Lane Length	1	8	320				1	0	Cont	0	7	0
PM NO BUILD Queue	1	67	125				1	5	25	0	47	100
PM BUILD Queue	1	67	125				1	5	25	0	57	125
Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	47	250				3	1,506	Cont	1	9	225
AM NO BUILD Queue	1	51	100				3	1,722	700	1	40	75
AM BUILD Queue	1	51	100				3	1,781	725	1	40	75
Existing Lane Length	1	111	250				3	2,528	Cont	1	6	225
PM NO BUILD Queue	1	120	200				3	3,041	>1,000	1	111	200
PM BUILD Queue	1	120	200				3	3,165	>1,000	1	111	200
Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	5	150				3	2,104	Cont	1	37	300
AM NO BUILD Queue	1	25	50				3	2,479	>1,000	1	58	100
AM BUILD Queue	1	25	50				3	2,546	>1,000	1	62	100
Existing Lane Length	1	4	150				3	2,047	Cont	1	97	300
PM NO BUILD Queue	1	72	125				3	2,338	>1,000	1	138	225
PM BUILD Queue	1	73	150				3	2,482	>1,000	1	150	225

Cycle Length: AM
110 PM
130

The left and right turn auxiliary lanes at this intersection are sufficient in length.

Intersection #2 – Dellyne Ave. (Learning Rd.) / Coors Blvd. – Pages A-63 thru A-72

The results of the analysis of the signalized intersection of Dellyne Ave. (Learning Rd.) / Coors Blvd. are summarized in the following table:

Dellyne Ave. (Learning Rd.) / Coors Blvd.	AM Peak Hour		PM Peak Hour	
2009	<u>NO BUILD</u>	<u>BUILD</u>	<u>NO BUILD</u>	<u>BUILD</u>
Andalucia Geometry	C-32.2	C-32.8	D-42.5	D-46.5

D - 40.0 - Bold Italicized LOS denotes that one or more movements in the analysis run operated at Level-of-Service "E" or worse.

Andalucia Geometry (Dellyne Ave. / Coors Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Dellyne Ave.	2	0	1	0	1
WB Learning Rd.	2	0	1	0	1
NB Coors Blvd.	1	0	3	1	0
SB Coors Blvd.	2	0	3	0	1

The intersection of Dellyne Ave. (Learning Rd.) / Coors Blvd. is currently being designed by Bohannon-Huston, Inc. in conjunction with the development of Andalucia, Tract 6. The geometry used in this analysis is consistent with the plan being developed by Bohannon-Huston, Inc.

This analysis demonstrates that the intersection of Dellyne Ave. (Learning Rd.) / Coors Blvd. as designed by Bohannon-Huston, Inc. will operate at acceptable levels-of-service for all conditions analyzed in this study.

The Queuing Analysis for this intersection results in the lanes length changes summarized in the following table:

Queueing Analysis Summary Sheet

Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Intersection: Dellyne Ave / Coors Blvd

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	156	200*	1	31	Cont	1	31	Cont	0	238	150
AM NO BUILD Queue	2	226	175	1	36	75	1	36	75	0	240	300
AM BUILD Queue	2	226	175	1	36	75	1	36	75	0	255	325
Existing Lane Length	2	0	200*	1	0	Cont	1	0	Cont	0	0	150
PM NO BUILD Queue	2	225	200	1	23	75	1	23	75	0	109	175
PM BUILD Queue	2	225	200	1	23	75	1	23	75	0	124	200

Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	48	200*	1	20	Cont	1	20	Cont	0	77	200
AM NO BUILD Queue	1	191	250	1	79	125	1	79	125	0	113	175
AM BUILD Queue	1	191	250	1	79	125	1	79	125	0	113	175
Existing Lane Length	1	4	200*	1	1	Cont	1	1	Cont	0	6	200
PM NO BUILD Queue	1	149	225	1	113	200	1	113	200	0	59	125
PM BUILD Queue	1	149	225	1	113	200	1	113	200	0	59	125

Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	70	325	3	1,781	Cont	3	1,781	Cont	1	66	225
AM NO BUILD Queue	1	82	125	3	2,044	800	3	2,044	800	1	91	150
AM BUILD Queue	1	95	150	3	2,079	825	3	2,079	825	1	91	150
Existing Lane Length	1	16	325	3	129	Cont	3	129	Cont	1	1	225
PM NO BUILD Queue	1	323	450	3	2,695	>1,000	3	2,695	>1,000	1	112	200
PM BUILD Queue	1	336	450	3	2,730	>1,000	3	2,730	>1,000	1	112	200

Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	186	275	3	1,380	Cont	3	1,380	Cont	1	36	250
AM NO BUILD Queue	1	240	300	3	1,554	625	3	1,554	625	1	49	100
AM BUILD Queue	1	240	300	3	1,590	650	3	1,590	650	1	49	100
Existing Lane Length	1	8	275	3	149	Cont	3	149	Cont	1	16	250
PM NO BUILD Queue	1	180	275	3	1,852	850	3	1,852	850	1	223	325
PM BUILD Queue	1	180	275	3	1,888	875	3	1,888	875	1	223	325

Cycle Length: **AM** 110 **PM** 130

* - Unequal Left Turn Lane Lengths for Dual Left Turns

The queuing analysis indicates that the northbound left turn lane on Coors Blvd. should be extended to a length of 450 feet.

Intersection #3 - Montano Rd. / Coors Blvd. – Pages A-73 thru A-84

The results of the analysis of the signalized intersection of Montano Rd. / Coors Blvd. are summarized in the following table:

Montano Rd. / Coors Blvd.	AM Peak Hour		PM Peak Hour		
	2006	<u>NO BUILD</u>	<u>BUILD</u>	<u>NO BUILD</u>	<u>BUILD</u>
Existing Geometry		<i>F-99.6</i>	<i>F-102</i>	<i>F-124</i>	<i>F-130</i>
Exist. Geom. – Add NB/SB LT, Thru Lane				<i>E-61.0</i>	<i>E-70.3</i>

D - 40.0 - Bold Italicized LOS denotes that one or more movements in the analysis run operated at Level-of-Service "E" or worse.

Existing Geometry (Montano Rd. / Coors Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Montano Rd.	2	0	2	0	1
WB Montano Rd.	2	0	1	1	0
NB Coors Blvd.	2	0	3	0	1
SB Coors Blvd.	2	0	3	0	1

This analysis demonstrates, as many other previous analyses, that there is a capacity shortfall at the existing intersection of Montano Rd. / Coors Blvd. This analysis has shown the resulting levels-of-service resulting from the addition of triple northbound / southbound left turn lanes and fourth northbound / southbound thru lanes on Coors Blvd. There is not sufficient right-of-way to construct the recommended improvements. This analysis is primarily to demonstrate what would be needed to provide capacity at the intersection if sufficient right-of-way existed.

The intersection of Montano Rd. / Coors Blvd. is designated as a grade separated interchange at some time in the future. There are no funds nor is there a definite plan in place to construct the interchange by any definable date.

Given that the development of Esperia Subdivision only increases the delays at Montano Rd. / Coors Blvd. by about 3 seconds in the AM Peak Hour and 6 seconds in the PM Peak Hour, no recommendation is made for improvements associated with this project.

The Queuing Analysis for this intersection results in the lanes length changes summarized in the following table:

Queueing Analysis Summary Sheet

Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Intersection: Montañó Rd / Coors Blvd

2009												
Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	74	300	2	743	Cont	2	274	300	1	274	300
AM NO BUILD Queue	2	75	75	2	898	550	1	354	400	1	354	400
AM BUILD Queue	2	75	75	2	898	550	1	371	425	1	371	425
Existing Lane Length	2	268	300	2	312	Cont	1	245	300	1	245	300
PM NO BUILD Queue	2	271	225	2	615	450	1	400	525	1	400	525
PM BUILD Queue	2	271	225	2	615	450	1	447	575	1	447	575
Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	166	350	2	252	Cont	0	59	0	0	59	0
AM NO BUILD Queue	2	220	175	2	299	225	0	63	100	0	63	100
AM BUILD Queue	2	228	175	2	299	225	0	63	100	0	63	100
Existing Lane Length	2	369	350	2	837	Cont	0	105	0	0	105	0
PM NO BUILD Queue	2	545	400	2	1,006	700	0	113	200	0	113	200
PM BUILD Queue	2	562	425	2	1,006	700	0	113	200	0	113	200
Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	213	650	3	1,238	Cont	1	310	500	1	310	500
AM NO BUILD Queue	2	409	275	3	1,531	625	1	557	600	1	557	600
AM BUILD Queue	2	422	300	3	1,545	625	1	566	600	1	566	600
Existing Lane Length	2	649	650	3	1,658	Cont	1	225	500	1	225	500
PM NO BUILD Queue	2	1,036	700	3	2,022	>1,000	1	522	650	1	522	650
PM BUILD Queue	2	1,087	725	3	2,047	>1,000	1	541	675	1	541	675
Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	286	410	3	1,288	Cont	1	56	350	1	56	350
AM NO BUILD Queue	2	363	250	3	1,480	600	1	61	100	1	61	100
AM BUILD Queue	2	363	250	3	1,490	625	1	61	100	1	61	100
Existing Lane Length	2	162	410	3	1,301	Cont	1	126	350	1	126	350
PM NO BUILD Queue	2	273	225	3	1,606	750	1	138	225	1	138	225
PM BUILD Queue	2	273	225	3	1,627	775	1	138	225	1	138	225
AM PM												
Cycle Length:	110	130										

analysis from the Andalucia, Tract 6 Traffic Study demonstrated that the northbound queues were shorter than what is shown in this report. The Poisson arrival method of calculating the queue lengths in this report do not consider the G/C ratio for each phase and, therefore, are probably conservatively high.

Intersection #4 –Sevilla Ave. (Maduri Ave.) / Coors Blvd. – Pages A-85 thru A-94

The results of the analysis of the signalized intersection of Sevilla Ave. (Maduri Ave.) / Coors Blvd. are summarized in the following table:

Sevilla Ave. (Maduri Ave.) / Coors Blvd.	AM Peak Hour		PM Peak Hour	
	2009	NO BUILD	BUILD	NO BUILD
Proposed Geometry (Tee Intersection)		A-4.4		A-6.7
Proposed Geometry (Full Intersection)			B-15.2	C-30.7

D - 40.0 - Bold Italicized LOS denotes that one or more movements in the analysis run operated at Level-of-Service "E" or worse.

Proposed Geometry (Sevilla Ave. / Coors Blvd.) – TEE Intersection

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Maduri Ave.	N/A	N/A	N/A	N/A	N/A
WB Sevilla Ave.	1	0	0	0	1
NB Coors Blvd.	0	0	3	0	1
SB Coors Blvd.	1	0	3	0	0

Proposed Geometry (Sevilla Ave. – Maduri Ave. / Coors Blvd.) – FULL Intersection

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Maduri Ave.	1	0	0	1	0
WB Sevilla Ave.	1	0	0	1	0
NB Coors Blvd.	1	0	3	0	1
SB Coors Blvd.	1	0	3	0	1

The intersection of Sevilla Ave. / Coors Blvd. is a new intersection constructed with the development of Andalucia South Residential Development as a signalized tee intersection. Development of the proposed Esperia Subdivision will create the fourth leg of the intersection.

The analysis presented in this study demonstrates that the operation of the intersection of Sevilla Ave. (Maduri Ave.) / Coors Blvd. will operate at acceptable levels-of-service for all conditions analyzed.

The Queuing Analysis for this intersection should determine the lane lengths to incorporate into the design and construction of the intersection:

Queueing Analysis Summary Sheet

Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Intersection: Sevilla Ave / Coors Blvd

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach		# Lanes	Vol.		Length (Ft.)		# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length		1	0		Design		1	0	Cont	0	0	0
AM NO BUILD Queue		1	33		75		1	0	0	0	43	75
AM BUILD Queue		1	82		125		1	0	0	0	115	175
Existing Lane Length		1	0		Design		1	0	Cont	0	0	0
PM NO BUILD Queue		1	20		50		1	0	0	0	26	75
PM BUILD Queue		1	163		250		1	1	0	0	184	275

Westbound				Left Turns			Thru Movements			Right Turns		
Approach		# Lanes	Vol.		Length (Ft.)		# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length		1	0		Design		1	0	Cont	0	0	0
AM NO BUILD Queue		1	47		100		1	0	0	0	42	75
AM BUILD Queue		1	47		100		1	0	0	0	42	75
Existing Lane Length		1	0		Design		1	0	Cont	0	0	0
PM NO BUILD Queue		1	29		75		1	0	0	0	45	100
PM BUILD Queue		1	29		75		1	1	0	0	45	100

Northbound				Left Turns			Thru Movements			Right Turns		
Approach		# Lanes	Vol.		Length (Ft.)		# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length		1	0		Design		3	1,549	Cont	1	0	Design
AM NO BUILD Queue		1	31		75		3	1,751	700	1	15	50
AM BUILD Queue		1	95		150		3	1,751	700	1	15	50
Existing Lane Length		1	0		Design		3	2,562	Cont	1	0	Design
PM NO BUILD Queue		1	118		200		3	2,907	>1,000	1	53	100
PM BUILD Queue		1	254		350		3	2,907	>1,000	1	53	100

Southbound				Left Turns			Thru Movements			Right Turns		
Approach		# Lanes	Vol.		Length (Ft.)		# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length		1	0		Design		3	2,146	Cont	1	0	Design
AM NO BUILD Queue		1	22		50		3	2,474	>1,000	1	0	0
AM BUILD Queue		1	22		50		3	2,474	>1,000	1	52	100
Existing Lane Length		1	0		Design		3	2,148	Cont	1	0	Design
PM NO BUILD Queue		1	58		125		3	2,495	>1,000	1	0	0
PM BUILD Queue		1	58		125		3	2,495	>1,000	1	127	200

AM
PM
 Cycle Length: 110 130

There are two criteria for design of the auxiliary lanes at the intersection of Sevilla Ave. / Coors Blvd. The first criteria is the queuing table above. The second criteria is the New Mexico Department of Transportation's *State Access Management* Manual. The posted speed limit on Coors Blvd. at this location is 45 M.P.H. Based on that speed limit, the minimum required length for a left turn lane at this intersection is 400 feet plus a 12.5:1 transition taper. The minimum required length for a right turn deceleration lane is 370

feet plus a 12.5:1 transition taper. In this instance, the New Mexico Department of Transportation's State Access Management Manual criteria should govern.

RESULTS OF UNSIGNALIZED INTERSECTION (DRIVEWAY) CAPACITY ANALYSES

IMPLEMENTATION YEARS (2009)

Intersection #5 – Maduri Ave. / Vidal Dr. – Pages A-95 thru A-97

The results of the analysis of the unsignalized intersection of Maduri Ave. / Vidal Dr. are summarized in the following table:

	BUILD Condition	
	AM Peak	PM Peak
2009		
Maduri Ave. / Vidal Dr.		
Minor Street (Vidal Dr.)		
NB Left	A - 9	A - 9
NB Thru	A - 9	A - 9
NB Right	A - 9	A - 9
Minor Street (Vidal Dr.)		
SB Left	B - 12	C - 18
SB Thru	B - 12	C - 18
SB Right	B - 12	C - 18
Major Street (Maduri Ave.)		
EB Left	A - 7	A - 8
WB Left	A - 7	A - 8

The intersection of Maduri Ave. / Vidal Dr. currently does not exist since it will be constructed with this project. Therefore, a BUILD Analysis only is necessary. This study demonstrates that the unsignalized intersection of Maduri Ave. / Vidal Dr. will operate at acceptable levels-of-service for all conditions analyzed.

Additionally, the calculated queue lengths in the HiCAP 2.0 reports were never more than 2 vehicles deep and in most cases only 1 vehicle.

It should be noted that Levels of Service (LOS) for unsignalized intersections cannot be compared directly with Levels of Service for signalized intersections.

LEVEL-OF-SERVICE CRITERIA FOR UNSIGNALIZED INTERSECTIONS

<u>Average Delay</u> <u>(secs)</u>	<u>Level-of-Service</u>
≤ 10	A
> 10 and ≤ 15	B
> 15 and ≤ 25	C
> 25 and ≤ 35	D
> 35 and ≤ 50	E

Generally speaking, a Level-of-Service D or better is an acceptable parameter for design purposes.

CONCLUSIONS

The results of this analysis of the adjacent transportation system associated with this proposed commercial / residential development indicate that there will be moderate impact along Coors Blvd. at the intersections analyzed in this study. The intersection of Dellyne Ave. (Learning Rd.) / Coors Blvd. will be reconstructed in the near future by the Andalucia, Tract 6 development project. Design of the intersection of Dellyne Ave. / Coors Blvd. is currently complete and construction of the improvements to the intersection should begin in the summer of 2006. Impacts of the proposed Esperia Subdivision to other intersections analyzed in this study are minor.

In summary, the proposed site development plans for these developments present minimized adverse impact to the adjacent transportation system if the recommendations are implemented as follows:

RECOMMENDATIONS

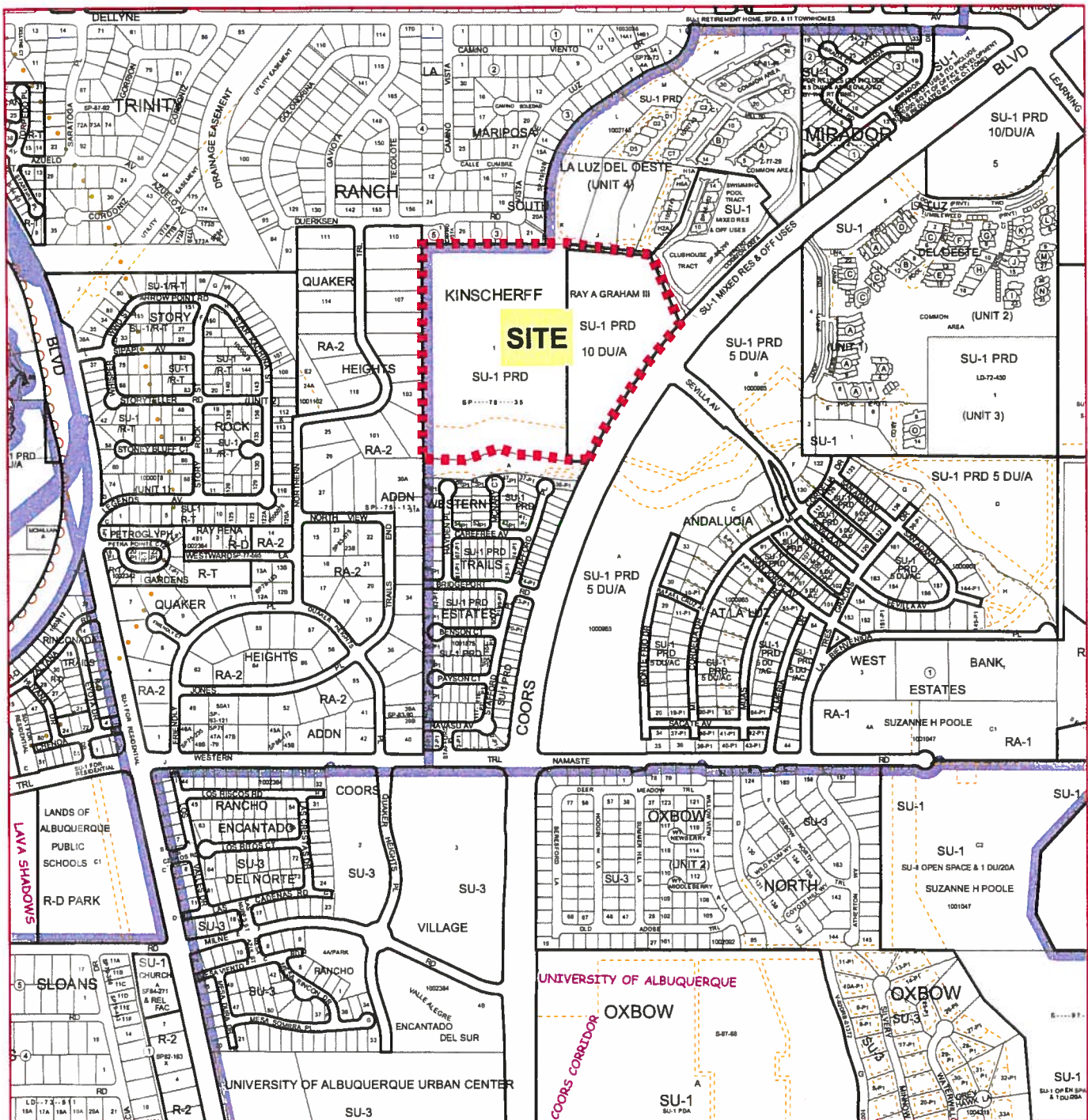
- All design and construction for this project shall insure that adequate site distances at the proposed driveways along Coors Blvd. are maintained.
- Driveways shall be constructed using a minimum of 25-foot radius curb returns or the minimum required by the City of Albuquerque Development Process Manual (D.P.M.).
- The signalized intersection of Sevilla Ave. (Maduri Ave.) / Coors Blvd. should be constructed as a four legged signalized intersection with a northbound left turn lane and a southbound right turn lane as defined in this study. The west leg of Maduri Ave. should be constructed with a dedicated left turn lane and a thru / right turn lane on the approach. Left and right turn lane lengths should be a minimum length as defined on Pages 15 and 16 of this study.
- Upon construction of the new access at Maduri Ave. / Coors Blvd. and the connection of paved street to La Luz del Oeste, the current access to La Luz del Oeste may be restricted to right-in, right-out only movements.
- The northbound left turn lane on Coors Blvd. at Dellyne should be extended to a length of 450 feet.

Appendix

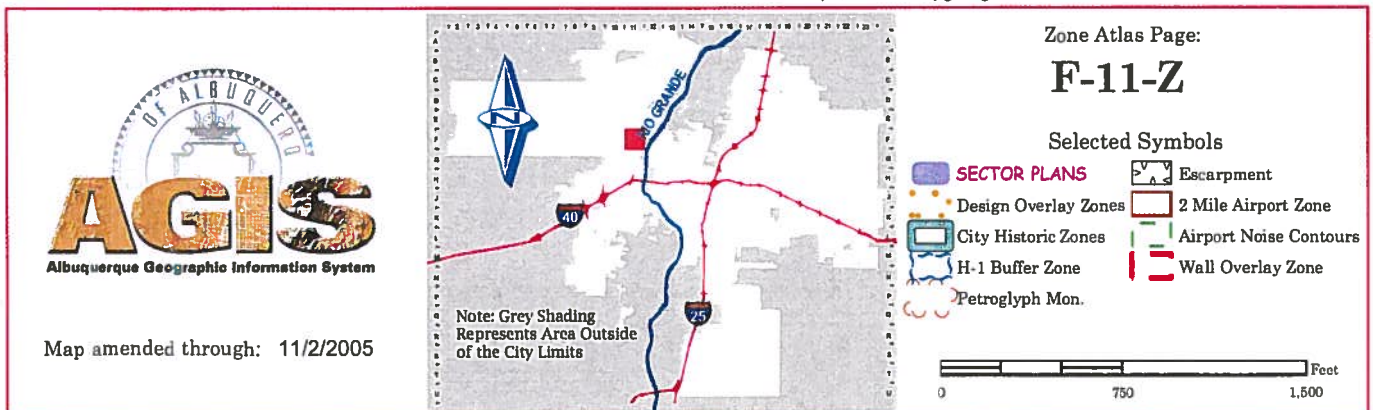
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Appendix

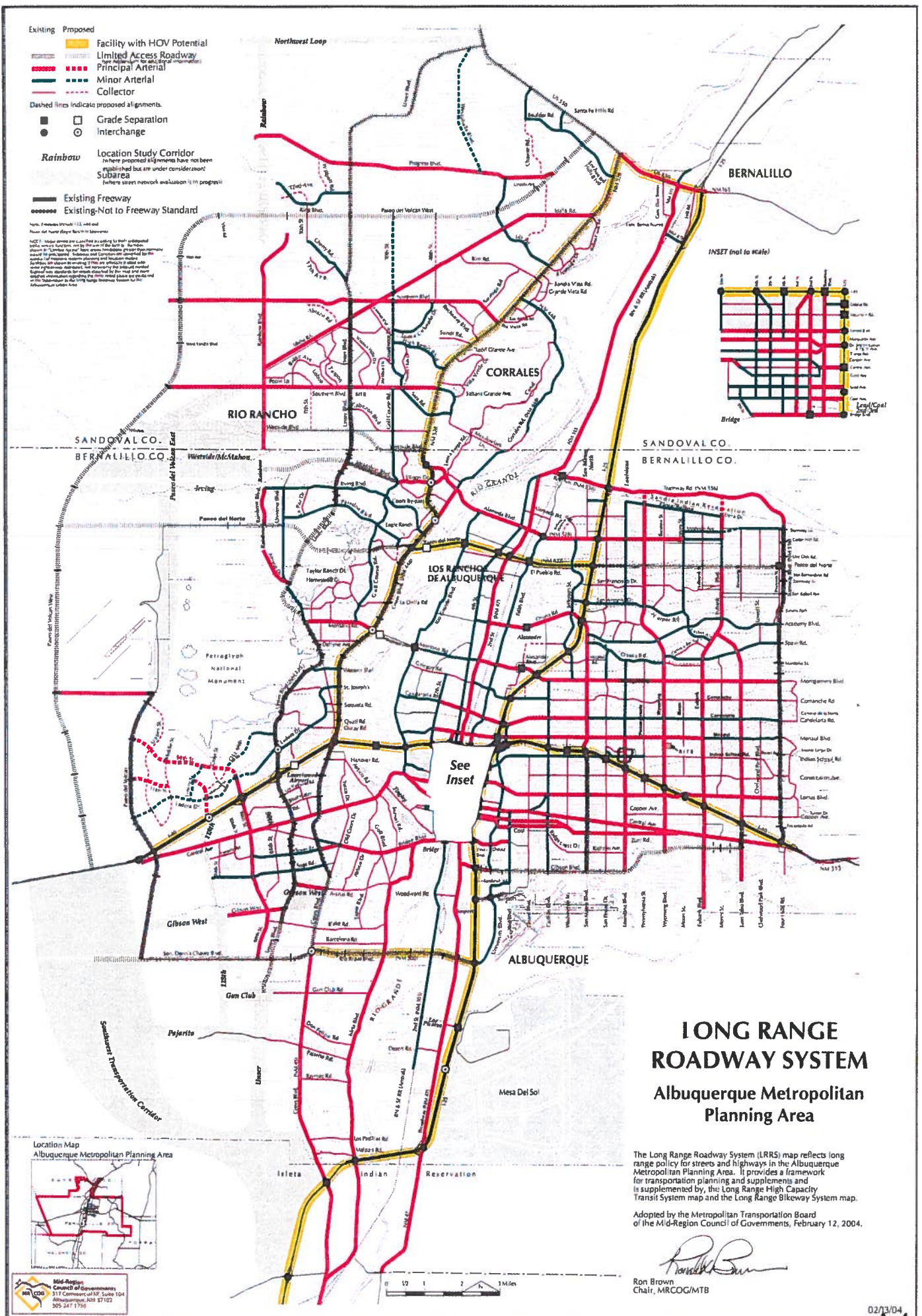
Appendix

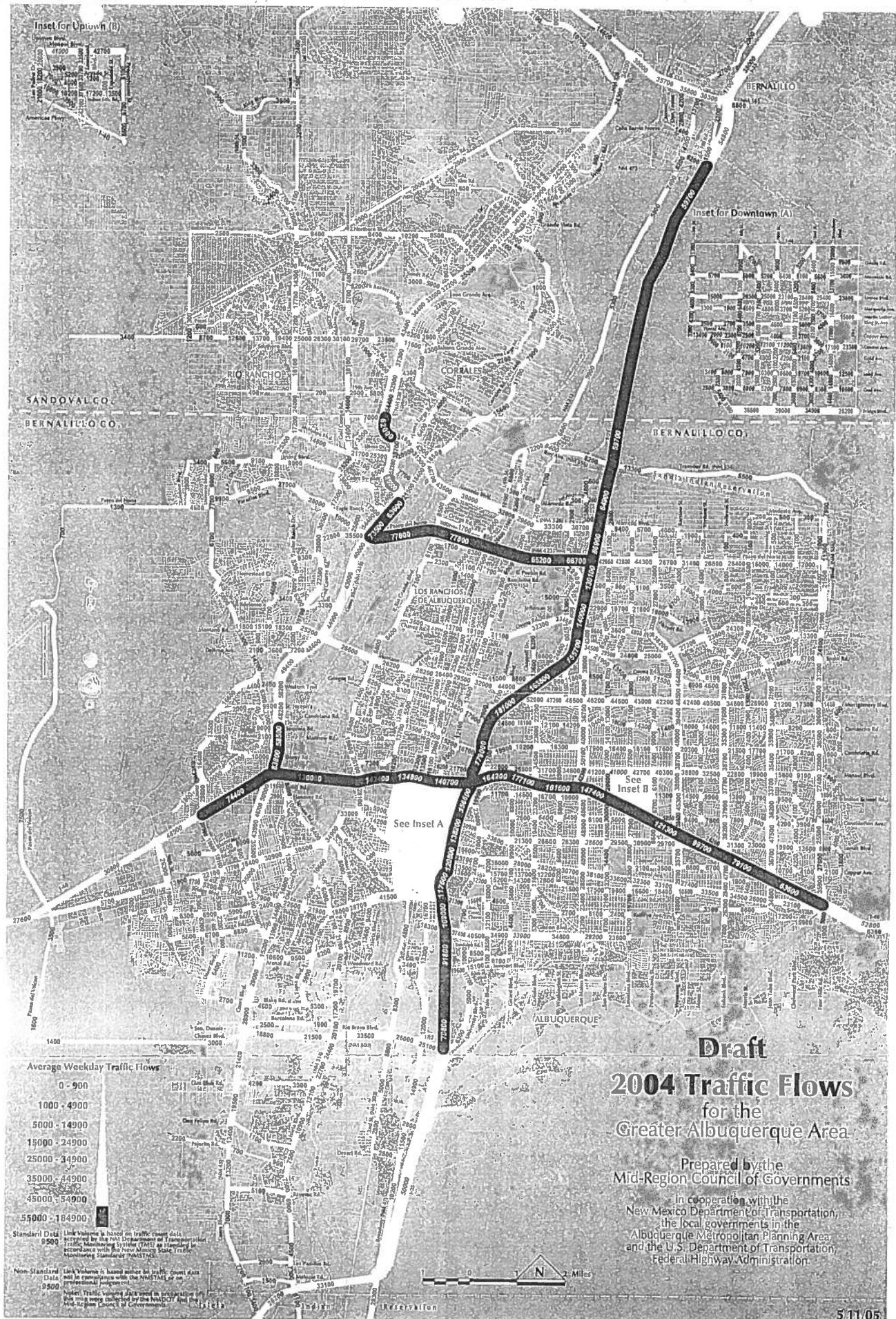


For more current information and more details visit: <http://www.cabq.gov/gis>









Mid-Region Council of Governments
809 Copper Ave. NW
Albuquerque, NM 87102
505-247-1750

McNaney Development (Sevilla Ave. / Coors Blvd.)
Trip Generation Data

USE (ITE CODE)	DESCRIPTION	24 HR VOL		A. M. PEAK HR.		P. M. PEAK HR.	
		GROSS	ENTER	EXIT	ENTER	EXIT	
Summary Sheet							
		Units					
Single-Family Detached Housing (210)	67.00	717	14	42	48	27	
Residential Condominium / Townhouse (230)	89.00	581	8	39	37	18	
Shopping Center (820)	39.20	3,695	54	35	162	176	
General Office Building (710)	16.80	338	40	5	17	81	
Subtotal		5,331	116	121	264	302	

McNaney Development (Sevilla Ave. / Coors Blvd.)
Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME	A. M. PEAK HOUR		P. M. PEAK HOUR	
		ENTER	EXIT	ENTER	EXIT
	GROSS				

Units

67.00	717	14	42	48	27
-------	-----	----	----	----	----

Single-Family Detached Housing (210)

Dwelling Units

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$\ln(T) = 0.92 \ln(X) + 2.707$$

50% Enter, 50% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$T = 0.7 (X) + 9.477$$

25% Enter, 75% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$\ln(T) = 0.901 \ln(X) + 0.527$$

64% Enter, 36% Exit

Comments:
 Tract No.

Based on ITE Trip Generation Manual - 7th Edition

McNaney Development (Sevilla Ave. / Coors Blvd.)
Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME	A. M. PEAK HOUR		P. M. PEAK HOUR	
		ENTER	EXIT	ENTER	EXIT

Units

89.00	581	8	39	37	18
-------	-----	---	----	----	----

Residential Condominium / Townhouse (230)

Dwelling Units

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$\ln(\tau) = 0.85 \ln(X) + 2.55$$

50% Enter, 50% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$\ln(\tau) = 0.8 \ln(X) + 0.26$$

17% Enter, 83% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$\ln(\tau) = 0.82 \ln(X) + 0.32$$

67% Enter, 33% Exit

Comments:
 Tract No.

Based on ITE Trip Generation Manual - 7th Edition

McNaney Development (Sevilla Ave. / Coors Blvd.)
Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME		A. M. PEAK HOUR		P. M. PEAK HOUR	
	GROSS		ENTER	EXIT	ENTER	EXIT
Shopping Center (820)		Units				
		39.20	54	35	162	176
		1,000 S.F.				

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$\ln(T) = 0.65 \ln(X) + 5.83$$

50% Enter, 50% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$\ln(T) = 0.6 \ln(X) + 2.29$$

61% Enter, 39% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$\ln(T) = 0.66 \ln(X) + 3.403$$

48% Enter, 52% Exit

Comments:
 Tract No.

Based on ITE Trip Generation Manual - 7th Edition

McNaney Development (Sevilla Ave. / Coors Blvd.)
Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME		A. M. PEAK HOUR		P. M. PEAK HOUR
	GROSS	ENTER	EXIT	ENTER	EXIT

Units

16.80

338	40	5	17	81
-----	----	---	----	----

1,000 S.F.

General Office Building (710)ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$\ln(T) = 0.77 \ln(X) + 3.65$$

50% Enter, 50% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$\ln(T) = 0.8 \ln(X) + 1.55$$

88% Enter, 12% Exit

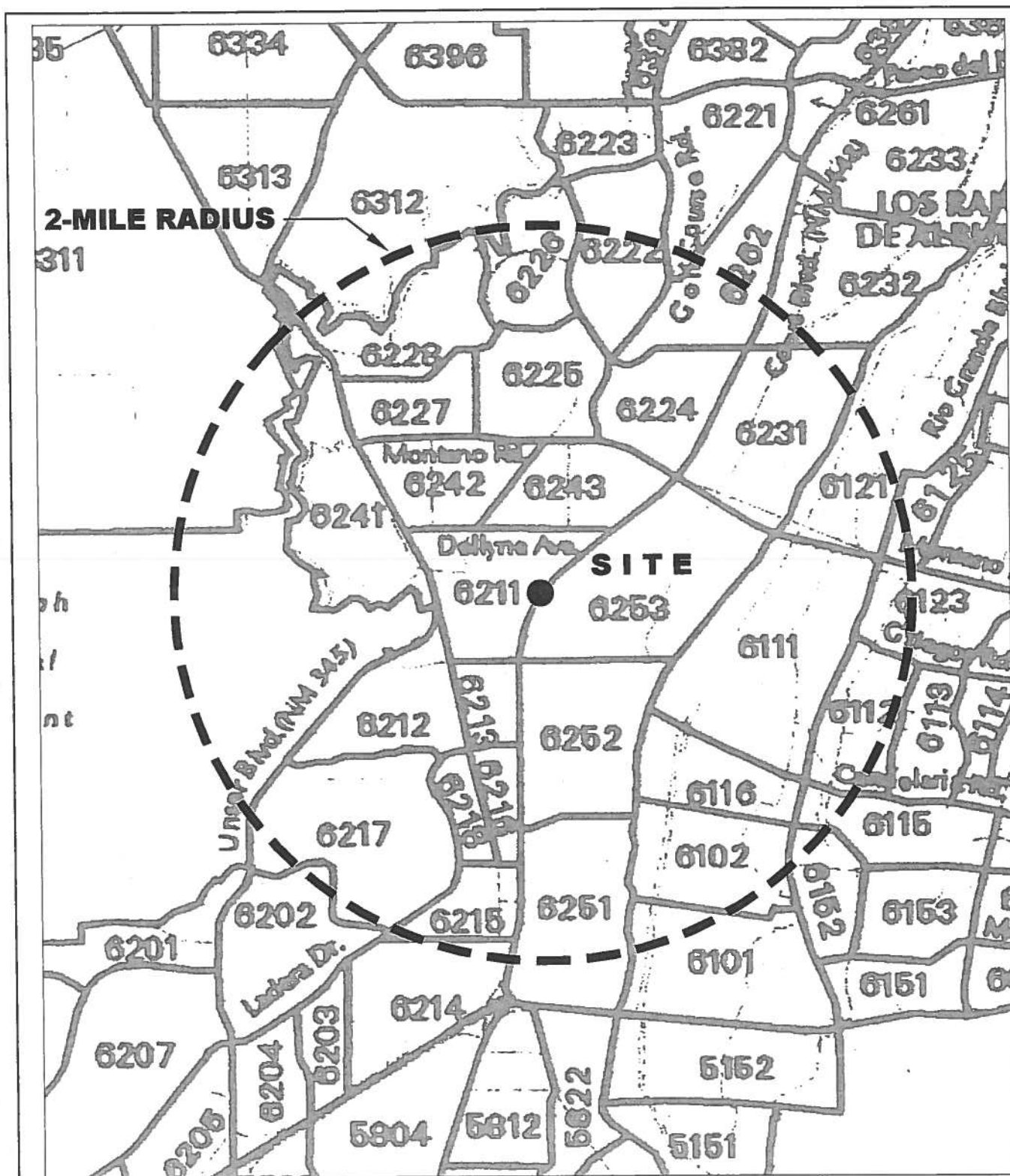
Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$T = 1.12 (X) + 78.81$$

17% Enter, 83% Exit

Comments:
 Tract No.

Based on ITE Trip Generation Manual - 7th Edition



DATA ANALYSIS SUBZONE (DASZ) MAP
Esperia Subdivision (Coors Blvd / Sevilla Ave)

Trip Distribution Table **Esperia Development (Sevilla Ave / Coors Blvd)**

Data Analysis Subzone Population Data for Determination of Local Trip Distribution for Proposed Retail Commercial Trips

2000 and 2025 Data Taken from Mid-Region Council of Governments' 2025 Socioeconomic
 2025 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico (S-03-01)

DASZ #	% Sub Area In Study	2000 Population	2025 Population	Interpolated Population for the Year	Population In Study	Population / Distance	Percent Population	(CN) Coors Blvd North			(ME) Montano Rd East			(LE) Learning Dr East			
								% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population	
Boundary Specified on DASZ Map																	
6101	10%	1931	2202	2,029	203	203	0.67%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6102	90%	1297	1346	1,315	1,184	1,184	3.31%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6111	100%	1080	1081	1,080	1,080	1,080	3.02%	0%	0.00%	0	50%	1.51%	540	0%	0.00%	0	
6112	85%	937	948	948	806	806	2.25%	0%	0.00%	0	50%	1.13%	403	0%	0.00%	0	
6115	15%	1121	1307	1,188	178	178	0.60%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6116	100%	684	657	674	674	674	1.88%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6121	30%	672	680	678	203	203	0.57%	0%	0.00%	0	100%	0.57%	203	0%	0.00%	0	
6123	30%	657	766	696	209	209	0.58%	0%	0.00%	0	100%	0.59%	209	0%	0.00%	0	
6125	20%	79	187	111	22	22	0.08%	0%	0.00%	0	100%	0.06%	22	0%	0.00%	0	
6152	10%	761	711	743	74	74	0.21%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6202	5%	884	1517	1,089	55	55	0.15%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6211	100%	1125	2073	1,466	1,466	1,466	4.10%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6212	100%	1628	2322	1,878	1,878	1,878	6.25%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6213	100%	0	379	136	136	136	0.38%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6214	5%	2217	3875	2,814	141	141	0.39%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6215	100%	1791	1743	1,774	1,774	1,774	4.98%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6216	100%	339	952	560	560	560	1.67%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6217	95%	2785	2701	2,761	2,623	2,623	7.33%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6218	100%	1742	2111	1,875	1,875	1,875	6.24%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6222	55%	3377	3107	3,280	1,804	1,804	5.04%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6224	100%	2421	3356	2,758	2,758	2,768	7.71%	50%	3.85%	1,379	0%	0.00%	0	0%	0.00%	0	
6225	100%	2045	1882	1,986	1,986	1,986	6.65%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6226	80%	1728	1628	1,692	1,354	1,354	3.78%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6227	100%	1097	1500	1,242	1,242	1,242	3.47%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6228	100%	1581	1597	1,574	1,574	1,574	4.40%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6231	95%	40	529	216	205	206	0.67%	50%	0.29%	103	50%	0.29%	103	0%	0.00%	0	
6232	10%	339	1378	713	71	71	0.20%	100%	0.00%	71	0%	0.00%	0	0%	0.00%	0	
6241	95%	2684	2543	2,633	2,501	2,501	6.99%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6242	100%	2072	2036	2,059	2,059	2,059	6.76%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6243	100%	2086	1942	2,041	2,041	2,041	6.71%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6251	80%	1817	1842	1,826	1,461	1,461	4.08%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6252	100%	433	1476	808	808	808	2.28%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6253	145	1622	1622	1,622	677	677	1.89%	0%	0.00%	0	30%	0.57%	203	35%	0.66%	237	
6262	45%	82	144	104	47	47	0.13%	100%	0.13%	47	0%	0.00%	0	0%	0.00%	0	
6311	10%	2	193	71	7	7	0.02%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6312	10%	1	1053	380	38	38	0.11%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
6314	20%	2	0	1	0	0	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
							1,800				1,800				1,883		
							4.47%				4.47%				4.70%		
							100.00%				100.00%				100.00%		
							35,774				35,774				47,886		
							237				237				237		
							0.66%				0.66%				0.66%		

Trip Distribution Table
Esperia Development (Sevilla Ave / Coors Blvd)

Data Analysis Subzone Population Data for determination of Local Trip Distribution for Proposed Retail Commercial Trips

2000 and 2025 Data Taken from Mid-Region Council of Governments' 2025 Socioeconomic
 2025 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico (S-03-07)

DASZ #	% Sub Area In Study	2000 Population		2025 Population	Interpolated Population for the Year 2009	Population In Study	Population / Distance	Percent Population	(NE) Namaste Rd East		(CS) Coors Blvd South		(WW) Western Trail West			
		2000	2025						% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population		
Boundary Specified on DASZ Map																
6101	10%	1931	2202		2,029	203	203	0.67%	0%	0.00%	0	100%	0.57%	203	0%	0.00%
6102	90%	1297	1346		1,315	1,184	1,184	3.31%	0%	0.00%	0	100%	3.31%	1,184	0%	0.00%
6111	100%	1080	1081		1,080	1,080	1,080	3.02%	0%	0.00%	0	50%	1.51%	540	0%	0.00%
6112	85%	954	937		948	806	806	2.26%	0%	0.00%	0	50%	1.13%	403	0%	0.00%
6115	15%	1121	1307		1,188	178	178	0.60%	0%	0.00%	0	100%	0.50%	178	0%	0.00%
6116	100%	684	657		674	674	674	1.88%	0%	0.00%	0	100%	1.88%	674	0%	0.00%
6121	30%	672	690		678	203	203	0.67%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6123	30%	657	696		678	209	209	0.68%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6125	20%	78	167		111	22	22	0.08%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6152	10%	761	711		743	74	74	0.21%	0%	0.00%	0	100%	0.21%	74	0%	0.00%
6202	5%	864	1517		1,099	55	55	0.15%	0%	0.00%	0	100%	0.15%	55	0%	0.00%
8211	100%	1125	2073		1,466	1,466	1,466	4.10%	0%	0.00%	0	50%	2.05%	733	0%	0.00%
6212	100%	1628	2322		1,878	1,878	1,878	6.25%	0%	0.00%	0	50%	2.62%	939	50%	2.82%
6213	100%	0	379		136	136	136	0.39%	0%	0.00%	0	0%	0.00%	0	100%	0.36%
6214	5%	2217	3875		2,814	141	141	0.39%	0%	0.00%	0	100%	0.39%	141	0%	0.00%
6215	100%	1791	1774		1,774	1,774	1,774	4.95%	0%	0.00%	0	100%	4.95%	1,774	0%	0.00%
6216	100%	339	952		560	560	560	1.67%	0%	0.00%	0	100%	1.57%	560	0%	0.00%
6217	95%	2795	2701		2,761	2,623	2,623	7.33%	0%	0.00%	0	100%	7.33%	2,623	0%	0.00%
6218	100%	1742	2111		1,875	1,875	1,875	5.24%	0%	0.00%	0	100%	5.24%	1,875	0%	0.00%
6222	55%	3377	3107		3,280	1,904	1,804	6.04%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6224	100%	2421	3358		2,758	2,758	2,758	7.71%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6225	100%	2045	1882		1,986	1,986	1,986	5.65%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6226	80%	1728	1629		1,692	1,354	1,354	3.79%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
8227	100%	1097	1500		1,242	1,242	1,242	3.47%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6228	100%	1581	1567		1,574	1,574	1,574	4.40%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6231	95%	40	529		216	205	205	0.67%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6232	10%	339	1378		713	71	71	0.20%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6241	95%	2694	2543		2,633	2,501	2,501	6.99%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6242	100%	2072	2036		2,059	2,059	2,059	6.76%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6243	100%	2086	1942		2,041	2,041	2,041	6.71%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6251	80%	1817	1842		1,826	1,461	1,461	4.09%	0%	0.00%	0	100%	4.06%	1,461	0%	0.00%
6252	100%	433	1478		808	808	808	2.26%	25%	0.56%	202	75%	1.69%	606	0%	0.00%
6253	100%	145	1622		677	677	677	1.89%	35%	0.66%	237	0%	0.00%	0	0%	0.00%
6262	45%	82	144		104	47	47	0.13%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6311	10%	2	183		71	7	7	0.02%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6312	10%	1	1053		380	38	38	0.11%	0%	0.00%	0	0%	0.00%	0	0%	0.00%
6314	20%	2	0		47,886	35,774	35,774	100.00%	0%	0.00%	0	0%	0.00%	0	100%	0.00%
							439								13,290	1,806
							1.23%								37.15%	5.05%

