CITY OF ALBUQUERQUE



September 1, 2016

J. Graeme Means, PE High Mesa Consulting Group 6010 – B Midway Park Blvd NE Albuquerque, NM 87109

Re: MacArthur ES Courtyard Drain 1100 Douglas MacArther Rd NW Request Permanent C.O. - Accepted Engineer's Stamp dated: 5/18/2016 (F14D038) Certification dated: 8-29-16

Dear Mr. Means,

PO Box 1293 Based on the Certification received 8/31/2016, the site is acceptable for release of Certificate of Occupancy by Hydrology.

If you have any questions, you can contact me at 924-3982 or Totten Elliott at 924-3982.

Albuquerque

M

Abiel Carrillo, P.E. Principal Engineer, Planning Department Development and Review Services

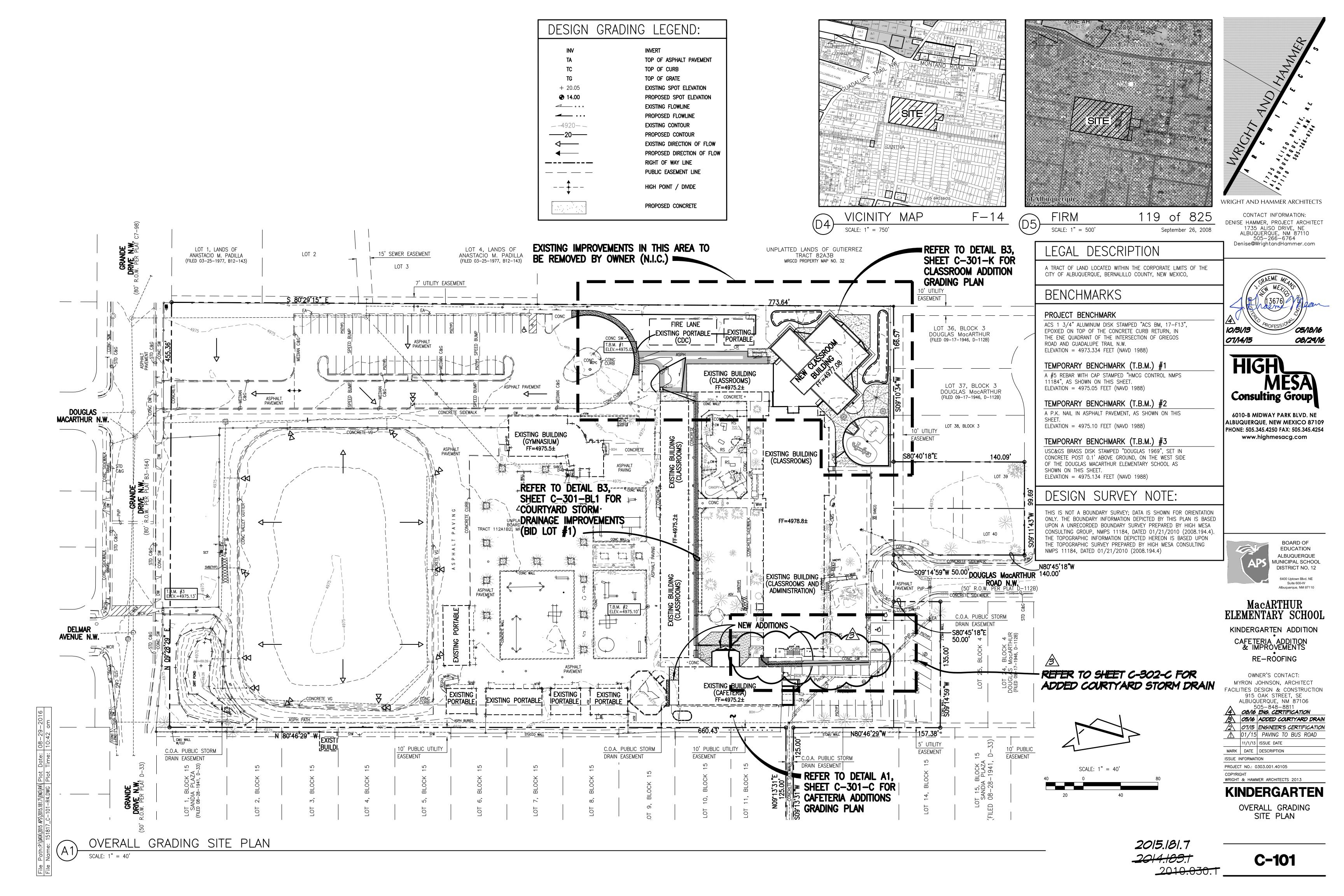
www.cabq.gov

New Mexico 8710

TE/AC C: email,

Sincerely,

Cordova, Camille C.; Miranda, Rachel; Sandoval, Darlene M.; Lois Blocker



DRAINAGE PLAN

I. INTRODUCTION AND EXECUTIVE SUMMARY

THIS PROJECT, LOCATED IN THE NORTH VALLEY OF THE ALBUQUERQUE METROPOLITAN AREA, REPRESENTS A MODIFICATION TO AN EXISTING ELEMENTARY SCHOOL SITE WITHIN AN INFILL AREA. THE PURPOSE OF THIS PROJECT IS TO PROVIDE IMPROVED DRAINAGE TO THE EXISTING COURTYARD, CONSTRUCT NEW ADDITIONS TO THE EXISTING CAFETERIA BUILDING, AND CONSTRUCT A NEW KINDERGARTEN CLASSROOM BUILDING ADDITION. THE DRAINAGE CONCEPT WILL BE TO MAINTAIN THE EXISTING DRAINAGE PATTERNS OF THE SITE AND RETAIN ANY INCREASE IN DEVELOPED RUNOFF GENERATED ONSITE.