Trip Distribution Table

Data Analysis Subzone Population Data for determination of Local Trip Distribution for Proposed Retail Commercial Trips

2000 and 2025 Data Taken from Mid-Region Council of Governments' 2025 Socioeconomic
2025 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico (S-03-01)

DASZ #	% Sub Area In Study	2000 Population	2025 Population	Interpolated Population for the Year	Population In Study	Population / Distance	Percent Population	(DW)			(MW)		
								% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population
Boundary Specified on DASZ Map													
6101	10%	1931	2202	2,029	203	203	0.67%	0%	0.00%	0	0%	0.00%	0
6102	90%	1297	1346	1,315	1,184	1,184	3.31%	0%	0.00%	0	0%	0.00%	0
6111	100%	1080	1081	1,080	1,080	1,080	3.02%	0%	0.00%	0	0%	0.00%	0
6112	85%	954	937	948	806	806	2.25%	0%	0.00%	0	0%	0.00%	0
6115	15%	1188	1307	1,188	178	178	0.50%	0%	0.00%	0	0%	0.00%	0
6116	100%	664	616	674	674	874	1.88%	0%	0.00%	0	0%	0.00%	0
6121	30%	672	680	678	203	203	0.67%	0%	0.00%	0	0%	0.00%	0
6123	30%	657	768	696	209	209	0.68%	0%	0.00%	0	0%	0.00%	0
6125	20%	78	167	111	22	22	0.06%	0%	0.00%	0	0%	0.00%	0
6152	10%	761	743	743	74	74	0.21%	0%	0.00%	0	0%	0.00%	0
6202	5%	884	1517	1,099	55	55	0.15%	0%	0.00%	0	0%	0.00%	0
6211	100%	1125	2073	1,466	1,466	1,466	4.10%	50%	2.05%	733	0%	0.00%	0
6218	100%	1628	2322	1,878	1,878	1,878	5.25%	0%	0.00%	0	0%	0.00%	0
6213	100%	0	379	136	136	136	0.38%	0%	0.00%	0	0%	0.00%	0
6214	5%	2217	3675	2,814	141	141	0.39%	0%	0.00%	0	0%	0.00%	0
6215	100%	1791	1743	1,774	1,774	1,774	4.98%	0%	0.00%	0	0%	0.00%	0
6216	100%	339	952	560	560	560	1.57%	0%	0.00%	0	0%	0.00%	0
6217	95%	2795	2701	2,761	2,623	2,623	7.33%	0%	0.00%	0	0%	0.00%	0
6218	100%	1742	2111	1,875	1,875	1,875	6.24%	0%	0.00%	0	0%	0.00%	0
6222	55%	3377	3107	3,280	1,804	1,804	6.04%	0%	0.00%	0	100%	5.04%	1,904
6224	100%	2421	3356	2,758	2,758	2,768	7.71%	0%	0.00%	0	0%	3.85%	1,379
6225	100%	1986	1882	1,986	1,986	1,986	6.65%	0%	0.00%	0	100%	5.55%	1,986
6226	80%	1728	1629	1,692	1,354	1,354	3.78%	0%	0.00%	0	100%	3.78%	1,354
6227	100%	1087	1500	1,242	1,242	1,242	3.47%	50%	1.74%	621	50%	1.74%	621
6228	100%	1581	1587	1,574	1,574	1,574	4.40%	50%	2.20%	787	50%	2.20%	787
6231	95%	40	529	216	205	206	0.67%	0%	0.00%	0	0%	0.00%	0
6232	10%	339	1378	713	71	71	0.20%	0%	0.00%	0	0%	0.00%	0
6241	95%	2884	2543	2,633	2,501	2,501	6.99%	75%	5.24%	1,876	25%	1.75%	625
6242	100%	2072	2036	2,059	2,059	2,069	5.76%	100%	5.76%	2,059	0%	0.00%	0
6243	100%	2086	1942	2,041	2,041	2,041	6.71%	70%	3.99%	1,428	30%	1.71%	612
6251	80%	1817	1842	1,826	1,461	1,461	4.08%	0%	0.00%	0	0%	0.00%	0
6252	100%	433	1476	808	808	808	2.26%	0%	0.00%	0	0%	0.00%	0
6253	145%	145	1622	677	677	677	1.69%	0%	0.00%	0	0%	0.00%	0
6262	45%	82	104	104	47	47	0.13%	0%	0.00%	0	0%	0.00%	0
6311	10%	2	183	71	7	7	0.02%	100%	0.02%	7	0%	0.00%	0
6312	10%	1	1053	380	38	38	0.11%	50%	0.05%	19	50%	0.05%	19
6314	20%	2	0	47,886	1	0	0.00%	0%	0.00%	0	0%	0.00%	0
							100.00%				7,630		
							35,774				9,188		
							35,774				21,059		
							47,886				25,689		

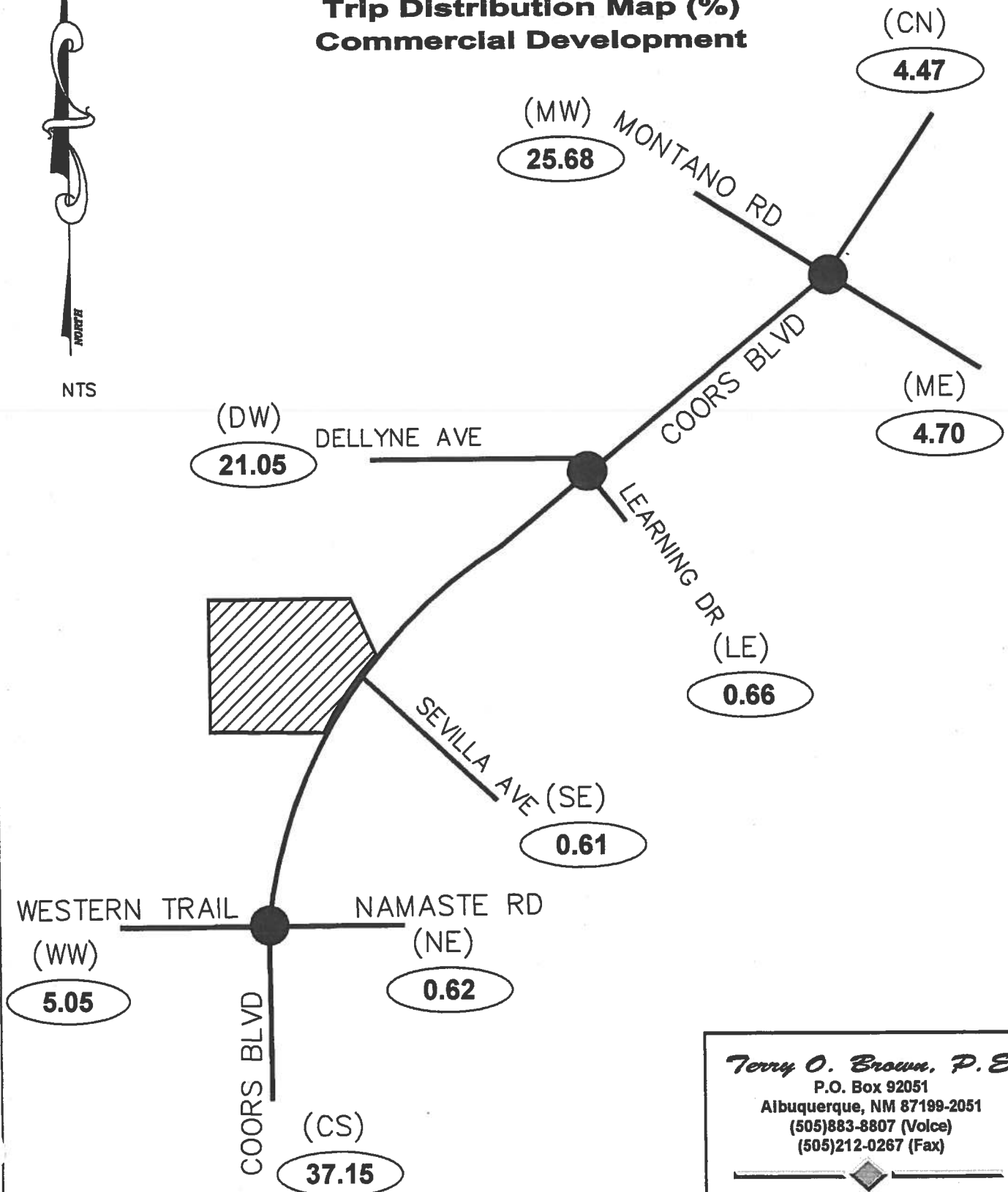
Esperia Development

(Sevilla Ave / Coors Blvd)

**Trip Distribution Map (%)
Commercial Development**



NTS



Terry O. Brown, P.E.

P.O. Box 92051

Albuquerque, NM 87199-2051

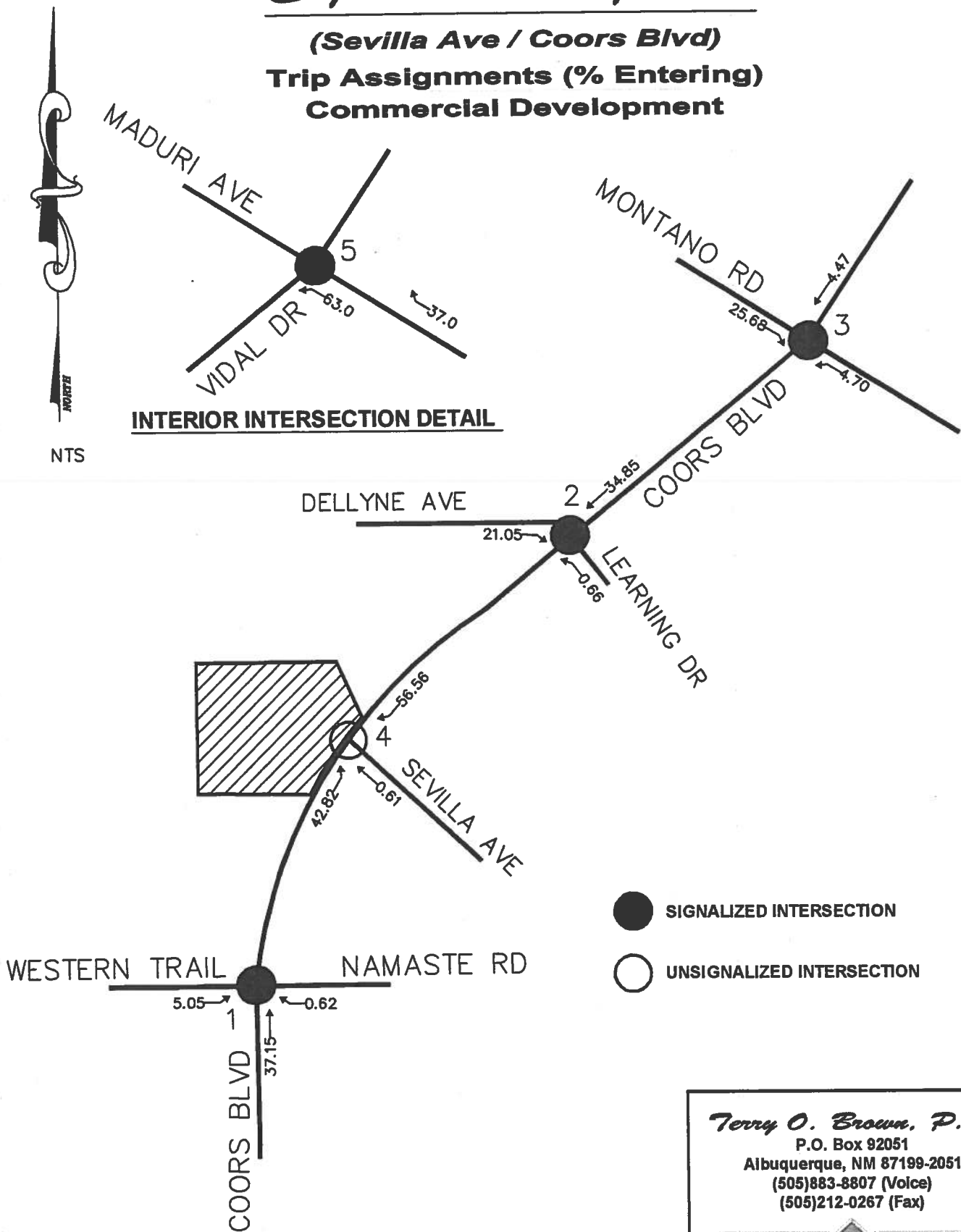
(505)883-8807 (Voice)

(505)212-0267 (Fax)

Esperia Development

(Sevilla Ave / Coors Blvd)

Trip Assignments (% Entering)
Commercial Development



Terry O. Brown, P.E.

P.O. Box 92051

Albuquerque, NM 87199-2051

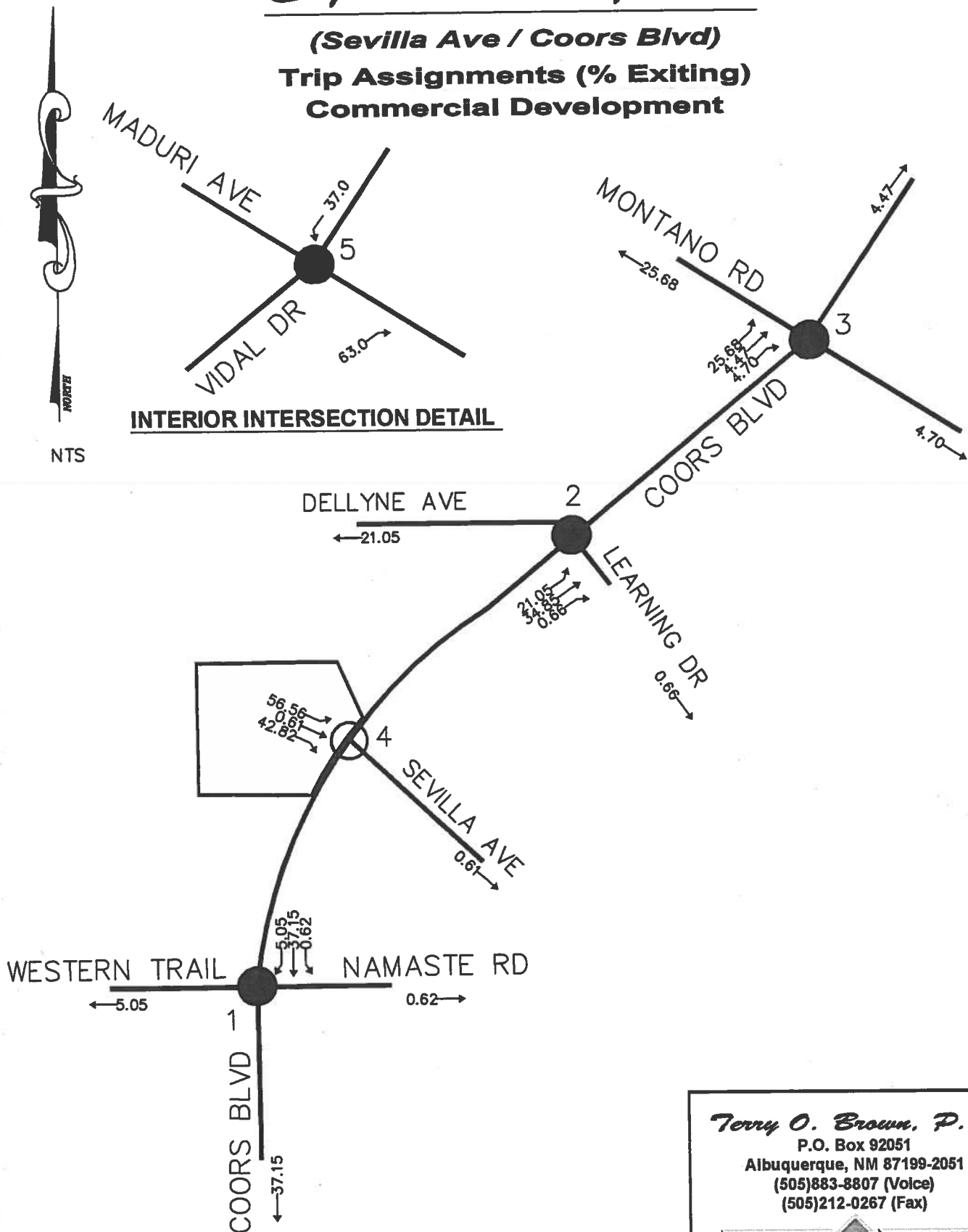
(505)883-8807 (Voice)

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Esperia Development

(Sevilla Ave / Coors Blvd)

Trip Assignments (% Exiting)
Commercial Development



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Subareas of State Planning and Development District 3

③ Subarea Identification Number

Note: Subarea boundaries extend to the county lines where not shown on map.

Sandoval Co.
Bernalillo Co.

Bernalillo Co.
Valencia Co.

Torrance Co.



Middle Rio Grande
Council of Governments
of New Mexico

317 Commercial N.E., Suite 200
Albuquerque, N.M. 87102 (505) 347-1750

3/96

Esperia Subdivision (Coors Blvd / Sevilla Ave)
Trip Distribution - Subarea Map

Figure 4

Trip Distribution Table

Esperia Development (Sevilla Ave / Coors Blvd)

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development

Data Taken from Middle Rio Grande Council of Governments' 2020 Socioeconomic Forecasts
for Data Analysis Subzones in State Planning and Development District 3 (TR-125) - Appendix "B"

Sub Area I.D.#	% Sub Area In Study	Population				Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	(CN) Coors Blvd North			(ME) Montano Rd East			(LE) Learning Dr East		
		2005	2010	2015	2025					% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population
1	100%	26,972	39,738	53,201	77,230	37,185	2,009	13.9	145	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,610	42,227	47,940	40,358	37,185	6.85	6,428	50%	1.78%	2,714	0%	0.00%	0	0%	0.00%	0
3	100%	7,865	8,728	9,336	9,591	8,555	8,555	8.2	1,043	100%	0.68%	1,043	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,836	15,923	16,527	14,826	14,826	16.15	908	100%	0.69%	908	0%	0.00%	0	0%	0.00%	0
5	100%	35,968	44,203	48,059	54,241	42,556	42,556	4.15	10,254	50%	3.35%	5,127	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,850	4,265	18,676	3,717	3,717	8.75	425	0%	0.28%	0	0%	0.00%	0	0%	0.00%	0
7	100%	48,565	59,615	64,196	75,089	57,405	57,405	1	57,405	9%	3.38%	5,166	1%	0.38%	574	0%	0.00%	0
8	100%	27,546	28,553	29,299	33,406	28,352	28,352	3.55	7,988	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
9	100%	1,678	1,888	2,055	2,438	1,846	1,846	17.4	108	0%	0.07%	0	0%	0.00%	0	0%	0.00%	0
10	100%	39,532	4,822	59,940	70,184	11,764	11,764	8.35	1,409	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
11	100%	32,051	33,202	37,130	36,279	32,416	32,416	8.15	3,977	50%	1.34%	2,044	50%	1.34%	2,044	0%	0.00%	0
12	100%	16,144	16,146	16,635	17,804	16,146	16,146	3.95	4,087	50%	0.62%	948	50%	0.62%	948	0%	0.00%	0
13	100%	8,715	10,146	10,348	11,137	9,860	9,860	5.2	1,886	50%	3.71%	5,665	50%	3.71%	5,665	0%	0.00%	0
14	100%	93,104	94,279	96,147	99,871	94,044	94,044	8.3	11,331	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
15	100%	24,691	25,262	25,949	25,919	25,148	25,148	2.8	8,981	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
16	100%	108,882	108,353	107,808	108,703	108,459	108,459	8.95	10,900	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
17	100%	20,920	21,196	22,042	22,645	21,680	21,680	4.2	5,182	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
18	100%	42,078	41,670	41,542	42,625	41,696	41,696	6.2	6,726	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
19	100%	59,027	58,888	60,441	60,385	60,475	60,475	10.75	6,628	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
20	100%	9,482	9,699	9,756	9,893	9,674	9,674	8.3	1,168	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
21	100%	6	6	6	6	6	6	11.5	1	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
22	100%	4,231	3,629	3,701	3,704	3,615	3,615	13.25	273	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
23	100%	18,140	20,380	21,613	24,186	20,069	20,069	16.3	1,231	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
24	100%	2,393	2,554	2,697	3,054	2,522	2,522	14.5	174	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
25	100%	1,009	1,062	1,127	1,252	1,051	1,051	17.55	60	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
26	100%	75,506	85,654	96,202	117,341	83,624	83,624	22	3,801	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
27	100%	20,955	22,276	23,694	26,710	22,012	22,012	17.1	1,287	50%	0.42%	644	0%	0.00%	0	0%	0.00%	0
28	100%	19,524	21,690	23,476	26,318	21,257	21,257	27	787	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
29	100%	11,390	13,771	16,206	20,579	13,289	13,289	50	266	100%	0.17%	266	0%	0.00%	0	0%	0.00%	0
		811,863	836,918	947,476	1,075,238	833,405	795,056		162,839	100.00%	100.00%	24,823	16.05%	8.98%	13,722	8.98%	0.00%	0.00%

* - Subarea in which the site is located.

Trip Distribution Table

Esperia Development (Serrilla Ave / Coors Blvd)

Sub Area Population Data:
For determination of Trip Distribution for Proposed Office Development

Data Taken from Middle Rio Grande Council of Governments' 2020 Socioeconomic Forecasts
for Data Analysis Subzones In State Planning and Development District 3 (TR-125) - Appendix "B"

Sub Area I.D.#	% Sub Area in Study	Population					Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	(NE)			(CS)			(WW)		
		2005	2010	2015	2025	2030					% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population
1	100%	26,972	39,738	53,201	77,230	37,185	2,009	145	13.9	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,610	42,227	47,940	40,358	37,185	6,428	6.85	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
3	100%	7,865	8,728	9,336	9,591	8,555	8,555	1,043	8.2	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,938	15,923	18,527	14,628	14,628	906	16.15	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
5	100%	35,968	44,203	48,059	54,241	42,558	42,558	10,264	4.15	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,950	4,265	18,676	3,717	3,717	426	8.75	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
7*	100%	48,565	59,615	64,196	75,089	57,405	57,405	7,406	3.55	0.39%	1%	0.39%	574	39%	14.65%	22,388	12%	4.51%	6,889
8	100%	27,546	28,553	29,299	33,406	28,352	28,352	7,308	17.4	0.00%	0%	0.00%	0	100%	5.23%	7,988	0%	0.00%	0
9	100%	1,678	1,888	2,055	2,438	1,846	1,846	106	17.4	0.00%	0%	0.00%	0	100%	0.92%	1,409	0%	0.00%	0
10	100%	39,532	4,822	59,940	70,184	11,764	11,764	3,977	8.35	0.00%	0%	0.00%	0	100%	2.60%	3,977	0%	0.00%	0
11	100%	32,051	33,202	37,130	38,279	16,146	16,146	4,087	3.95	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
12	100%	16,144	16,146	16,635	17,804	9,860	9,860	1,896	5.2	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
13	100%	8,715	10,146	10,348	11,137	9,404	9,404	1,331	8.3	0.00%	0%	0.00%	0	50%	2.94%	4,491	0%	0.00%	0
14	100%	93,104	94,279	96,147	99,871	25,148	25,148	8,981	2.8	0.00%	0%	0.00%	0	100%	7.13%	10,900	0%	0.00%	0
15	100%	24,691	25,262	25,949	25,919	108,459	108,459	9,95	9.95	0.00%	0%	0.00%	0	100%	3.38%	5,162	0%	0.00%	0
16	100%	108,882	108,353	107,808	106,703	21,680	21,680	5,182	4.2	0.00%	0%	0.00%	0	100%	4.40%	6,725	0%	0.00%	0
17	100%	20,920	21,196	22,042	22,645	41,696	41,696	6,726	6.2	0.00%	0%	0.00%	0	100%	3.68%	5,628	0%	0.00%	0
18	100%	42,078	41,670	41,542	42,625	60,475	60,475	6,828	10.75	0.00%	0%	0.00%	0	100%	0.76%	1,168	0%	0.00%	0
19	100%	59,027	58,888	60,441	60,385	9,674	9,674	1,188	8.3	0.00%	0%	0.00%	0	100%	0.00%	1	0%	0.00%	0
20	100%	9,482	9,699	9,756	9,893	6	6	1	11.5	0.00%	0%	0.00%	0	100%	0.18%	273	0%	0.00%	0
21	100%	6	6	2,463	9,511	3,615	3,615	273	13.25	0.00%	0%	0.00%	0	100%	0.81%	1,231	0%	0.00%	0
22	100%	4,231	3,629	3,701	3,704	20,069	20,069	1,231	16.3	0.00%	0%	0.00%	0	100%	0.11%	174	0%	0.00%	0
23	100%	18,140	20,360	21,613	24,186	2,522	2,522	174	14.5	0.00%	0%	0.00%	0	100%	0.04%	60	0%	0.00%	0
24	100%	2,363	2,554	2,697	3,054	1,051	1,051	80	17.55	0.00%	0%	0.00%	0	100%	2.49%	3,801	0%	0.00%	0
25	100%	1,008	1,052	1,127	1,252	83,624	83,624	3,801	22	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
26	100%	75,506	85,654	96,202	117,341	22,012	22,012	1,287	17.1	0.00%	0%	0.00%	0	100%	0.52%	787	0%	0.00%	0
27	100%	20,955	22,276	23,694	26,710	21,257	21,257	266	27	0.00%	0%	0.00%	0	100%	0.00%	0	0%	0.00%	0
28	100%	19,524	21,650	23,476	26,318	13,289	13,289	266	50	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
29	100%	11,390	13,771	16,206	20,579	833,405	833,405	162,839	674	0.38%	0%	0.38%	674	0%	50.18%	76,887	0%	4.51%	6,889
		811,863	836,916	947,476	1,075,238		795,056						50.18%			50.18%			4.51%

* - Subarea in which the site is located.

Trip Distribution Table

Esperia Development (Sevilla Ave / Coors Blvd)

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development

Data Taken from Middle Rio Grande Council of Governments' 2020 Socioeconomic Forecasts
for Data Analysis Subzones in State Planning and Development District 3 (TR-125) - Appendix "B"

Sub Area i.d.#	% Sub Area in Study	Population					Interpolated Population for the Year	Population in Study	Dist. (MI.)	Population / Distance	(DW)			(MW)		
		2005	2010	2015	2020	2025					% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population
1	100%	26,972	39,738	53,201	77,230	101,259	37,185	2,009	13.9	145	100%	0.09%	145	0%	0.00%	0
2	100%	39,348	40,610	42,227	47,940	53,658	37,185	37,185	6.85	5,428	50%	1.78%	2,714	0%	0.00%	0
3	100%	7,865	8,728	9,338	9,591	9,844	8,555	8,555	8.2	1,043	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,938	15,923	18,527	21,131	14,626	14,626	18.15	806	0%	0.00%	0	0%	0.00%	0
5	100%	35,968	44,203	48,059	54,241	60,423	42,558	42,558	4.15	10,284	50%	3.35%	5,127	0%	0.00%	0
6	100%	2,784	3,950	4,265	4,865	5,465	3,717	3,717	8.75	425	0%	0.00%	0	0%	0.00%	0
7*	100%	48,565	59,615	64,196	75,089	85,982	57,405	57,405	1	57,405	14%	5.26%	8,037	24%	9.01%	13,777
8	100%	27,546	28,553	29,298	33,406	38,514	28,352	28,352	3.55	7,986	0%	0.00%	0	0%	0.00%	0
9	100%	1,678	1,888	2,055	2,438	2,821	1,846	1,846	17.4	106	0%	0.00%	0	0%	0.00%	0
10	100%	39,532	4,822	59,940	70,184	80,428	11,764	11,764	8.35	1,409	0%	0.00%	0	0%	0.00%	0
11	100%	32,051	33,202	37,130	38,279	42,416	32,416	32,416	8.15	3,977	0%	0.00%	0	0%	0.00%	0
12	100%	16,144	16,146	16,635	17,804	19,073	16,146	16,146	3.95	4,087	0%	0.00%	0	0%	0.00%	0
13	100%	8,715	10,146	10,348	11,137	11,926	9,860	9,860	5.2	1,886	0%	0.00%	0	0%	0.00%	0
14	100%	93,104	94,279	96,147	99,671	103,195	94,044	94,044	8.3	11,331	0%	0.00%	0	0%	0.00%	0
15	100%	24,691	25,262	25,949	25,919	26,588	25,148	25,148	2.8	8,861	0%	0.00%	0	0%	0.00%	0
16	100%	108,882	108,353	107,806	106,703	105,600	108,459	108,459	9.95	10,900	0%	0.00%	0	0%	0.00%	0
17	100%	20,920	21,196	22,042	22,645	23,248	21,680	21,680	4.2	5,162	0%	0.00%	0	0%	0.00%	0
18	100%	42,078	41,670	41,542	42,825	44,108	41,686	41,686	6.2	6,725	0%	0.00%	0	0%	0.00%	0
19	100%	59,027	58,888	60,441	60,385	60,475	60,475	60,475	10.75	5,628	0%	0.00%	0	0%	0.00%	0
20	100%	9,482	9,689	9,756	9,893	10,030	9,674	9,674	8.3	1,166	0%	0.00%	0	0%	0.00%	0
21	100%	6	6	2,463	9,511	16,511	6	6	11.5	1	0%	0.00%	0	0%	0.00%	0
22	100%	4,231	3,629	3,701	3,704	3,704	3,615	3,615	13.25	273	0%	0.00%	0	0%	0.00%	0
23	100%	18,140	20,390	21,613	24,186	26,759	20,069	20,069	16.3	1,231	0%	0.00%	0	0%	0.00%	0
24	100%	2,393	2,554	2,697	3,054	3,411	2,522	2,522	14.5	174	0%	0.00%	0	0%	0.00%	0
25	100%	1,009	1,062	1,127	1,252	1,377	1,051	1,051	17.55	60	0%	0.00%	0	0%	0.00%	0
26	100%	75,506	85,654	96,202	117,341	138,480	83,624	83,624	22	3,801	0%	0.00%	0	0%	0.00%	0
27	100%	20,955	22,276	23,694	26,710	29,726	22,012	22,012	17.1	1,287	50%	0.42%	644	0%	0.00%	0
28	100%	19,524	21,690	23,476	26,318	29,154	21,257	21,257	27	787	0%	0.00%	0	0%	0.00%	0
29	100%	11,360	13,771	16,206	20,579	25,004	13,289	13,289	50	266	0%	0.00%	0	0%	0.00%	0
		811,863	836,918	947,476	1,075,238	1,206,566	833,405	795,056		152,839		10.90%	16,668		9.01%	13,777
													10,90%			9,01%

* - Subarea in which the site is located.

Trip Distribution Table

Esperia Development (Sevilla Ave / Coors Blvd)

Sub Area Population Data:

For determination of Trip Distribution for Proposed R&S Development

Data Taken from Middle Rio Grande Council of Governments' 2020 Socioeconomic Forecasts

for Data Analysis Subzones in State Planning and Development District 3 (TR-125) - Appendix "B"

Sub Area I.D.#	% Sub Area In Study	Population				Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	Coors Blvd North			(ME) Montano Rd East			(LE) Learning Dr East		
		2005	2010	2015	2025					% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population
1	100%	26,972	39,738	53,201	77,230	37,185	2,009	20.5	98	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,610	42,227	47,940	40,358	37,185	11.75	3,166	50%	0.87%	1,582	0%	0.00%	0	0%	0.00%	0
3	100%	7,865	8,728	9,336	9,591	8,555	8,555	9.75	877	100%	0.54%	877	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,938	15,923	18,527	14,928	14,928	14.25	1,026	100%	0.63%	1,026	0%	0.00%	0	0%	0.00%	0
5	100%	35,968	44,203	48,059	54,241	42,558	42,558	8.75	4,864	50%	1.49%	2,432	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,950	4,265	18,676	3,717	3,717	16	232	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
7*	100%	48,565	59,815	64,198	75,089	57,405	57,405	7.5	7,654	9%	0.42%	689	1%	0.05%	77	0%	0.00%	0
8	100%	27,548	28,553	29,299	33,408	28,352	28,352	7	4,060	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
9	100%	39,532	4,922	37,130	38,279	1,846	1,846	23.5	79	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
10	100%	32,051	33,202	37,130	38,279	32,416	32,416	8.5	3,814	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
11	100%	16,144	16,146	16,635	17,804	16,146	16,146	8	2,691	50%	1.65%	1,345	50%	0.83%	1,345	0%	0.00%	0
12	100%	8,715	10,146	10,348	11,137	9,860	9,860	5.25	1,878	50%	0.81%	938	50%	0.58%	938	0%	0.00%	0
13	100%	93,104	94,279	96,147	99,871	94,044	94,044	4.25	15,874	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
14	100%	24,691	25,262	25,949	25,919	25,148	25,148	4.25	6,817	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
15	100%	108,882	108,353	107,806	106,703	108,459	108,459	3	36,163	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
16	100%	20,920	21,198	22,042	22,645	21,880	21,880	3.75	5,781	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
17	100%	42,078	41,870	41,542	42,825	41,888	41,888	3.75	41,886	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
18	100%	59,027	58,888	60,441	60,385	60,475	60,475	3.75	16,127	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
19	100%	9,482	9,698	9,756	9,883	9,674	9,674	7.25	1,334	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
20	100%	6	6	2,463	9,511	6	6	8.25	1	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
21	100%	4,231	3,628	3,701	3,704	3,615	3,615	7.5	482	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
22	100%	18,140	20,390	21,813	24,186	20,069	20,069	10.75	1,867	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
23	100%	2,353	2,554	2,897	3,054	2,522	2,522	15.5	163	0%	0.10%	0	0%	0.00%	0	0%	0.00%	0
24	100%	1,009	1,062	1,127	1,252	1,051	1,051	18.75	63	0%	0.04%	0	0%	0.00%	0	0%	0.00%	0
25	100%	75,508	85,654	98,202	117,341	83,824	83,824	22.5	3,717	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
26	100%	20,955	22,276	23,694	26,710	22,012	22,012	26.25	839	50%	0.81%	419	0%	0.00%	0	0%	0.00%	0
27	100%	19,524	21,690	23,478	26,318	21,257	21,257	22.25	966	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
28	100%	11,360	13,771	16,206	20,579	13,289	13,289	22.25	897	100%	0.37%	597	0%	0.00%	0	0%	0.00%	0
29	100%	811,863	836,916	947,476	1,075,238	833,405	785,056		162,838	100.00%	100.00%	17,746	8.08%	8.08%	13,167	0.00%	0.00%	0
												10.90%			8.08%			0.00%

* - Subarea in which the site is located.

Trip Distribution Table

Esperia Development (Sevilla Ave / Coors Blvd)

Sub Area Population Data:
For determination of Trip Distribution for Proposed Re-S. Development

Data Taken from Middle Rio Grande Council of Governments' 2020 Socioeconomic Forecasts
for Data Analysis Subzones in State Planning and Development District 3 (TR-125) - Appendix "B"

Sub Area I.D.#	% Sub Area In Study	Population					Population in Study	Dist. (Mi.)	Population / Distance	(NE)			(CS)			(WW)		
		2005	2010	2015	2025	Interpolated Population for the Year				% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population
1	100%	26,972	39,738	53,201	77,230	37,185	2,009	20.5	98	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,610	42,227	47,940	40,358	37,185	11.75	3,166	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
3	100%	7,865	8,728	9,338	9,591	8,555	8,555	9.75	877	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,936	16,923	18,527	14,628	14,628	14.25	1,026	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
5	100%	35,988	44,203	48,059	54,241	42,556	42,556	8.75	4,864	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,850	4,265	4,676	3,717	16	16	232	0%	0.00%	0	100%	0.14%	232	0%	0.00%	0
7*	100%	48,566	59,615	64,198	75,089	57,405	57,405	7.5	7,654	1%	0.05%	77	39%	1.83%	2,885	12%	0.56%	918
8	100%	27,548	28,553	29,269	33,408	28,352	28,352	23.5	1,208	0%	0.00%	0	100%	2.49%	4,050	0%	0.00%	0
9	100%	39,532	4,822	59,940	70,184	11,784	11,784	11.25	1,048	0%	0.00%	0	100%	0.64%	1,048	0%	0.00%	0
10	100%	32,051	33,202	37,130	38,279	32,416	32,416	8.5	3,814	0%	0.00%	0	100%	2.34%	3,814	0%	0.00%	0
11	100%	16,144	16,146	18,835	17,804	16,146	16,146	6	2,891	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
12	100%	8,715	10,148	10,348	11,137	9,660	9,660	5.25	1,878	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
13	100%	83,104	94,279	96,147	98,871	94,044	94,044	4.25	6,917	0%	0.00%	0	50%	1.82%	2,859	0%	0.00%	0
14	100%	24,691	25,282	25,949	25,919	25,148	25,148	4.25	6,917	0%	0.00%	0	100%	22.20%	38,153	0%	0.00%	0
15	100%	108,882	108,353	107,808	106,703	108,459	108,459	3	36,163	0%	0.00%	0	100%	3.55%	5,781	0%	0.00%	0
16	100%	20,920	21,186	22,042	22,845	21,690	21,690	3.75	5,781	0%	0.00%	0	100%	25.81%	41,698	0%	0.00%	0
17	100%	42,078	41,670	41,542	42,625	41,696	41,696	3.75	16,127	0%	0.00%	0	100%	9.90%	18,127	0%	0.00%	0
18	100%	59,027	58,888	60,441	60,395	60,475	60,475	3.75	16,127	0%	0.00%	0	100%	0.82%	1,334	0%	0.00%	0
19	100%	9,482	9,698	9,768	9,883	9,674	9,674	7.25	1,334	0%	0.00%	0	100%	0.00%	1	0%	0.00%	0
20	100%	6	6	2,463	9,511	6	6	8.25	1	0%	0.00%	0	100%	0.30%	482	0%	0.00%	0
21	100%	4,231	3,629	3,701	3,704	3,615	3,615	7.5	482	0%	0.00%	0	100%	1.15%	1,887	0%	0.00%	0
22	100%	18,140	20,390	21,613	24,186	20,089	20,089	10.75	1,887	0%	0.00%	0	100%	0.10%	163	0%	0.00%	0
23	100%	2,393	2,554	2,697	3,054	2,522	2,522	15.5	163	0%	0.00%	0	100%	0.04%	63	0%	0.00%	0
24	100%	1,009	1,062	1,127	1,262	1,061	1,061	16.75	63	0%	0.00%	0	100%	2.28%	3,717	0%	0.00%	0
25	100%	75,508	85,654	96,202	117,341	83,624	83,624	22.5	3,717	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
26	100%	20,955	22,276	23,694	26,710	22,012	22,012	26.25	839	0%	0.00%	0	100%	0.59%	955	0%	0.00%	0
27	100%	19,524	21,690	23,476	26,318	21,257	21,257	22.25	966	0%	0.00%	0	100%	0.00%	0	0%	0.00%	0
28	100%	11,360	13,771	16,208	20,579	13,288	13,288	22.25	697	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
29	100%	811,963	836,916	947,476	1,075,238	833,405	795,056		162,838	0.05%	0.05%	77	0%	75.84%	123,602	0%	0.56%	918
										0.05%			75.84%			0.56%		

* - Subarea in which the site is located.

Trip Distribution Table

Esperia Development (Sevilla Ave / Coors Blvd)

Sub Area Population Data:

For determination of Trip Distribution for Proposed R_{25} Development

Data Taken from Middle Rio Grande Council of Governments' 2020 Socioeconomic Forecasts
for Data Analysis Subzones in State Planning and Development District 3 (TR-129) - Appendix "B"

Sub Area I.D.#	% Sub Area In Study	Population					Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	(DW)			(MW)		
		2005	2010	2015	2025	2009					% Utilizing	% Population / Dist Utilizing	Population	% Utilizing	% Population / Dist Utilizing	Population
1	100%	26,972	39,738	53,201	77,230	37,185	2,009	2,009	20.5	98	100%	0.08%	98	0%	0.00%	0
2	100%	39,348	40,610	42,227	47,940	40,358	37,185	37,185	11.75	3,165	50%	0.97%	1,562	0%	0.00%	0
3	100%	7,985	8,728	9,336	9,591	8,555	8,555	8,555	9.75	877	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,938	15,923	18,527	14,928	14,928	14,928	14.25	1,026	0%	0.00%	0	0%	0.00%	0
5	100%	35,988	44,203	48,059	54,241	42,558	42,558	42,558	8.75	4,864	50%	1.49%	2,432	0%	0.00%	0
6	100%	2,784	3,950	4,265	18,878	3,717	3,717	3,717	16	232	0%	0.00%	0	0%	0.00%	0
7*	100%	48,565	59,615	64,198	75,089	57,405	57,405	57,405	7.5	7,654	14%	0.69%	1,072	24%	1.13%	1,837
8	100%	27,548	28,553	29,289	33,406	28,352	28,352	28,352	7	4,050	0%	0.00%	0	0%	0.00%	0
9	100%	1,678	1,888	2,055	2,438	1,846	1,846	1,846	23.5	79	0%	0.00%	0	0%	0.00%	0
10	100%	32,051	33,202	37,130	38,278	32,416	32,416	32,416	11.75	1,048	0%	0.00%	0	0%	0.00%	0
11	100%	16,144	16,148	18,635	17,804	18,146	18,146	18,146	8.5	3,814	0%	0.00%	0	0%	0.00%	0
12	100%	8,715	10,148	10,348	11,137	9,860	9,860	9,860	5.25	1,578	0%	0.00%	0	0%	0.00%	0
13	100%	93,104	94,278	96,147	99,871	94,044	94,044	94,044	6	15,674	0%	0.00%	0	0%	0.00%	0
14	100%	24,891	25,262	25,949	25,919	25,148	25,148	25,148	4.25	5,917	0%	0.00%	0	0%	0.00%	0
15	100%	108,882	108,353	107,806	106,703	108,459	108,459	108,459	3	36,163	0%	0.00%	0	0%	0.00%	0
16	100%	20,820	21,186	22,042	22,645	21,680	21,680	21,680	3.75	5,781	0%	0.00%	0	0%	0.00%	0
17	100%	42,078	41,570	41,542	42,625	41,988	41,988	41,988	1	41,988	0%	0.00%	0	0%	0.00%	0
18	100%	59,027	58,888	60,441	60,385	60,475	60,475	60,475	3.75	16,127	0%	0.00%	0	0%	0.00%	0
19	100%	9,482	9,989	9,756	9,893	9,674	9,674	9,674	7.25	1,334	0%	0.00%	0	0%	0.00%	0
20	100%	6	6	2,463	9,511	6	6	6	6.25	1	0%	0.00%	0	0%	0.00%	0
21	100%	4,231	3,829	3,701	3,704	3,615	3,615	3,615	7.5	482	0%	0.00%	0	0%	0.00%	0
22	100%	18,140	20,360	21,813	24,186	20,089	20,089	20,089	10.75	1,867	0%	0.00%	0	0%	0.00%	0
23	100%	2,393	2,554	2,697	3,054	2,522	2,522	2,522	16.5	163	0%	0.00%	0	0%	0.00%	0
24	100%	1,009	1,082	1,127	1,252	1,051	1,051	1,051	16.75	63	0%	0.00%	0	0%	0.00%	0
25	100%	75,508	85,654	98,202	117,341	83,624	83,624	83,624	22.5	3,717	0%	0.00%	0	0%	0.00%	0
26	100%	20,955	22,278	23,684	26,710	22,012	22,012	22,012	26.25	839	50%	0.28%	419	0%	0.00%	0
27	100%	19,524	21,690	23,476	26,318	21,257	21,257	21,257	22.25	966	0%	0.00%	0	0%	0.00%	0
28	100%	11,360	13,771	16,206	20,578	13,289	13,289	13,289	22.25	597	0%	0.00%	0	0%	0.00%	0
29	100%	811,883	838,916	947,476	1,075,238	833,405	795,056	795,056	162,838	162,838	0%	3.44%	5,603	0%	1.13%	1,837
													3,44%			1,13%

* - Subareas in which the site is located.

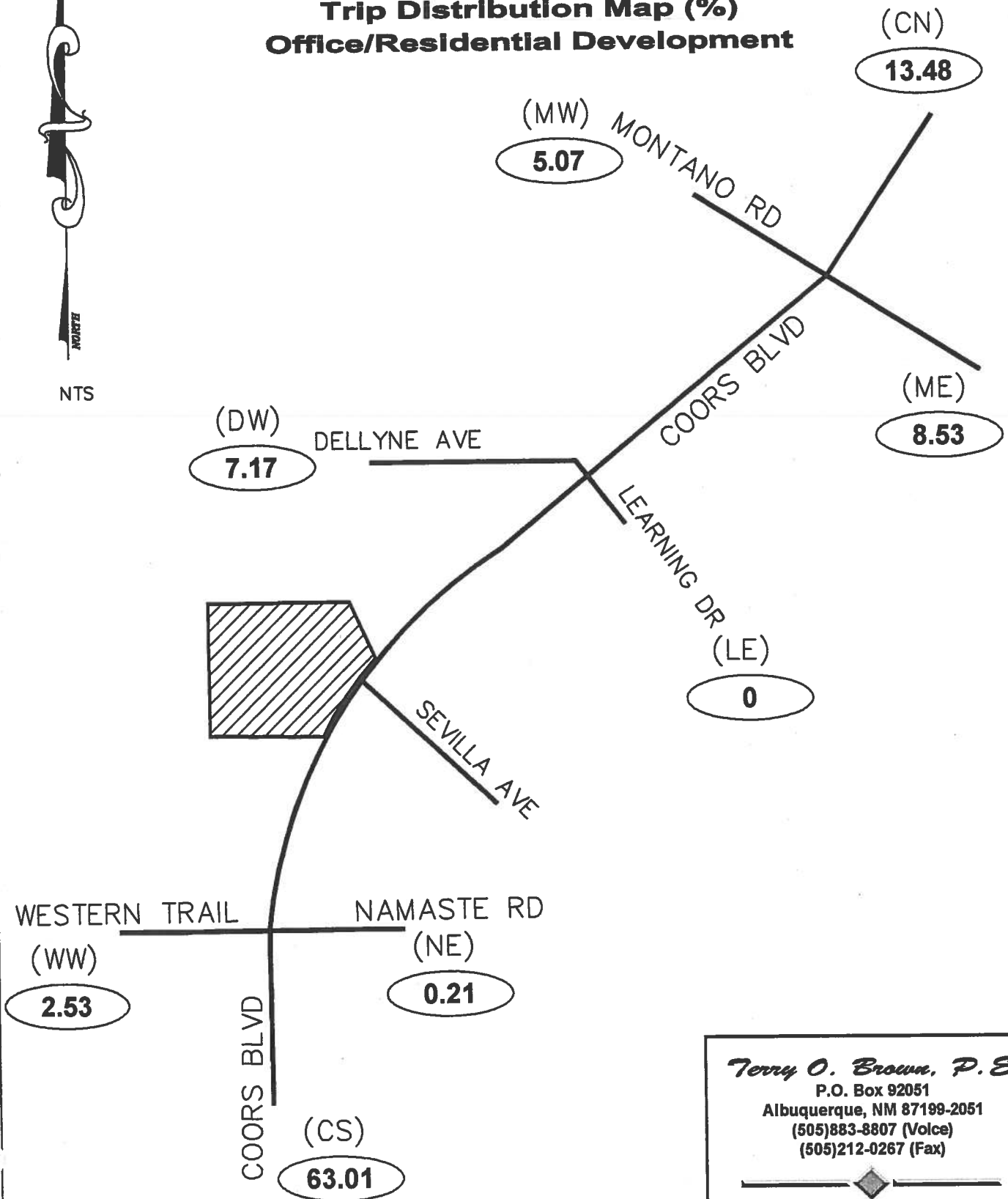
Esperia Development

(Sevilla Ave / Coors Blvd)

Trip Distribution Map (%)
Office/Residential Development



NTS



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Albuquerque, NM 87199-2051

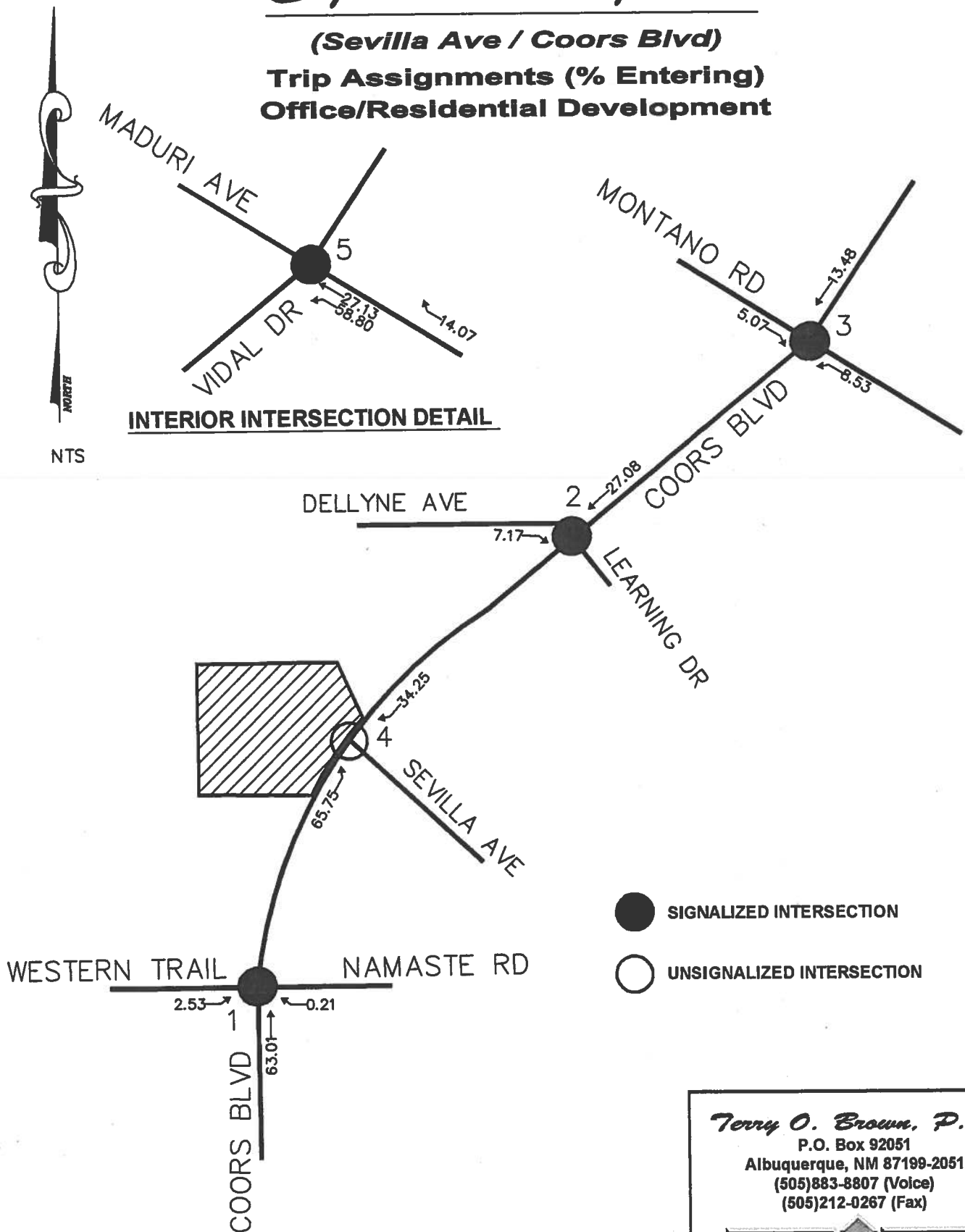
(505)883-8807 (Voice)

(505)212-0267 (Fax)

Esperia Development

(Sevilla Ave / Coors Blvd)

**Trip Assignments (% Entering)
Office/Residential Development**



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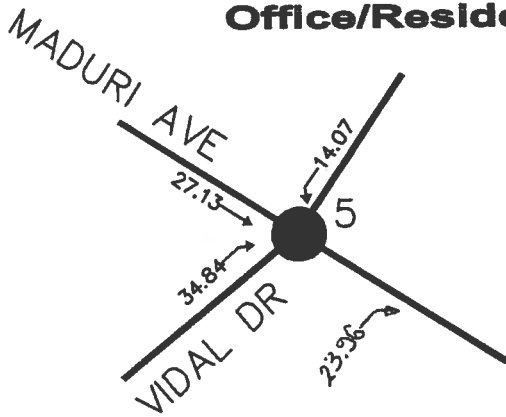
Esperia Development

(Sevilla Ave / Coors Blvd)

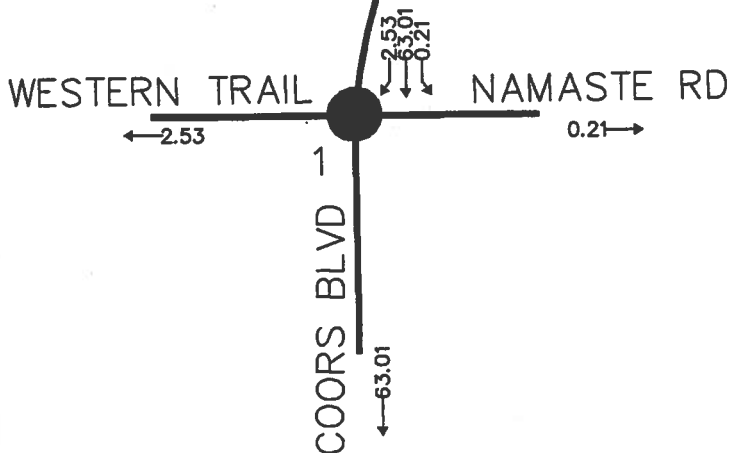
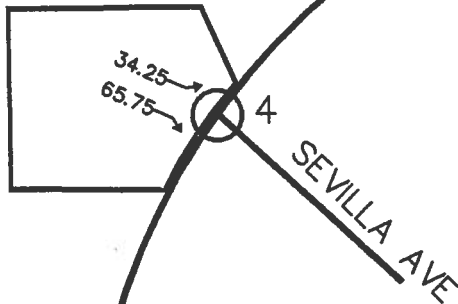
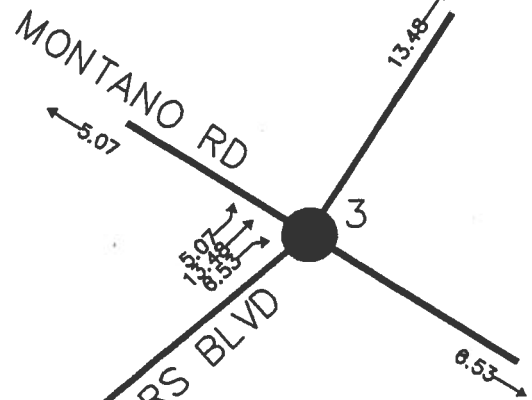
**Trip Assignments (% Exiting)
Office/Residential Development**



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INTERIOR INTERSECTION DETAIL



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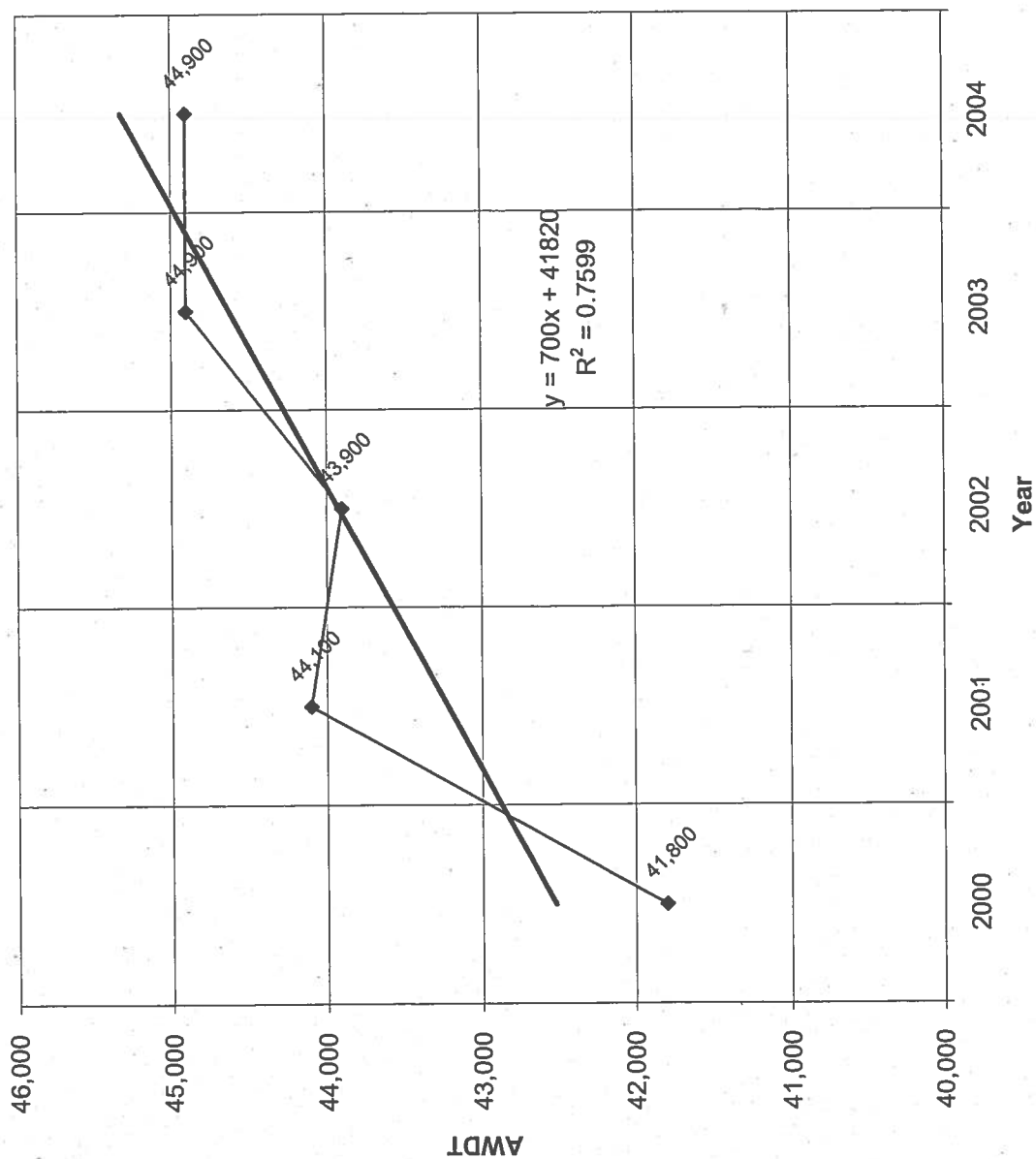
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Esperia Development
Historic Growth Rate Table

Traffic Flows from MRCOG Map

	2000	2001	2002	2003	2004
Coors Blvd North of Montañó Rd	41,800	44,100	43,900	44,900	44,900
Montañó Rd West of Coors Blvd	24,100	24,800	22,500	23,000	25,200
Montañó Rd East of Coors Blvd	24,700	26,400	25,600	25,700	26,600
Coors Blvd South of Montañó Rd	45,100	48,400	49,800	49,000	48,600
Dellyne Ave West of Coors Blvd	6,200	6,400	6,600	7,000	7,200
Coors Blvd South of Dellyne Ave	47,100	49,000	46,700	47,800	49,400
Western Trail West of Coors Blvd	2,100	2,200	2,500	2,400	2,400
Coors Blvd South of Western Trail	46,700	48,600	47,100	48,600	50,200

Historic Growth Chart Coors Blvd North of Montañó Rd (2000-2004)



◆ Coors Blvd North of Montañó Rd
 — Linear (Coors Blvd North of Montañó Rd)

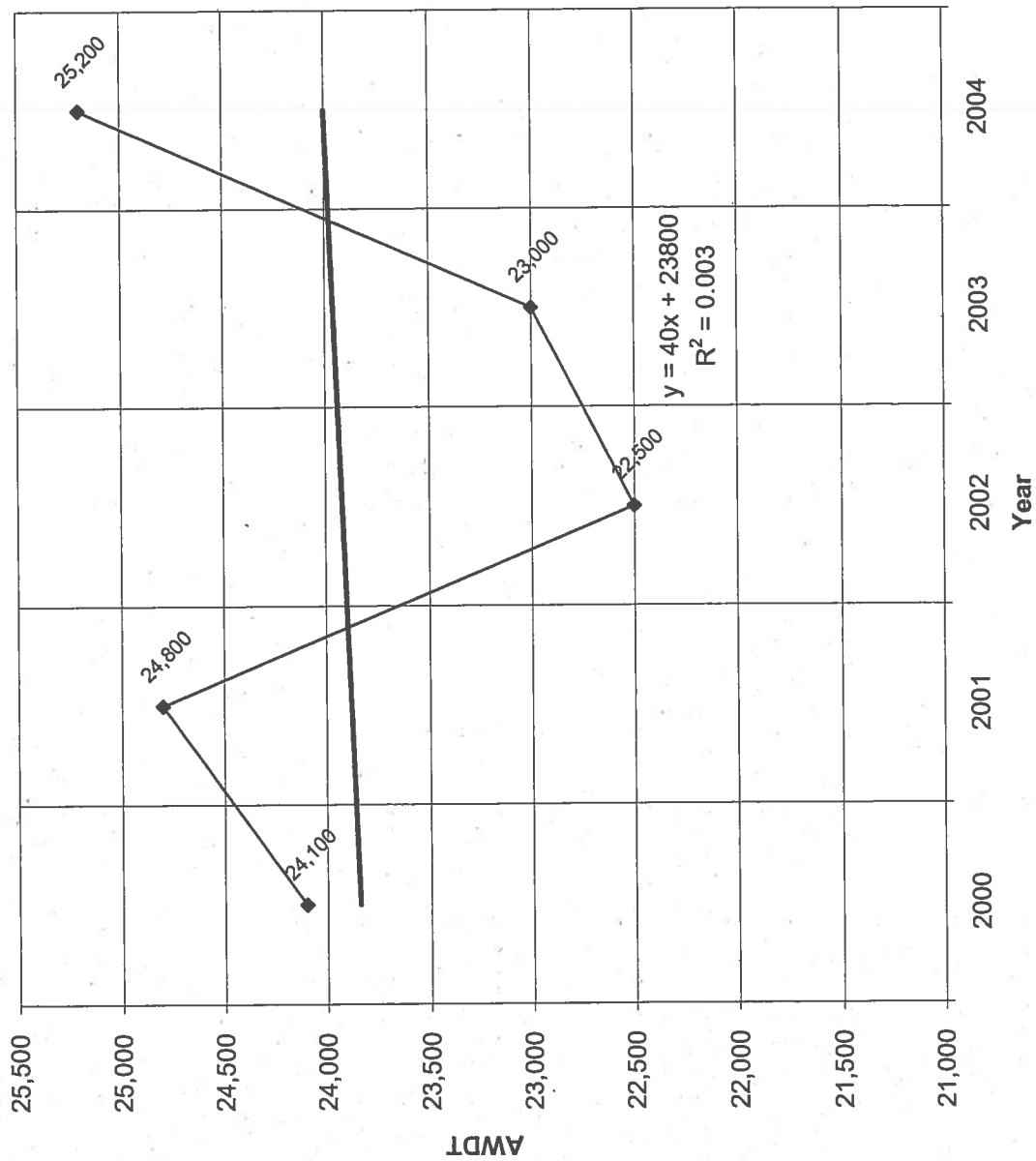
Annual Increase (Trendline) = 700 veh./day

Growth Rate = 700 / 44900

Growth Rate = .0155

Use 1.6% Growth Rate

Historic Growth Chart Montaña Rd West of Coors Blvd (2000-2004)



— Montaña Rd West of Coors Blvd
— Linear (Montaña Rd West of Coors Blvd)

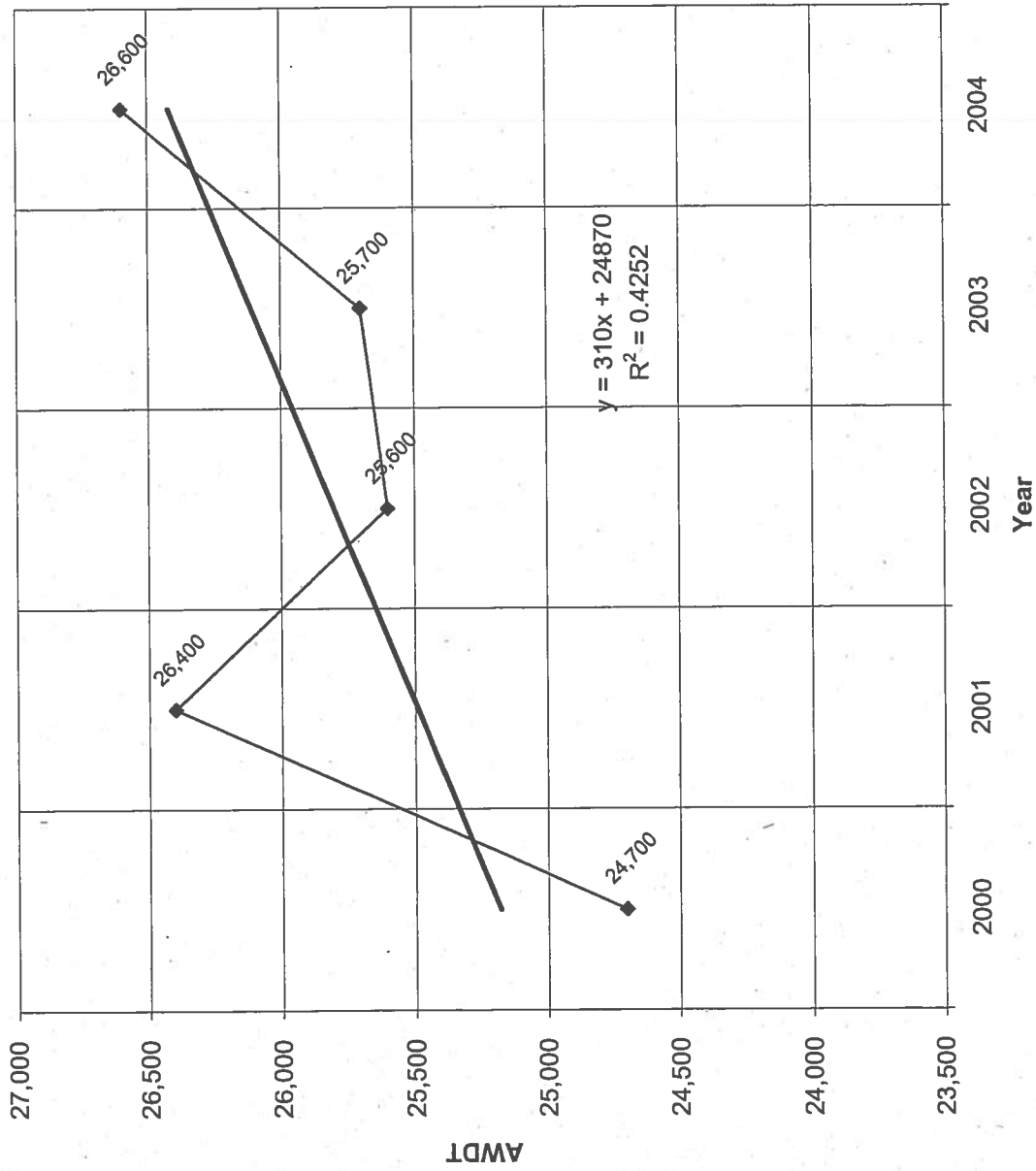
Annual Increase (Trendline) = 40 veh./day

Growth Rate = $40 / 25200$

Growth Rate = .0015

Use 0.2% Growth Rate

Historic Growth Chart Montaña Rd East of Coors Blvd (2000-2004)



—◆— Montaña Rd East of Coors Blvd
 — Linear (Montaña Rd East of Coors Blvd)

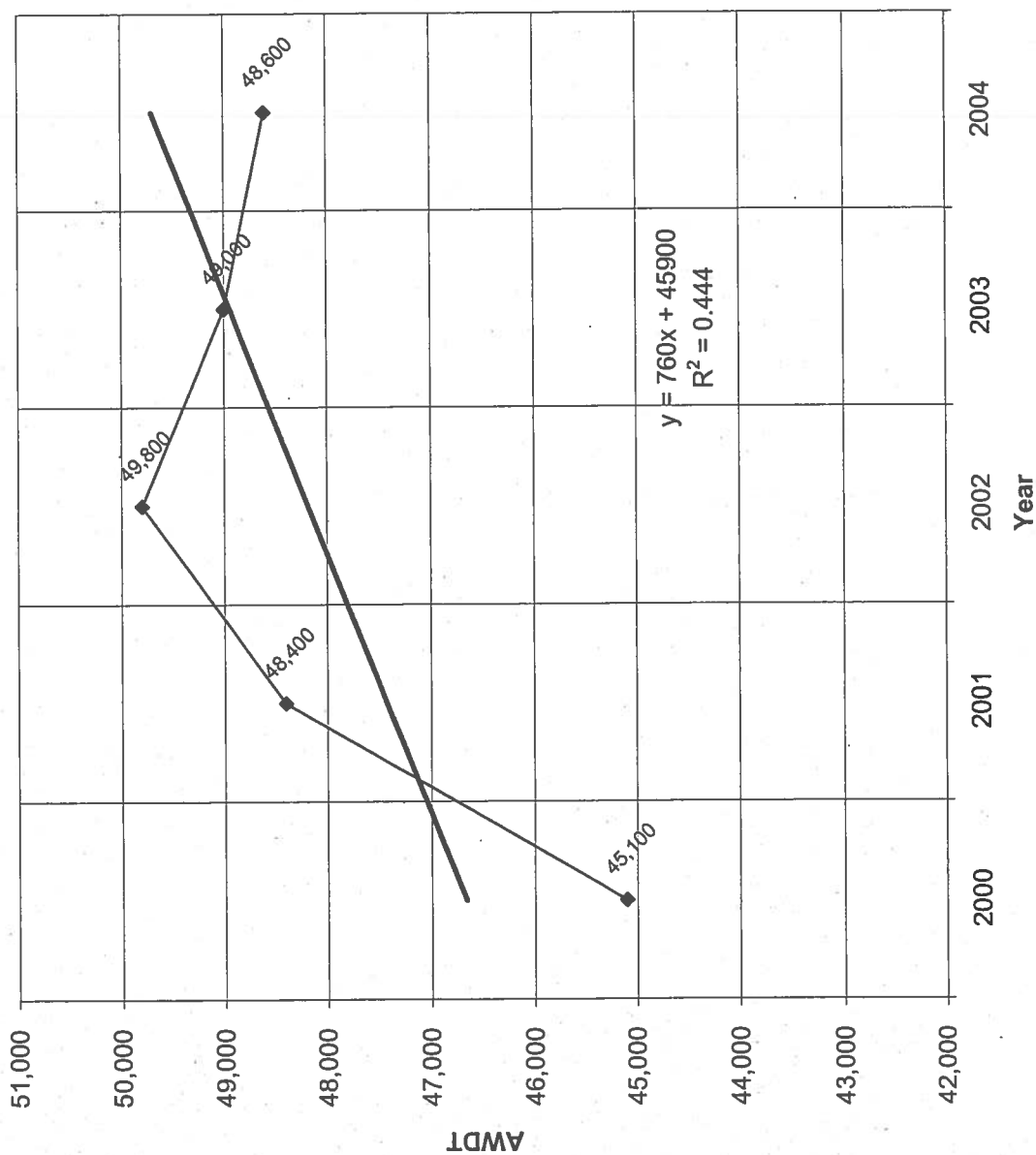
Annual Increase (Trendline) = 310 veh./day

Growth Rate = 310 / 26600

Growth Rate = .0116

Use 1.2% Growth Rate

Historic Growth Chart Coors Blvd South of Montañó Rd (1999-2003)



◆ Coors Blvd South of Montañó Rd
 — Linear (Coors Blvd South of Montañó Rd)

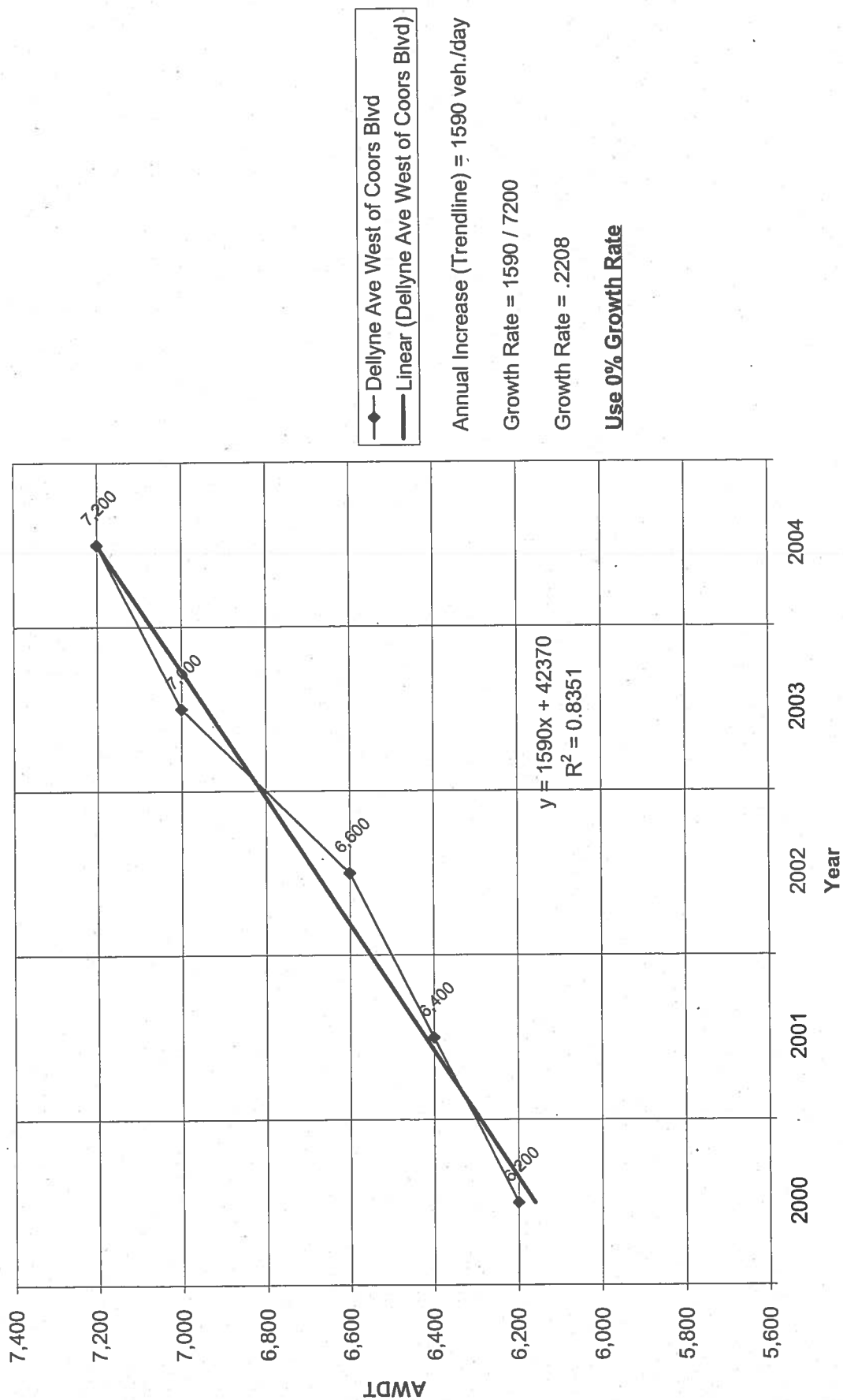
Annual Increase (Trendline) = 760 veh./day

Growth Rate = 760 / 48600

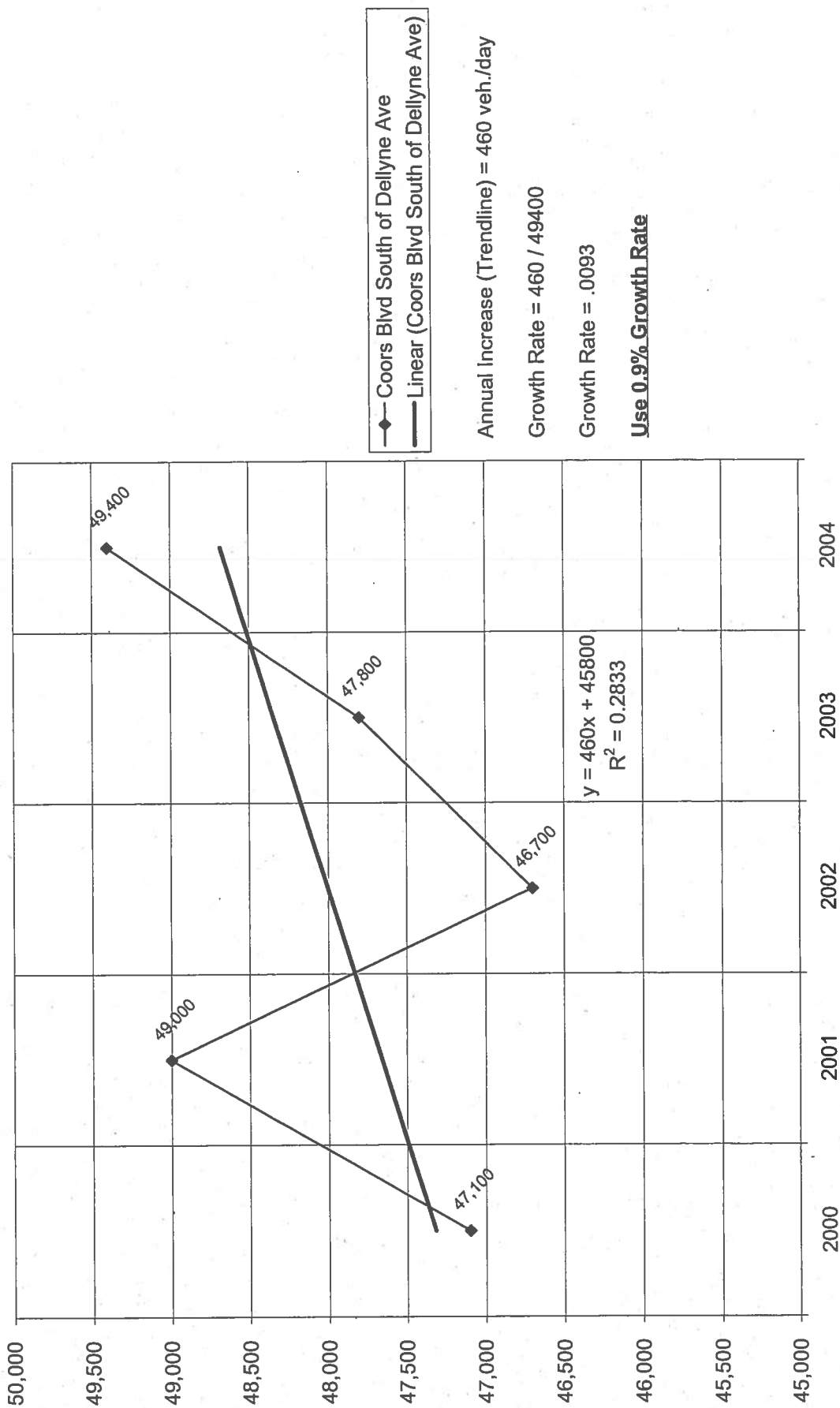
Growth Rate = .0156

Use 1.6% Growth Rate

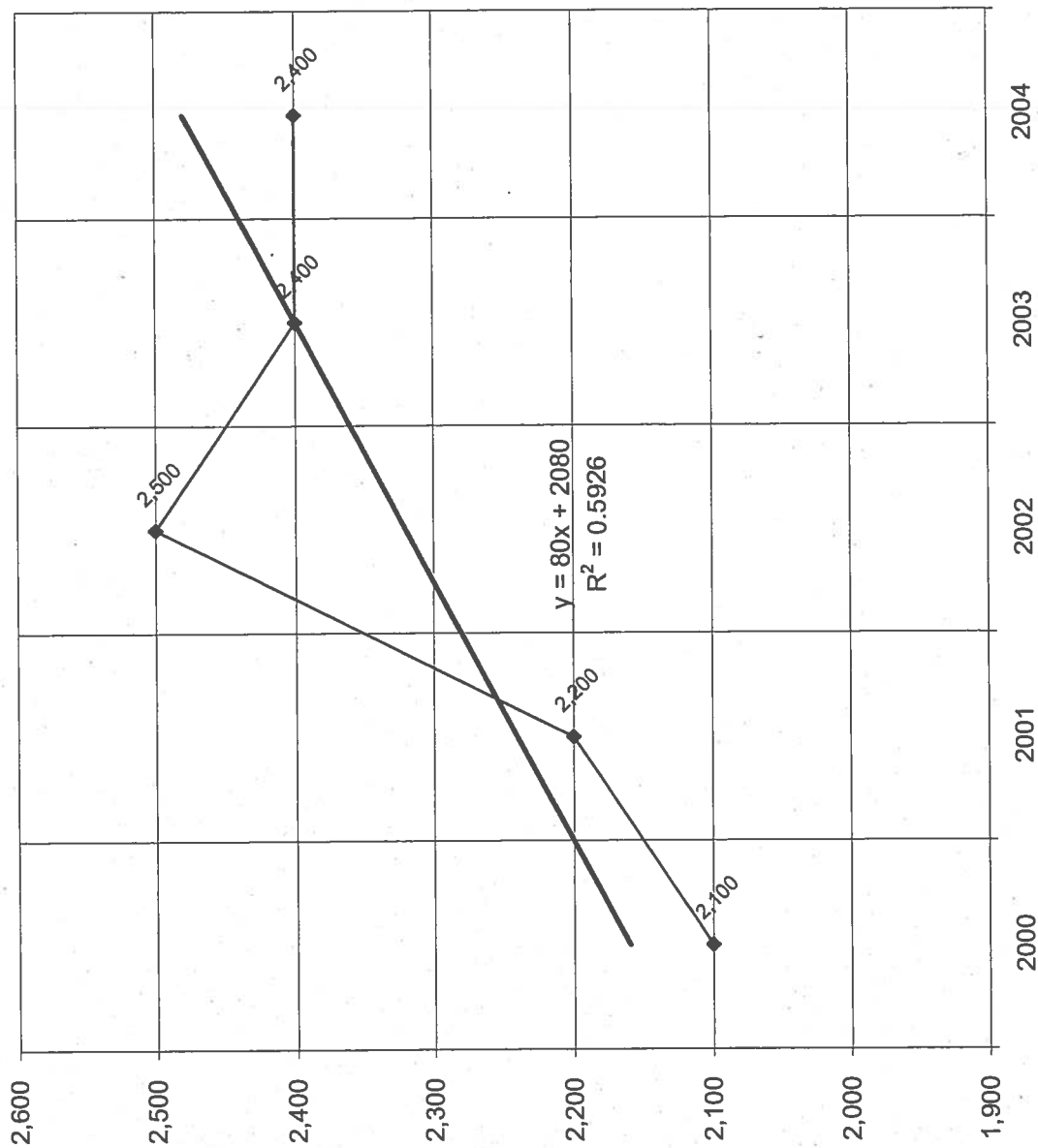
Historic Growth Chart Dellyne Ave West of Coors Blvd (1999-2003)



Historic Growth Chart Coors Blvd South of Dellyne Ave (1999-2003)



Historic Growth Chart Western Trail West of Coors Blvd (1999-2003)



◆ Western Trail West of Coors Blvd
 — Linear (Western Trail West of Coors Blvd)

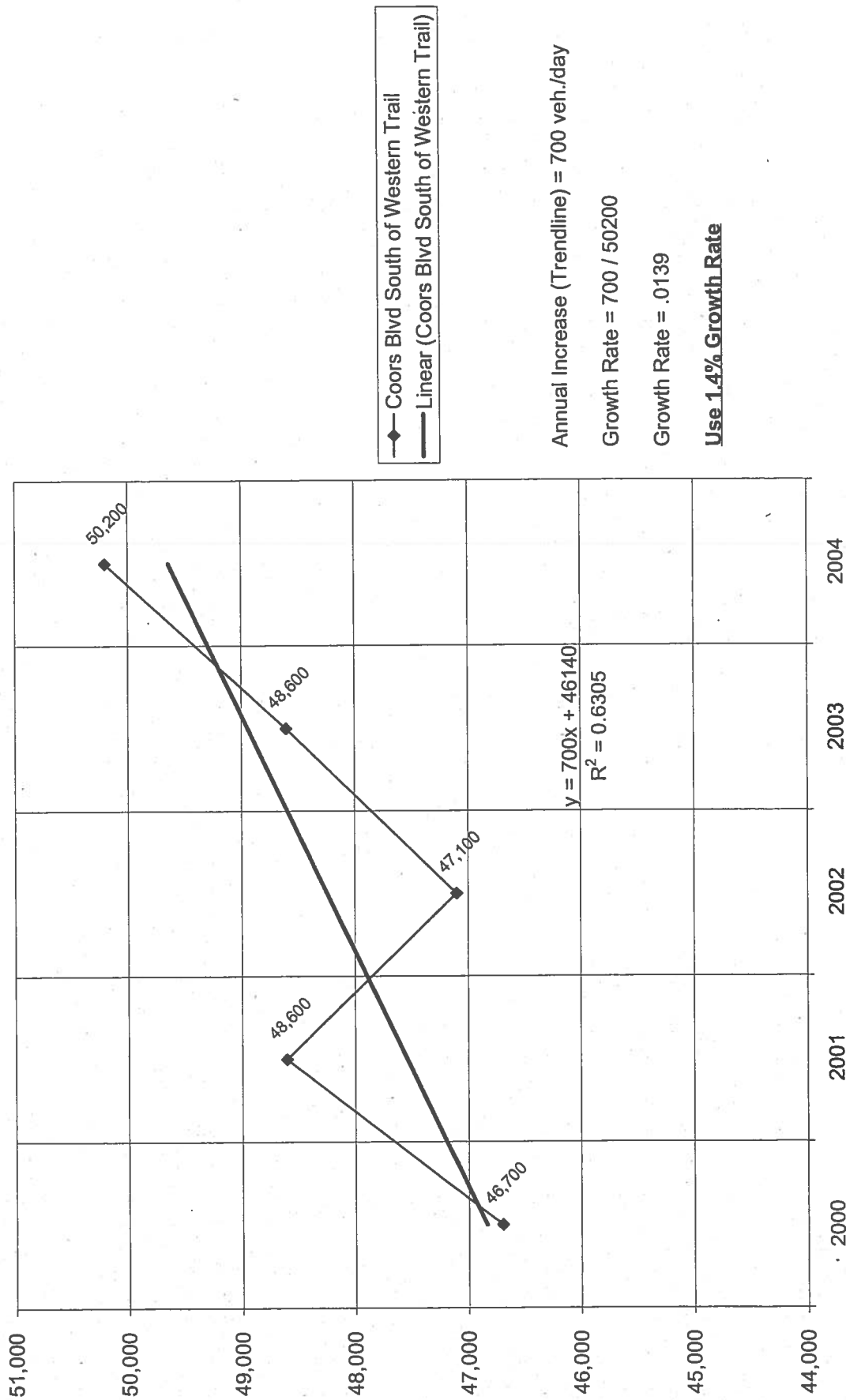
Annual Increase (Trendline) = 80 veh./day

Growth Rate = 80 / 2400

Growth Rate = .0333

Use 3.3% Growth Rate

Historic Growth Chart Coors Blvd South of Western Trail (1999-2003)



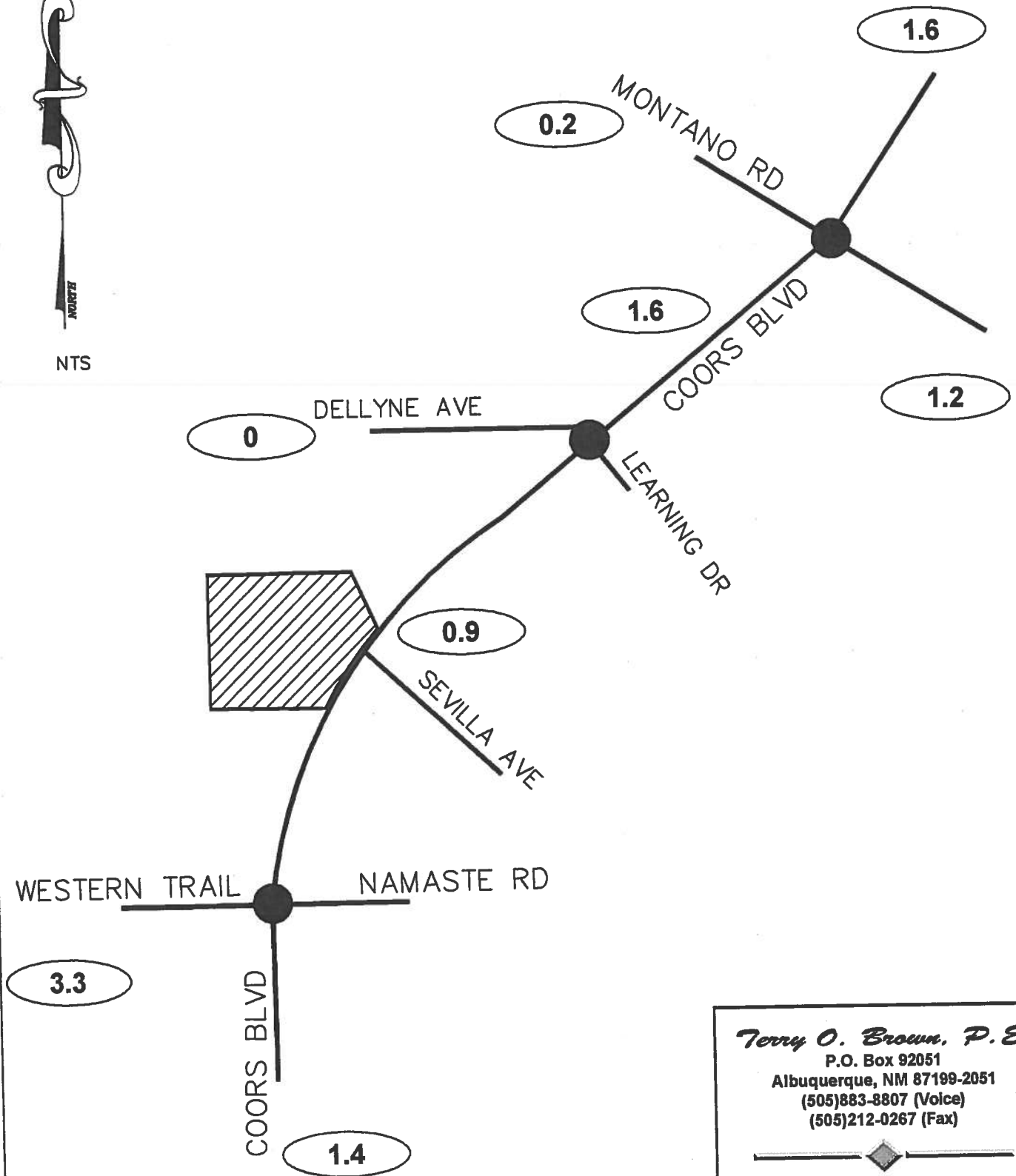
Esperia Development

(Sevilla Ave / Coors Blvd)

Growth Rate Map (%)



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Esperia Development (Sevilla Ave / Coors Blvd)
Projected Turning Movements SUMMARY
PROPOSED DEVELOPMENT (2009) - 100% Development

INTERSECTION: Summary

Western Trail / Coors Blvd

(1)

2.0% Truck

Existing (2006)

2009 (NO BUILD - A.M.)