THIS SUBMITTAL IS MADE IN SUPPORT OF BUILDING PERMIT WITHIN THE JURISDICTION OF THE CITY OF ALBUQUERQUE.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP. THE SCHOOL SITE IS LOCATED NEAR THE INTERSECTION OF DOUGLAS MACARTHUR NW AND GRANDE DRIVE NW. THE PROPERTY IS UNPLATTED. AS SHOWN BY PANEL 119 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, SEPTEMBER 26, 2008, THIS SITE LIES WITHIN A DESIGNATED ZONE X FLOOD HAZARD ZONE, AN AREA OF 1% ANNUAL CHANCE FLOOD WITH AVERAGE DEPTHS LESS THAN 1 FOOT.

III. BACKGROUND DOCUMENTS

THE PREPARATION OF THIS PLAN RELIED UPON THE FOLLOWING DOCUMENTS:

- TOPOGRAPHIC SURVEY PREPARED BY HIGH MESA CONSULTING GROUP (NMPS 11184) DATED 01-21-2010. THIS REFERENCED SURVEY PROVIDES THE BASIS FOR THE EXISTING CONDITIONS OF THE PROJECT SITE.
- GRADING AND DRAINAGE PLAN FOR DOUGLAS MACARTHUR SCHOOL PREPARED BY WILSON & COMPANY, DATED 06-17-1994. THIS PLAN SUPPORTED CONSTRUCTION OF THE MINI-GYMNASIUM AND PAVED BUS DROP OFF LOOP (NOW EXISTING) AND IDENTIFIED THE DRAINAGE BASINS 101, 102, AND 103 FOR THE SITE. THE PROPOSED IMPROVEMENTS LIE WITHIN BASIN 101 AND 102 OF THE 1994 PLAN.

IV. EXISTING CONDITIONS

THE PROPOSED PROJECT SITE INCLUDES THREE AREAS OF CONSTRUCTION, ALL WITHIN THE EASTERN PORTION OF THE SITE. THESE AREAS ARE 1) THE CAFETERIA BUILDING, 2) THE EXISTING COURTYARD, AND 3) THE EXISTING PLAYGROUND AND CLASSROOM PORTABLES AREA AT THE NORTHEAST CORNER OF THE SITE. THE ENTIRE SITE IS GENERALLY FLAT WITH MINIMAL TOPOGRAPHY. EXHIBITING POORLY DEFINED DRAINAGE PATTERNS.