2009 (BUILD - A.M.)

0.84			0.38			0.84			0.89			PHF
Eastbound (Western Trail)			Westbound (Western Trail)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
46	0	43	2	0	1	49	1,569	9	5	2,161	38	
67	3	47	93	8	59	51	1,722	40	25	2,479	58	
72	3	47	93	8	59	51	1,781	40	25	2,546	62	

Existing (2006)

2009 (NO BUILD - P.M.)

2009 (BUILD - P.M.)

0.54			0.47			0.88			0.99			PHF
Eastbound (Western Trail)			Westbound (Western Trail)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
30	0	54	9	0	8	116	2,634	6	4	2,102	100	
70	10	59	67	5	47	120	3,041	111	72	2,338	138	
81	10	59	67	5	57	120	3,165	111	73	2,482	150	

Dellyne Ave / Coors Blvd

(2)

4.8% Truck

Existing (2006)

2009 (NO BUILD - A.M.)

2009 (BUILD - A.M.)

0.78			0.36			0.84			0.92			PHF
Eastbound (Dellyne Ave)			Westbound (Dellyne Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
156	31	238	53	22	85	72	1,829	68	195	1,446	38	
226	36	240	191	79	113	82	2,044	91	240	1,554	49	
226	36	255	191	79	113	95	2,079	91	240	1,590	49	

Existing (2006)

2009 (NO BUILD - P.M.)

2009 (BUILD - P.M.)

0.87			0.55			0.91			0.96			PHF
Eastbound (Dellyne Ave)			Westbound (Dellyne Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
87	15	100	23	5	34	310	2,449	28	92	1,623	178	
225	23	109	149	113	59	323	2,695	112	180	1,852	223	
225	23	124	149	113	59	336	2,730	112	180	1,888	223	

Montaño Rd / Coors Blvd

(3)

1.6% Truck

Existing (2006)

2009 (NO BUILD - A.M.)

2009 (BUILD - A.M.)

0.66			0.77			0.84			0.85			PHF
Eastbound (Montaño Rd)			Westbound (Montaño Rd)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
74	747	276	172	261	61	223	1,297	325	300	1,350	59	
75	898	354	220	299	63	409	1,531	557	363	1,480	61	
75	898	371	228	299	63	422	1,545	566	363	1,490	61	

Existing (2006)

2009 (NO BUILD - P.M.)

2009 (BUILD - P.M.)

0.97			0.96			0.90			0.96			PHF
Eastbound (Montaño Rd)			Westbound (Montaño Rd)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
270	314	246	382	867	109	680	1,738	236	170	1,363	132	
271	615	400	545	1,006	113	1,036	2,022	522	273	1,606	138	
271	615	447	562	1,006	113	1,087	2,047	541	273	1,627	138	

Esperia Development (Sevilla Ave / Coors Blvd)
Projected Turning Movements SUMMARY
PROPOSED DEVELOPMENT (2009) - 100% Development

INTERSECTION: S u m m a r y

Sevilla Ave / Coors Blvd

(4)

0% Truck

Existing (2006)

2009 (NO BUILD - A.M.)

2009 (BUILD - A.M.)

0.85			0.85			0.89			0.89			PHF
Eastbound (Sevilla Ave)			Westbound (Sevilla Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	1,591	0	0	2,204	0	
33	0	43	47	0	42	31	1,751	15	22	2,474	0	
82	0	115	47	0	42	95	1,751	15	22	2,474	52	

Existing (2006)

2009 (NO BUILD - P.M.)

2009 (BUILD - P.M.)

0.85			0.85			0.99			0.99			PHF
Eastbound (Sevilla Ave)			Westbound (Sevilla Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	2,631	0	0	2,206	0	
20	0	26	29	0	45	118	2,907	53	58	2,495	0	
163	1	184	29	1	45	254	2,907	53	58	2,495	127	

Maduri Ave / Vidal Dr

(5)

3% Truck

Existing (2006)

2009 (NO BUILD - A.M.)

2009 (BUILD - A.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Maduri Ave)			Westbound (Maduri Ave)			Northbound (Vidal Dr)			Southbound (Vidal Dr)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	31	0	0	0	76	0	0	
0	23	0	70	17	31	0	0	30	101	0	0	

Existing (2006)

2009 (NO BUILD - P.M.)

2009 (BUILD - P.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Maduri Ave)			Westbound (Maduri Ave)			Northbound (Vidal Dr)			Southbound (Vidal Dr)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	118	0	0	0	46	0	0	
0	34	0	162	28	118	0	0	44	129	0	0	

Esperia Development (Sevilla Ave / Coors Blvd)
Projected Turning Movements Worksheet
Western Trail / Coors Blvd

INTERSECTION: Western Trail (1)

E-W Street: Western Trail

N-S Street: Coors Blvd

2003

2009

Growth Rates

Existing Volumes
 Background Traffic Growth

Subtotal

La Luz del Oeste, Unit 4

Andalucia Tract 6

Andalucia Res. - Phase 1

Subtotal (NO BUILD - A.M.)

Percent Office / Res. Trips Generated(Entering)

Percent Office / Res. Trips Generated(Exiting)

Percent Commercial Trips Generated(Entering)

Percent Commercial Trips Generated(Exiting)

Total Trips Generated

Total AM Peak Hour BUILD Volumes

3.30%			3.30%			1.40%			0.90%		
Eastbound (Western Trail)			Westbound (Western Trail)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
42	0	39	2	0	1	47	1,506	9	5	2,104	37
8	0	8	0	0	0	4	127	1	0	114	2
50	0	47	2	0	1	51	1,633	10	5	2,218	39
0	0	0	0	0	0	0	31	0	0	106	1
17	0	0	0	0	1	0	43	0	1	110	17
0	3	0	91	8	57	0	15	30	19	45	1
67	3	47	93	8	59	51	1,722	40	25	2,479	58
2.53%	0.00%	0.00%	0.00%	0.00%	0.21%	0.00%	63.01%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.21%	63.01%	2.53%
5.05%	0.00%	0.00%	0.00%	0.00%	0.62%	0.00%	37.15%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.62%	37.15%	5.05%
5	0	0	0	0	0	0	59	0	0	57	4
72	3	47	93	8	59	51	1,781	40	25	2,546	62

Eastbound (Western Trail)			Westbound (Western Trail)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
27	0	49	8	0	7	111	2,528	6	4	2,047	97
5	0	10	2	0	1	9	212	1	0	111	5
32	0	59	10	0	8	120	2,740	7	4	2,158	102
1	0	0	0	0	0	0	117	0	0	64	0
36	0	0	0	0	3	0	132	0	3	88	35
1	10	0	57	5	36	0	52	104	65	28	1
70	10	59	67	5	47	120	3,041	111	72	2,338	138
2.53%	0.00%	0.00%	0.00%	0.00%	0.21%	0.00%	63.01%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.21%	63.01%	2.53%
5.05%	0.00%	0.00%	0.00%	0.00%	6.20%	0.00%	37.15%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.62%	37.15%	5.05%
11	0	0	0	0	10	0	124	0	1	144	12
81	10	59	67	5	57	120	3,165	111	73	2,482	150

Total PM Peak Hour BUILD Volumes

Entering Exiting

62 86 A.M.

102 126 P.M.

54 35 A.M.

162 176 P.M.

Number of Office/Residential Trips Generated

Number of Commercial Trips Generated

100% Office / Residential Development

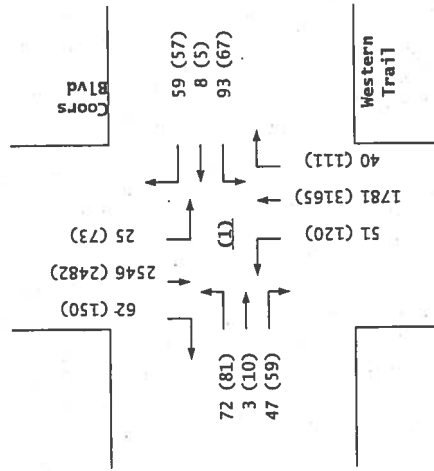
100% Commercial Development

Eastbound (Western Trail)			Westbound (Western Trail)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
46	0	43	2	0	1	49	1,569	9	5	2,161	38
30	0	54	9	0	8	116	2,634	6	4	2,102	100

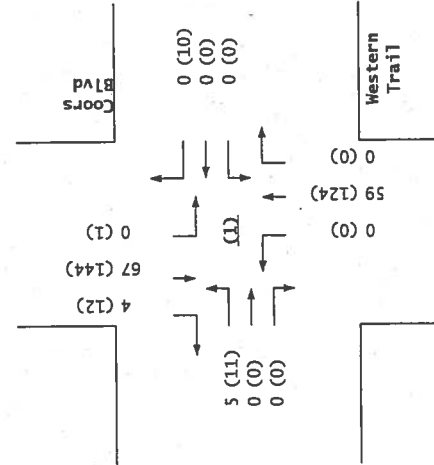
2006 AM Peak Hr. Volumes

2006 PM Peak Hr. Volumes

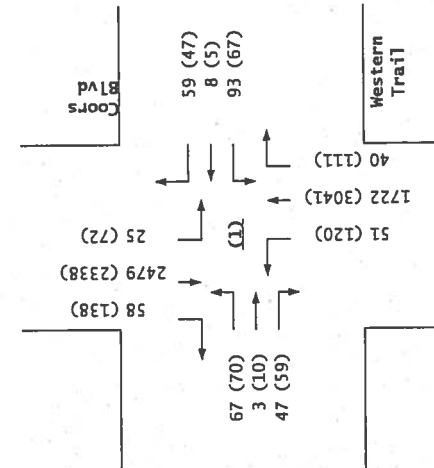
2009
BUILD



Trips



2009
NO BUILD



Esperia Development (Sevilla Ave / Coors Blvd)
Projected Turning Movements Worksheet
Dellyne Ave / Coors Blvd

INTERSECTION :

E-W Street: Dellyne Ave
N-S Street: Coors Blvd

(2)

Year of Existing Counts
Implementation Year

2003
2009

Growth Rates

	0.00%			3.30%			0.90%			1.60%		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	156	31	238	48	20	77	70	1,781	66	186	1,380	36
Background Traffic Growth	0	0	0	0	4	15	4	96	4	18	132	3
Subtotal	156	31	238	58	24	92	74	1,877	70	204	1,512	39
La Luz del Oeste, Unit 4	0	0	1	0	0	0	5	28	0	0	7	0
Andalucia Tract 6	70	5	0	133	55	21	0	56	21	36	8	10
Andalucia Res. - Phase 1	0	0	1	0	0	0	3	83	0	0	27	0
Subtotal (NO BUILD - A.M.)	226	36	240	191	79	113	82	2,044	91	240	1,554	49
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	7.17%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	27.08%	0.00%
Percent Office / Res. Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	7.17%	27.08%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	21.05%	0.65%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	34.85%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	21.05%	34.85%	0.66%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	15	0	0	0	13	35	0	0	36	0
Total AM Peak Hour BUILD Volumes	226	36	255	191	79	113	95	2,079	91	240	1,590	49

	Eastbound (Dellyne Ave)			Westbound (Dellyne Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	87	15	100	21	5	31	302	2,385	27	88	1,549	170
Background Traffic Growth	0	0	0	0	1	6	16	129	1	8	149	16
Subtotal	87	15	100	25	6	37	318	2,514	28	96	1,698	186
La Luz del Oeste, Unit 4	0	0	5	0	0	0	3	17	0	0	31	0
Andalucia Tract 6	138	8	0	124	107	22	0	113	84	84	29	37
Andalucia Res. - Phase 1	0	0	4	0	0	0	2	51	0	0	94	0
Subtotal (NO BUILD - P.M.)	225	23	109	149	113	59	323	2,695	112	180	1,852	223
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	7.17%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	27.08%	0.00%
Percent Office / Res. Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	7.17%	27.08%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	7.17%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	27.08%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	7.17%	27.08%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	15	0	0	0	13	35	0	0	36	0
Subtotal AM Pk Hr. BUILD Volumes	225	23	124	149	113	59	336	2,730	112	180	1,888	223
Pass-by Trip Adjustments	0	0	0	0	0	0	0	0	0	0	0	0
Total AM Peak Hour BUILD Volumes	225	23	124	149	113	59	336	2,730	112	180	1,888	223

Entering Exiting

62 86 A.M.
102 126 P.M.
54 35 A.M.
162 176 P.M.

Number of Residential Trips Generated

100% Office / Residential Development

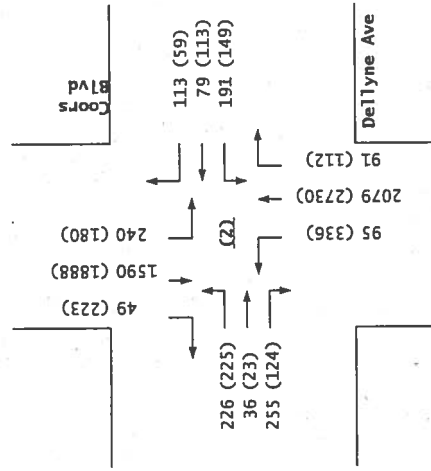
Number of Commercial Trips Generated

100% Commercial Development

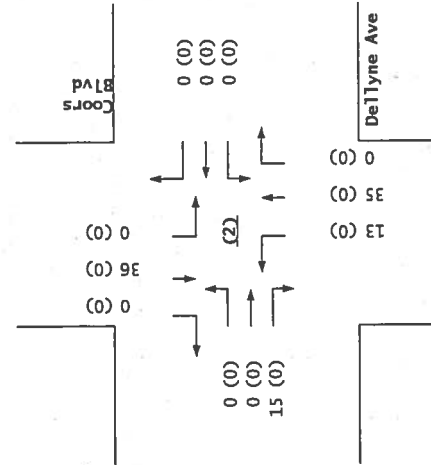
Eastbound (Dellyne Ave)			Westbound (Dellyne Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
156	31	238	53	22	85	72	1,829	68	195	1,446	38
87	15	100	23	5	34	310	2,449	28	92	1,623	178

2006 AM Peak Hr. Volumes
2006 PM Peak Hr. Volumes

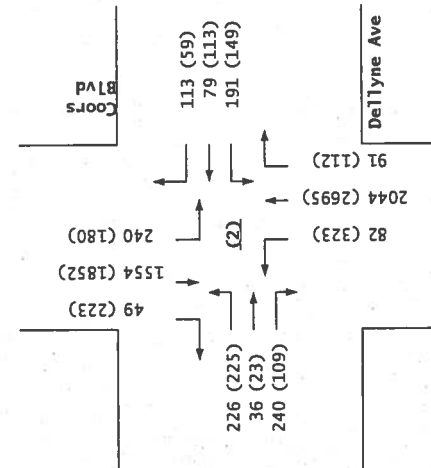
2009
BUILD



Trips



2009
NO BUILD



Esperia Development (Sevilla Ave / Coors Blvd)
Projected Turning Movements Worksheet
Montaño Rd / Coors Blvd

INTERSECTION: Montaño Rd (3)
E-W Street: Coors Blvd
N-S Street: Coors Blvd

Year of Existing Counts
Implementation Year

2003
2009

Growth Rates

	0.20%			1.20%			1.60%			1.60%		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	74	743	274	166	252	59	213	1,238	310	286	1,288	56
Background Traffic Growth	1	9	3	12	18	4	20	119	30	27	124	5
Subtotal	75	752	277	178	270	63	233	1,357	340	313	1,412	61
La Luz del Oeste, Unit 4	0	0	0	3	0	0	2	15	11	0	4	0
Andalucia Tract 6	0	146	75	27	29	0	167	121	168	50	52	0
Andalucia Res. - Phase 1	0	0	2	12	0	0	7	38	38	0	12	0
Subtotal (NO BUILD - A.M.)	75	898	354	220	299	63	409	1,531	557	363	1,480	61
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	5.07%	8.53%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	13.48%	0.00%
Percent Office / Res. Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	5.07%	13.48%	8.53%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	25.68%	4.70%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	4.47%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.68%	4.47%	4.70%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	17	8	0	0	13	14	9	0	10	0
Total AM Peak Hour BUILD Volumes	75	898	371	228	299	63	422	1,545	566	363	1,490	61

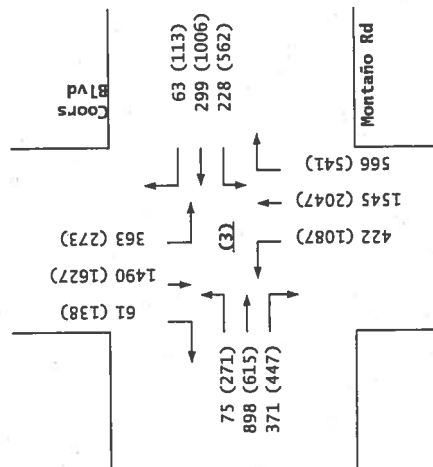
	0.20%			1.20%			1.60%			1.60%		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	268	312	245	369	837	105	649	1,668	225	162	1,301	126
Background Traffic Growth	3	4	3	27	60	8	62	159	22	16	125	12
Subtotal	271	316	248	396	897	113	711	1,817	247	178	1,426	138
La Luz del Oeste, Unit 4	0	0	2	12	0	0	1	9	7	0	17	0
Andalucia Tract 6	0	299	142	94	109	0	320	173	245	95	120	0
Andalucia Res. - Phase 1	0	0	8	43	0	0	4	23	23	0	43	0
Subtotal (NO BUILD - P.M.)	271	615	400	545	1,006	113	1,036	2,022	522	273	1,606	138
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	5.07%	8.53%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	13.48%	0.00%
Percent Office / Res. Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	5.07%	13.48%	8.53%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	25.68%	4.70%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	4.47%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.68%	4.47%	4.70%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	47	17	0	0	51	25	19	0	21	0
Total PM Peak Hour BUILD Volumes	271	615	447	562	1,006	113	1,087	2,047	541	273	1,627	138

Number of Office/Residential Trips Generated
Number of Commercial Trips Generated

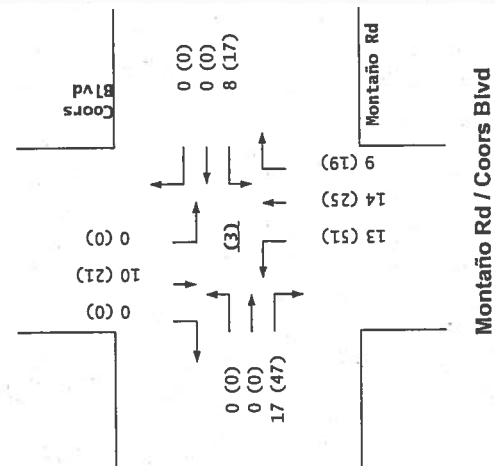
	0.20%			1.20%			1.60%			1.60%		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	74	747	276	172	261	61	223	1,297	325	300	1,350	59
Background Traffic Growth	270	314	246	382	867	109	680	1,738	236	170	1,363	132
Subtotal	344	1,061	522	554	1,128	170	903	3,035	561	470	2,713	191
La Luz del Oeste, Unit 4	0	0	0	3	0	0	2	15	11	0	4	0
Andalucia Tract 6	0	146	75	27	29	0	167	121	168	50	52	0
Andalucia Res. - Phase 1	0	0	2	12	0	0	7	38	38	0	12	0
Subtotal (NO BUILD - A.M.)	344	1,061	522	554	1,128	170	903	3,035	561	470	2,713	191
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	5.07%	8.53%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	13.48%	0.00%
Percent Office / Res. Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	5.07%	13.48%	8.53%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	25.68%	4.70%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	4.47%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.68%	4.47%	4.70%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	47	17	0	0	51	25	19	0	21	0
Total AM Peak Hour BUILD Volumes	344	1,061	522	554	1,128	170	903	3,035	561	470	2,713	191

2006 AM Peak Hr. Volumes
2006 PM Peak Hr. Volumes

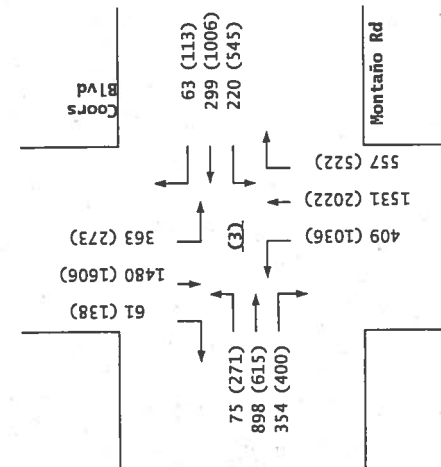
2009
BUILD



Trips



2009
NO BUILD



Esperia Development (Sevilla Ave / Coors Blvd)
Projected Turning Movements Worksheet

Sevilla Ave / Coors Blvd

(4)

E-W Street: Sevilla Ave
N-S Street: Coors Blvd

2003

2009

INTERSECTION :

Year of Existing Counts
Implementation Year

Growth Rates

	0.90%			0.90%			0.90%			0.90%		
	Eastbound (Sevilla Ave)			Westbound (Sevilla Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	0	0	0	0	1,549	0	0	2,146	0
Background Traffic Growth	0	0	0	0	0	0	0	84	0	0	116	0
Subtotal	0	0	0	0	0	0	0	1,633	0	0	2,262	0
La Luz del Oeste, Unit 4	33	0	43	0	0	0	31	0	0	0	64	0
Andalucia Tract 6	0	0	0	0	0	13	0	61	0	13	129	0
Andalucia Res. - Phase 1	0	0	0	47	0	29	0	57	15	9	19	0
Subtotal (NO BUILD - A.M.)	33	0	43	47	0	42	31	1,751	15	22	2,474	0
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	65.75%	0.00%	0.00%	0.00%	0.00%	34.25%
Percent Office / Res. Trips Generated(Exiting)	34.25%	0.00%	65.75%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.61%	0.00%	42.82%	0.00%	0.00%	0.00%	0.00%	56.56%
Percent Commercial Trips Generated(Exiting)	56.56%	0.61%	42.82%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	49	0	72	0	0	0	64	0	0	0	52	0
Total AM Peak Hour BUILD Volumes	82	0	115	47	0	42	95	1,751	15	22	2,474	52

	Eastbound (Sevilla Ave)			Westbound (Sevilla Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	0	0	0	0	2,562	0	0	2,148	0
Background Traffic Growth	0	0	0	0	0	0	0	138	0	0	116	0
Subtotal	0	0	0	0	0	0	0	2,700	0	0	2,264	0
La Luz del Oeste, Unit 4	20	0	26	0	0	0	118	0	0	0	39	0
Andalucia Tract 6	0	0	0	0	0	27	0	171	0	26	127	0
Andalucia Res. - Phase 1	0	0	0	29	0	18	0	36	53	32	65	0
Subtotal (NO BUILD - P.M.)	20	0	26	29	0	45	118	2,907	53	58	2,495	0
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	65.75%	0.00%	0.00%	0.00%	0.00%	34.25%
Percent Office / Res. Trips Generated(Exiting)	34.25%	0.00%	65.75%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.61%	0.00%	42.82%	0.00%	0.00%	0.00%	0.00%	56.56%
Percent Commercial Trips Generated(Exiting)	56.56%	0.61%	42.82%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	143	1	158	0	1	0	136	0	0	0	127	0
Total PM Peak Hour BUILD Volumes	163	1	184	29	1	45	254	2,907	53	58	2,495	127

Entering Exiting

62 86 A.M.

Number of Office/Residential Trips Generated

102 126 P.M.

Number of Office/Residential Trips Generated

54 35 A.M.

Number of Commercial Trips Generated

162 176 P.M.

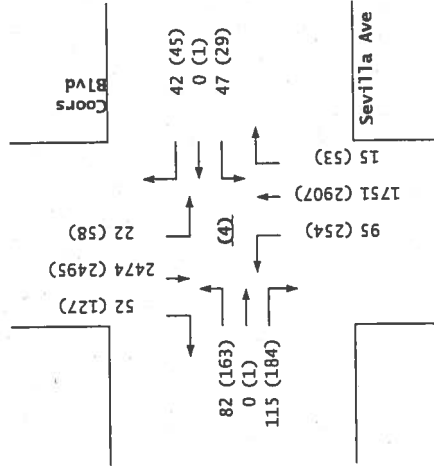
Number of Commercial Trips Generated

	Eastbound (Sevilla Ave)			Westbound (Sevilla Ave)			Northbound (Coors Blvd)			Southbound (Coors Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
	0	0	0	0	0	0	0	1,591	0	0	2,204	0
2006 AM Peak Hr. Volumes	0	0	0	0	0	0	0	1,591	0	0	2,204	0
2006 PM Peak Hr. Volumes	0	0	0	0	0	0	0	2,631	0	0	2,206	0

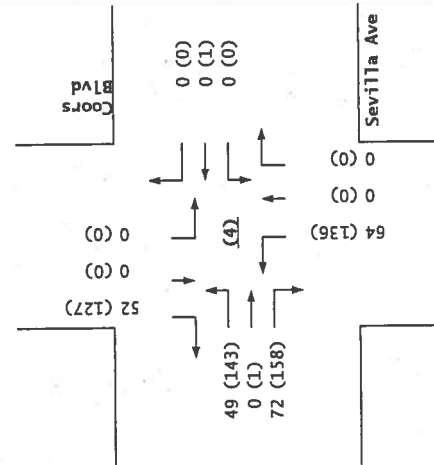
2006 AM Peak Hr. Volumes

2006 PM Peak Hr. Volumes

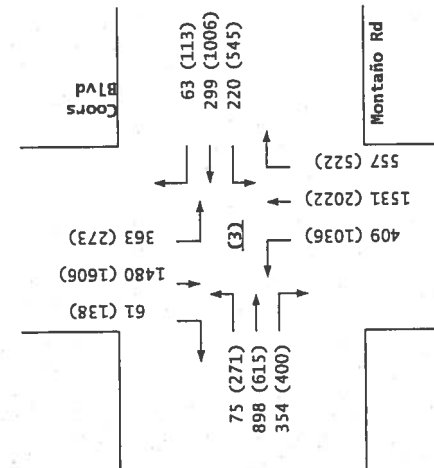
2009
BUILD



Trips



2009
NO BUILD



Sevilla Ave / Coors Blvd

Esperia Development (Sevilla Ave / Coors Blvd)
Projected Turning Movements Worksheet
Maduri Ave / Vidal Dr

(5)

E-W Street: **Maduri Ave**
 N-S Street: **Vidal Dr**

Year of Existing Counts: **2006**
 Implementation Year: **2009**

Growth Rates

	0.90%			0.90%			0.90%			0.90%		
	Eastbound (Maduri Ave)			Westbound (Maduri Ave)			Northbound (Vidal Dr)			Southbound (Vidal Dr)		
Existing Volumes	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Background Traffic Growth	0	0	0	0	0	0	0	0	0	0	0	0
Subtotal	0	0	0	0	0	0	0	0	0	0	0	0
La Luz del Oeste, Unit 4	0	0	0	0	0	31	0	0	0	76	0	0
Subtotal (NO BUILD - A.M.)	0	0	0	0	0	31	0	0	0	76	0	0
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	0.00%	58.80%	27.13%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office / Res. Trips Generated(Exiting)	0.00%	27.13%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	34.84%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	63.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	37.00%	0.00%	0.00%
Total Trips Generated	0	23	0	20	17	31	0	0	30	25	0	0
Total AM Peak Hour BUILD Volumes	0	23	0	20	17	31	0	0	30	101	0	0

	Eastbound (Maduri Ave)			Westbound (Maduri Ave)			Northbound (Vidal Dr)			Southbound (Vidal Dr)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Background Traffic Growth	0	0	0	0	0	0	0	0	0	0	0	0
Subtotal	0	0	0	0	0	0	0	0	0	0	0	0
La Luz del Oeste, Unit 4	0	0	0	0	0	118	0	0	0	46	0	0
Subtotal (NO BUILD - P.M.)	0	0	0	0	0	118	0	0	0	46	0	0
Percent Office / Res. Trips Generated(Entering)	0.00%	0.00%	0.00%	58.80%	27.13%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office / Res. Trips Generated(Exiting)	0.00%	27.13%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	34.84%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	63.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	37.00%	0.00%	0.00%
Total Trips Generated	0	34	0	162	28	118	0	0	44	83	0	0
Total PM Peak Hour BUILD Volumes	0	34	0	162	28	118	0	0	44	129	0	0

Entering: **62** Exiting: **86** A.M.
102 **126** P.M.

Number of Office/Residential Trips Generated

100% Office / Residential Development
 100% Commercial Development

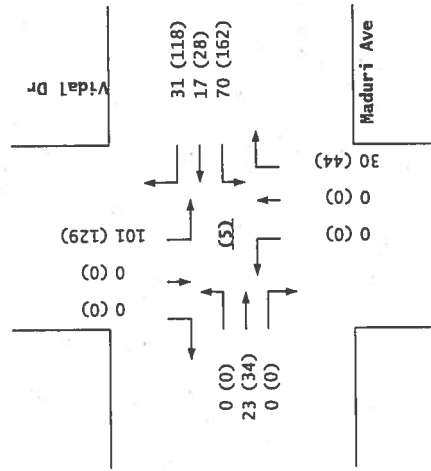
Number of Commercial Trips Generated

54 **35** A.M.
162 **176** P.M.

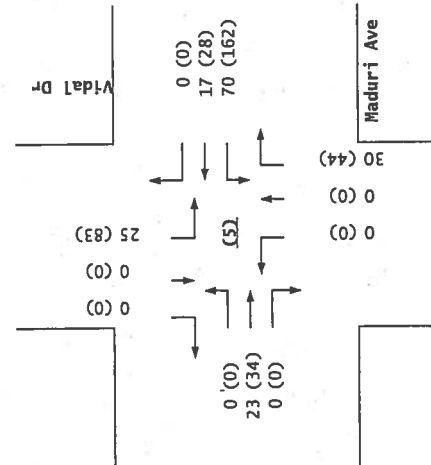
	Eastbound (Maduri Ave)			Westbound (Maduri Ave)			Northbound (Vidal Dr)			Southbound (Vidal Dr)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2006 AM Peak Hr. Volumes	0	0	0	0	0	0	0	0	0	0	0	0
2006 PM Peak Hr. Volumes	0	0	0	0	0	0	0	0	0	0	0	0

2006 AM Peak Hr. Volumes
 2006 PM Peak Hr. Volumes

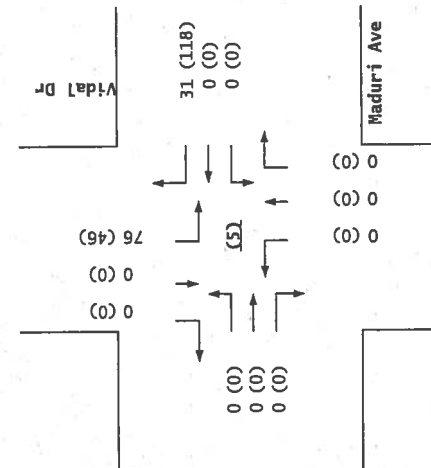
2009
BUILD



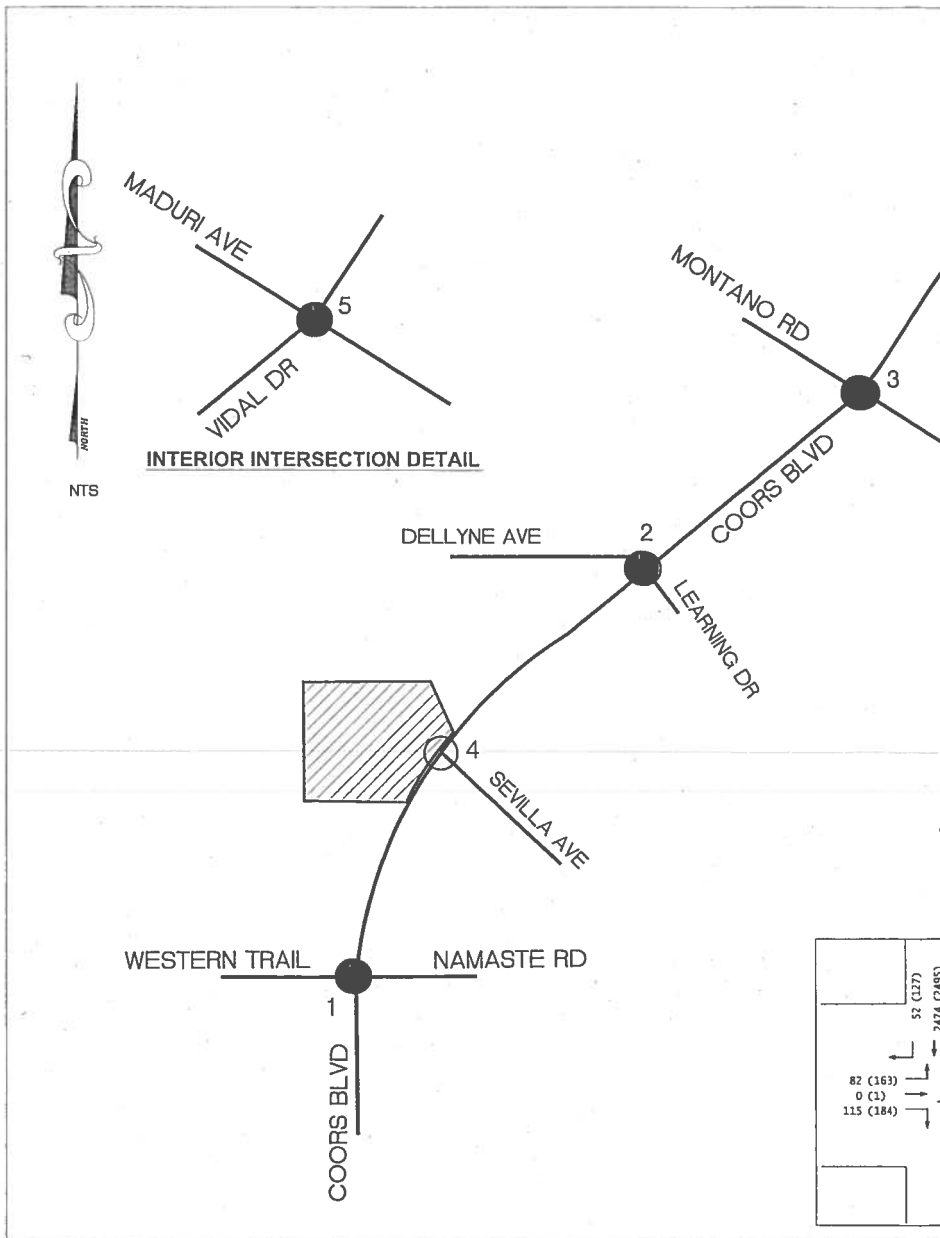
Trips

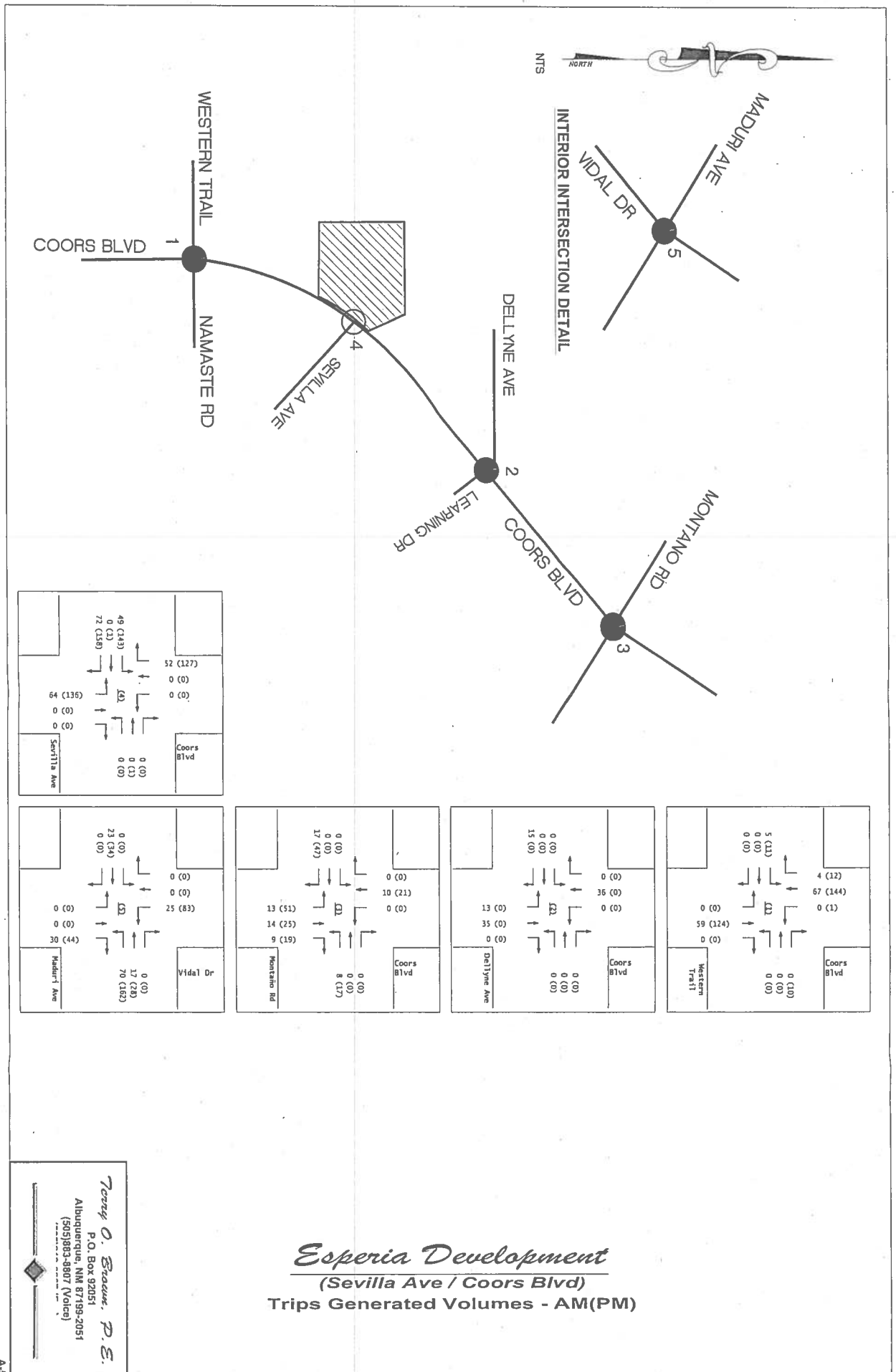


2009
NO BUILD



Maduri Ave / Vidal Dr





Esperia Development
 (Sevilla Ave / Coors Blvd)
 Trips Generated Volumes - AM(PM)

Terry O. Shaum, P.E.
 P.O. Box 92051
 Albuquerque, NM 87199-2051
 (505) 863-9887 (Voice)

Analysis of
Intersection #1:
Western Trail / Coors Blvd.

Queueing Analysis Summary Sheet

Project:
Intersection:

Esperia Development (Sevilla Ave / Coors Blvd)
Western Trail / Coors Blvd

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	42	200				1	0	Cont	0	39	0
AM NO BUILD Queue	2	67	75				1	3	0	0	47	100
AM BUILD Queue	2	72	75				1	3	0	0	47	100
Existing Lane Length	2	27	200				1	0	Cont	0	49	0
PM NO BUILD Queue	2	70	100				1	10	50	0	59	125
PM BUILD Queue	2	81	100				1	10	50	0	59	125
Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	2	320				1	0	Cont	0	1	0
AM NO BUILD Queue	1	93	150				1	8	25	0	59	100
AM BUILD Queue	1	93	150				1	8	25	0	59	100
Existing Lane Length	1	8	320				1	0	Cont	0	7	0
PM NO BUILD Queue	1	67	125				1	5	25	0	47	100
PM BUILD Queue	1	67	125				1	5	25	0	57	125
Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	47	250				3	1,506	Cont	1	9	225
AM NO BUILD Queue	1	51	100				3	1,722	700	1	40	75
AM BUILD Queue	1	51	100				3	1,781	725	1	40	75
Existing Lane Length	1	111	250				3	2,528	Cont	1	6	225
PM NO BUILD Queue	1	120	200				3	3,041	>1,000	1	111	200
PM BUILD Queue	1	120	200				3	3,165	>1,000	1	111	200
Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	5	150				3	2,104	Cont	1	37	300
AM NO BUILD Queue	1	25	50				3	2,479	>1,000	1	58	100
AM BUILD Queue	1	25	50				3	2,546	>1,000	1	62	100
Existing Lane Length	1	4	150				3	2,047	Cont	1	97	300
PM NO BUILD Queue	1	72	125				3	2,338	>1,000	1	138	225
PM BUILD Queue	1	73	150				3	2,482	>1,000	1	150	225

Cycle Length: AM PM
 110 130

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Western Trail / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 1-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		Western Trail E-W STREET							
SB TOTAL 2,562 58 RIGHT 25 LEFT 2,479 THRU	N TR? <table border="1" style="width: 100%; text-align: center;"> <tr> <td>R</td> <td>T</td> <td>L</td> </tr> <tr> <td>1</td> <td>3</td> <td>1</td> </tr> </table>	R	T	L	1	3	1	59 RIGHT 160 WB TOTAL 8 THRU 93 LEFT	0 R 1 T 1 L NO. LANES TR? Y
R	T	L							
1	3	1							
NO. LANES <table border="1" style="width: 100%; text-align: center;"> <tr> <td>2</td> <td>L</td> </tr> <tr> <td>1</td> <td>T</td> </tr> <tr> <td>0</td> <td>R</td> </tr> </table> TR? Y		2	L	1	T	0	R	67 LEFT 3 THRU 47 RIGHT 117 EB TOTAL	
2	L								
1	T								
0	R								
1 L 3 T 1 R L T R TR? N		51 LEFT 40 RIGHT 1,722 THRU 1,813 NB TOTAL							
EB LT = 34 WB TH = 67 101 WB LT = 93 EB TH = 50 143*		NB LT = 51 SB TH = 826 877* SB LT = 25 NB TH = 574 599							
MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400		CAPACITY LEVEL UNDER NEAR OVER							
143 + 877 = 1,020 E-W CRITICAL N-S CRITICAL		STATUS? UNDER							
TES: <u>Existing Geometry</u>									

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Western Trail / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 1-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

SB TOTAL 2,633		Coors Blvd N-S STREET		59 RIGHT 8 THRU 93 LEFT		160 WB TOTAL
62	25	N TR?	R	T	L	
RIGHT	THRU	LEFT	1	3	1	

NO. LANES

2	L	
1	T	
0	R	

TR? Y

0	R	
1	T	
1	L	

TR? Y

NO. LANES

EB TOTAL 122		72 LEFT 3 THRU 47 RIGHT		131 LTR		Western Trail E-W STREET	
				51 LEFT	1,781 THRU	40 RIGHT	
					1,872 NB TOTAL		

EB LT = 36 WB TH = 67 103	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = 51 SB TH = 849 900*
WB LT = 93 EB TH = 50 143*			SB LT = 25 NB TH = 594 619

143 + 900 = 1,043 STATUS? UNDER
 E-W CRITICAL N-S CRITICAL

TES: Existing Geometry

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Western Trail / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 1-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		47 RIGHT 5 THRU 67 LEFT	119 WB TOTAL
SB TOTAL 2,548	N TR?		
138 RIGHT 2,338 THRU 72 LEFT	R T L 1 3 1		
NO. LANES 2 L 1 T 0 R		TR? Y	
		0 R 1 T 1 L NO. LANES	
70 LEFT 10 THRU 59 RIGHT		1 3 1 L T R TR? N	
EB TOTAL 139		Western Trail E-W STREET	
		120 LEFT 3,041 THRU 111 RIGHT	3,272 NB TOTAL

EB LT = 35 WB TH = 52 87	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = 120 SB TH = 779 899
WB LT = 67 EB TH = 69 136*			SB LT = 72 NB TH = 1,014 1,086*

136	+	1,086	=	1,222	STATUS? NEAR
E-W CRITICAL		N-S CRITICAL			
TES: Existing Geometry					

SIGNALIZED INTERSECTION **PLANNING APPLICATION WORKSHEET**

Intersection: Western Trail / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 1-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		
SB TOTAL 2,705	N TR?	57 RIGHT
150 RIGHT 2,482 THRU 73 LEFT	R 1 T 3 L 1	5 THRU 129 WB TOTAL
		67 LEFT

NO. LANES

2	L
1	T
0	R

TR?