RUNOFF FROM THE EXISTING CAFETERIA BUILDING LACKS WELL DEFINED DRAINAGE AS THE SITE IS GENERALLY FLAT. OVERFLOW APPEARS TO SHEET FLOW IN A WESTERLY DIRECTION TO ULTIMATELY DRAIN TO AN EXISTING DETENTION POND AT THE SOUTHWEST CORNER OF THE SCHOOL SITE THAT DISCHARGES TO THE EXISTING PUBLIC STORM DRAIN WITHIN GRANDE DRIVE NW.

THE EXISTING COURTYARD GRADUALLY SLOPES FROM EAST TO WEST, SHEET FLOWING RUNOFF TOWARD THE EXISTING BUILDING AND CAUSING FLOODING OF THE EXISTING BUILDING ENTRANCES. AN EXISTING STORM INLET LIES AT THE NORTHWEST CORNER OF THE MAIN CLASSROOM BUILDING; THIS INLET COLLECTS OVERFLOW RUNOFF FROM THE COURTYARD, AS WELL AS SURFACE FLOWS FROM THE NORTHEAST PORTABLE CLASSROOMS. RUNOFF THAT DRAINS TO THIS INLET IS CONVEYED VIA PRIVATE STORM DRAIN TO ULTIMATELY DISCHARGE TO A PUBLIC STORM DRAIN IN GRANDE DRIVE NW.

RUNOFF GENERATED WITHIN THE NORTHEAST PLAYGROUND APPEARS TO POND IN A LOW POINT IN THIS AREA, WITH OVERFLOW SHEETFLOWING TO THE SOUTHEAST TO AN ONSITE LOW AREA IMMEDIATELY NORTH OF DOUGLAS MACARTHUR ROAD NW; THIS AREA ULTIMATELY OVERFLOWS TO THE PUBLIC STROM DRAIN WITHIN DOUGLAS MACARTHUR ROAD NW.

THERE ARE NO OFFSITE FLOWS IMPACTING THE PROJECT; THE AREAS SURROUNDING THE SCHOOL EXHIBIT PARALLEL TOPOGRAPHY TO THE SCHOOL SITE AND THEREFORE DO NOT CONTRIBUTE OFFSITE FLOWS.

V. DEVELOPED CONDITIONS

THE PROPOSED CONSTRUCTION INCLUDES THREE AREAS: 1) BUILDING ADDITIONS TO THE CAFETERIA BUILDING, 2) DRAINAGE IMPROVEMENTS TO THE COURTYARD, AND 3) A NEW KINDERGARTEN CLASSROOM BUILDING ADDITION AND GRAVEL FIRE LANE AT THE NORTHEAST CORNER OF THE SITE.

THE SMALL ADDITIONS TO THE EXISTING CAFETERIA BUILDING WILL REPLACE IMPERVIOUS PAVEMENT WITH IMPERVIOUS BUILDING. NO CALCULATIONS WERE PREPARED FOR THIS AREA AS THERE IS NO INCREASE IN RUNOFF GENERATED BY THIS DEVELOPMENT. RUNOFF WILL CONTINUE TO DRAIN AWAY FROM THE BUILDING ON PAVED SURFACES.

THE DRAINAGE IMPROVEMENTS TO THE EXISTING COURTYARD CONSIST OF THE REMOVAL AND REPLACEMENT OF A PORTION OF THE EXISTING SIDEWALK WITH THE INCLUSION OF A NEW TRENCH DRAIN. THE NEW TRENCH DRAIN WILL INTERCEPT AND COLLECT THE EXISTING RUNOFF THAT CURRENTLY FLOODS THE EXISTING BUILDING ENTRANCES. RUNOFF COLLECTED WITHIN THE TRENCH DRAIN WILL BE CONVEYED VIA PRIVATE STORM DRAIN EXTENSION TO THE EXISTING STORM INLET AT THE NORTHWEST CORNER OF THE EXISTING BUILDING, AND ULTIMATELY DISCHARGED TO THE PUBLIC STORM DRAIN IN GRANDE DRIVE NW. NO CALCULATIONS WERE PREPARED FOR THIS AREA AS THERE IS NO INCREASE IN RUNOFF GENERATED BY THIS DEVELOPMENT.

A NEW KINDERGARTEN CLASSROOM BUILDING ADDITION AND GRAVEL FIRE LANE WILL REPLACE THE EXISTING PORTABLE CLASSROOMS AND PLAYGROUND LOCATED AT THE NORTHEAST CORNER OF THE SCHOOL SITE. THESE IMPROVEMENTS WILL RESULT IN A MINIMAL INCREASE IN DEVELOPED RUNOFF GENERATED BY THE SITE. TWO SHALLOW (6□ +/-) WATER HARVESTING AREAS TO THE NORTHWEST AND EAST OF THE NEW ADDITION ARE SIZED TO RETAIN THE INCREASE IN RUNOFF GENERATED BY THE SITE. IN ADDITION, A NEW CURB AND GUTTER IS PROPOSED ALONG THE EASTERN EDGE OF THE SCHOOL SITE TO PROVIDE POSITIVE DRAINAGE OF OVERFLOW RUNOFF. THE CURB AND GUTTER WILL CONVEY OVERFLOW RUNOFF SOUTH TO THE EXISTING LOW AREA NORTH OF DOUGLAS MACARTHUR ROAD NW, MAINTAINING THE EXISTING DRAINAGE PATTERNS ALREADY ESTABLISHED FOR THIS AREA OF THE SITE.

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING AND PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0□ INTERVALS, 2.) THE LIMIT AND CHARACTER OF THE EXISTING AND PROPOSED IMPROVEMENTS. AND 3.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN. THE PROPOSED GRADING AND DRAINAGE IMPROVEMENTS WILL MAINTAIN AND IMPROVE THE CURRENT DRAINAGE PATTERNS FOR THE SITE, RETAINING ONSITE ANY INCREASE IN DEVELOPED RUNOFF GENERATED BY THE IMPROVEMENTS.

VII. CALCULATIONS

CALCULATIONS ANALYZING THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT HAVE BEEN PREPARED FOR THE PORTION OF THE SITE AFFECTED BY THE NEW KINDERGARTEN CLASSROOM ADDITION BUILDING AND FIRE LANE IMPROVEMENTS. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. IN ADDITION, THE AVERAGE END-AREA METHOD HAS BEEN USED TO QUANTIFY THE VOLUME OF RUNOFF RETAINED WITHIN THE PROPOSED WATER HARVESTING AREAS. AS DEMONSTRATED BY THESE CALCULATIONS, THE PROPOSED IMPROVEMENTS WILL RESULT IN A MINIMAL INCREASE IN PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED BY THIS PROJECT, WHICH WILL BE MITIGATED BY THE RETENTION OF THE INCREASE WITHIN THE NEW WATER HARVESTING AREAS.

VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVALUATIONS CONTAINED HEREIN:

- 1. THE PROPOSED IMPROVEMENTS WILL MAINTAIN OR IMPROVE THE EXISTING DRAINAGE PATTERNS OF THIS PORTION OF THE EXISTING ELEMENTARY SCHOOL SITE. 2. THE PROPOSED IMPROVEMENTS TO THE CAFETERIA AND COURTYARD WILL REPLACE EXISTING IMPERVIOUS LAND TREATMENT WITH NEW IMPERVIOUS LAND TREATMENT.
- MAINTAINING THE EXISTING VOLUME AND PEAK RATE OF DISCHARGE GENERATED BY THESE AREAS. 3. THE PROPOSED IMPROVEMENTS TO THE KINDERGARTEN CLASSROOM ADDITION WILL RESULT IN A MINIMAL INCREASE IN VOLUME AND PEAK RATE OF DISCHARGE
- GENERATED BY THIS AREA. 4. THE PROPOSED WATER HARVESTING AREAS ARE SIZED TO RETAIN THE INCREASE IN VOLUME OF RUNOFF GENERATED BY THE KINDERGARTEN CLASSROOM ADDITION. 5. THE PROPOSED IMPROVEMENTS WILL NOT ADVERSELY IMPACT DOWNSTREAM PROPERTIES OR DOWNSTREAM DRAINAGE CONDITIONS.