Y

0	R
1	T
1	L

NO. LANES

TR?

Y

	81 LEFT
150 EB TOTAL	10 THRU
	59 RIGHT

1		3		1
L		T		R

TR?

N

Western Trail E-W STREET	
	3,165 THRU
120 LEFT	111 RIGHT
3,396 NB TOTAL	

EB LT = 41 WB TH = 62 103	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = 120 SB TH = 827 947
WB LT = 67 EB TH = 69 136*			SB LT = 73 NB TH = 1,055 1,128*

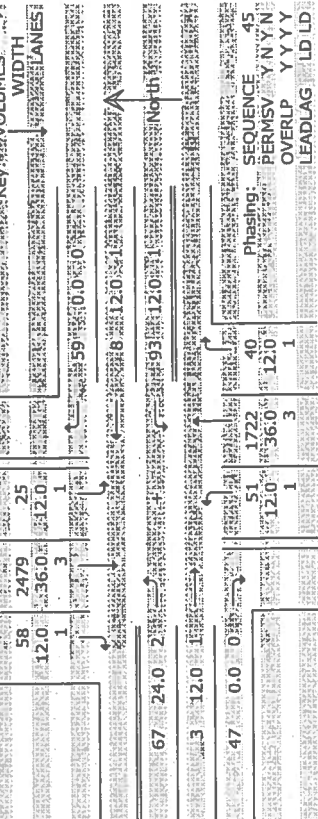
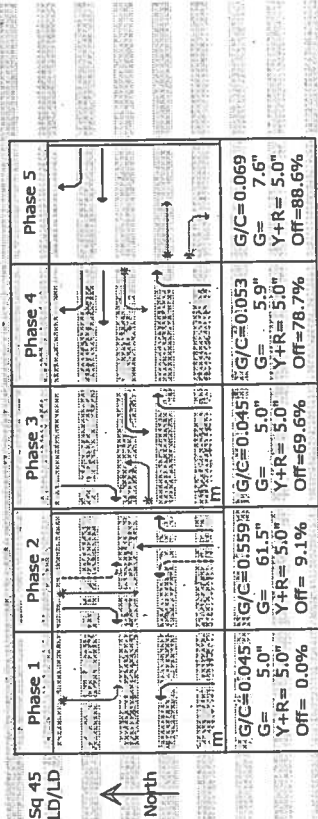
136 E-W CRITICAL	+	1,128 N-S CRITICAL	=	1,264	STATUS? NEAR
TES: Existing Geometry					

SIGNAL2000/TEAPAC (Ver 2.70.07) - Capacity Analysis Summary

SIGNAL2000/TEAPAC (Ver 2.70.07) - HCM Input Worksheet

Intersection # 1 -
Degree of Saturation (V/S) 0.82 Vehicle Delay 30.3 Level of Service C

Area Location Type: NONGBD



C=110 sec G=85.0 sec = 77.3% Y=25.0 sec = 22.7% Ped=0.0 sec = 0.0%

Lane Group	Width/Lanes	g/C	Reqd	Used	Service Rate @D _s (vph) @E	Adj Volume	V/C	HCM Delay	Queue Model 1
------------	-------------	-----	------	------	---------------------------------------	------------	-----	-----------	---------------

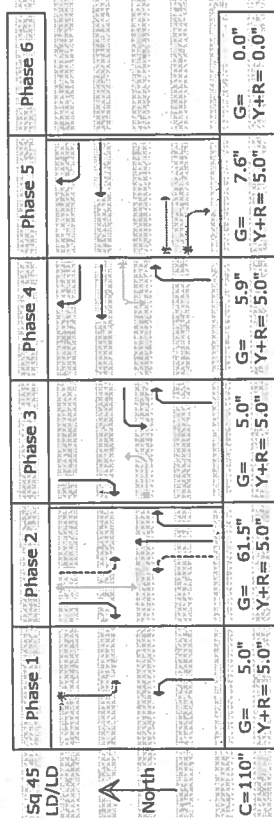
RT	12/1	0.074	0.650	1030	1030	65	0.063	7.0	A
TH	36/3	0.527	0.559	2838	2838	2785	0.981	36.5	*D+ 1454 ft
LT	12/1	0.000	0.045	125	139	28	0.189	13.7	*B+ 18 ft

RT	12/1	0.059	0.749	1186	1186	48	0.040	3.6	A
TH	36/3	0.401	0.559	2838	2838	2050	0.722	18.8	B
LT	12/1	0.010	0.045	120	136	61	0.412	27.2	C+

RT+TH	12/1	0.123	0.168	205	251	134	0.494	42.9	D+
LT	12/1	0.144	0.144	187	234	186	0.727	54.9	*D

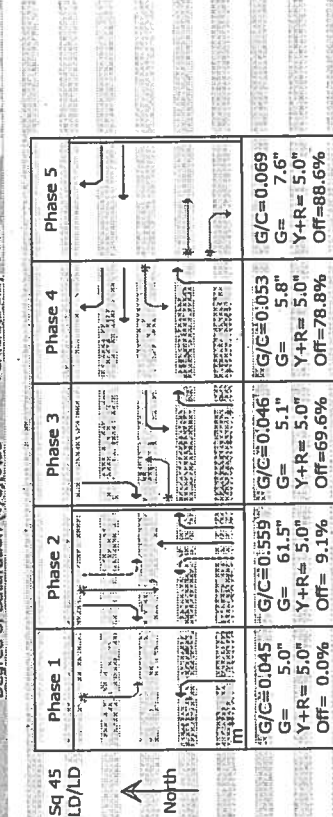
RT+TH	12/1	0.069	0.069	61	87	60	0.541	54.8	*D
LT	24/2	0.044	0.045	84	128	80	0.513	54.2	*D

	SB	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Pk-hr fact, PHF	.89	.89	.89	.89	.89	.89	.89	.89	.89	.89	.89	.89
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Strup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



SIGNAL2000/TEAPAC Ver 2.70.07 - Capacity Analysis Summary

Area Location: Type: NONGBD:



0.0%

Queue Model 1

0115-2700/97/0005-0000\$05.00/0

38 ft	1569 ft	18 ft
-------	---------	-------

1000

19 ft	790 ft	48 ft
-------	--------	-------

1998

177 ft
268 ft

Journal of Management Inquiry

90 ft
68 ft

Sq 45 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
						
C=110° G= 5.0° Y+R= 5.0°	G= 61.5° Y+R= 5.0°	G= 5.1° Y+R= 5.0°	G= 5.8° Y+R= 5.0°	G= 7.6° Y+R= 5.0°	G= 0.0° Y+R= 0.0°	

Esperia Development
Analysis of Western Trail / Coors Blvd - [1_09PXX]
2009 PM Peak NOBUILD Conditions

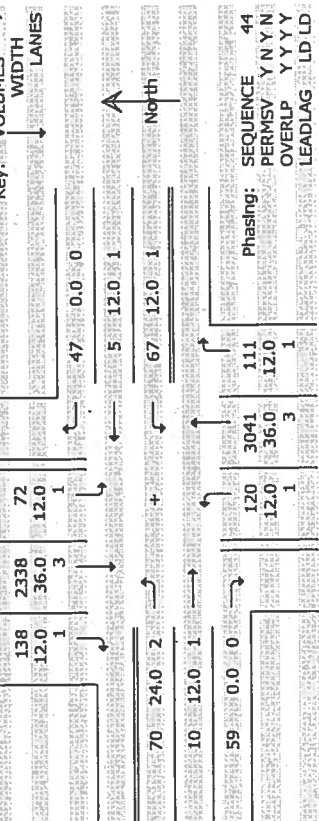
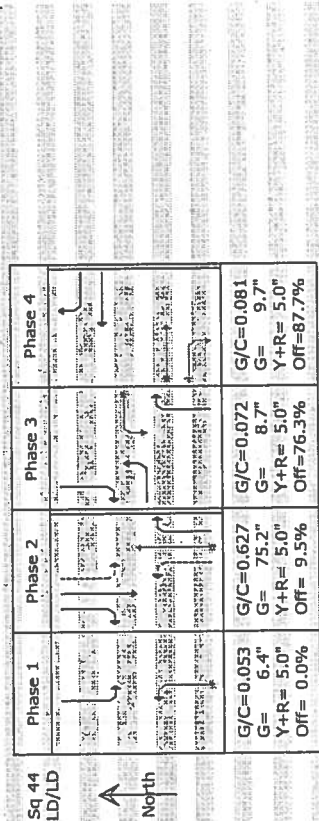
02/27/06
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SIGNAL2000/TEAPACIV4/2.7/071 - Capacity Analysis Summary

SIGNAL2000/TEAPACIV4/2.7/071 - HCM Input Worksheet

Intersection # 1 -
Degree of Saturation (v/c) 0.89 Vehicle Delay 46.2 Level of Service D

Area Local on Type: NONCBD
Key: VOLUMES → WIDTH LANCES



C=120 sec G=100.0 sec = 83.3% Y=20.0 sec = 16.7% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	g/C	Used	Service Rate @ E (veh/h)	Adj Volume	v/c	HCM Delay	L	Queue Model 1
------------	-------------	-----	------	--------------------------	------------	-----	-----------	---	---------------

Heavy veh, %HV	Pk-hr fact, PHF	Pretimed or Act	Strup lost, l	Ext eff grm, e	Arrival typ, AT	Ped vol, vped	Bike vol, vbic	Parking locatns	Park mnvrs, Nm	Bus stops, NB	Grade, %G	EB	TH	LT
2.0	2.0	2.0	2.0	2.0	2.0	0	0	NO	0	0	0	0	0	0
99	99	99	99	99	99	0	0	0	0	0	0	0	0	0
A	A	A	A	A	A	0	0	0	0	0	0	0	0	0
2.0	2.0	2.0	2.0	2.0	2.0	0	0	0	0	0	0	0	0	0
2.0	2.0	2.0	2.0	2.0	2.0	0	0	0	0	0	0	0	0	0
3	3	3	3	3	3	0	0	0	0	0	0	0	0	0

RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
12/1	36/3	12/1	0.138	0.459	0.021	0.741	0.627	0.053	1173	3181	119
12/1	12/1	12/1	0.118	0.118	0.118	0.118	0.118	0.118	139	2362	73
A	B	C	4.5	16.6	32.3	0.107	1.086	0.872	126	3456	136
64 ft	875 ft	56 ft	16.6	32.3	0.468	4.4	67.4	68.8	126	3456	136

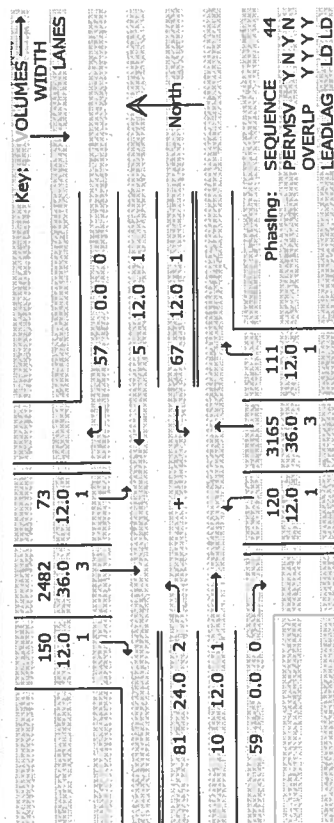
RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
12/1	36/3	12/1	0.129	0.644	0.060	0.741	0.627	0.053	1173	3181	127
12/1	12/1	12/1	0.107	0.107	0.107	0.107	0.107	0.107	1173	3181	143
A	B	C	4.4	67.4	68.8	0.107	1.086	0.872	126	3456	136
58 ft	2228 ft	201 ft	4.4	67.4	68.8	0.107	1.086	0.872	126	3456	136

RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
12/1	36/3	12/1	0.087	0.095	0.072	0.081	0.081	0.072	58	102	49
12/1	12/1	12/1	0.087	0.095	0.072	0.081	0.081	0.072	58	102	49
E+	E+	E+	57.4	69.5	69.5	57.4	69.5	69.5	70	89	89
111 ft	149 ft	149 ft	57.4	69.5	69.5	57.4	69.5	69.5	70	89	89

RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
12/1	36/3	12/1	0.103	0.066	0.072	0.081	0.081	0.072	59	103	215
12/1	12/1	12/1	0.103	0.066	0.072	0.081	0.081	0.072	59	103	215
E+	E+	E+	69.3	54.0	54.0	69.3	54.0	54.0	92	93	93
154 ft	74 ft	74 ft	69.3	54.0	54.0	69.3	54.0	54.0	92	93	93

TEPCALZOD/TEPCALZOD Ver 2.70.07 - Capacity Analysis Summary

Area Location Type: ONCBD

[illegible]

Sq 44 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
G = 7.1" Y+R = 5.0" C = 130°	G = 83.8" Y+R = 5.0"	G = 9.0" Y+R = 5.0"	G = 10.0" Y+R = 5.0"	G = 0.0" Y+R = 0.0"	G = 0.0" Y+R = 0.0"	G = 0.0" Y+R = 0.0"

C=130 sec	G=110.0 sec $\approx$ 84.6%	Y=20.0 sec = 15.4%	Ped= 0.0 sec = 0.0%
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Lane Group	Width, Lanes	g/C Req'd Used	Service Rate @D _{vph} @E _h	Adj Volume	HCM Delay v/c	L S	Queue Model 1
------------	--------------	-------------------	---	---------------	---------------------	--------	------------------

B Approach				17.1 B					
RT	12/1	0.59	0.753	192	152	0.128	4.4	A	73 ft
TH	36/3	0.487	0.645	3272	2507	0.766	17.3	B	1001 ft
LT	12/1	0.026	0.055	113	74	0.481	35.8	D+	60 ft

IB Approach		71.1		E	
TH	12/13	0.142	0.753	1192	1126
TH	36/3	0.670	0.645	3272	3597
L	12/13	0.067	0.055	120	138
				0.106	0.083
				4.4	4.4
				*E	*E
				73.2	77.4
				2493 ft	218 ft

PB Approach				74.9		E
RT+TH	12/1	0.118	0.077	1	93	148 ft
LT	12/1	0.116	0.070	1	91	163 ft
						*E
						78.2
						0.724
						83
						0.669
						71.5
						E

FB Approach		68.4		E					
RT+TH	12/1	0.121	0.077	1	94	0.736	78.9	*E	169 ft
LT	24/2	0.099	0.070	1	157	108	59.5	E+	95 ft

Analysis of
Intersection #2:
Dellyne Ave. (Learning Rd.) / Coors Blvd.

Queueing Analysis Summary Sheet

Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Intersection: Dellyne Ave / Coors Blvd

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	156	200*	1	31	Cont	1	31	Cont	0	238	150**
AM NO BUILD Queue	2	226	175	1	36	75	1	36	75	0	240	300
AM BUILD Queue	2	226	175	1	36	75	1	36	75	0	255	325
Existing Lane Length	2	0	200*	1	0	Cont	1	0	Cont	0	0	150**
PM NO BUILD Queue	2	225	200	1	23	75	1	23	75	0	109	175
PM BUILD Queue	2	225	200	1	23	75	1	23	75	0	124	200

Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	48	200*	1	20	Cont	1	20	Cont	0	77	200
AM NO BUILD Queue	1	191	250	1	79	125	1	79	125	0	113	175
AM BUILD Queue	1	191	250	1	79	125	1	79	125	0	113	175
Existing Lane Length	1	4	200*	1	1	Cont	1	1	Cont	0	6	200
PM NO BUILD Queue	1	149	225	1	113	200	1	113	200	0	59	125
PM BUILD Queue	1	149	225	1	113	200	1	113	200	0	59	125

Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	70	325	3	1,781	Cont	3	1,781	Cont	1	66	225
AM NO BUILD Queue	1	82	125	3	2,044	800	3	2,044	800	1	91	150
AM BUILD Queue	1	95	150	3	2,079	825	3	2,079	825	1	91	150
Existing Lane Length	1	16	325	3	129	Cont	3	129	Cont	1	1	225
PM NO BUILD Queue	1	323	450	3	2,695	>1,000	3	2,695	>1,000	1	112	200
PM BUILD Queue	1	336	450	3	2,730	>1,000	3	2,730	>1,000	1	112	200

Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	1	186	275	3	1,380	Cont	3	1,380	Cont	1	36	250
AM NO BUILD Queue	1	240	300	3	1,554	625	3	1,554	625	1	49	100
AM BUILD Queue	1	240	300	3	1,590	650	3	1,590	650	1	49	100
Existing Lane Length	1	8	275	3	149	Cont	3	149	Cont	1	16	250
PM NO BUILD Queue	1	180	275	3	1,852	850	3	1,852	850	1	223	325
PM BUILD Queue	1	180	275	3	1,888	875	3	1,888	875	1	223	325

Cycle Length: **AM** 110 **PM** 130

* - Unequal Left Turn Lane Lengths for Dual Left Turns

** - Thru Lane queue governs

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Dellyne Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 1-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

SB TOTAL <u>1,843</u>		<u>Coors Blvd</u> N-S STREET		<u>113</u> RIGHT		<u>383</u> WB TOTAL
49 RIGHT		☐ N TR?		<u>79</u> THRU		
240 LEFT		R T L <u>1</u> <u>3</u> <u>1</u>		<u>191</u> LEFT		
THRU						

NO. LANES

2	L
1	T
0	R

TR? ☐ Y

0	R	TR? ☐ Y
1	T	
1	L	

NO. LANES

226 LEFT		<u>1</u> <u>3</u> <u>1</u> L T R		<u>Dellyne Ave</u> E-W STREET	
<u>502</u> EB TOTAL		TR? ☐ N		<u>2,044</u> THRU	
36 THRU				<u>82</u> LEFT	
240 RIGHT				<u>91</u> RIGHT	
				<u>2,217</u> NB TOTAL	

EB LT = <u>113</u> WB TH = <u>192</u> <u>305</u>	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = <u>82</u> SB TH = <u>518</u> <u>600</u>
WB LT = <u>191</u> EB TH = <u>276</u> <u>467*</u>			SB LT = <u>240</u> NB TH = <u>681</u> <u>921*</u>

<u>467</u> E-W CRITICAL	+	<u>921</u> N-S CRITICAL	=	<u>1,388</u>	STATUS? <u>NEAR</u>
NOTES: <u>Proposed Andalucia Geometry (under Design)</u>					

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Dellyne Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 1-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

SB TOTAL <u>1,879</u>		Coors Blvd N-S STREET <u>N</u> TR? R T L 1 3 1		113 RIGHT <u>79</u> THRU <u>191</u> LEFT		<u>383</u> WB TOTAL
49 RIGHT <u>1,590</u> THRU 240 LEFT						

NO. LANES

2	L
1	T
0	R

TR? Y

0	R
1	T
1	L

NO. LANES

TR? Y

517	EB TOTAL	226 LEFT <u>36</u> THRU <u>255</u> RIGHT
-----	----------	---

1	3	1
L	T	R

 TR?
N

Dellyne Ave E-W STREET	
95 LEFT <u>2,079</u> THRU 91 RIGHT	<u>2,265</u> NB TOTAL

EB LT = <u>113</u> WB TH = <u>192</u> <u>305</u> WB LT = <u>191</u> EB TH = <u>291</u> <u>482*</u>	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = <u>95</u> SB TH = <u>530</u> <u>625</u> SB LT = <u>240</u> NB TH = <u>693</u> <u>933*</u>
---	---	--	--

482 E-W CRITICAL	+	933 N-S CRITICAL	=	1,415	STATUS? <u>***OVER***</u>
TES: <u>Proposed Andalucia Geometry (under Design)</u>					

SIGNALIZED INTERSECTION **PLANNING APPLICATION WORKSHEET**

Intersection: Dellyne Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 1-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd		59	
N-S STREET		RIGHT	
N		113	
TR?		321	
R	T	THRU	WB TOTAL
1	3	149	
		LEFT	
SB TOTAL			
2,255			
223	180		
RIGHT	LEFT		
THRU			

NO. LANES

2	L
1	T
0	R

TR?

Y

0	R
1	T
1	L

NO. LANES

TR?

Y

Dellyne Ave		323	
E-W STREET		THRU	
357		2,695	
EB TOTAL		3,130	
225	23	323	112
LEFT	THRU	LEFT	RIGHT
109	109		
RIGHT		NB TOTAL	

EB LT = 113 WB TH = 172 285* WB LT = 149 EB TH = 132 281	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = 323 SB TH = 617 940 SB LT = 180 NB TH = 898 1,078*
---	---	--	---

285	+	1,078	=	1,363	STATUS? NEAR
E-W CRITICAL N-S CRITICAL					
TES: Proposed Andalucia Geometry (under Design)					

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Dellyne Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 1-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET								
SB TOTAL 2,291	N TR?	59 RIGHT 113 THRU 149 LEFT						
223 RIGHT 1,888 THRU 180 LEFT	<table border="1" style="margin: auto;"> <tr> <td>R</td> <td>T</td> <td>L</td> </tr> <tr> <td style="text-align: center;">1</td> <td style="text-align: center;">3</td> <td style="text-align: center;">1</td> </tr> </table>	R	T	L	1	3	1	321 WB TOTAL
R	T	L						
1	3	1						

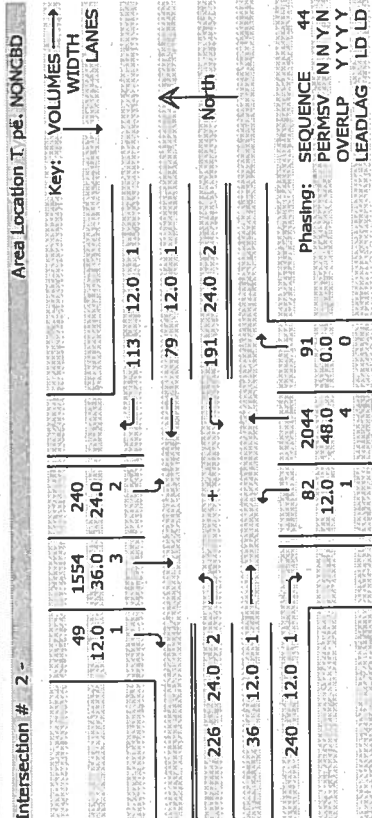
NO. LANES <table border="1" style="width: 100%;"> <tr><td style="text-align: center;">2</td><td style="text-align: center;">L</td></tr> <tr><td style="text-align: center;"> </td><td style="text-align: center;"> </td></tr> <tr><td style="text-align: center;">1</td><td style="text-align: center;">T</td></tr> <tr><td style="text-align: center;"> </td><td style="text-align: center;"> </td></tr> <tr><td style="text-align: center;">0</td><td style="text-align: center;">R</td></tr> </table>	2	L			1	T			0	R	TR? Y	NO. LANES <table border="1" style="width: 100%;"> <tr><td style="text-align: center;">0</td><td style="text-align: center;">R</td></tr> <tr><td style="text-align: center;"> </td><td style="text-align: center;"> </td></tr> <tr><td style="text-align: center;">1</td><td style="text-align: center;">T</td></tr> <tr><td style="text-align: center;"> </td><td style="text-align: center;"> </td></tr> <tr><td style="text-align: center;">1</td><td style="text-align: center;">L</td></tr> </table>	0	R			1	T			1	L
2	L																					
1	T																					
0	R																					
0	R																					
1	T																					
1	L																					

225 LEFT 23 THRU 124 RIGHT	<table border="1" style="margin: auto;"> <tr> <td style="text-align: center;">1</td> <td style="text-align: center;"> </td> <td style="text-align: center;">3</td> <td style="text-align: center;"> </td> <td style="text-align: center;">1</td> </tr> <tr> <td style="text-align: center;">L</td> <td style="text-align: center;"> </td> <td style="text-align: center;">T</td> <td style="text-align: center;"> </td> <td style="text-align: center;">R</td> </tr> </table> TR? N	1		3		1	L		T		R	Dellyne Ave E-W STREET 2,730 THRU 3,178 NB TOTAL
1		3		1								
L		T		R								
372 EB TOTAL	336 LEFT	112 RIGHT										

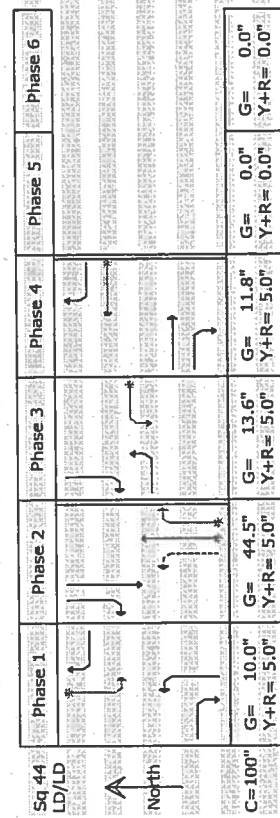
EB LT = 113 WB TH = 172 285	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER
WB LT = 149 EB TH = 147 296*		

296 E-W CRITICAL	+	1,090 N-S CRITICAL	=	1,386	STATUS? <u>NEAR</u>
NOTES: Proposed Andalusia Geometry (under Design)					

SIGNAL 2000/TEAPAC Ver 2.701071 - HCM Input Worksheet



	SB	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Pk-hr fact, PHF	.92	.92	.92	.50	.50	.50	.84	.84	.84	.78	.78	.78
Prelim'd or Act	A	A	A	A	A	A	A	A	A	A	A	A
Strutp lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



SIGNAL 2000/TEAPAC Ver 2.701071 - Capacity Analysis Summary

Intersection Averages for Int. # 2 - Degree of Saturation (v/c) 0.78 Vehicle Delay 32.2 Level of Service C

Sq 44 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4
	</			

C=100 sec G= 80.0 sec = 80.0% Y=20.0 sec = 20.0% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	g/C Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	Queue Model 1
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SB Approach

	RT	TH	LT
12/1	0.059	0.631	0.973
36/3	0.342	0.445	2197
24/2	0.100	0.100	262
	53	1689	261
	0.054	0.769	0.777
	7.1	25.1	154.8
	A	C	D
	28.0 ft	669 ft	1195 ft

NB Approach

	RT+TH	LT
48/4	0.380	0.445
12/1	0.027	0.100
	2911	218
	2541	98
	0.873	0.398
	28.4	15.6
	C	B
	1836 ft	75 ft

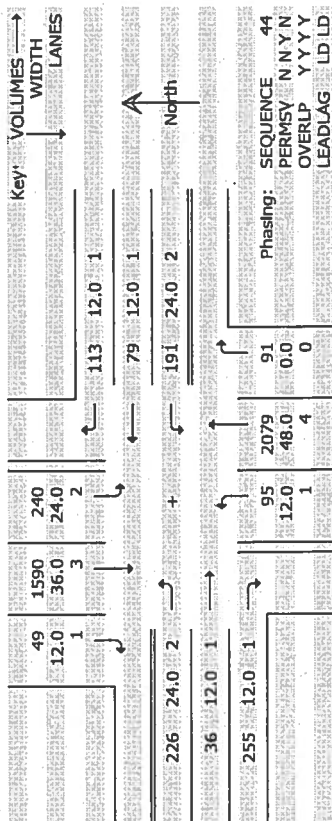
WB Approach

	RT	TH	LT
12/1	0.182	0.269	0.413
12/1	0.118	0.118	159
24/2	0.136	0.136	383
	226	413	382
	0.546	0.735	0.836
	32.9	54.9	54.8
	C	*D	*D
	257 ft	219 ft	285 ft

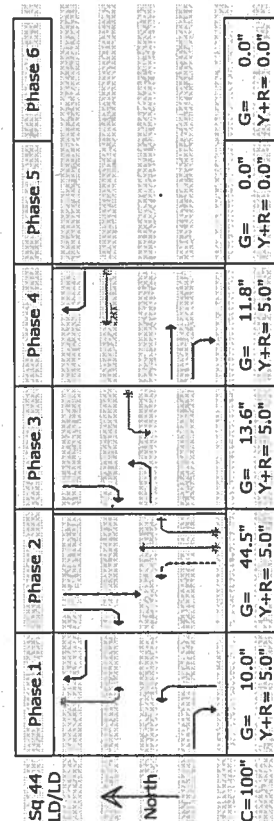
EB Approach

	RT	TH	LT
12/1	0.233	0.269	0.413
12/1	0.046	0.118	159
24/2	0.109	0.136	383
	308	413	290
	0.744	0.214	0.635
	40.6	40.4	43.7
	D+	D+	D+
	382 ft	56 ft	196 ft

SIGNAL2000/TEAPAC Ver 2.70.07 HCM Input Worksheet
Intersection # 2 - Area Location Type: NONCBD

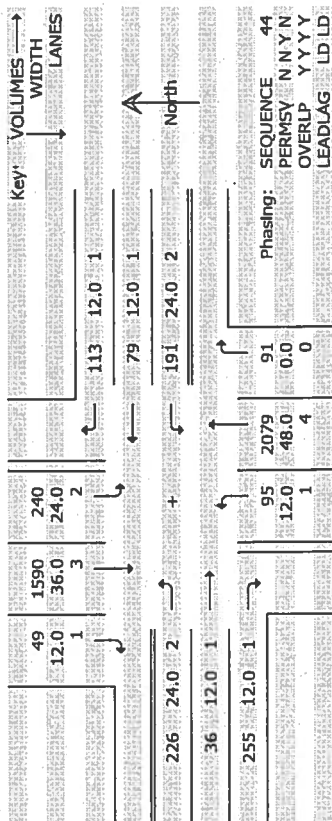


	SB	TH	RT	LT	WB	TH	RT	LT	NB	TH	RT	LT	EB	TH	RT	LT
Heavy veh, %HV	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Pk-hr fact, PHF	.92	.92	.92	.92	.50	.50	.50	.50	.84	.84	.84	.84	.78	.78	.78	.78
Predimed or Act	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
Strup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff gm, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

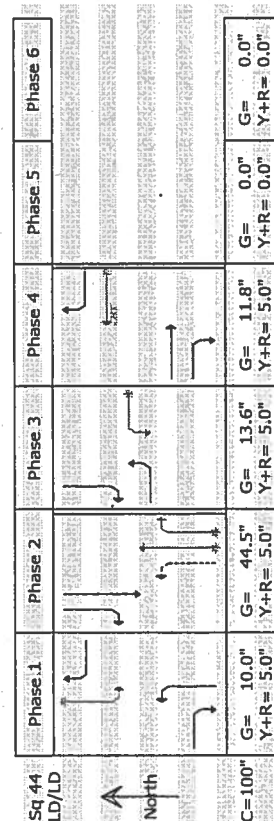


SIGNAL2000/TEAPAC Ver 2.70.07 HCM Input Worksheet

Intersection # 2 - Area Location Type: NONCBD

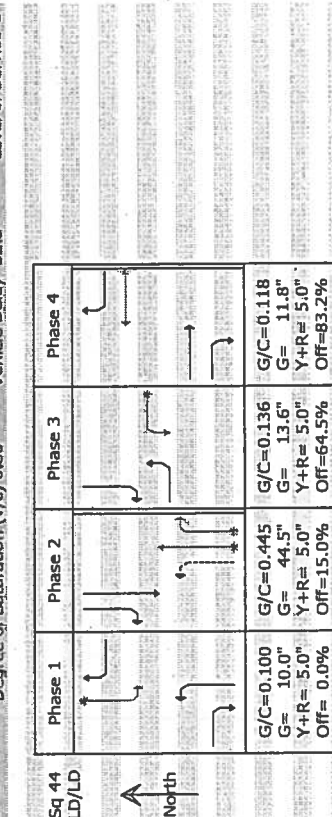


	SB	TH	RT	LT	WB	TH	RT	LT	NB	TH	RT	LT	EB	TH	RT	LT
Heavy veh, %HV	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Pk-hr fact, PHF	.92	.92	.92	.92	.50	.50	.50	.50	.84	.84	.84	.84	.78	.78	.78	.78
Predimed or Act	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
Strup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff gm, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



SIGNAL2000/TEAPAC Ver 2.70.07 Capacity Analysis Summary

Intersection Averages for Int # 2 - Degree of Saturation (v/c) 0.80 Vehicle Delay 32.8 Level of Service C



C=100 sec G= 80.0 sec = 80.0% Y=20.0 sec = 20.0% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	Req'd	g/C Used	Service Rate @D (vph)	Adj Volume	v/c	HCM Delay	Queue Model 1
------------	-------------	-------	----------	-----------------------	------------	-----	-----------	---------------

SB Approach							28.9	C
RT	12/1	0.059	0.631	973	973	53	0.054	7.1 A
TH	36/3	0.349	0.445	2197	2197	1728	0.787	25.7 C+
LT	24/2	0.100	0.100	262	319	261	0.777	54.8 *D

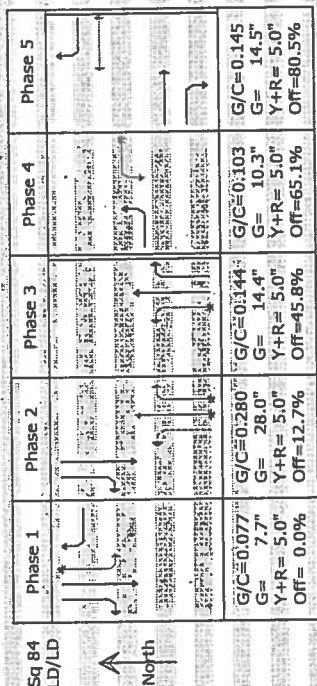
NB Approach							28.7	C
RT+TH	48/4	0.386	0.445	2911	2911	2583	0.887	29.2 *C
LT	12/1	0.035	0.100	218	242	113	0.459	16.7 B

WB Approach							48.4	D
RT	12/1	0.182	0.269	367	413	226	0.546	32.9 C
TH	12/1	0.118	0.118	159	195	158	0.735	54.9 *D
LT	24/2	0.136	0.136	383	449	382	0.836	54.8 *D

EB Approach							43.6	D+
RT	12/1	0.245	0.269	367	413	327	0.790	43.9 D+
TH	12/1	0.046	0.118	159	195	46	0.214	40.4 D+
LT	24/2	0.109	0.136	383	449	290	0.635	43.7 D+

SIGNA 3000/TEAPACT Ver 2.70.077 - Capacity Analysis Summary

Intersection	Averages for Int #	Degree of Saturation (v/c)	Vehicle Delay	Level of Service D+
2 -		0.90	42.5	



Lane Group	Width/ Lanes	g/C Reqd	Service Rate @D (vph) @E	Adj Volume	HCM Delay	L S	Queue Model 1
					v/c		

USB Approach		39.8				D+				
RT	12/1	0.186	0.408	603	628	232	0.369	21.0	C+	212 ft
TH	36/3	0.385	0.408	2013	2013	1929	0.958	40.6	D+	957 ft
LT	24/2	0.077	0.077	189	237	188	0.726	54.9	*D	141 ft

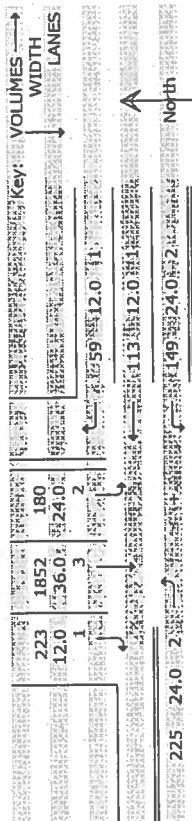
NB Approach		42.4		D+	
RT+TH	48/4	0.454	0.474	3103	3085
LT	12/1	0.144	0.144	356	355
				404	355
				0.872	0.872
				40.9	40.9
				54.8	54.8
				*D	*D
				463 ft	463 ft
				1205 ft	1205 ft

WB Approach				50.1					D
RT	12/1	0.102	0.273	374	420	107	0.255	28.7	C
TH	12/1	0.145	0.145	206	247	205	0.777	54.8	*D
LT	24/2	0.103	0.103	272	329	271	0.783	54.9	*D

EB Approach		48.5					D		
RT	12/1	0.115	0.145	170	206	125	0.558	D+	160 ft
TH	12/1	0.029	0.145	206	247	26	0.098	D+	30 ft
LT	24/2	0.100	0.103	272	329	259	0.749	D	190 ft

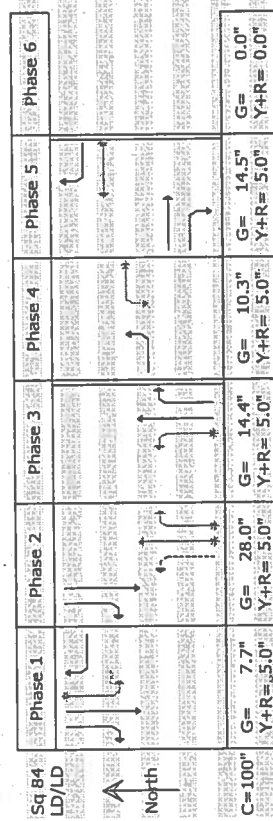
©/ENAP 0000738 ©/ENAP 700 071-FCM Input Worksheet

Intersection # 2 - Area Location Type: NONCBD



109	12.0	1	323	2695	112	Phasing:	SEQUENCE	84
			12.0	48.0	0.0		PERMSV	N N Y N
					0		OVERLP	Y Y Y Y
		1		4			LEADLAG	LD LD

	SB		WB		NB		EB	
	RT	LT	RT	TH	RT	TH	RT	TH
Heavy veh, %HV	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Pk-hr fact, PHF	.96	.96	.96	.55	.55	.55	.91	.87
Pretimed or Act	A	A	A	A	A	A	A	A
Startup lost, I ₁	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3
Ped vol, vped	0		0	0	0	0	0	0
Bike vol, vbic	0		0	0	0	0	0	0
Parking locatns	NO		NO	NO	NO	NO	NO	NO
Park minvrs, Nim	0		0	0	0	0	0	0
Bus stops, NB	0		0	0	0	0	0	0
Grade, %G	.0		0	0	.0	.0	.0	.0

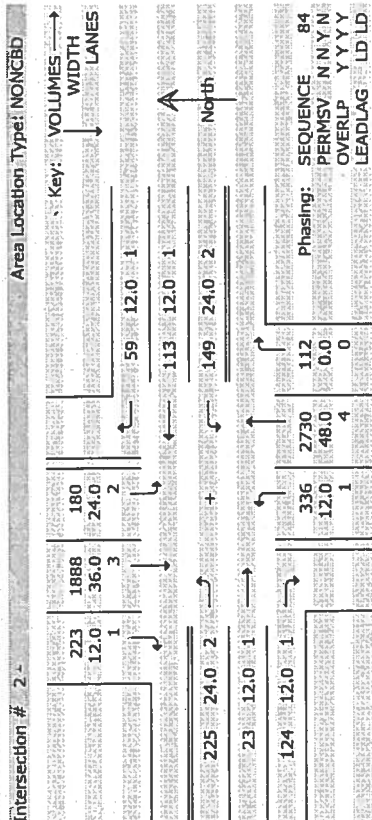


Esperita Development
Analysis of Delyne Ave / Coors Blvd - [2_09P8X]
2009 PM Peak BUILD Conditions

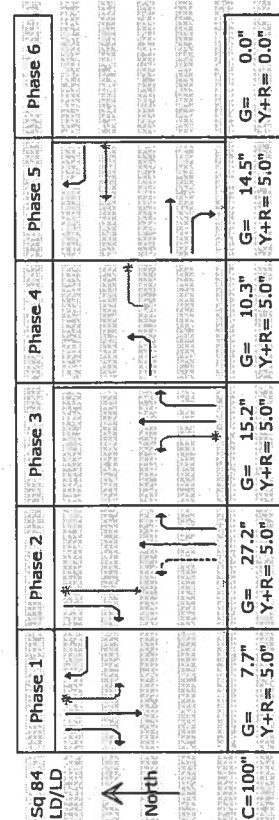
02/27/06
09:28:50

SIGNAL2000/TEAPAC Vol 2/01071 - HCM Input Worksheet

SIGNAL2000/TEAPAC Vol 2/01071 - Capacity Analysis Summary



	SB	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
PK-hr fact, PHF	.96	.96	.96	.55	.55	.55	.91	.91	.91	.87	.87	.87	.87	.87	.87	.87	.87	.87
Prelim'd or Act	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, lt	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mvmrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

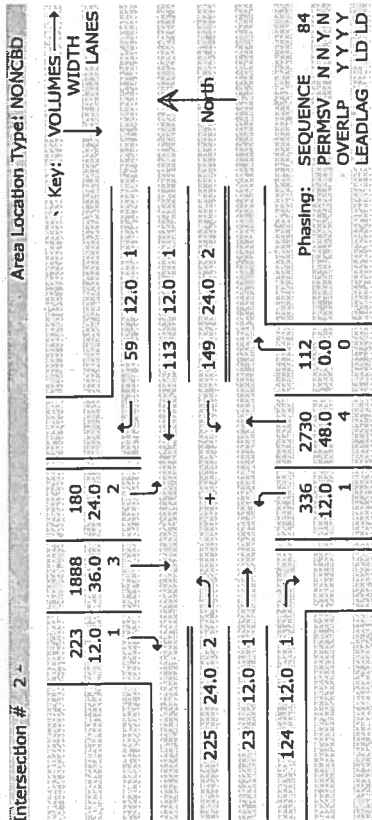


Esperita Development
Analysis of Delyne Ave / Coors Blvd - [2_09P8X]
2009 PM Peak BUILD Conditions

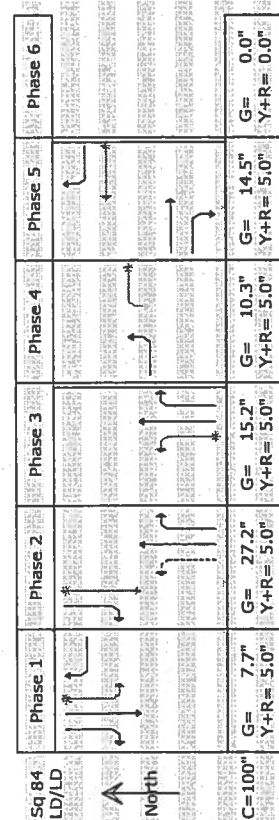
02/27/06
09:28:50

SIGNAL2000/TEAPAC Vol 2/01071 - HCM Input Worksheet

SIGNAL2000/TEAPAC Vol 2/01071 - Capacity Analysis Summary



	SB	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
PK-hr fact, PHF	.96	.96	.96	.55	.55	.55	.91	.91	.91	.87	.87	.87	.87	.87	.87	.87	.87	.87
Prelim'd or Act	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, lt	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mvmrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



Analysis of
Intersection #3:
Montano Rd. / Coors Blvd.

Queueing Analysis Summary Sheet

Project:
Intersection:

Esperia Development (Sevilla Ave / Coors Blvd)
Montaño Rd / Coors Blvd

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	74	300	2	743	Cont	2	743	Cont	1	274	300
AM NO BUILD Queue	2	75	75	2	898	550	2	898	550	1	354	400
AM BUILD Queue	2	75	75	2	898	550	2	898	550	1	371	425
Existing Lane Length	2	268	300	2	312	Cont	2	312	Cont	1	245	300
PM NO BUILD Queue	2	271	225	2	615	450	2	615	450	1	400	525
PM BUILD Queue	2	271	225	2	615	450	2	615	450	1	447	575

Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	166	350	2	252	Cont	2	252	Cont	0	59	0
AM NO BUILD Queue	2	220	175	2	299	225	2	299	225	0	63	100
AM BUILD Queue	2	228	175	2	299	225	2	299	225	0	63	100
Existing Lane Length	2	369	350	2	837	Cont	2	837	Cont	0	105	0
PM NO BUILD Queue	2	545	400	2	1,006	700	2	1,006	700	0	113	200
PM BUILD Queue	2	562	425	2	1,006	700	2	1,006	700	0	113	200

Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	213	650	3	1,238	Cont	3	1,238	Cont	1	310	500
AM NO BUILD Queue	2	409	275	3	1,531	625	3	1,531	625	1	557	600
AM BUILD Queue	2	422	300	3	1,545	625	3	1,545	625	1	566	600
Existing Lane Length	2	649	650	3	1,658	Cont	3	1,658	Cont	1	225	500
PM NO BUILD Queue	2	1,036	700	3	2,022	>1,000	3	2,022	>1,000	1	522	650
PM BUILD Queue	2	1,087	725	3	2,047	>1,000	3	2,047	>1,000	1	541	675

Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
Existing Lane Length	2	286	410	3	1,288	Cont	3	1,288	Cont	1	56	350
AM NO BUILD Queue	2	363	250	3	1,480	600	3	1,480	600	1	61	100
AM BUILD Queue	2	363	250	3	1,490	625	3	1,490	625	1	61	100
Existing Lane Length	2	162	410	3	1,301	Cont	3	1,301	Cont	1	126	350
PM NO BUILD Queue	2	273	225	3	1,606	750	3	1,606	750	1	138	225
PM BUILD Queue	2	273	225	3	1,627	775	3	1,627	775	1	138	225

AM PM
 Cycle Length: 110 130

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Montaño Rd / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 3-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

SB TOTAL <u>1,904</u>		Coors Blvd N-S STREET N TR?		63 RIGHT <u>299</u>		582 WB TOTAL
61 RIGHT	363 LEFT	R 1	T 3	L 2	THRU <u>220</u>	
THRU <u>1,480</u>					LEFT	

NO. LANES

2	L
2	T
1	R

TR?