3 MACARTHUR ES 2016 COURTYARD DRAIN

AS A FOLLOW-UP TO THE RECENTLY COMPLETED KINDERGARTEN AND CAFETERIA ADDITION PROJECTS, A NEW STORM DRAIN SERVING THE COURTYARD AREA IS PROPOSED TO PROTECT THE MECHANICAL ROOM AND CAFETERIA BUILDING FROM FLOODING ATTRIBUTABLE TO PRE-EXISTING ROOF DRAINAGE FROM THE MAIN CLASSROOM BUILDING. THE DOOR TO THE MECHANICAL ROOM IS SET BELOW THE SURROUNDING AREA, AND ROOF DRAINAGE THAT DISCHARGES TO GRADE IMMEDIATELY NEXT TO THE DOOR RE-ENTERS THE BUILDING. ALSO, LARGE RAINFALL EVENTS IN THE SUMMER OF 2015 OVERWHELMED THE NEW TRENCH DRAIN RESULTING IN WATER ENTERING THE NEW CAFETERIA ADDITION.

A NEW STORM INLET IS PROPOSED WITH A GRATE ELEVATION BELOW THE MECHANICAL ROOM DOOR. A NEW 12" STORM DRAIN WILL BE CONSTRUCTED UNDER THE COVERED WALKWAY THAT LEADS TO THE EAST PARKING AREA, WITH A CONNECTION TO AN EXISTING ON-SITE STORM DRAIN MANHOLE. THIS PLAN IS SUBMITTED AS A REVISION TO THE PREVIOUSLY APPROVED AND CERTIFIED PLAN.