N

0	R
2	T
2	L

NO. LANES

TR?

Y

75 LEFT <u>898</u>	2 L	3 T	1 R	TR? N	Montaño Rd E-W STREET 1,531 THRU <u>409</u>	557 RIGHT
1,327 EB TOTAL					LEFT <u>2,497</u>	NB TOTAL
THRU <u>354</u>						
RIGHT						

EB LT = <u>38</u> WB TH = <u>181</u> 219	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = <u>205</u> SB TH = <u>493</u> 698*
WB LT = <u>110</u> EB TH = <u>449</u> 559*			SB LT = <u>182</u> NB TH = <u>510</u> 692

559 E-W CRITICAL	+	698 N-S CRITICAL	=	1,257	STATUS? <u>NEAR</u>
NOTES: Existing Geometry					

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Montaño Rd / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 3-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

SB TOTAL <u>1,914</u>		Coors Blvd N-S STREET <u>N</u> TR? R T L 1 3 2		63 RIGHT <u>299</u> THRU <u>228</u> LEFT		<u>590</u> WB TOTAL
61 RIGHT <u>1,490</u> THRU	363 LEFT					

NO. LANES

2	L
2	T
1	R

TR? N

0	R
2	T
2	L

NO. LANES

TR? Y

1,344 EB TOTAL	75 LEFT <u>898</u> THRU <u>371</u> RIGHT
-------------------	---

2	3	1
L	T	R

TR? N

Montaño Rd E-W STREET		
422 LEFT	1,545 THRU <u>2,533</u> NB TOTAL	566 RIGHT

EB LT = 38
 WB TH = 181
219

WB LT = 114
 EB TH = 449
563*

MAXIMUM SUM
OF CRITICAL VALUES

0 TO 1,200
 1,201 TO 1,400
 >1,400

CAPACITY
LEVEL

UNDER
NEAR
OVER

NB LT = 211
 SB TH = 497
708*

SB LT = 182
 NB TH = 515
697

563 + 708 = 1,271 STATUS? NEAR
 E-W CRITICAL N-S CRITICAL

NOTES: Existing Geometry

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Montaño Rd / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 3-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		Montaño Rd E-W STREET																				
SB TOTAL 2,017	N TR?	113 RIGHT 1,006	1,664 WB TOTAL																			
138 RIGHT 1,606	273 LEFT THRU	THRU 545	LEFT																			
<table border="1" style="margin: auto;"> <tr> <td>R</td> <td>T</td> <td>L</td> </tr> <tr> <td style="text-align: center;">1</td> <td style="text-align: center;">3</td> <td style="text-align: center;">2</td> </tr> </table>		R	T	L	1	3	2	<table border="1" style="margin: auto;"> <tr> <td>0</td> <td>R</td> </tr> <tr> <td></td> <td></td> </tr> <tr> <td>2</td> <td>T</td> </tr> <tr> <td></td> <td></td> </tr> <tr> <td>2</td> <td>L</td> </tr> </table>		0	R			2	T			2	L			
R	T	L																				
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0	R																					
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NO. LANES <table border="1" style="margin: auto;"> <tr><td>2</td><td>L</td></tr> <tr><td></td><td></td></tr> <tr><td>2</td><td>T</td></tr> <tr><td></td><td></td></tr> <tr><td>1</td><td>R</td></tr> </table>		2	L			2	T			1	R	TR? Y										
2	L																					
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TR? N		NO. LANES																				
<table border="1" style="margin: auto;"> <tr> <td>2</td> <td></td> <td>3</td> <td></td> <td>1</td> </tr> <tr> <td>L</td> <td></td> <td>T</td> <td></td> <td>R</td> </tr> </table>		2		3		1	L		T		R	TR? N										
2		3		1																		
L		T		R																		
<table border="1" style="margin: auto;"> <tr> <td>271</td> </tr> <tr> <td>LEFT</td> </tr> <tr> <td> 1,286 </td> </tr> <tr> <td>EB TOTAL</td> </tr> <tr> <td>THRU</td> </tr> <tr> <td> 400 </td> </tr> <tr> <td>RIGHT</td> </tr> </table>		271	LEFT	1,286	EB TOTAL	THRU	400	RIGHT	<table border="1" style="margin: auto;"> <tr> <td>2,022</td> <td></td> <td>522</td> </tr> <tr> <td>THRU</td> <td></td> <td>RIGHT</td> </tr> <tr> <td> 1,036 </td> <td></td> <td> 3,580 </td> </tr> <tr> <td>LEFT</td> <td></td> <td>NB TOTAL</td> </tr> </table>		2,022		522	THRU		RIGHT	1,036		3,580	LEFT		NB TOTAL
271																						
LEFT																						
1,286																						
EB TOTAL																						
THRU																						
400																						
RIGHT																						
2,022		522																				
THRU		RIGHT																				
1,036		3,580																				
LEFT		NB TOTAL																				

EB LT = 136
 WB TH = 560

695

*

WB LT = 273
 EB TH = 308

580

MAXIMUM SUM
OF CRITICAL VALUES

0 TO 1,200
 1,201 TO 1,400
 >1,400

CAPACITY
LEVEL

UNDER
NEAR
OVER

NB LT = 518
 SB TH = 535

1,053

*

SB LT = 137
 NB TH = 674

811

695 + 1,053 = 1,748

E-W CRITICAL

N-S CRITICAL

STATUS? ***OVER***

NOTES: Existing Geometry

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Montaño Rd / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 3-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		Montaño Rd E-W STREET	
SB TOTAL 2,038	N TR?	113 RIGHT 1,006	1,681 WB TOTAL
138 RIGHT	273 LEFT	THRU 562	
THRU		LEFT	
R T L 1 3 2		0 R TR? Y	
NO. LANES 2 L		2 T	
2 T		2 L	
TR? N		NO. LANES	
1 R		2 L	
271 LEFT		2,047 THRU	
1,333 EB TOTAL		3,675 NB TOTAL	
615 THRU		1,087 LEFT	
447 RIGHT		541 RIGHT	

EB LT = 136
 WB TH = 560

695

*

WB LT = 281
 EB TH = 308

589

MAXIMUM SUM
OF CRITICAL VALUES

0 TO 1,200
 1,201 TO 1,400
 >1,400

CAPACITY
LEVEL

UNDER
NEAR
OVER

NB LT = 544
 SB TH = 542

1,086

*

SB LT = 137
 NB TH = 682

819

695 + 1,086 = 1,781

E-W CRITICAL N-S CRITICAL

STATUS? ***OVER***

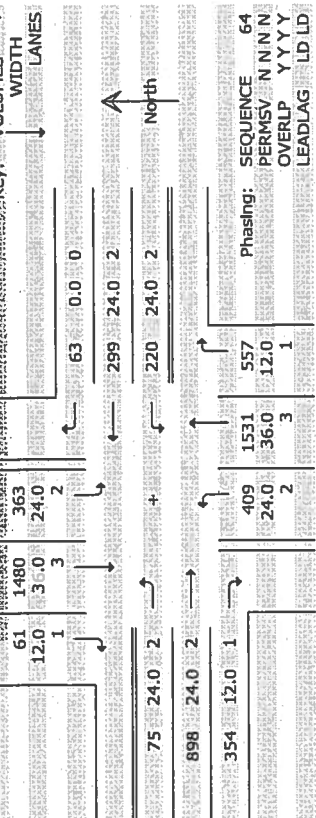
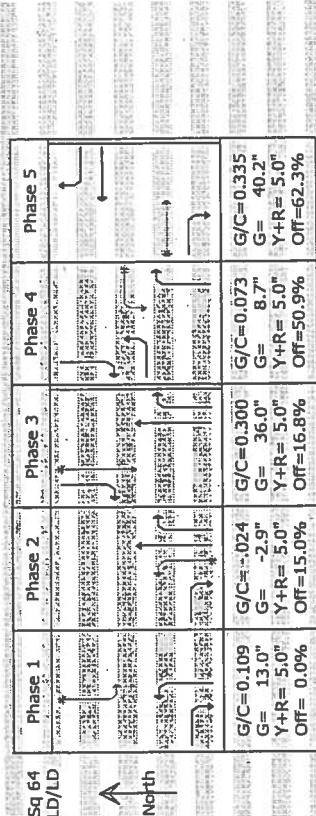
NOTES: Existing Geometry

SIGNAL 2000/ITEAPAC (Ver 2.70.07) - Capacity Analysis Summary

SIGNAL 2000/ITEAPAC (Ver 2.70.07) - HCM Input Worksheet

Intersection # 3 - Area Location Type: NONCBB
Degree of Saturation (V/S) 1.03 Vehicle Delay 99.6 Level of Service F

Intersection # 3 - Area Location Type: NONCBB
Key: VOLUMES → WIDTH LANES



C=120 sec G= 95.0 sec = 79.2% Y=25.0 sec = 20.8% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	g/C Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	L	Queue Model 1
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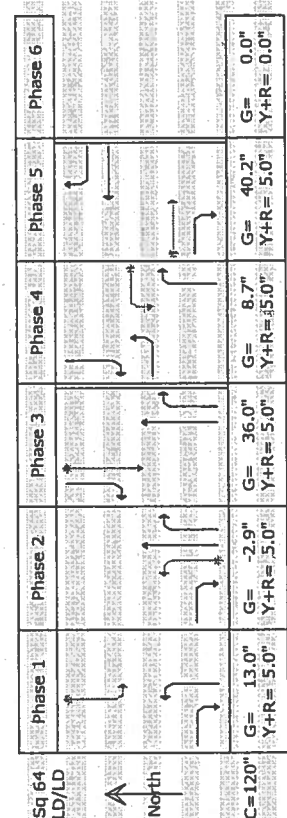
RT	12/1	0.090	0.414	618	72	0.109	21.7	C+	69 ft
TH	36/3	0.349	0.300	1457	1741	1.141	113.9	*F	1286 ft
LT	24/2	0.160	0.109	241	427	1.142	144.4	*F	505 ft

RT	12/1	0.439	0.431	650	686	0.966	59.5	E+	1046 ft
TH	36/3	0.364	0.317	1557	1615	1.129	107.5	F	1319 ft
LT	24/2	0.176	0.126	302	407	1.122	133.3	*F	555 ft

RT+TH	24/2	0.171	0.335	1106	1162	0.404	30.9	C	287 ft
LT	24/2	0.122	0.073	123	217	1.139	155.3	*F	353 ft

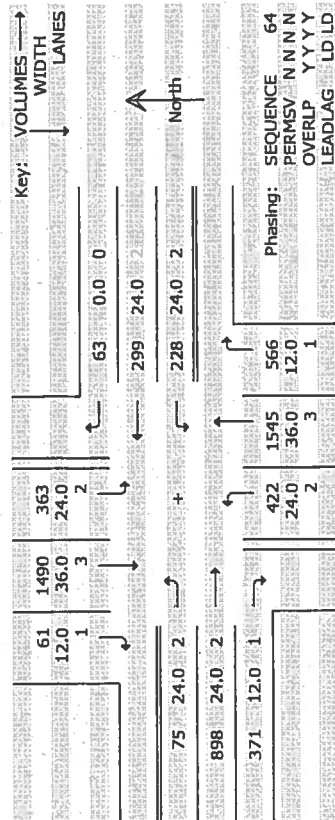
RT	12/1	0.369	0.503	780	799	0.671	24.6	C+	595 ft
TH	24/2	0.391	0.335	1137	1193	1.141	113.8	*F	1421 ft
LT	24/2	0.071	0.073	123	217	0.454	54.7	D	92 ft

RT	TH	LT	WB	TH	LT	RT	TH	LT	EB	TH	LT
1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6
.85	.85	.85	.77	.77	.84	.84	.84	.84	.66	.66	.66
A	A	A	A	A	A	A	A	A	A	A	A
2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
3	3	3	3	3	3	3	3	3	3	3	3
0	0	0	0	0	0	0	0	0	0	0	0
NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
0	0	0	0	0	0	0	0	0	0	0	0
.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

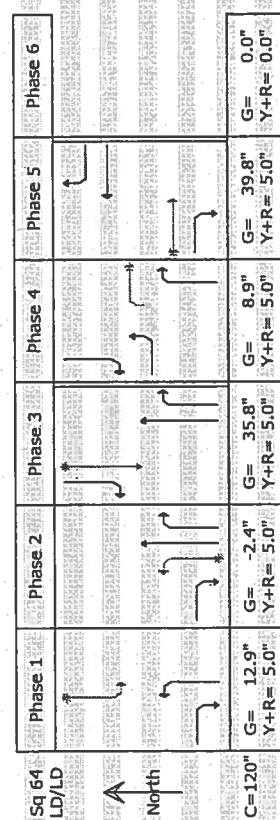


SIGNAL2000/TEAPAC Ver 2.70.071 - HCM Input Worksheet

Intersection # 3 - Area Location Type: NONCBD

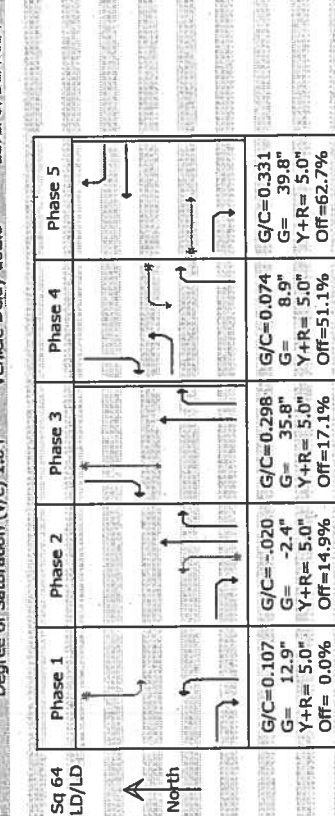


	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6
Pk-hr fact, PHF	.85	.85	.85	.77	.77	.77	.84	.84	.84	.66	.66	.66
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Strup lost, lt	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff gmn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park minvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



SIGNAL2000/TEAPAC Ver 2.70.071 - Capacity Analysis Summary

Intersection Averages for Int # 3 - Degree of Saturation (v/c) 1.04 Vehicle Delay 102.0 Level of Service F



C=120 sec G= 95.0 sec = 79.2% Y=25.0 sec = 20.8% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	Reqd	g/c Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	L	Queue Model 1
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SB Approach	RT	TH	LT	12/1	36/3	24/2	0.090	0.414	0.298	0.107	619	1451	237	340	72	1753	427	1.109	1.153	1.154	21.6	119.1	149.3	C+	*F	*F	69 ft	1314 ft	512 ft
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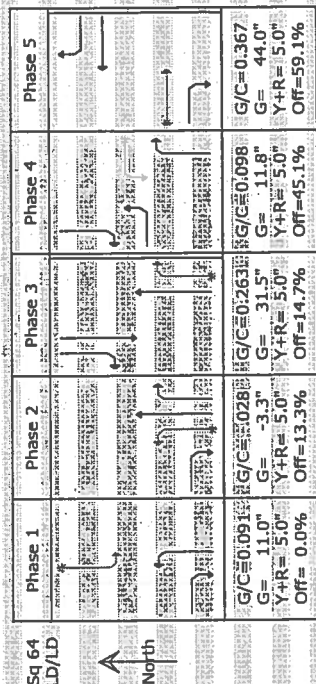
NB Approach	RT	TH	LT	12/1	36/3	24/2	0.445	0.436	0.320	0.129	658	1575	313	419	693	1631	502	0.973	1.128	1.128	60.5	106.7	134.8	E+	*F	*F	1071 ft	1327 ft	574 ft
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WB Approach	RT+TH	LT	24/2	24/2	0.171	0.331	0.074	1091	130	223	470	0.409	1.152	158.9	31.3	C	288 ft
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EB Approach	RT	TH	LT	12/1	24/2	24/2	0.383	0.502	0.391	0.074	779	1122	130	223	562	1361	114	0.704	1.153	0.444	25.8	119.2	54.4	C+	*F	D	640 ft	1443 ft	92 ft
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SIGNAL2000/TEAPAC Ver 2.70.07 - Capacity Analysis Summary

Intersection	Averages for Int. #	Degree of Saturation (v/c)	Vehicle Delay	Level of Service
3 -	0.91	61:0	E+	



C=120 sec G=95.0 sec = 79.2% Y=25.0 sec = 20.8% Ped=0.0 sec = 0.0%

Lane Group	"Width"/ Lanes	g/G Reqd Used	Service Rate @D (vph) @E	Adj Volume	HCM Delay	Queue Model 1
L	S			v/c		

5B Approach		62.4					E+			
RT	12/1	0.090	0.403	597	640	72	0.112	22.5	C+	71 ft
TH	48/4	0.271	0.263	1696	1786	1753	0.982	61.0	E+	821 ft
LT	36/3	0.116	0.091	293	441	427	0.905	74.8	*E	270 ft*

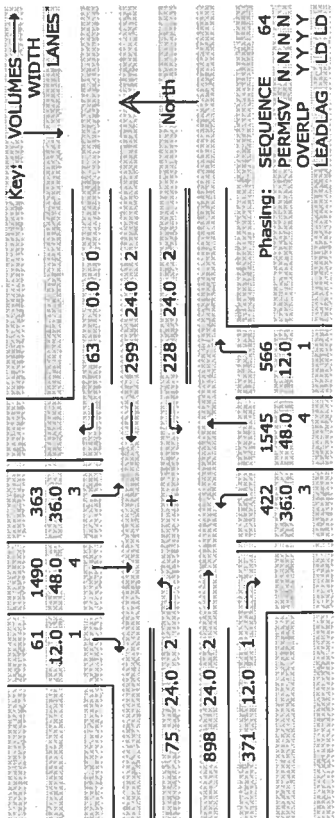
NB Approach		65.0 E+							
RT	12/1	0.445	0.417	623	662	674	1.018	74.6	*E 1148 ft
TH	48/4	0.282	0.277	1801	1880	1839	0.978	58.9	E+ 851 ft
LT	36/3	0.130	0.105	367	517	502	0.923	74.7	*E 317 ft

VIB Approach				46.0 D						
RT+TH	24/2	0.171	0.367	1230	1272	470	0.369	28.0	C	273 ft
LT	24/2	0.124	0.098	206	307	296	0.873	74.7	*E	271 ft

EB Approach		59/2				E+				
RT	12/1	0.383	0.514	801	817	562	0.688	24.4	C+	624 ft.
TH	24/2	0.391	0.367	1266	1307	1361	1.041	74.3	*E	1245 ft.
LT	24/2	0.071	0.098	206	307	114	0.336	51.0	D	88 ft.

[illegible]

Intersection # 3 - Area Location Type: NONCBD



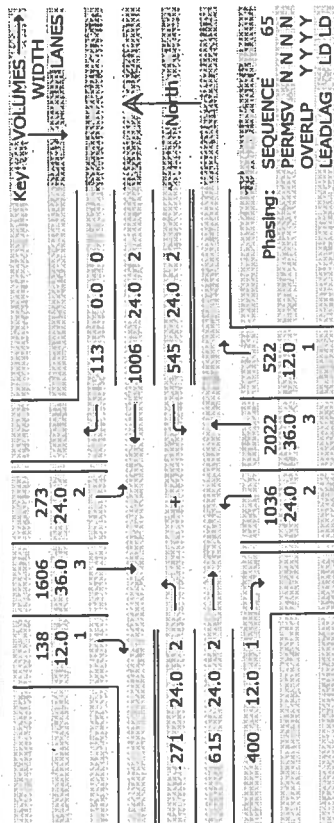
	SB		WB		NB		EB	
	RT	LT	RT	LT	RT	LT	RT	LT
Heavy veh, %HV	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6
Peak-hr fact, PHF	.85	.85	.77	.77	.84	.84	.66	.66
Pretimed or Act	A	A	A	A	A	A	A	A
Startup lost, I1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Start-up eff, g ₁	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3
Ped vol, vped	0		0		0		0	
Bike vol, v _{bic}	0		0		0		0	
Parking locatns	NO		NO		NO		NO	
Park mnvrs, Nm	0		0		0		0	
Bus stops, NB	0		0		0		0	
Grade, %G	.0		.0		.0		.0	

Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
G= 11.0" Y+R= 5.0"	G= -3.3" Y+R= 5.0"	G= 31.5" Y+R= 5.0"	G= 11.8" Y+R= 5.0"	G= 44.0" Y+R= 5.0"	G= 0.0" Y+R= 0.0"

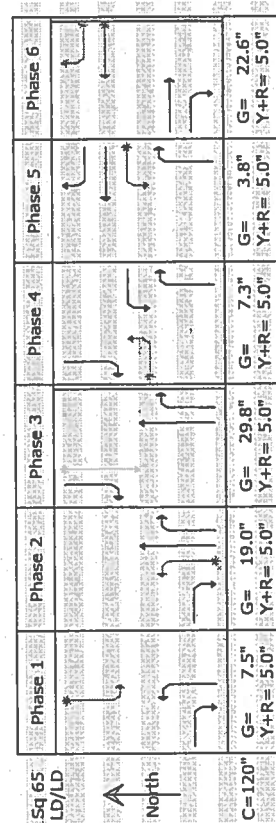
Esperia Development
Analysis of Montano Rd / Coors Blvd - [3_09PNX]
2009 PM Peak NOBUILD Conditions

02/27/06
09:58:22

SIGNAL2000/TEAPAC/Vol 2.70.071 - HCM Input Worksheet
Intersection # 3 - Area Location Type: JNONGBIB



	SB	TH	RT	LT	WB	RT	TH	LT	NB	TH	RT	LT	EB	TH	RT	LT
Heavy veh, %HV	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6
Pk-hr fact, PHF	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
Startup lost, l	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Parking locals	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvs, Nm	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



Esperia Development
Analysis of Montano Rd / Coors Blvd - [3_09PNX]
2009 PM Peak NOBUILD Conditions

02/27/06
09:58:22

SIGNAL2000/TEAPAC/Vol 2.70.071 - Capacity Analysis Summary

Intersection Averages for Int # 3 -
Degree of Saturation (v/c) 1.09 Vehicle Delay 124.0 Level of Service F

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 65 LD/LD	12/1 36/3 24/2	12/1 36/3 24/2	12/1 36/3 24/2	12/1 36/3 24/2	12/1 36/3 24/2	12/1 36/3 24/2
G/C=0.062 G= 7.5" Y+R= 5.0" Off= 0.0%	G/C=0.158 G= 19.0" Y+R= 5.0" Off=10.4%	G/C=0.248 G= 29.8" Y+R= 5.0" Off=30.4%	G/C=0.061 G= 7.3" Y+R= 5.0" Off=59.4%	G/C=0.032 G= 3.8" Y+R= 5.0" Off=69.7%	G/C=0.188 G= 22.6" Y+R= 5.0" Off=77.0%	G/C=0.188 G= 22.6" Y+R= 5.0" Off=77.0%

C=120 sec G= 90.0 sec = 75.0% Y=30.0 sec = 25.0% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	g/C Reqd	Used	Service Rate @D (vph) @E	Adj Volume	v/c	HCM Delay	L Queue	Model 1 S
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SB Approach

RT	12/1	0.141	0.351	504	558	144	0.258	28.0	C 159 ft
TH	36/3	0.338	0.248	1165	1265	1673	1.323	195.9	*F 1506 ft
LT	24/2	0.121	0.062	78	181	284	1.321	229.4	*F 418 ft

NB Approach

RT	12/1	0.394	0.625	993	993	580	0.584	14.2	B+ 511 ft
TH	36/3	0.437	0.448	2284	2284	2247	0.984	47.8	D 1300 ft
LT	24/2	0.349	0.262	817	905	1151	1.272	175.3	*F 1391 ft

WB Approach

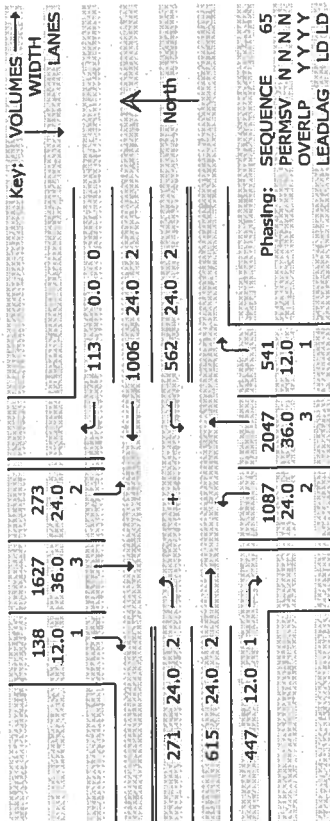
RT+TH	24/2	0.347	0.262	829	917	1166	1.272	175.1	*F 1432 ft
LT	24/2	0.198	0.135	333	439	568	1.224	170.8	*F 709 ft

EB Approach

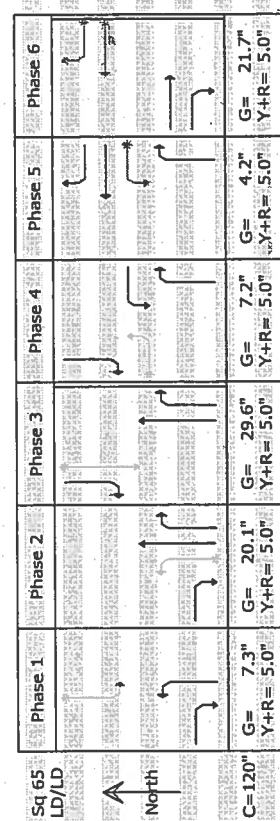
RT	12/1	0.300	0.492	761	782	412	0.527	21.6	C+ 422 ft
TH	24/2	0.209	0.188	552	662	634	0.946	70.6	E 573 ft
LT	24/2	0.120	0.061	73	177	279	1.322	230.5	*F 412 ft

SIGNAL2000/TEAPAC Ver 2.70.071 - HCM Input Worksheet

Intersection # 3 - Area Location Type: NONGAD

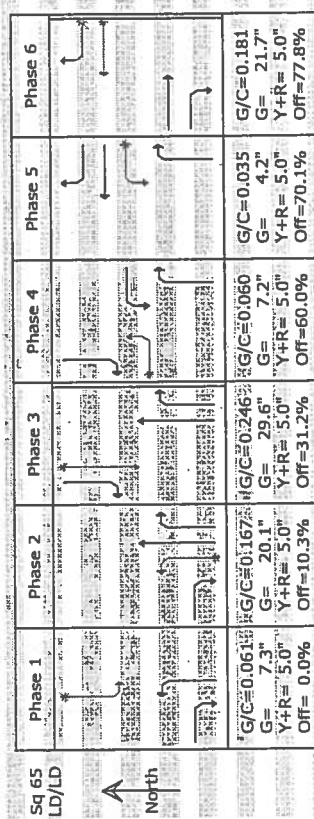


	SB	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	EB
Heavy veh, %HV	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6
PK-hr fact, PHF	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96	.96
Predimed or Act	A	A	A	A	A	A	A	A	A	A	A	A	A
Strup lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0



SIGNAL2000/TEAPAC Ver 2.70.071 - Capacity Analysis Summary

Intersection Averages for Int # 3 -
Degrees of Saturation (V/S) 1.11 Vehicle Delay 130.0 Level of Service F



C=120 sec G= 90.0 sec = 75.0% Y=30.0 sec = 25.0% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	Req'd	g/C	Used	Service Rate @D (vph) @E	Adj Volume	V/c	HCM Delay	L	Queue Model 1
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SB Approach	RT	TH	LT	12/1	36/3	24/2	0.141	0.341	0.121	0.348	0.246	0.061	553	1255	176	144	1695	284	2007	F

NB Approach	RT	TH	LT	12/1	36/3	24/2	0.405	0.442	0.363	0.634	0.455	0.270	1007	2320	848	932	1208	11296	82.7	F

WB Approach	RT+TH	LT	24/2	24/2	0.347	0.257	0.136	811	901	1166	1294	185.2	178.8	743	1464	F

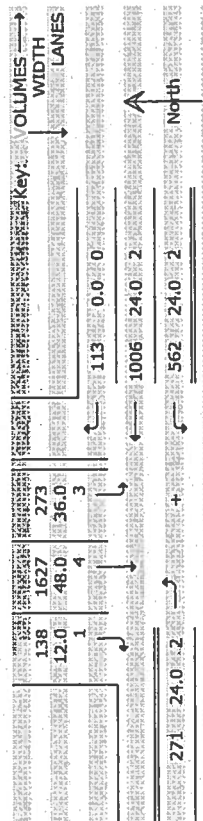
EB Approach	RT	TH	LT	12/1	36/3	24/2	0.327	0.209	0.120	0.492	0.181	0.060	761	632	173	461	634	279	94.0	F

Esperia Development
Analysis of Montalvo Rd / Coors Blvd - [3_09PB1]
2009 PM PK BUILD Cond - Triple NB/SB LT, 4th NB/SB Thru Lanes

03/02/06
20:51:40

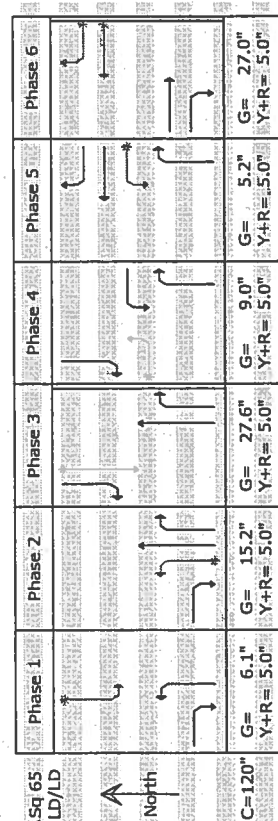
SIGNAL2008/TEAPAC(V6.027) HCM Input Worksheet

Intersection # 3 - Area Location Type: NONCBD



SEQUENCE	65
PERMSV	NNNN
OVERLP	Y.Y.Y
LEADLAG	LD LD

	SB		WB		NB		EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6	1.6
PK-hr fact, PHF	.96	.96	.96	.96	.96	.96	.97	.97	.97
Prelim'd or Act	A	A	A	A	A	A	A	A	A
Strutp lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff gm, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0



Esperia Development
Analysis of Montalvo Rd / Coors Blvd - [3_09PB1]
2009 PM PK BUILD Cond - Triple NB/SB LT, 4th NB/SB Thru Lanes

03/02/06
20:51:40

SIGNAL2008/TEAPAC(V6.027) Capacity Analysis Summary

Intersection Averages for Int # 3 - Degree of Saturation (v/c) 0.93 Vehicle Delay 70.3 Level of Service E

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 65 LD/LD						
	G/C=0.051 G= 6.1" Y+R= 5.0" Off= 0.0%	G/C=0.126 G= 15.2" Y+R= 5.0" Off= 9.2%	G/C=0.230 G= 27.6" Y+R= 5.0" Off= 26.0%	G/C=0.075 G= 9.0" Y+R= 5.0" Off= 53.2%	G/C=0.043 G= 5.2" Y+R= 5.0" Off= 64.8%	G/C=0.225 G= 27.0" Y+R= 5.0" Off= 73.3%

C=120 sec G= 90.0 sec = 75.0% Y= 30.0 sec = 25.0% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	Reqd	Used	g/C	Service Rate @D (vph) @E	Adj Volume	V/c	HCM Delay	L	Queue Model 1
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SB Approach										
	RT	TH	LT							
	12/1	48/4	36/3	0.141	0.263	0.090	0.346	0.230	0.051	
	551	1564	52	496	1448	52	144	1695	284	
	28.4	95.3	136.8	0.261	1.084	1.084	0.261	1.084	1.084	
	C	*F	*F	C	*F	*F	C	*F	*F	
	160 ft	908 ft	229 ft							

NB Approach										
	RT	TH	LT							
	12/1	48/4	36/3	0.405	0.339	0.252	0.599	0.398	0.219	
	953	2704	1013	953	2704	1013	601	2274	1208	
	16.8	35.2	93.8	0.631	0.841	1.068	0.631	0.841	1.068	
	B	D+	*F	B	D+	*F	B	D+	*F	
	573 ft	848 ft	804 ft							

WB Approach										
	RT+TH	LT								
	24/2	24/2	0.347	0.310	0.160	0.1087	1166	585		
	426	533	426	426	533	426	1087	585		
	107.3	106.2	107.3	107.3	106.2	107.3	107.3	106.2		
	*F	*F	*F	*F	*F	*F	*F	*F		
	1134 ft	605 ft								

EB Approach										
	RT	TH	LT							
	12/1	24/2	24/2	0.327	0.209	0.120	0.485	0.225	0.075	
	748	697	130	771	801	223	461	634	279	
	23.7	49.3	136.4	0.598	0.792	1.086	0.598	0.792	1.086	
	C+	D	*F	C+	D	*F	C+	D	*F	
	497 ft	494 ft	325 ft							

Analysis of
Intersection #4:
Sevilla Ave. (Maduri Ave.) / Coors Blvd.

Queueing Analysis Summary Sheet

Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Intersection: Sevilla Ave / Coors Blvd

2009

Eastbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	0	<i>Design</i>	1	0	<i>Cont</i>	1	0	<i>Cont</i>	0	0	0
AM NO BUILD Queue	1	33	75	1	0	0	1	0	0	0	43	75
AM BUILD Queue	1	82	125	1	0	0	1	0	0	0	115	175
<i>Existing Lane Length</i>	1	0	<i>Design</i>	1	0	<i>Cont</i>	1	0	<i>Cont</i>	0	0	0
PM NO BUILD Queue	1	20	50	1	0	0	1	0	0	0	26	75
PM BUILD Queue	1	163	250	1	1	0	1	1	0	0	184	275
Westbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	0	<i>Design</i>	1	0	<i>Cont</i>	1	0	<i>Cont</i>	0	0	0
AM NO BUILD Queue	1	47	100	1	0	0	1	0	0	0	42	75
AM BUILD Queue	1	47	100	1	0	0	1	0	0	0	42	75
<i>Existing Lane Length</i>	1	0	<i>Design</i>	1	0	<i>Cont</i>	1	0	<i>Cont</i>	0	0	0
PM NO BUILD Queue	1	29	75	1	0	0	1	0	0	0	45	100
PM BUILD Queue	1	29	75	1	1	0	1	1	0	0	45	100
Northbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	0	<i>Design</i>	3	1,549	<i>Cont</i>	3	1,549	<i>Cont</i>	1	0	0
AM NO BUILD Queue	1	31	75	3	1,751	700	3	1,751	700	1	15	50
AM BUILD Queue	1	95	150	3	1,751	700	3	1,751	700	1	15	50
<i>Existing Lane Length</i>	1	0	<i>Design</i>	3	2,562	<i>Cont</i>	3	2,562	<i>Cont</i>	1	0	0
PM NO BUILD Queue	1	118	200	3	2,907	>1,000	3	2,907	>1,000	1	53	100
PM BUILD Queue	1	254	350	3	2,907	>1,000	3	2,907	>1,000	1	53	100
Southbound				Left Turns			Thru Movements			Right Turns		
Approach	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)	# Lanes	Vol.	Length (Ft.)
<i>Existing Lane Length</i>	1	0	<i>Design</i>	3	2,146	<i>Cont</i>	3	2,146	<i>Cont</i>	1	0	0
AM NO BUILD Queue	1	22	50	3	2,474	>1,000	3	2,474	>1,000	1	0	0
AM BUILD Queue	1	22	50	3	2,474	>1,000	3	2,474	>1,000	1	52	100
<i>Existing Lane Length</i>	1	0	<i>Design</i>	3	2,148	<i>Cont</i>	3	2,148	<i>Cont</i>	1	0	0
PM NO BUILD Queue	1	58	125	3	2,495	>1,000	3	2,495	>1,000	1	0	0
PM BUILD Queue	1	58	125	3	2,495	>1,000	3	2,495	>1,000	1	127	200

Cycle Length: AM 110 PM 130

SIGNALIZED INTERSECTION **PLANNING APPLICATION WORKSHEET**

Intersection: Sevilla Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 3-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		
SB TOTAL 2,496	N TR?	42 RIGHT 0 THRU 47 LEFT
0 RIGHT 2,474 THRU	R T L 1 3 1	89 WB TOTAL

NO. LANES

1	L								
1	T								
0	R								
					TR?	Y			

0	R								
1	T								
1	L								
					TR?	Y			

	Sevilla Ave E-W STREET	
33 LEFT 76 EB TOTAL 0 THRU 43 RIGHT	1 3 1 L T R N	1,751 THRU 31 LEFT 15 RIGHT 1,797 NB TOTAL

EB LT = 33 WB TH = 42 75 WB LT = 47 EB TH = 43 90*	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = 31 SB TH = 825 856* SB LT = 22 NB TH = 584 606
---	---	--	---

90	+	856	=	946	
E-W CRITICAL		N-S CRITICAL			STATUS? <u>UNDER</u>

NOTES: Proposed Geometry

SIGNALIZED INTERSECTION PLANNING APPLICATION WORKSHEET

Intersection: Sevilla Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 3-Mar-06
 Time Period Analyzed: AM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		Sevilla Ave E-W STREET								
SB TOTAL <u>2,548</u> 52 22 RIGHT LEFT <u>2,474</u> THRU	<u>N</u> TR? <table style="width: 100%; text-align: center;"> <tr> <td>R</td> <td>T</td> <td>L</td> </tr> <tr> <td>1</td> <td>3</td> <td>1</td> </tr> </table>	R	T	L	1	3	1	42 RIGHT 0 THRU 47 LEFT 89 WB TOTAL		
R	T	L								
1	3	1								
NO. LANES <table style="width: 100%;"> <tr> <td style="width: 50%; text-align: center;">1</td> <td style="width: 50%; text-align: center;">L</td> </tr> <tr> <td style="text-align: center;">1</td> <td style="text-align: center;">T</td> </tr> <tr> <td style="text-align: center;">0</td> <td style="text-align: center;">R</td> </tr> </table> TR? <u>Y</u>		1	L	1	T	0	R	0 R 1 T 1 L NO. LANES TR? <u>Y</u>		
1	L									
1	T									
0	R									
EB TOTAL <u>197</u> 82 LEFT 0 THRU 115 RIGHT		<table style="width: 100%; text-align: center;"> <tr> <td>1</td> <td>3</td> <td>1</td> </tr> <tr> <td>L</td> <td>T</td> <td>R</td> </tr> </table> TR? <u>N</u> 1,751 THRU 95 15 LEFT RIGHT <u>1,861</u> NB TOTAL		1	3	1	L	T	R	
1	3	1								
L	T	R								

EB LT = <u>82</u> WB TH = <u>42</u> <u>124</u> WB LT = <u>47</u> EB TH = <u>115</u> <u>162</u>*	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER		NB LT = <u>95</u> SB TH = <u>825</u> <u>920</u>* SB LT = <u>22</u> NB TH = <u>584</u> <u>606</u>
---	--	--	--	--

162	+	920	=	1,082		STATUS?	UNDER
E-W CRITICAL		N-S CRITICAL					

NOTES: Proposed Geometry

SIGNALIZED INTERSECTION **PLANNING APPLICATION WORKSHEET**

Intersection: Sevilla Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 NO BUILD

Date: 3-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

Coors Blvd N-S STREET		Sevilla Ave E-W STREET												
SB TOTAL 2,553 0 RIGHT 58 LEFT 2,495 THRU NO. LANES <table border="1" style="width:100%;"> <tr><td>1</td><td>L</td></tr> <tr><td>1</td><td>T</td></tr> <tr><td>0</td><td>R</td></tr> </table> TR? Y EB TOTAL 46 20 LEFT 0 THRU 26 RIGHT	1	L	1	T	0	R	TR? N <table border="1" style="width:100%;"> <tr><td>R</td><td>T</td><td>L</td></tr> <tr><td>1</td><td>3</td><td>1</td></tr> </table> TR? N 1 L 3 T 1 R	R	T	L	1	3	1	45 RIGHT 0 THRU 29 LEFT 74 WB TOTAL 0 R 1 T 1 L NO. LANES TR? Y 118 LEFT 53 RIGHT 2,907 THRU 3,078 NB TOTAL
1	L													
1	T													
0	R													
R	T	L												
1	3	1												

EB LT = 20
 WB TH = 45
65*

WB LT = 29
 EB TH = 26
55

MAXIMUM SUM
OF CRITICAL VALUES

0 TO 1,200
 1,201 TO 1,400
 >1,400

CAPACITY
LEVEL

UNDER
NEAR
OVER

NB LT = 118
 SB TH = 832
950

SB LT = 58
 NB TH = 969
1,027*

65 + 1,027 = 1,092 STATUS? **UNDER**

E-W CRITICAL N-S CRITICAL

NOTES: Proposed Geometry

SIGNALIZED INTERSECTION **PLANNING APPLICATION WORKSHEET**

Intersection: Sevilla Ave / Coors Blvd
 Analyst: TOB
 Project: Esperia Development (Sevilla Ave / Coors Blvd)
 Condition: 2009 BUILD

Date: 3-Mar-06
 Time Period Analyzed: PM Peak Hr.
 City / State: Albuquerque, NM

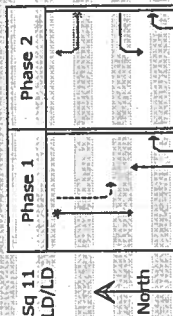
Coors Blvd N-S STREET												
SB TOTAL 2,680	N TR?	45 RIGHT 1 THRU 29 LEFT										
127 RIGHT 2,495 THRU	58 LEFT 1	75 WB TOTAL										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">R</td> <td style="width: 33%;">T</td> <td style="width: 33%;">L</td> </tr> <tr> <td style="text-align: center;">1</td> <td style="text-align: center;">3</td> <td style="text-align: center;">1</td> </tr> </table>			R	T	L	1	3	1				
R	T	L										
1	3	1										
NO. LANES <table border="1" style="width:100%; border-collapse: collapse;"> <tr><td style="width: 33%;">1</td><td style="width: 33%;">L</td></tr> <tr><td> </td><td> </td></tr> <tr><td>1</td><td>T</td></tr> <tr><td> </td><td> </td></tr> <tr><td>0</td><td>R</td></tr> </table>		1	L			1	T			0	R	TR? Y
1	L											
1	T											
0	R											
NO. LANES <table border="1" style="width:100%; border-collapse: collapse;"> <tr><td style="width: 33%;">0</td><td style="width: 33%;">R</td></tr> <tr><td> </td><td> </td></tr> <tr><td>1</td><td>T</td></tr> <tr><td> </td><td> </td></tr> <tr><td>1</td><td>L</td></tr> </table>		0	R			1	T			1	L	NO. LANES
0	R											
1	T											
1	L											
EB TOTAL 348		163 LEFT 1 THRU 184 RIGHT										
1 L		3 T										
1 R		1 R										
TR? N		Sevilla Ave E-W STREET										
		2,907 THRU 3,214 NB TOTAL										
		254 LEFT										
		53 RIGHT										

EB LT = 163 WB TH = 46 209	MAXIMUM SUM OF CRITICAL VALUES 0 TO 1,200 1,201 TO 1,400 >1,400	CAPACITY LEVEL UNDER NEAR OVER	NB LT = 254 SB TH = 832 1,086*
WB LT = 29 EB TH = 185 214*			SB LT = 58 NB TH = 969 1,027

214 E-W CRITICAL	+	1,086 N-S CRITICAL	=	1,300	STATUS? <u>NEAR</u>
NOTES:					

SIGNAL2000/TEAPAC Ver 2.70.b7 - Capacity Analysis Summary

Inte section Averages for Int #	Degree of Saturation (v/c)	Vehicle Delay	Level of Service A
4	0.57	4.4	



$G/C = 0.840$ $G = 100.8''$ $Y+R = 5.0''$ $Off = 0.0\%$	$G/C = 0.077$ $G = 9.2''$ $Y+R = 5.0''$ $Off = 88.1\%$
--	---

C=120 sec **G=110.0 sec = 91.7%** **Y=10.0 sec = 8.3%** **Ped=0.0 sec = 0.0%**

Lane Group	Width/ Lanes	g/c Reqd / Used	Service Rate @ 10 (yph) @ Eff	Adj Volume	v/c	HCM Delay	L S	Queue Model 1
------------	-----------------	--------------------	----------------------------------	---------------	-----	--------------	--------	------------------