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CALCULATIONS

I. <u>SITE CHARACTERISTICS</u> A. PRECIPITATION ZONE = 2
B. $P_{6,100} = P_{360} =$ 2.35
C. TOTAL PROJECT AREA $(A_T) = 27,280$ SF 0.63 AC
D. LAND TREATMENTS
1. EXISTING LAND TREATMENT
TREATMENT AREA (SF/AC) % A
D 10,660 / 0.25 40
2. DEVELOPED LAND TREATMENT
TREATMENT AREA (SF/AC) %
B C 13,880 / 0.32 51 D 13,400 / 0.31 49
II. HYDROLOGY
A. EXISTING CONDITION
a. VOLUME E _w = (E _A A _A +E _B A _B +E _C A _C +E _D A _D)/A _T
$E_{W} = (E_{A}A_{A} + E_{B}A_{B} + E_{C}A_{C} + E_{D}A_{D})/A_{T}$ $E_{W} = ((0.00^{*}0.53) + (0.23^{*}0.78) + (0.15^{*}1.13) + (0.25^{*}2.12))/0.63 = 1.40 \text{ IN}$ $V_{100} = (E_{W}/12)A_{T} = (1.40/12)0.63 = 0.0732 \text{ AC-FT} = 3,170 \text{ CF}$
b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^*1.56) + (0.23^*2.28) + (0.15^*3.14) + (0.25^*4.7)) = 2.2 CFS$
B. DEVELOPED CONDITION
B. <u>DEVELOPED CONDITION</u> a. VOLUME $E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$ $E_W = ((0.00^{\circ}0.53) + (0.00^{\circ}0.78) + (0.32^{\circ}1.13) + (0.31^{\circ}2.12))/0.63 = 1.62$ IN $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849$ AC-FT = 3,680 CF
a. VOLUME $E_{W} = (E_{A}A_{A} + E_{B}A_{B} + E_{C}A_{C} + E_{D}A_{D})/A_{T}$ $E_{W} = ((0.00^{*}0.53) + (0.00^{*}0.78) + (0.32^{*}1.13) + (0.31^{*}2.12))/0.63 = 1.62 \text{ IN}$ $V_{100} = (E_{W}/12)A_{T} = (1.62/12)0.63 = 0.0849 \text{ AC-FT} = 3,680 \text{ CF}$ b. PEAK DISCHARGE $Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B} + Q_{PC}A_{C} + Q_{PD}A_{D}$
a. VOLUME $E_{W} = (E_{A}A_{A}+E_{B}A_{B}+E_{C}A_{C}+E_{D}A_{D})/A_{T}$ $E_{W} = ((0.00^{*}0.53) + (0.00^{*}0.78) + (0.32^{*}1.13) + (0.31^{*}2.12))/0.63 = 1.62 \text{ IN}$ $V_{100} = (E_{W}/12)A_{T} = (1.62/12)0.63 = 0.0849 \text{ AC-FT} = 3,680 \text{ CF}$ b. PEAK DISCHARGE $Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B} + Q_{PC}A_{C} + Q_{PD}A_{D}$ $Q_{P} = Q_{100} = ((0.00^{*}1.56) + (0.00^{*}2.28) + (0.32^{*}3.14) + (0.31^{*}4.7)) = 2.5 \text{ CFS}$ c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA
a. VOLUME $E_W = (E_AA_A + E_BA_B + E_CA_C + E_DA_D)/A_T$ $E_W = ((0.00^*0.53) + (0.00^*0.78) + (0.32^*1.13) + (0.31^*2.12))/0.63 = 1.62$ IN $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849$ AC-FT = 3,680 CF b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^*1.56) + (0.00^*2.28) + (0.32^*3.14) + (0.31^*4.7)) = 2.5$ CFS c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 460
a. VOLUME $E_W = (E_AA_A + E_BA_B + E_CA_C + E_DA_D)/A_T$ $E_W = ((0.00^*0.53) + (0.00^*0.78) + (0.32^*1.13) + (0.31^*2.12))/0.63 = 1.62$ IN $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849$ AC-FT = 3,680 CF b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^*1.56) + (0.00^*2.28) + (0.32^*3.14) + (0.31^*4.7)) = 2.5$ CFS c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF)
a. VOLUME $E_W = (E_AA_A + E_BA_B + E_CA_C + E_DA_D)/A_T$ $E_W = ((0.00^*0.53) + (0.00^*0.78) + (0.32^*1.13) + (0.31^*2.12))/0.63 = 1.62$ IN $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849$ AC-FT = 3,680 CF b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^*1.56) + (0.00^*2.28) + (0.32^*3.14) + (0.31^*4.7)) = 2.5$ CFS c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 460 280 280 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 840
a. VOLUME $E_W = (E_AA_A + E_BA_B + E_CA_C + E_DA_D)/A_T$ $E_W = ((0.00^*0.53) + (0.00^*0.78) + (0.32^*1.13) + (0.31^*2.12))/0.63 = 1.62$ IN $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849$ AC-FT = 3,680 CF b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^*1.56) + (0.00^*2.28) + (0.32^*3.14) + (0.31^*4.7)) = 2.5$ CFS c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF)