5B Approach

TH	36/3	0.535	0.840	220	4220	2780	0.659	3.8	*A	577 ft
L†	12/1	0.104	0.840	127	138	25	0.179	2.4	A	10 ft

JA Annmach

RT	12/1	0.057	1.000	1568	17	0.011	0.0	1 ft
TH	36/3	0.394	0.840	4220	1967	0.466	2.6	310 ft

WB Approach

RT	12/1	0.077	0.077	50	94	49	0.405	55.0	*D
LT	12/1	0.076	0.077	58	106	55	0.407	54.8	D

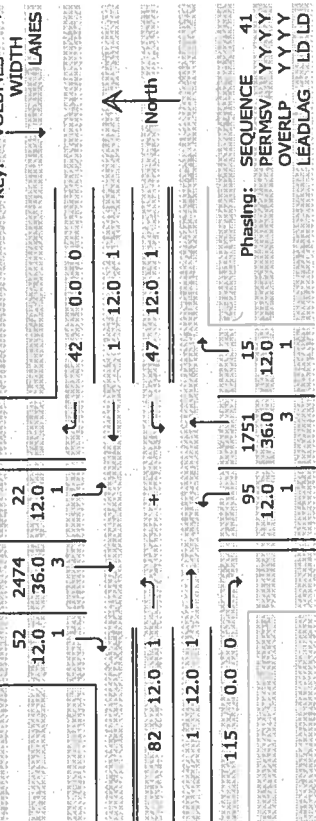
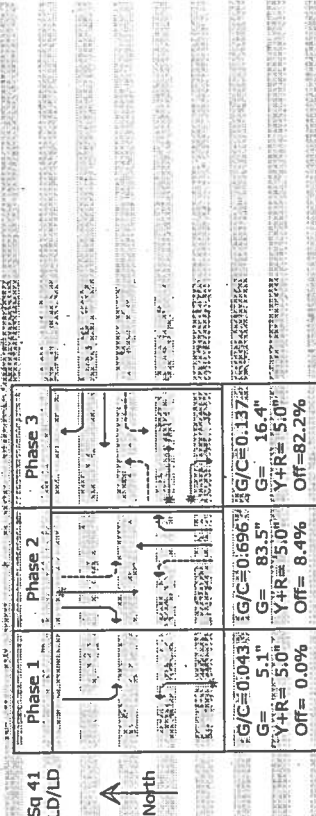
RT	12/1	0.077	0.077	50	94	49	0.405	55.0	*D	77 ft
LT	12/1	0.076	0.077	58	106	55	0.407	54.8	D	86 ft

SIGNAL2000/TEAPAC Ver 2.70.071 - Capacity Analysis Summary

SIGNAL2000/TEAPAC Ver 2.70.071 - HCM Input Worksheet

Intersection # 4 - Area Location Type: NONCBD
Degree of Saturation (V/G) 0.68 Vehicle Delay 15.2 Level of Service B

Intersection # 4 - Area Location Type: NONCBD
Key: VOLUMES → WIDTH LANES



C=120 sec G=105.0 sec = 87.5% Y=15.0 sec = 12.5% Ped=0.0 sec = 0.0%

C=120 sec G=105.0 sec = 87.5% Y=15.0 sec = 12.5% Ped=0.0 sec = 0.0%

Lane Group	Width/Lanes	Reqd	g/G Used	Service Rate @D (vph) @E	Adj Volume	V/c	HCM Delay	L	Queue Model 1
------------	-------------	------	----------	--------------------------	------------	-----	-----------	---	---------------

Lane Group	Width/Lanes	Reqd	g/G Used	Service Rate @D (vph) @E	Adj Volume	V/c	HCM Delay	L	Queue Model 1
------------	-------------	------	----------	--------------------------	------------	-----	-----------	---	---------------

RT	12/1	0.082	0.696	1091	1091	0.053	5.8	A	30 ft
TH	36/3	0.535	0.696	3497	3497	0.795	13.8	*B+	1016 ft
LT	12/1	0.000	0.043	151	151	0.146	6.2	A	12 ft

RT	12/1	0.082	0.696	1091	1091	0.053	5.8	A	30 ft
TH	36/3	0.535	0.696	3497	3497	0.795	13.8	*B+	1016 ft
LT	12/1	0.000	0.043	151	151	0.146	6.2	A	12 ft

RT	12/1	0.057	0.696	1091	1091	0.016	5.6	A	9 ft
TH	36/3	0.394	0.696	3497	3497	0.562	9.3	A	541 ft
LT	12/1	0.043	0.043	108	107	0.787	54.4	*D	124 ft

RT	12/1	0.057	0.696	1091	1091	0.016	5.6	A	9 ft
TH	36/3	0.394	0.696	3497	3497	0.562	9.3	A	541 ft
LT	12/1	0.043	0.043	108	107	0.787	54.4	*D	124 ft

RT+TH	12/1	0.077	0.137	137	187	0.233	46.7	D	71 ft
LT	12/1	0.109	0.137	78	108	0.414	49.5	D	83 ft

RT+TH	12/1	0.077	0.137	137	187	0.233	46.7	D	71 ft
LT	12/1	0.109	0.137	78	108	0.414	49.5	D	83 ft

RT+TH	12/1	0.137	0.137	137	187	0.633	54.9	*D	210 ft
LT	12/1	0.123	0.137	113	156	0.525	50.9	D	146 ft

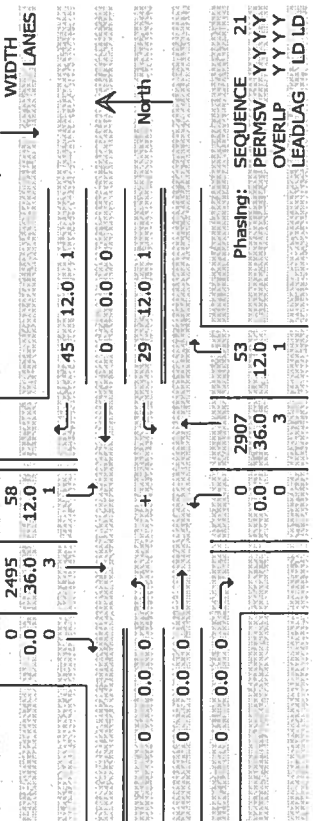
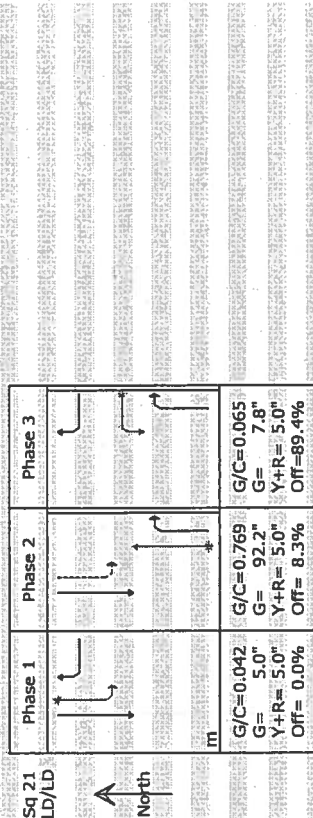
RT+TH	12/1	0.137	0.137	137	187	0.633	54.9	*D	210 ft
LT	12/1	0.123	0.137	113	156	0.525	50.9	D	146 ft

SIGNAL2000/TEAPAC/VOL270.D72 Capacity Analysis Summary

SIGNAL2000/TEAPAC/VOL270.D71 - HCM Input Worksheet

Intersection # 4 -
 Degree of Saturation (V/c) 0.67 Vehicle Delay 6.7 Level of Service A

Area Location Type: NONC-BB



C=120 sec G=105.0 sec = 87.5% Y=15.0 sec = 12.5% Ped= 0.0 sec = 0.0%

Sequence 21
 PERMSV YYY Y
 OVERLP YYY Y
 LEADLAG LD LD

Lane Group	Width/Lanes	g/G	Reqd	Used	Service Rate @D (vph) @E	Adj Volume	V/c	HCM Delay	L	Queue Model
------------	-------------	-----	------	------	--------------------------	------------	-----	-----------	---	-------------

Heavy veh, %HV	PK-hr fact, PHF	Pretimed or Act	Strup lost, l1	Ext eff gm, e	Arrival typ, AT	Ped vol, vped	Blke vol, vblc	Parking locatns	Park mnvrs, Nm	Bus stops, NB	Grade, %G
----------------	-----------------	-----------------	----------------	---------------	-----------------	---------------	----------------	-----------------	----------------	---------------	-----------

SB Approach												3.2		A
TH	36/3	0.490	0.852	4281	4281	2520	0.589	2.9	A	444 ft				
LT	12/1	0.008	0.042	106	120	59	0.440	16.6	*B	30 ft				

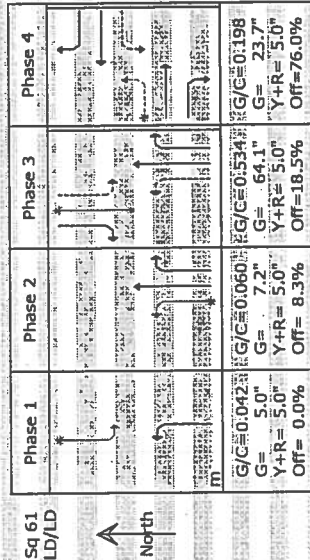
NB Approach	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
RT	12/1	0.080	0.875	1372	1372	3862	2936	0.760	8.6	*A	893 ft	
TH	36/3	0.562	0.769	3862	2936	0.760	8.6	*A	893 ft			

WB Approach												B1.5				A				
RT	12/1	0.080	0.875	1372	1372	54	0.039	1.0	A	12 ft										
TH	36/3	0.562	0.769	3862	3862	2936	0.760	8.6	*A	893 ft										

Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
G= 5.0"	G= 92.2"	G= 7.8"	G= 0.0"	G= 0.0"	G= 0.0"
Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 0.0"	Y+R= 0.0"	Y+R= 0.0"

SIGNAL2000/TEAPACTVer 2.70.07] - Capacity Analysis Summary

Intersection Averages for Int #	Level of Service C
4	30.7
Degree of Saturation (v/c)	0.87
Vehicle Delay	



C=170 sec G=100.0 sec = 83.3% Y=20.0 sec = 16.7% Ped= 0.0 sec = 0.0%

Line Group	Width/ Lanes	g/C		Service Rate @B (vnh) @E	Adj Volume	HCM Delay	Queue Model 1
		Reqd	Used		v/c	L	S

SB Approach

	RT	12/1	0.131	0.534	827	838	128	0.153	14.3	B+	103 ft
TH		36/3	0.490	0.534	2684	2684	2520	0.939	33.5	*C	1325 ft
LT		12/1	0.012	0.042	110	123	59	0.440	24.5	*C+	59 ft

NB: Approach

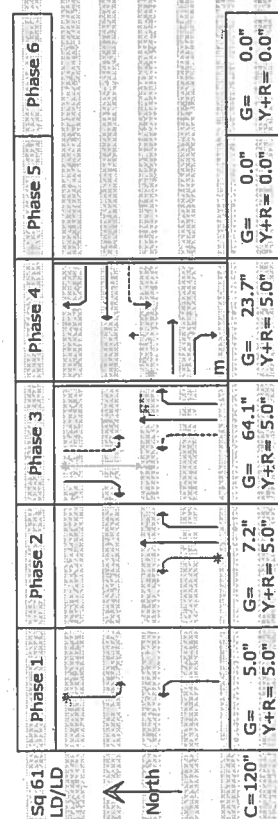
TH	12/1	0.080	0.635	996 ²⁴	996	54	0.054	8.3	A	33 ft
RT	36/3	0.562	0.635	3193	3193	2936	0.920	24.1	C+	1416 ft
LT	12/1	0.143	0.143	258	298	257	0.824	55.0	*D	318 ft

A/B Approach

[illegible]

FB Approach

RT+TH LT	12/1 12/1	0.188 0.198	0.198 0.198	233 193	289 241	217 192	0.698 0.727	51.5 54.8	D *D	324 ft 295 ft
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Analysis of
Intersection #5:
Maduri Ave. / Vidal Rd.

CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information		Site Information	
Analyst	Nancy	Jurisdiction/Date	City of ABQ 2/25/2006
Agency or Company	Terry Brown, P.E.	Major Street	Maduri Ave
Analysis Period/Year	AM Peak Hour 2009	Minor Street	Vidal Dr
Comment	2009 AM Peak BUILD Conditions		

Input Data

Lane Configuration	EB			WB			NB			SB		
Lane 1 (curb)	LTR			LTR			LTR			LTR		
Lane 2												
Lane 3												
Lane 4												
Lane 5												
	EB			WB			NB			SB		
Movement	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	1	23	1	70	17	31	1	1	30	101	1	1
PHF	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Percent of heavy vehicles, HV	3	3	3	3	3	3	3	3	3	3	3	3
Flow rate	1	27	1	82	20	36	1	1	35	119	1	1
Flare storage (# of vehs)												
Median storage (# of vehs)							1			1		
Signal upstream of Movement 2 _____ft												

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
NB	1	LTR	37	1010	0.037	0	8.7	A	8.7
	2								A
	3								
SB	1	LTR	121	657	0.184	1	11.7	B	11.7
	2								
	3								B
EB	①		1	1542	0.001	0	7.3	A	
WB	④		82	1579	0.052	0	7.4	A	

CHAPTER 17 - TWSC - UNSIGNALIZED INTERSECTIONS WORKSHEET

Analysis Summary

General Information

Analyst Nancy
 Agency or Company Terry Brown, P.E.
 Analysis Period/Year PM Peak Hour 2009
 Comment 2009 PM Peak BUILD Conditions

Site Information

Jurisdiction/Date City of ABQ 2/25/2006
 Major Street Maduri Ave
 Minor Street Vidal Dr

Input Data

Lane Configuration	EB			WB			NB			SB		
Lane 1 (curb)	LTR			LTR			LTR			LTR		
Lane 2												
Lane 3												
Lane 4												
Lane 5												
	EB			WB			NB			SB		
Movement	1 (LT)	2 (TH)	3 (RT)	4 (LT)	5 (TH)	6 (RT)	7 (LT)	8 (TH)	9 (RT)	10 (LT)	11 (TH)	12 (RT)
Volume (veh/h)	1	34	1	162	28	118	1	1	44	129	1	1
PHF	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Percent of heavy vehicles, HV	3	3	3	3	3	3	3	3	3	3	3	3
Flow rate	1	40	1	191	33	139	1	1	52	152	1	1
Flare storage (# of vehs)												
Median storage (# of vehs)							1			1		
Signal upstream of Movement 2												
Length of study period (h)	0.25											

Output Data

	Lane	Movement	Flow Rate (veh/h)	Capacity (veh/h)	v/c	Queue Length (veh)	Control Delay (s)	LOS	Approach Delay and LOS
NB	1	LTR	54	975	0.055	0	8.9	A	8.9
	2								
	3								A
SB	1	LTR	154	425	0.363	2	18.2	C	18.2
	2								
	3								C
EB	①		1	1399	0.001	0	7.6	A	
WB	④		191	1562	0.122	0	7.6	A	

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5 - 5_09PBX
 1 of 1

Data Entry Sheet
Determination of Warrants for Deceleration Lanes
NM DOT State Access Management Manual Criteria
Sevilla Ave / Coors Blvd

Project Information:

Project Name: **Esperia Development**
 Project Location: **Sevilla Ave / Coors Blvd**
 Implementation Year: **2009**
 Project Environment: **Urban Multi-Lane**

Street Information:

Major Street Name: **Coors Blvd**
 Minor Street Name: **Sevilla Ave**

Intersection Information:

	Orientation	Prevailing Speed	No. Lanes Each Direction
Sevilla Ave	Eastbound	25	N/A
Coors Blvd	North-South	45	3

Determine Case:

Case

- 1 Urban Two-Lane Highway - Use Table 17.B.1
- 2 Urban Multi-Lane Highway - Use Table 17.B-2
- 3 Rural Two Lane Highway - Use Table 17.B-3 and 17.B-5
- 4 Rural Multi-Lane Highway - Use Table 17.B-4 and 17.B-6

Coors Blvd is Case **2**
 Speed Category **45 to 55**

SB Right Turn Volumes

2009 AM Pk. Hr. NO BUILD	0
2009 AM Pk. Hr. BUILD	52
2009 PM Pk. Hr. NO BUILD	0
2009 PM Pk. Hr. BUILD	127

SB Thru Volumes

2474
2474
2495
2495

NB Left Turn Volumes

2009 AM Pk. Hr. NO BUILD	31
2009 AM Pk. Hr. BUILD	95
2009 PM Pk. Hr. NO BUILD	118
2009 PM Pk. Hr. BUILD	254

NB Thru Volumes

1751
1751
2907
2907

Determination of Warrants for Auxiliary Lanes

Project Name: **Esperia Development**
 Name of Highway: **Coors Blvd**
 Name of Cross Street: **Sevilla Ave**

Determination of Warrants for: **Eastbound Driveway**

Implementation Year Volumes - 2009 Posted Speed Limit: 45

Right Turn Deceleration Lane - Implementation Year Volumes

Condition	Year	Projected Right Turn Volume	Warrant Volume In thru Lane	Projected Volume In thru Lane	✓ if Met	Lane Length (Deceleration)*	Adjustment Factor for Grade**	Lane Length (Storage)***	Total Lane Length	Taper Ratio
AM Peak Hour NO BUILD	2009	-	-	825		N/A		-	N/A	N/A
AM Peak Hour BUILD	2009	52	1	825	✓	400	1.00	-	400	12.5:1
PM Peak Hour NO BUILD	2009	-	-	832		N/A		-	N/A	N/A
PM Peak Hour BUILD	2009	127	1	832	✓	400	1.00	-	400	12.5:1

Based on Table 17.B-2 (Criteria for Deceleration Lanes on Urban Multi-Lane Highways)

Left Turn Deceleration Lane - Implementation Year Volumes

Condition	Year	Projected Left Turn Volume	Warrant Volume In thru Lane	Projected Volume In thru Lane	✓ if Met	Lane Length (Deceleration)*	Adjustment Factor for Grade**	Lane Length (Storage)***	Total Lane Length	Taper Ratio
AM Peak Hour NO BUILD	2009	31	108	584	✓	400	1.00	50	450	12.5:1
AM Peak Hour BUILD	2009	95	1	584	✓	400	1.00	125	525	12.5:1
PM Peak Hour NO BUILD	2009	118	1	969	✓	400	1.00	150	550	12.5:1
PM Peak Hour BUILD	2009	254	1	969	✓	400	1.00	325	725	12.5:1

Based on Table 17.B-2 (Criteria for Deceleration Lanes on Urban Multi-Lane Highways)

* Lane Length Requirements based on Table 18.K-1 (Deceleration and Acceleration Lengths)

** Enter Grade Adjustment Factor from Table 18.K-2 or other criteria.

*** Lane Storage Length is Based on a calculated 3-minute queue based on average arrival rate per minute.

= Volume/Hr. divided by 60 times three (rounded) times 25 feet per vehicle.

Lane Storage Length for right turn decel lanes is zero unless there is a stop condition.

Notes and Comments:

1. This warrant sheet is for the eastbound Sevilla Ave at 100% Development of the Project

Table 17.B-2
Criteria For Deceleration Lanes On
URBAN MULTI-LANE HIGHWAYS

Turning Volume ¹ (vph)	LEFT-TURN DECELERATION LANE			RIGHT-TURN DECELERATION LANE		
	Minimum Directional Volume in the Through Lane (vphpl) ²			Minimum Directional Volume in the Through Lane (vphpl) ²		
	≤30 mph	35 to 40 mph	45 to 55 mph	≤30 mph	35 to 40 mph	45 to 55 mph
<5	Not Required	Not Required	Not Required	Not Required	Not Required	Not Required
5	Not Required	490	420	1,200	730	450
10	420	370	300	820	490	320
15	360	290	220	600	350	240
20	310	230	160	460	260	180
25	270	190	130	360	230	150
30	240	160	110	290	200	130
35	210	130	100	260	180	120
40	180	120	Required	240	170	110
45	160	110	Required	220	160	Required
50	140	Required	Required	200	Required	Required
55	120	Required	Required	190	Required	Required
≥56	Required	Required	Required	Required	Required	Required
	<i>Left-turn Deceleration Lanes are Required on Urban Multi-lane Highways for the following Left-turn Volumes:</i> • ≤30 mph : 56 vph or more • 35 to 40 mph : 46 vph or more • 45 to 55 mph : 36 vph or more			<i>Right-turn Deceleration Lanes are Required on Urban Multi-lane Highways for the following Right-turn Volumes:</i> • ≤30 mph : 56 vph or more • 35 to 40 mph : 46 vph or more • 45 to 55 mph : 41 vph or more		

Notes:

1. Use linear interpolation for turning volumes between 5 and 55 vph.
2. The volume in the adjacent through lane includes through vehicles and turning vehicles.

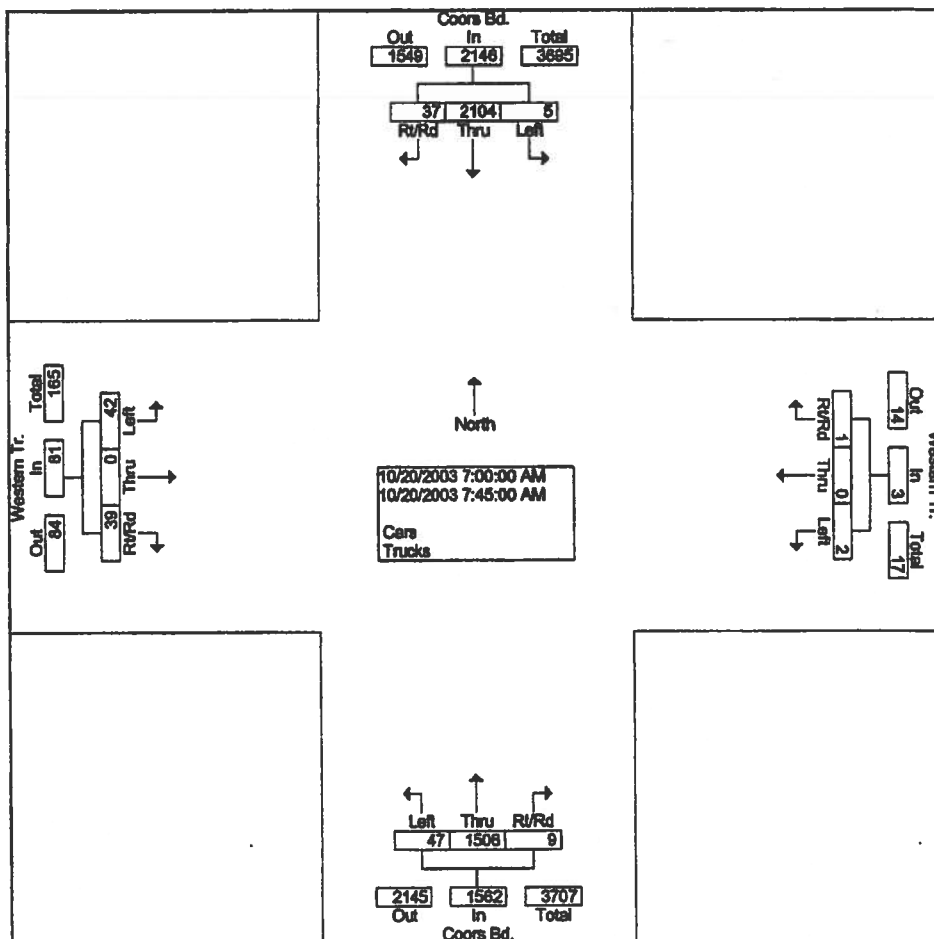
Table 18.K-1
Deceleration and Acceleration Lengths (feet)

Table 18.K-1 Deceleration and Acceleration Lengths (feet)											
Speed Change Lane Condition	Posted Speed (mph)										
	25	30	35	40	45	50	55	60	65	70	
Deceleration Distance											
Stop Condition	150	200	250	325	400	475	550	650	725	850	
Slow to 15 MPH	130	175	230	300	370	450	525	620	700	820	
Deceleration Taper											
Length for 12-foot Lane	50	75	100	125	150	175	200	225	250	250	
Straight Line Ratios (L:W)	4:1	6:1	8:1	10.5:1	12.5:1	14.5:1	16.5:1	18.5:1	21:1	21:1	
Acceleration Lane Length											
Acceleration Taper	N/A	190	270	380	550	760	960	1,170	1,380	1,590	
Length of 12-foot Lane	N/A	100	120	150	170	180	230	270	300	300	
Straight Line Ratios (L:W)	N/A	8:1	10:1	12.5:1	14:1	15:1	19:1	22.5:1	25:1	25:1	

Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Western Trail and Coors Bd.
Site Code : 00025585
Start Date : 10/20/2003
Page No : 3

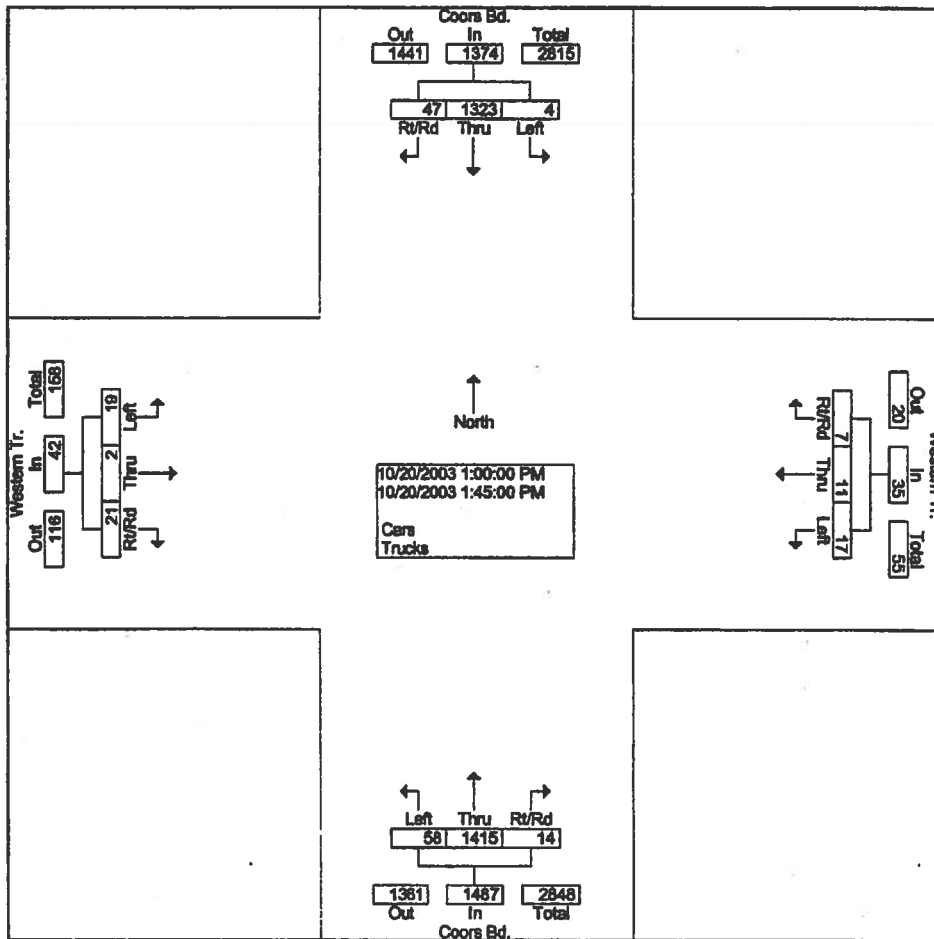
	Coors Bd. From North					Western Tr. From East					Coors Bd. From South					Western Tr. From West					
Start Time	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Int. Total
Peak Hour From 08:45 to 09:30 - Peak 1 of 1 Intersection 07:00																					
Volume	5	210	37	0	2148	2	0	0	1	3	47	150	9	0	1582	42	0	39	0	81	3782
Percent	0.2	98.0	1.7	0.0		86.7	0.0	0.0	33.3		3.0	98.4	0.8	0.0		51.9	0.0	48.1	0.0		
Volume	5	210	37	0	2148	2	0	0	1	3	47	150	9	0	1582	42	0	39	0	81	3782
Volume	1	573	14	0	588	1	0	0	0	1	9	429	0	0	438	10	0	11	0	21	1048
Peak Factor																					0.905
High Int.	07:15					07:15					07:45					07:15					
Volume	0	593	7	0	600	1	0	0	1	2	11	452	2	0	465	14	0	10	0	24	
Peak Factor					0.894					0.375					0.840					0.844	



Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Western Trall and Coors Bd.
Site Code : 00025585
Start Date : 10/20/2003
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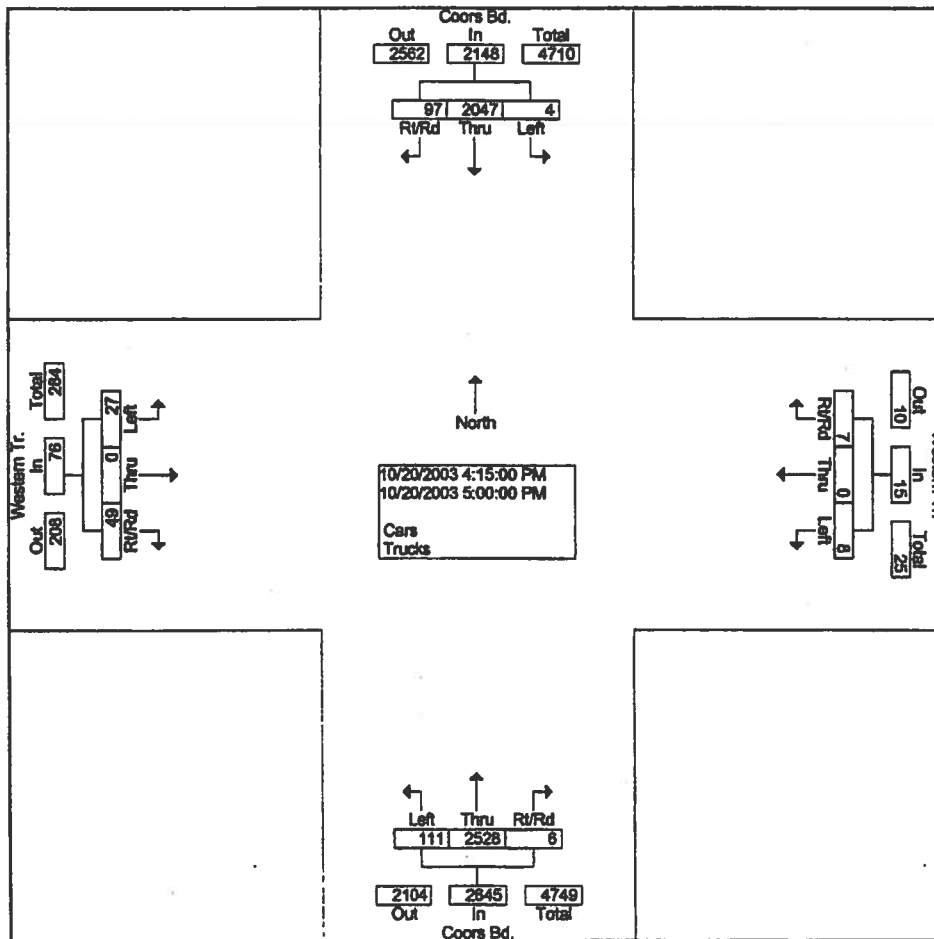
	Coors Bd. From North					Western Tr. From East					Coors Bd. From South					Western Tr. From West					
Start Time	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Int. Total
Peak Hour From 11:00 to 13:45 - Peak 1 of 1																					
Intersection 13:00																					
Volume	4	132	47	0	1374	17	11	0	7	35	58	141	14	0	1487	19	2	21	0	42	2938
Percent	0.3	96.3	3.4	0.0		48.8	31.4	0.0	20.0		3.9	95.2	0.9	0.0		45.2	4.8	50.0	0.0		
Volume	4	132	47	0	1374	17	11	0	7	35	58	141	14	0	1487	19	2	21	0	42	2938
Volume	2	348	10	0	380	1	9	0	1	11	12	357	4	0	373	7	1	7	0	15	759
Peak Factor																					0.968
High Int. Volumes	13:00					13:15					13:15					13:00					
Peak Factor					0.954					0.729					0.958						0.700



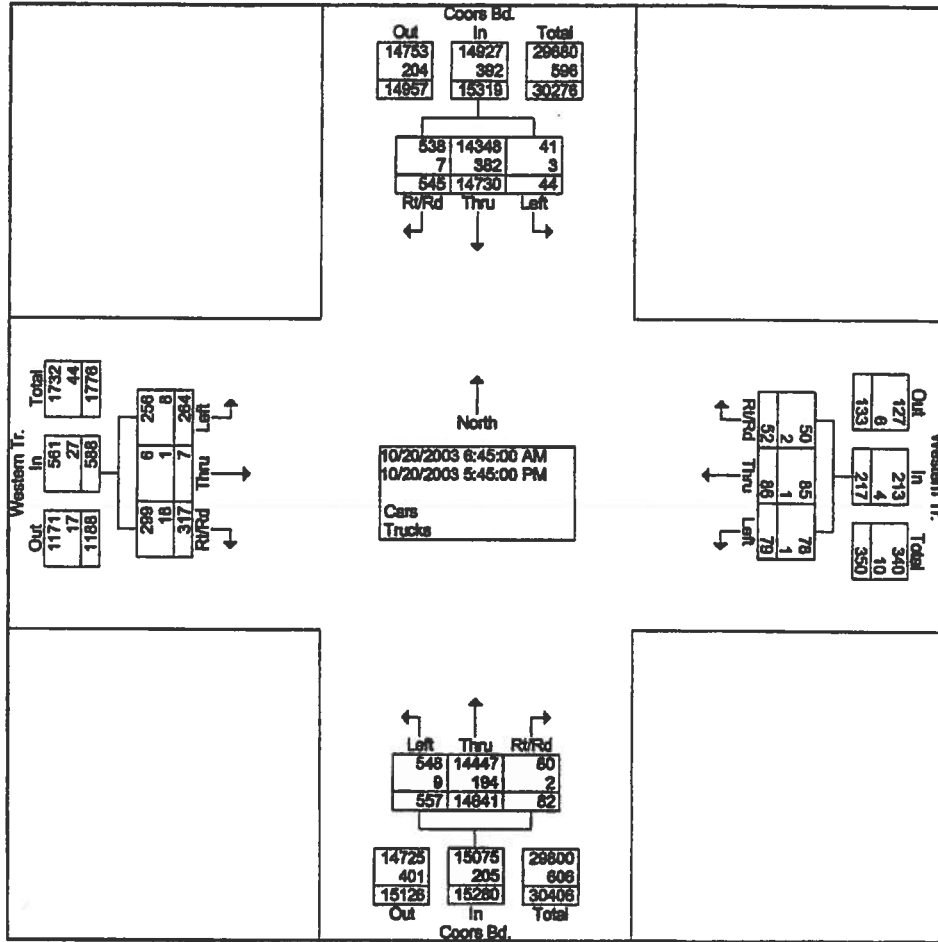
Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Western Trail and Coors Bd.
Site Code : 00025585
Start Date : 10/20/2003
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	Coors Bd. From North					Western Tr. From East					Coors Bd. From South					Western Tr. From West					
Start Time	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Int. Total
Peak Hour From 15:00 to 17:45 - Peak 1 of 1																					
Intersection 18:15																					
Volume	4	204	97	0	2148	8	0	0	7	15	111	252	8	0	2645	27	0	49	0	76	4884
Percent	0.2	95.3	4.5	0.0		53.3	0.0	0.0	48.7		4.2	95.8	0.2	0.0		35.5	0.0	64.5	0.0		
Volume	4	204	97	0	2148	8	0	0	7	15	111	252	8	0	2645	27	0	49	0	76	4884
Volume	2	504	37	0	543	2	0	0	1	3	28	727	0	0	753	9	0	10	0	19	1318
Peak Factor																					0.928
High Int.	17:00					16:30					17:00					18:45					
Volume	2	504	37	0	543	3	0	0	5	8	28	727	0	0	753	11	0	24	0	35	
Peak Factor					0.989					0.469				0.678						0.543	



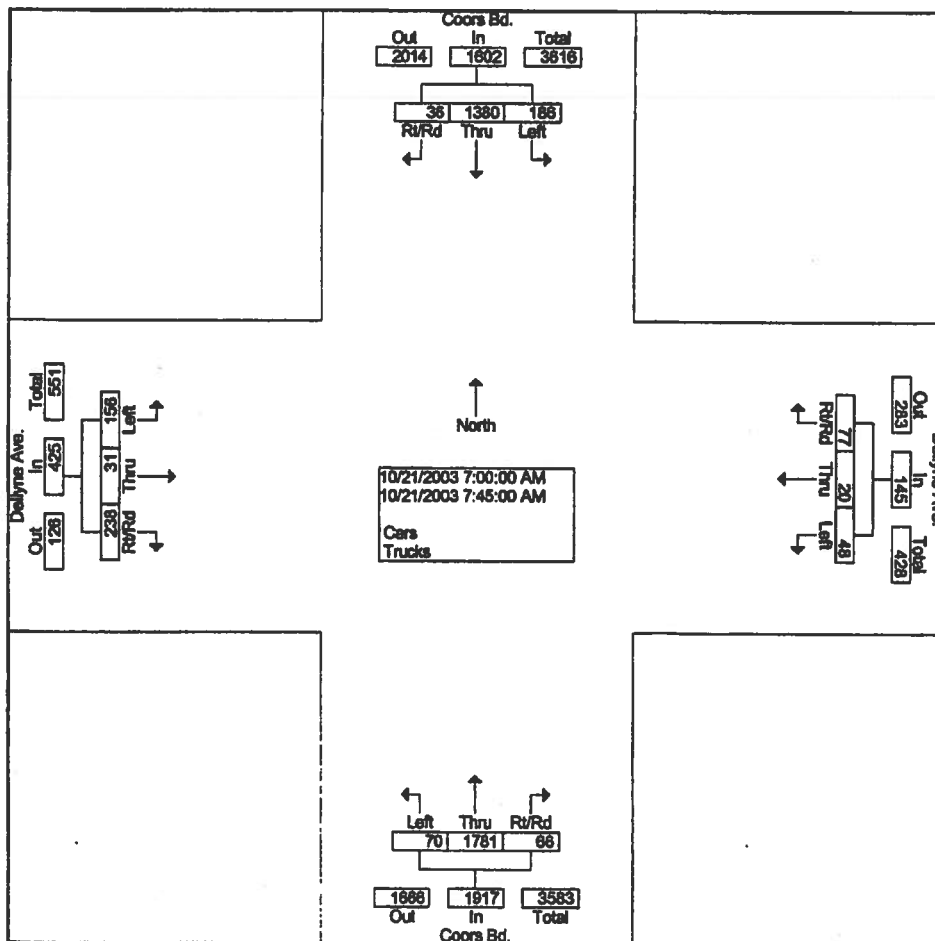
Total	2	189	115	0	2014	13	21	0	5	39	119	248	2	0	2587	22	0	42	0	64	4704
		7										8									
Grand Total	44	147	544	1	15319	79	86	2	50	217	557	148	81	1	15280	284	7	308	11	588	31404
		30										41									
Approch %	0.3	96.2	3.6	0.0		36.4	39.6	0.9	23.0		3.6	95.8	0.5	0.0		44.9	1.2	52.0	1.9		
Total %	0.1	48.9	1.7	0.0	48.8	0.3	0.3	0.0	0.2	0.7	1.8	46.6	0.3	0.0	48.7	0.8	0.0	1.0	0.0	1.9	



**Mid-Region Council of Governments
Intersection Turning Movement Analysis**

File Name : Dellyne Av. and Coors Bd.
Site Code : 00025143
Start Date : 10/21/2003
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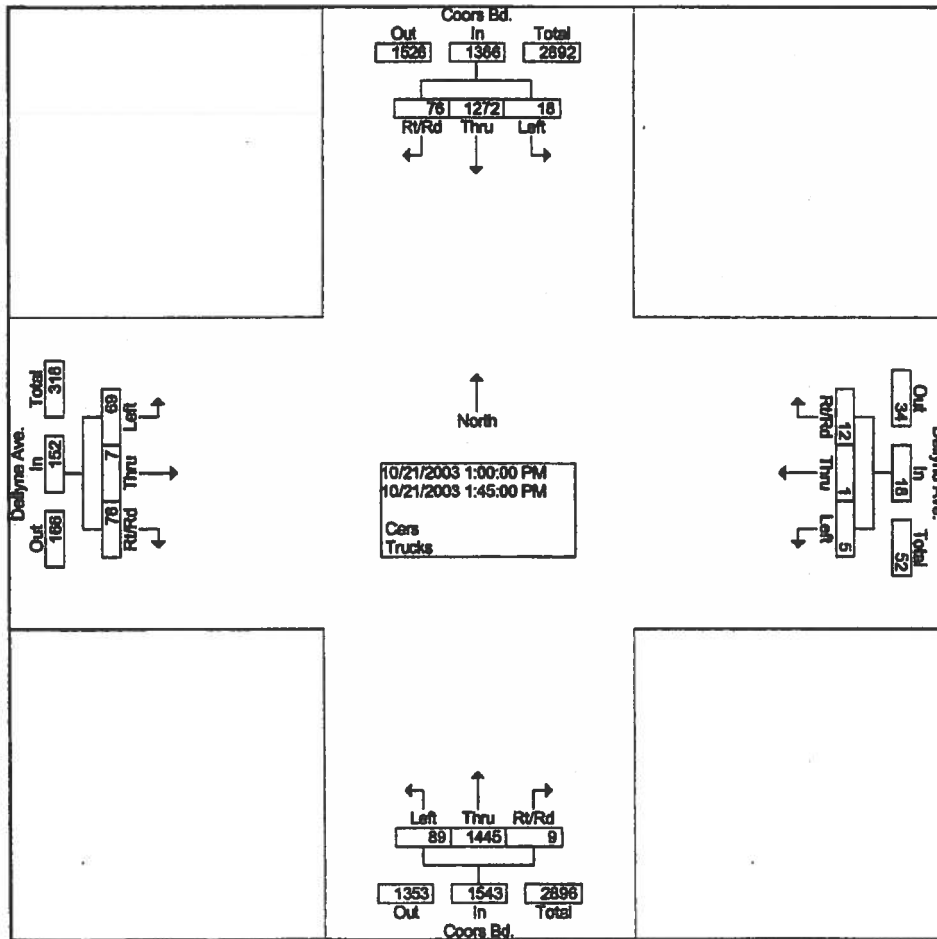
	Coors Bd. From North					Dellyne Ave. From East					Coors Bd. From South					Dellyne Ave. From West					
Start Time	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Int. Total
Peak Hour From 08:45 to 09:45 - Peak 1 of 1																					
Intersection 07:00																					
Volume	188	1380	35	1	1602	48	20	55	22	145	70	1781	54	12	1917	158	31	170	88	425	4089
Percent	11.8	88.1	2.2	0.1		33.1	13.8	37.9	15.2		3.7	92.9	2.8	0.6		38.7	7.3	40.0	18.0		
Volume	188	1380	35	1	1602	48	20	55	22	145	70	1781	54	12	1917	158	31	170	88	425	4089
Volume	91	309	2	0	402	33	17	40	10	100	19	528	28	3	574	20	20	50	12	102	1178
Peak Factor																					0.888
High Int. Volume	07:00					07:30					07:30					07:00					
Volume	16	414	7	0	437	33	17	40	10	100	19	528	28	3	574	55	3	49	30	137	
Peak Factor					0.918					0.383					0.835					0.778	



Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Dellyne Av. and Coors Bd.
Site Code : 00025143
Start Date : 10/21/2003
Page No : 4

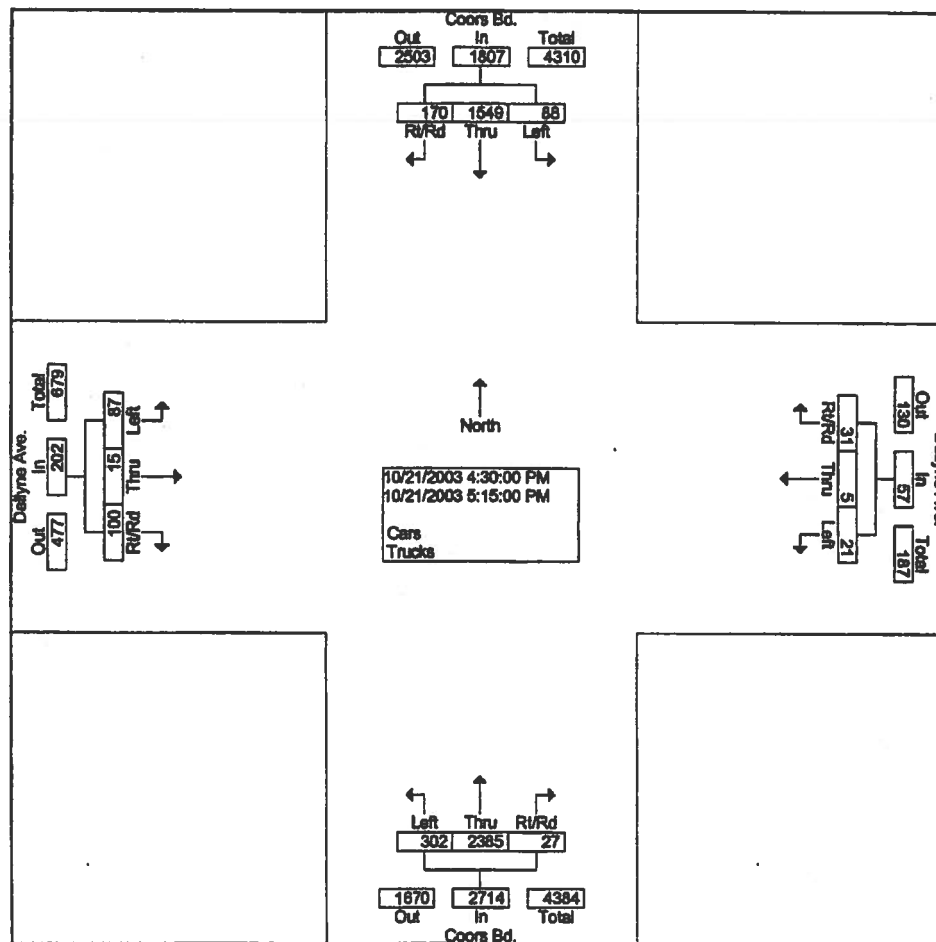
	Coors Bd. From North					Dellyne Ave. From East					Coors Bd. From South					Dellyne Ave. From West					
Start Time	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Int. Total
Peak Hour From 11:00 to 13:45 - Peak 1 of 1																					
Intersection 13:00																					
Volume	18	127	72	4	1366	5	1	4	8	18	89	144	4	5	1543	89	7	50	26	152	3079
Percent	1.3	93.1	5.3	0.3		27.8	5.8	22.2	44.4		5.8	93.8	0.3	0.3		45.4	4.8	32.9	17.1		
Volume	18	127	72	4	1366	5	1	4	8	18	89	144	4	5	1543	89	7	50	26	152	3079
Volume	5	305	15	2	327	2	0	1	0	3	25	430	1	0	458	16	1	14	1	32	818
Peak Factor																					0.941
High Int. Volume	13:00					13:30					13:45					13:30					
Peak Factor	1	335	21	1	358	1	0	1	8	8	25	430	1	0	458	23	1	16	5	45	
					0.954					0.563					0.848						0.844



**Mid-Region Council of Governments
Intersection Turning Movement Analysis**

File Name : Delyne Av. and Coors Bd.
Site Code : 00025143
Start Date : 10/21/2003
Page No : 5

	Coors Bd. From North					Delyne Ave. From East					Coors Bd. From South					Delyne Ave. From West					
Start Time	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Int. Total
Peak Hour From 15:00 to 17:45 - Peak 1 of 1																					
Intersection 16:30																					
Volume	88	154	170	0	1807	21	5	13	18	57	302	238	27	0	2714	87	15	70	30	202	4780
Percent	4.9	85.7	9.4	0.0		36.8	8.8	22.8	31.6		11.1	87.9	1.0	0.0		43.1	7.4	34.7	14.9		
Volume	88	154	170	0	1807	21	5	13	18	57	302	238	27	0	2714	87	15	70	30	202	4780
Volume	9	432	32	0	473	5	0	2	4	11	83	856	9	0	748	24	1	16	2	43	1275
Peak Factor																					0.937
High Int. 16:30						17:00					18:30					17:00					
Volume	9	432	32	0	473	10	3	4	9	26	83	856	9	0	748	18	5	25	10	58	
Peak Factor					0.955					0.548					0.907					0.871	

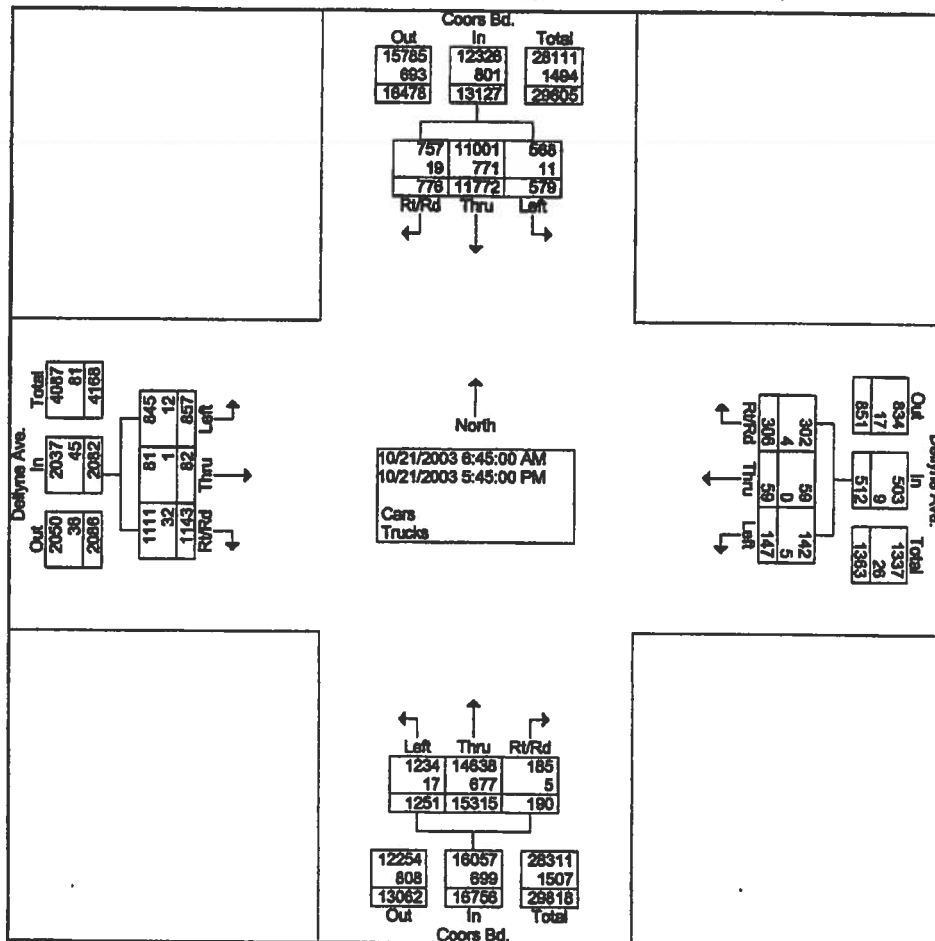


Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Dellyne Ave. and Coors Bd.
Site Code : 00025143
Start Date : 10/21/2003
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Groups Printed- Cars - Trucks

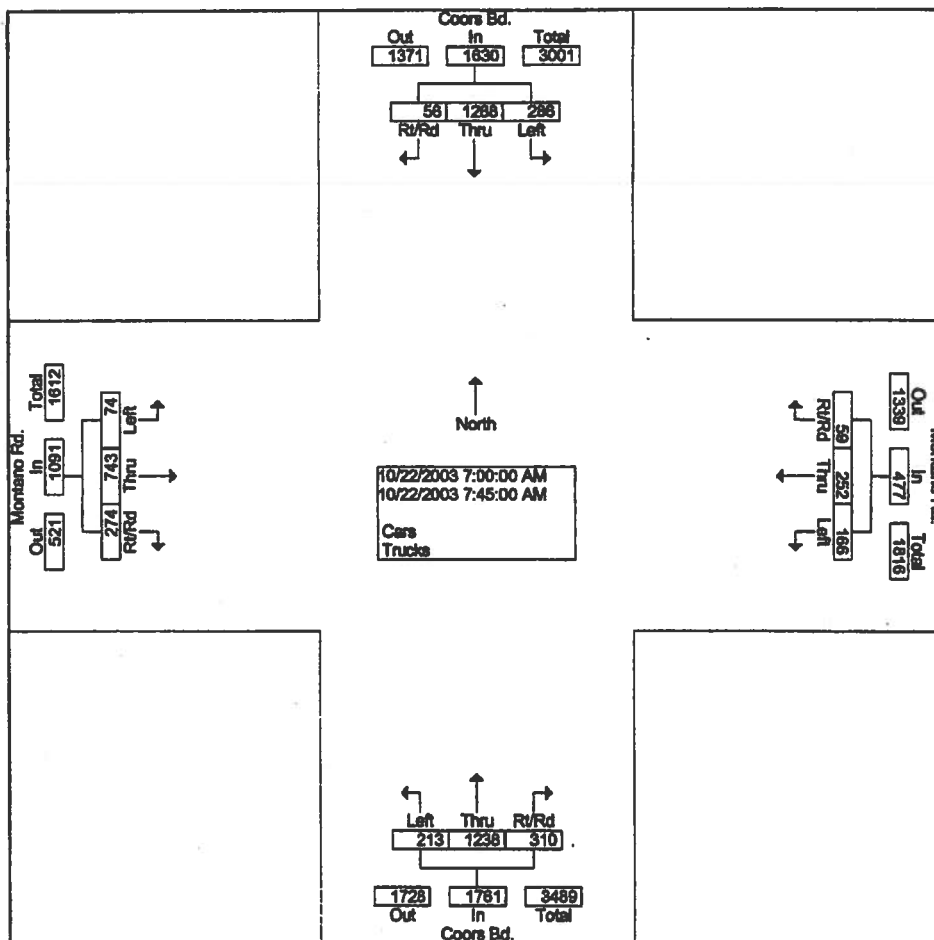
Start Time	Coors Bd. From North					Dellyne Ave. From East					Coors Bd. From South					Dellyne Ave. From West					Int. Total
	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	Left	Thru	Right	Rt/Rd	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
17:00	19	384	48	0	459	10	3	4	9	26	68	589	9	0	684	18	5	25	10	58	1207
17:15	27	375	57	0	459	5	1	5	2	13	73	542	5	0	620	23	8	18	5	54	1148
17:30	21	351	47	0	419	10	6	17	10	43	58	523	14	0	595	25	0	15	8	48	1105
17:45	41	358	35	0	434	2	1	4	4	11	52	487	1	1	521	13	0	10	18	41	1007
Total	108	1478	185	0	1771	27	11	30	25	93	249	2121	29	1	2400	79	13	68	41	201	4485
Grand Total	579	11772	754	22	13127	147	59	178	128	512	1251	15315	188	22	18758	857	82	883	480	2082	32477
Approch %	4.4	89.7	5.7	0.2		28.7	11.5	34.8	25.0		7.5	91.4	1.0	0.1		41.2	3.9	31.8	23.1		
Total %	1.8	36.2	2.3	0.1	40.4	0.5	0.2	0.5	0.4	1.6	3.9	47.2	0.5	0.1	51.6	2.8	0.3	2.0	1.5	6.4	



Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Montano Rd. and Coors Bd.
Site Code : 00025335
Start Date : 10/22/2003
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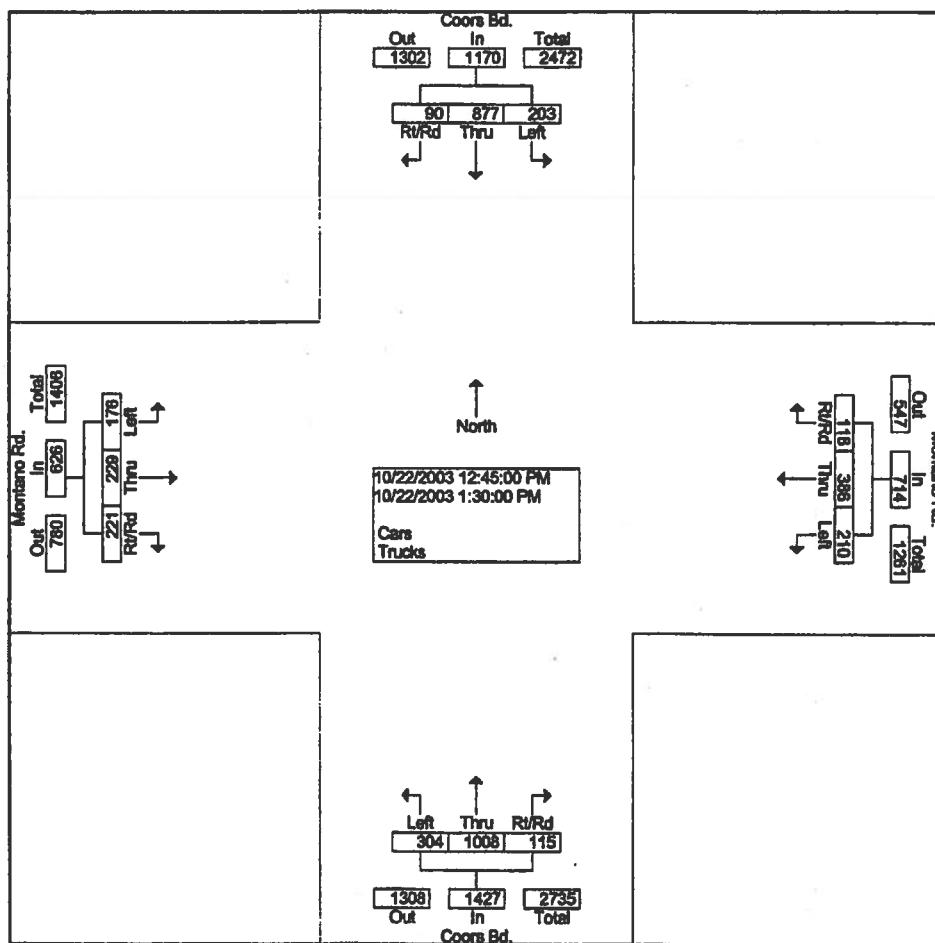
	Coors Bd. From North					Montano Rd. From East					Coors Bd. From South					Montano Rd. From West					Int. Total
Start Time	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	
Peak Hour From 08:45 to 09:30 - Peak 1 of 1																					
Intersection 07:00																					
Volume	286	128	30	28	1830	186	252	46	13	477	213	123	271	39	1781	74	743	230	44	1091	4959
Percent	17.5	79.0	1.8	1.6		34.8	52.8	9.6	2.7		12.1	70.3	15.4	2.2		6.8	68.1	21.1	4.0		
Volume	286	128	30	28	1830	186	252	46	13	477	213	123	271	39	1781	74	743	230	44	1091	4959
Volume	79	388	9	7	481	48	53	11	5	117	50	254	72	12	388	13	209	96	13	331	1317
Peak Factor																					0.941
High Int. Volume	07:15					07:30					07:30					07:00					
Volume	79	388	9	7	481	57	79	17	2	155	50	405	60	15	530	31	274	77	31	413	
Peak Factor					0.847					0.789					0.831					0.680	



Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Montano Rd. and Coors Bd.
Site Code : 00025335
Start Date : 10/22/2003
Page No : 4

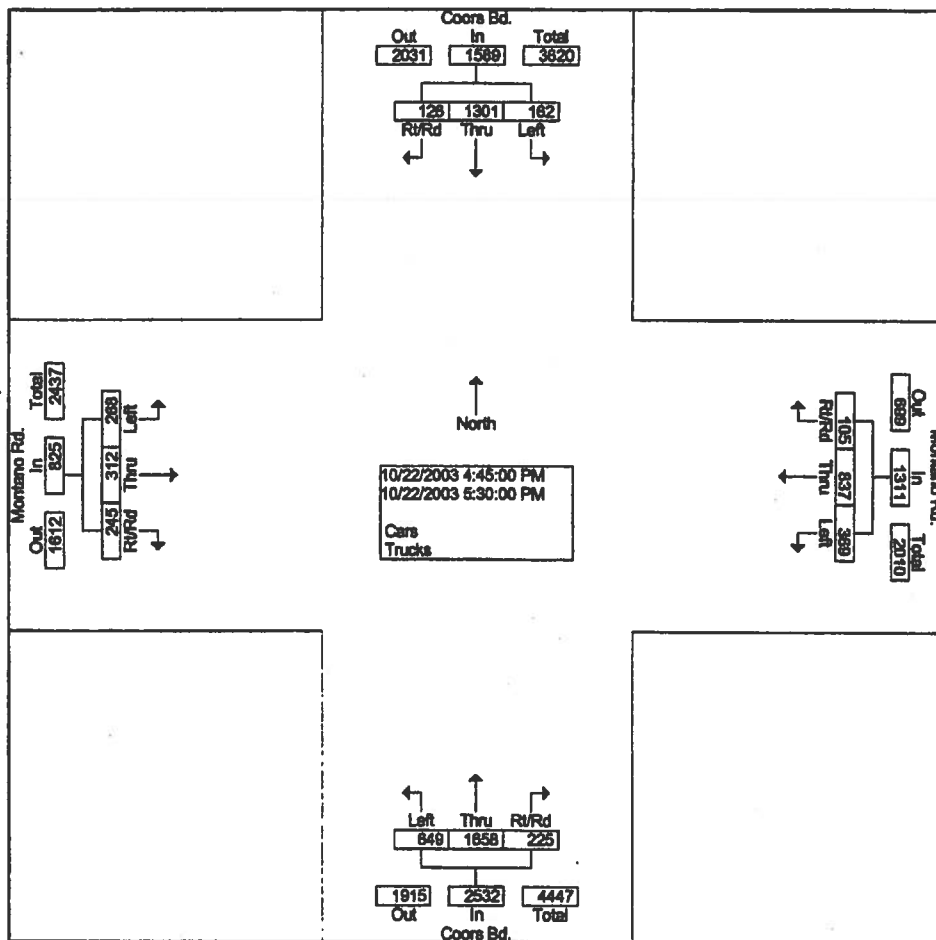
	Coors Bd. From North					Montano Rd. From East					Coors Bd. From South					Montano Rd. From West					
Start Time	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Int. Total
Peak Hour From 11:00 to 13:45 - Peak 1 of 1 Intersection 12:45																					
Volume	203	877	83	7	1170	210	386	110	8	714	304	1008	89	48	1427	176	229	122	99	826	3937
Percent	17.4	75.0	7.1	0.6		29.4	54.1	15.4	1.1		21.3	70.8	4.8	3.2		28.1	38.6	19.5	15.8		
Volume	203	877	83	7	1170	210	386	110	8	714	304	1008	89	48	1427	176	229	122	99	826	3937
Volume	58	209	11	1	279	58	129	23	2	212	82	249	23	20	374	49	59	35	28	171	1036
Peak Factor																					0.950
High Int.	13:00					13:30					13:30					13:00					
Volume	48	258	32	0	338	58	129	23	2	212	82	249	23	20	374	48	73	35	22	178	
Peak Factor					0.865					0.842					0.854					0.889	



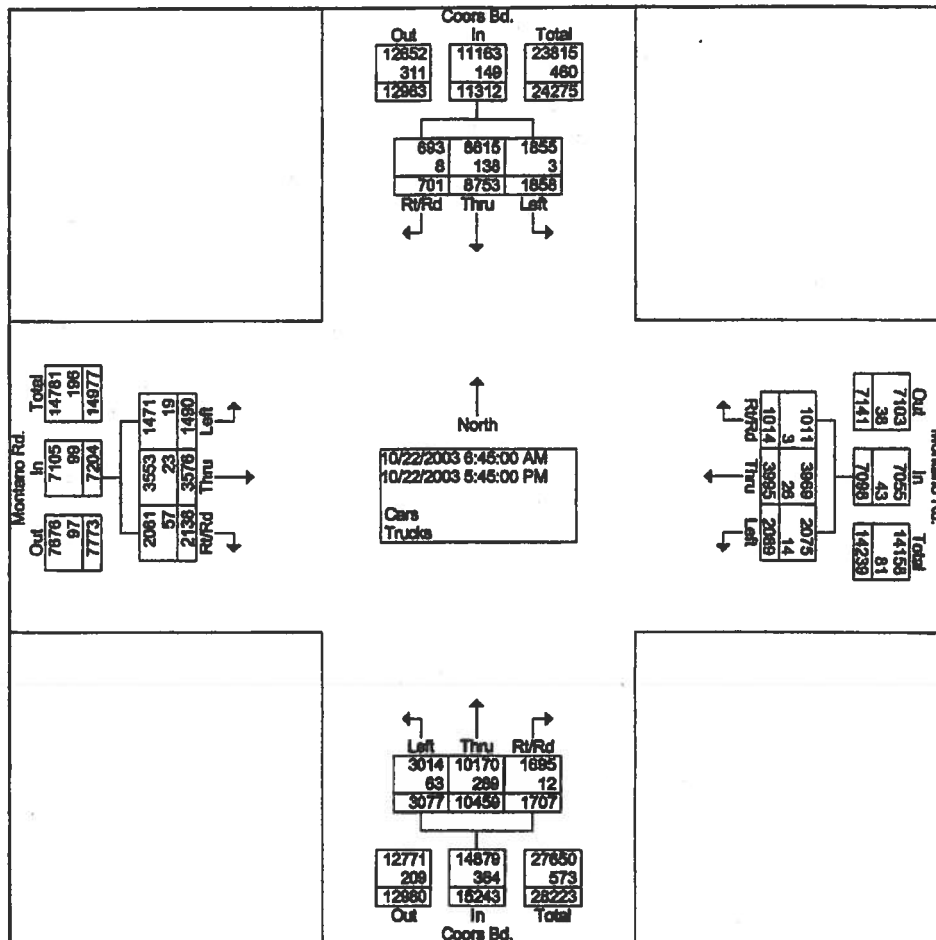
Mid-Region Council of Governments
Intersection Turning Movement Analysis

File Name : Montano Rd. and Coors Bd.
Site Code : 00025335
Start Date : 10/22/2003
Page No : 5

	Coors Bd. From North					Montano Rd. From East					Coors Bd. From South					Montano Rd. From West					
Start Time	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Left	Thru	Right	Rt/R d	App. Total	Int. Total
Peak Hour From 15:00 to 17:45 - Peak 1 of 1 Intersection 18:45																					
Volume	182	130 1	113	13	1589	369	837	90	15	1311	849	165 8	167	58	2532	288	312	203	42	825	6257
Percent	10.2	81.9	7.1	0.8		28.1	83.8	8.9	1.1		25.6	85.5	8.8	2.3		32.5	37.8	24.6	5.1		
Volume	182	130 1	113	13	1589	369	837	90	15	1311	849	165 8	167	58	2532	288	312	203	42	825	6257
Volume	32	329	32	4	397	104	207	23	1	335	160	482	48	13	701	74	88	55	15	212	1645
Peak Factor																					0.951
High Int.	18:45					17:30					17:15					17:15					
Volume	54	331	25	4	414	94	227	18	4	343	160	482	48	13	701	74	88	55	15	212	
Peak Factor					0.980					0.958					0.803					0.973	



Approch %	18.4	77.4	4.7	1.5		29.4	56.3	12.2	2.1		20.2	68.8	8.2	3.0		20.7	49.6	22.0	7.7	
Total %	4.5	21.4	1.3	0.4	27.7	5.1	9.8	2.1	0.4	17.4	7.5	25.6	3.1	1.1	37.3	3.6	8.8	3.9	1.4	17.6



Intersection Data SheetIntersection: **Western Trail / Coors Blvd**Posted Speed Limit (E-W Street): 35Date: 9/8/2005**Eastbound Approach: Western Trail**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
2	0	0	1	0

185' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y(only)	Y	N

Is there a right turn slip lane that by-passes the traffic signal?

No

Westbound Approach: Western Trail

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	0	0	1	0

300' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y(only)	Y	N

Is there a right turn slip lane that by-passes the traffic signal?

No

Posted Speed Limit (N-S Street): 45**Northbound Approach: Coors Blvd**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	stripes	3	0	1

175' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	N

Is there a right turn slip lane that by-passes the traffic signal?

No

Southbound Approach: Coors Blvd

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	0	3	0	1

115' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	N

Is there a right turn slip lane that by-passes the traffic signal?

No

Intersection Data SheetIntersection: **Dellyne Ave / Coors Blvd**Posted Speed Limit (E-W Street): 35Date: 9/8/2005**Eastbound Approach: Dellyne Ave**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	0	0	1	0

75' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
N	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Westbound Approach: Learning Dr.

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	0	1	0	1

125' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
N	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Posted Speed Limit (N-S Street): 45**Northbound Approach: Coors Blvd**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	0	3	0	1

275' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	N

Is there a right turn slip lane that by-passes the traffic signal?

No

Southbound Approach: Coors Blvd

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	0	3	0	1

275' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	N

Is there a right turn slip lane that by-passes the traffic signal?

No

Intersection Data SheetIntersection: **Montano Rd / Coors Blvd**Posted Speed Limit (E-W Street): 40Date: 9/82005**Eastbound Approach: Montano Rd**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
2	0	2	0	1

300' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y(only)	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Westbound Approach: Montano Rd

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
2	0	1	1	0

450' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y(only)	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Posted Speed Limit (N-S Street):

Northbound Approach: Coors Blvd

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
2	0	3	0	1

775' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y(only)	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Southbound Approach: Coors Blvd

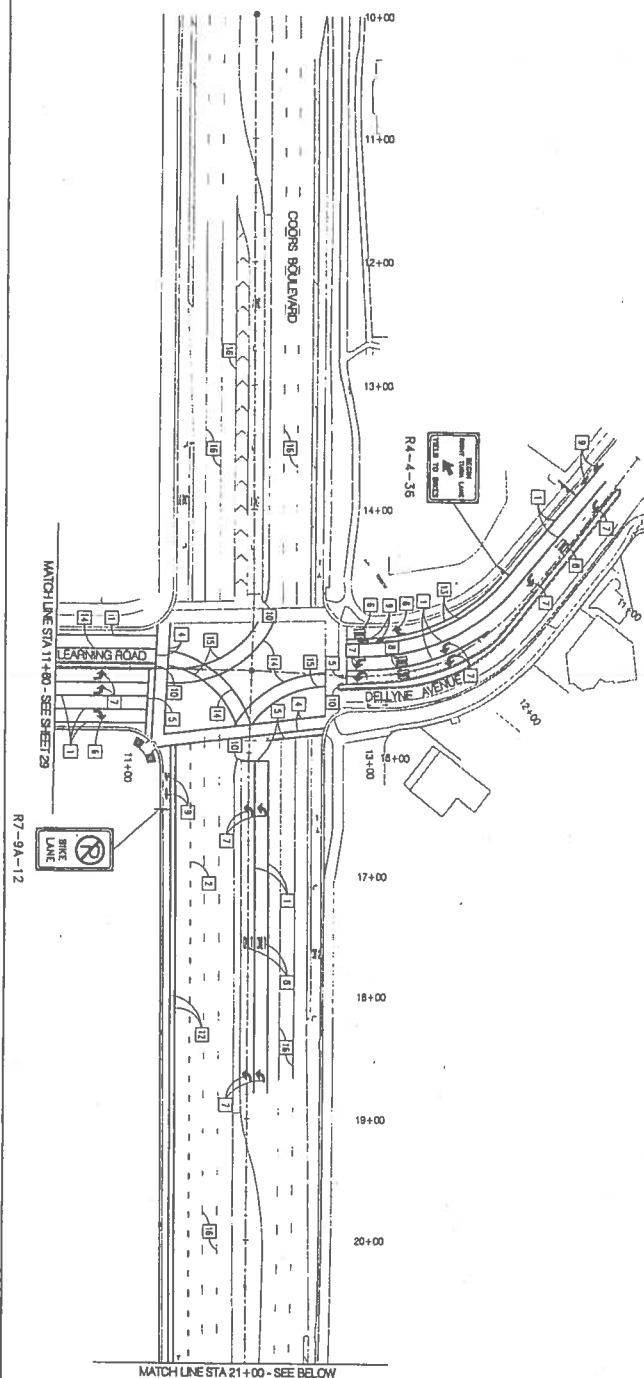
Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
2	0	3	0	1

450' lane

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y(only)	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

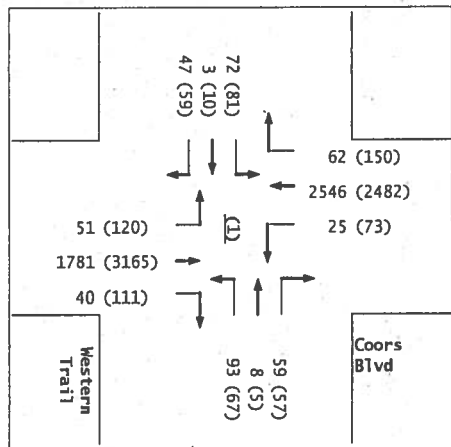
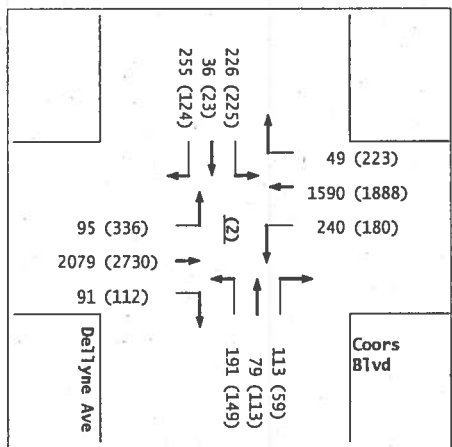
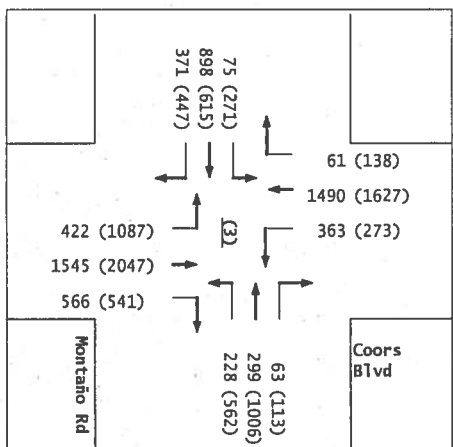
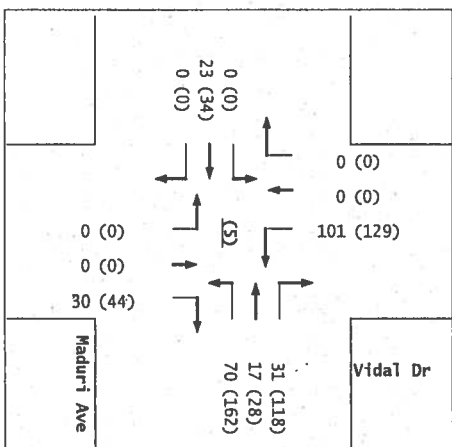
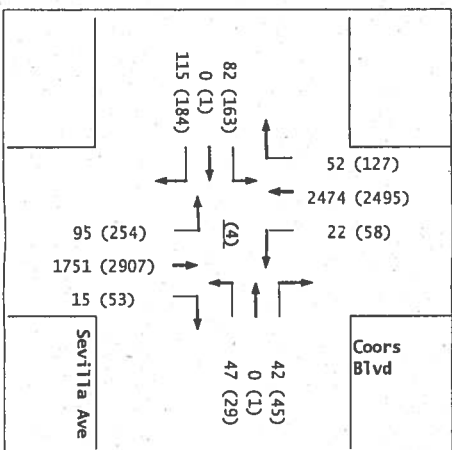
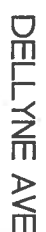


No.	Date	REMARKS	By
REVISIONS			
DESIGN			
Designed By: GSB		DATE: 10/05	
Drawn By: CW		DATE: 10/05	

5TH FLOOR TELEPHONE LIST

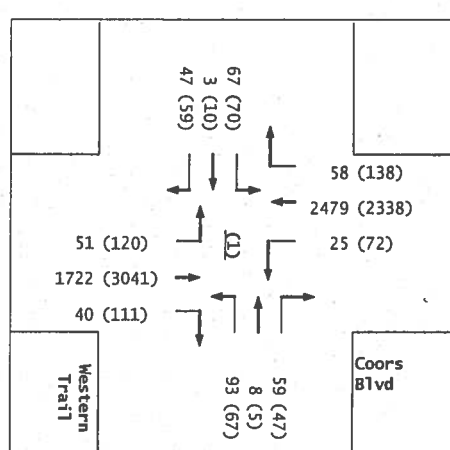
Allred, Stan	83695	Finance	Price, David	83670	Water Resources
Alvarez, Jose	83625	HR	Robinson, Roy	83621	Gen. Mgr. WUA
Baca, Crystal	83660	Water Resources	Roth, Frank	82511	Authority
Baca-Moya, Karen	83653	HR	Rudolph, Elaine	83677	HR
Bell, Eugene	78271	Safety	Sanchez, Mark	82504	Exec. Director
Ceniceros, Claude	83615	Conservation	Sivinski, Sharon	83245	Conservation
Chavez, Darlene	83627	Administration	Smith, Annette	83638	Finance
Chavez, Tim	83639	ITD	Stomp, John	83631	Water Resources
Crawford, Susan	83662	HR	Strong, Robert	82719	Utility Design
Cummings, Michael	78226	Safety	Tefera, Abebe	83681	Water Resources
Daves, Gary	83688	Water Rights	Vigil, Patrick	83669	HR
Davis, Kim	83628	Finance	Westcott, Anna	83603	Water Waste
De Gruyter, Kent	83682	ITD	Winslow, Mark	83684	UE&P
Dodd, Morgan	82721	ITD	Wintrobe, Cliff	83667	Finance
Dupree, Anita	82563	Contract Manager	Yuhas, Katherine	83633	Conservation
Edwards, Carol	83604	Water Waste			
Framel, Chris	82907	ITD			
Garcia, Arnold	83685	Human Resources			
Garcia, Darlyn	83691	HR	Employee Health	768-4630	
Harvey, Brian	83605	Water Waste	New Services	924-3952	
Hern, Phil	37088	Safety	Odor Hotline	873-7010	
Jenkins, Patty	82500	Authority	One Stop	924-3920	
Kolberg, Charles	83646	Attorney	Risk Management	768-3080	
Korak, Kathe	82670	ITD/Pager 251-1406	Roach Hotline	873-7009	
Landers, Susan	83637	Finance	Wasted Water	768-3640	
Larribas, Reba	83686	Human Resources	Water Quality	857-8260	
Lieuwen, Andrew	82570	Water Rights	Customer Service	768-2800	
Lopez, Rachel	83673	Finance	Conference Rooms	83648	Inside
Lucas, Nick	83697	HR	Front Fax	768-3629	
Maestas, Allen	83602	Water Waste	Back Fax	768-3630	
Martinez, Louis	83675	UE&P	HR Fax	768-3610	
Maxwell, Vicki	83650	Administration	Mark Sanchez Fax	768-2580	
Miles, Cindy	83634	Conservation			
Montoya, Anthony	82713	Utility Design			
Montoya, Judy	82561	Human Resources			
Montoya, Miriam	83663	Conservation			
Mora, Sal	83651	Conservation			
Morris, David	82506	Public Affairs Mgr.			
Musinski, Nancy	82729	Utility Design			
Ortiz, Tom	82508	Authority			
Pearson, Ted	83624	Attorney			
Pena, Floyd	82722	ITD			
Pera, Charlene	83694	Finance			

Last Updated
7/17/2009



Esperia Development
(Sevilla Ave / Coors Blvd)
2007 BUILD Volumes - AM(PM)

Terry O. Braun, P.E.
P.O. Box 92051
Albuquerque, NM 87199-2051
(505)883-8807 (Voice)

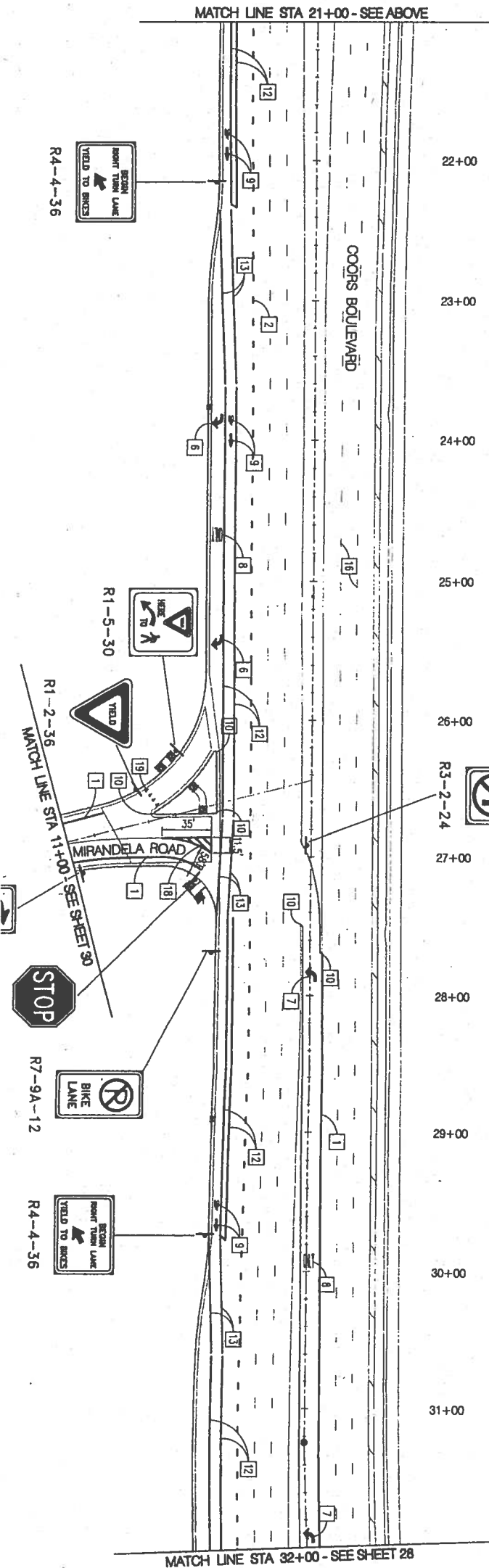
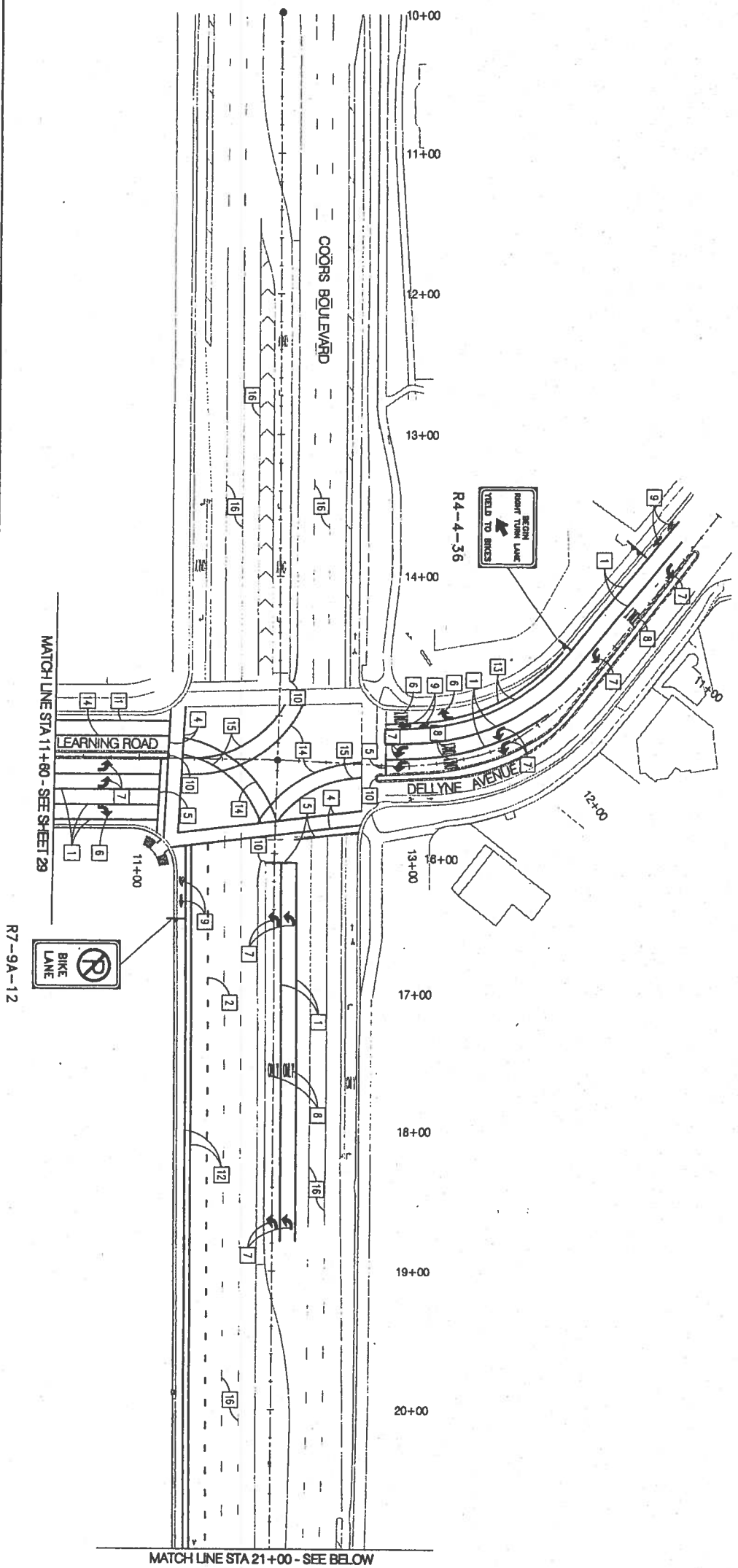


Terry O. Brown, P.E.
P.O. Box 92051
Albuquerque, NM 87199-2051
(505)883-8807 (Voice)



Trips Generated Volumes - AM(PM)

(503)883-8807 (Voice)



- KEYED NOTES**
- 1. INSTALL NEW 4" SOLID WHITE REFLECTORIZED PAINTED MARKING
 - 2. INSTALL NEW 4" DASHED WHITE REFLECTORIZED PAINTED MARKING (10' LINE, 30' SPACE)
 - 3. INSTALL NEW 4" SOLID YELLOW REFLECTORIZED PAINTED MARKING
 - 4. INSTALL NEW 12" SOLID WHITE REFLECTORIZED PLASTIC MARKING
 - 5. INSTALL NEW 24" SOLID WHITE REFLECTORIZED PLASTIC MARKING
 - 6. INSTALL NEW RETROREFLECTIVE PREFORMED PLASTIC PAVEMENT MARKING, RIGHT ARROW
 - 7. INSTALL NEW RETROREFLECTIVE PREFORMED PLASTIC PAVEMENT MARKING, LEFT ARROW
 - 8. INSTALL NEW RETROREFLECTIVE PREFORMED PLASTIC PAVEMENT MARKING, WORD, ONLY
 - 9. INSTALL NEW RETROREFLECTIVE PREFORMED PLASTIC PAVEMENT MARKING, ARROW AND SYMBOL, FOR BICYCLE LANES
 - 10. PAINT CURB FACE YELLOW (REFLECTORIZED PAVEMENT MARKING)
 - 11. INSTALL NEW 4" SOLID WHITE REFLECTORIZED PAINTED MARKING CHEROON (20' APART)
 - 12. INSTALL NEW 6" SOLID WHITE REFLECTORIZED PAINTED MARKING
 - 13. INSTALL NEW 6" WHITE REFLECTORIZED PAINTED MARKING DOTTED LINE (2' LINE, 6' SPACE)
 - 14. INSTALL NEW 4" WHITE REFLECTORIZED PAINTED MARKING DOTTED LINE (2' LINE, 4' SPACE)
 - 15. INSTALL NEW 4" YELLOW REFLECTORIZED PAINTED MARKING DOTTED LINE (2' LINE, 4' SPACE)
 - 16. EXISTING PAVEMENT MARKINGS TO REMAIN
 - 17. INSTALL NEW 4" DOUBLE YELLOW REFLECTORIZED PAINTED MARKING
 - 18. INSTALL NEW 6" SOLID YELLOW REFLECTORIZED PAINTED MARKING
 - 19. INSTALL YIELD PAVEMENT MARKING

- NOTES**
- 1. ALL SIGNING & STRIPING SHALL BE INSTALLED IN ACCORDANCE WITH MUTCD LATEST EDITION
 - 2. MEDIAN NOSES SHALL BE PAINTED YELLOW FROM THE PC TO THE PT.
 - 3. CONTRACTOR SHALL OBLITERATE ANY CONFLICTING STRIPING, EXCAVATION OF STRIPING TO BE ACCOMPLISHED WITH HIGH PRESSURE WASH WITH BLACKOUT IF NECESSARY.



Bohannon & Huston

Consulting Engineers
7600 Jefferson St. NE Albuquerque, NM 87109-4335
ENGINEERING & SPATIAL DATA & ADVANCED TECHNOLOGIES

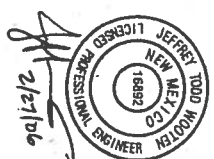


COORS BOULEVARD STRIPING & SIGNING PLAN
NORTH ANDALUCCA AT LA LUZ
STA. 10+00 TO STA. 32+00

Design Review Committee City Engineer Approval

Update

1A-117

ENGINEER'S SEAL			SURVEY INFORMATION			BENCH MARKS		AS-BUILT INFORMATION	
 2/27/06			FIELD NOTES					CONTRACTOR	DATE
			NO.	BY	DATE			WORK	DATE
								INSPECTOR'S	DATE
								ACCEPTANCE BY	DATE
								FIELD	DATE
								VERIFICATION BY	DATE
								DRAWINGS	DATE
								CONDUCTED BY	DATE
								MICRO-FILM INFORMATION	
								RECORDED BY	DATE
								NO.	

REMARKS		By	
DESIGN			
REVISIONS			
No.	Date		
Designed By: GSB		DATE: 10/05	
Drawn By: CW		DATE: 10/05	
Checked By: BJS		DATE: 10/05	