a. VOLUME $E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$ $E_W = ((0.00^{+}0.53) + (0.00^{+}0.78) + (0.32^{+}1.13) + (0.31^{+}2.12))/0.63 = 1.62 IN$ $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849 AC-FT = 3,680 CF$ b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^{+}1.56) + (0.00^{+}2.28) + (0.32^{+}3.14) + (0.31^{+}4.7)) = 2.5 CFS$ c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 460 280 280 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 840 490 490
a. VOLUME $E_W = (E_AA_A + E_BA_B + E_CA_C + E_DA_D)/A_T$ $E_W = ((0.00^{\circ}0.53) + (0.00^{\circ}0.78) + (0.32^{\circ}1.13) + (0.31^{\circ}2.12))/0.63 = 1.62$ IN $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849$ AC-FT = 3,680 CF b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^{\circ}1.56) + (0.00^{\circ}2.28) + (0.32^{\circ}3.14) + (0.31^{\circ}4.7)) = 2.5$ CFS c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 460 280 280 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 840 490 490 4976 1120 iii. TOTAL RETENTION CAPACITY
a. VOLUME $E_W = (E_AA_A + E_BA_B + E_CA_C + E_DA_D)/A_T$ $E_W = ((0.00^{\circ}0.53) + (0.00^{\circ}0.78) + (0.32^{*1}.13) + (0.31^{*2}.12))/0.63 = 1.62$ IN $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849$ AC-FT = 3.680 CF b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^{*1}.56) + (0.00^{*2}.28) + (0.32^{*3}.14) + (0.31^{*4}.7)) = 2.5$ CFS c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 460 280 280 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 840 490 490 4976 1120 iii. TOTAL RETENTION CAPACITY $V_{POND} = 280 + 490 = 770$ CF
a. VOLUME $E_{W} = (E_{A}A_{A}+E_{B}A_{B}+E_{C}A_{C}+E_{D}A_{D})/A_{T}$ $E_{W} = ((0.00^{+}0.53) + (0.00^{+}0.78) + (0.32^{+}1.13) + (0.31^{+}2.12))/0.63 = 1.62 \text{ IN}}{V_{100} = (E_{W}/12)A_{T} = (1.62/12)0.63 = 0.0849 \text{ AC-FT} = 3.680 \text{ CF}}$ b. PEAK DISCHARGE $Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B} + Q_{PC}A_{C} + Q_{PD}A_{D}$ $Q_{P} = Q_{100} = ((0.00^{+}1.56) + (0.00^{+}2.28) + (0.32^{+}3.14) + (0.31^{+}4.7)) = 2.5 \text{ CFS}$ c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 460 280 280 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 840 490 490 4976 1120 iii. TOTAL RETENTION CAPACITY $V_{POND} = 280 + 490 = 770 \text{ CF}$ C. <u>COMPARISON</u> a. VOLUME
a. VOLUME E _W = (E _A A _A +E _B A _B +E _C A _C +E _D A _D)/A _T E _W = ((0.00*0.53) + (0.00*0.78) + (0.32*1.13) + (0.31*2.12))/0.63 = 1.62 IN V ₁₀₀ = (E _W /12)A _T = (1.62/12)0.63 = 0.0849 AC-FT = 3,680 CF b. PEAK DISCHARGE Q _P = Q _P A _A A + Q _{PB} A _B + Q _{PC} A _C + Q _{PD} A _D Q _P = Q ₁₀₀ = ((0.00*1.56) + (0.00*2.28) + (0.32*3.14) + (0.31*4.7)) = 2.5 CFS c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 840 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4976.5 840 490 490 4976 1120 iii. TOTAL RETENTION CAPACITY V _{POND} = 280 + 490 = 770 CF C. <u>COMPARISON</u> a. VOLUME $\Delta V_{100} = 3680 - 3170 = 510$ CF (INCREASE) b. PEAK DISCHARGE
a. VOLUME $E_W = (E_AA_A + E_BA_B + E_CA_C + E_DA_D)/A_T$ $E_W = ((0.00^{+}0.53) + (0.00^{+}0.78) + (0.32^{+}1.13) + (0.31^{+}2.12))/0.63 = 1.62 IN$ $V_{100} = (E_W/12)A_T = (1.62/12)0.63 = 0.0849 AC-FT = 3.680 CF$ b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$ $Q_P = Q_{100} = ((0.00^{+}1.56) + (0.00^{+}2.28) + (0.32^{+}3.14) + (0.31^{+}4.7)) = 2.5 CFS$ c. WATER HARVESTING RETENTION PONDING (AVERAGE END-AREA METHOD) i. NORTHWEST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 460 280 280 4976 660 ii. EAST WATER HARVESTING AREA ELEV AREA (SF) VOLUME (CF) Σ VOLUME (CF) 4975.5 840 490 490 4976 1120 iii. TOTAL RETENTION CAPACITY $V_{POND} = 280 + 490 = 770 CF$ C. <u>COMPARISON</u> a. VOLUME $AV_{100} = 3680 - 3170 = 510 CF$ (INCREASE) b. PEAK DISCHARGE $\Delta Q_{100} = 2.5 - 2.2 = 0.3 CFS$ (INCREASE) c. RETENTION CAPACITY VS INCREASE IN VOLUME GENERATED

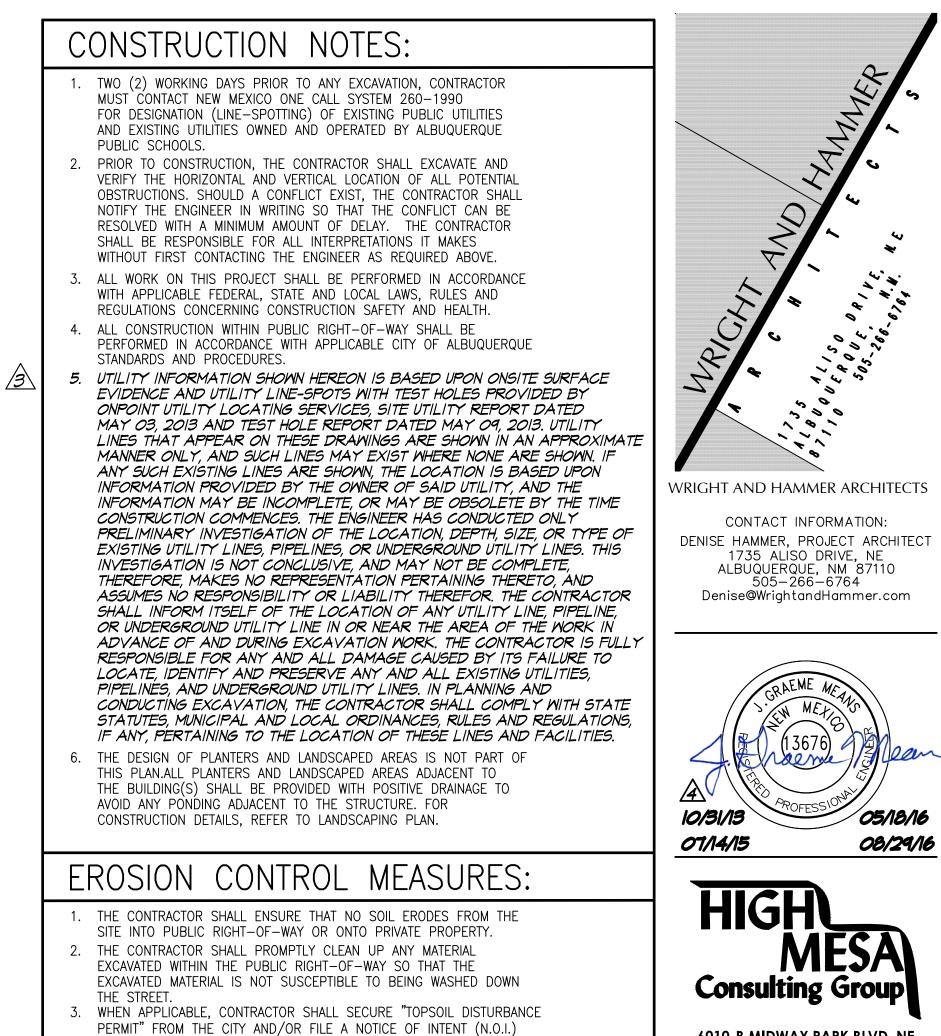
HEREBY **IPLIANCE** N DATED IENT WAS OBTAINED 08/15/2016 BY HIGH MESA CONSULTING GROUP UNDER THE DIRECTION OF CHARLES G. CALA, JR, NMPS 11184, AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

THIS CERTIFICATION IS SUBMITTED TO SUPPORT A RECOMMENDATION FOR PERMANENT CERTIFICATE OF OCCUPANCY FOR THE NEW CAFETERIA AND TO DOCUMENT COMPLETION OF THE IMPROVEMENTS FOR THE OWNER. THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THIS CERTIFICATION DOES NOT ADDRESS ADA COMPLIANCE WHICH IS BEYOND THE SCOPE OF GRADING AND DRAINAGE. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE. J. Eraeme Mean

J. GRAEME MEANS, NMPE 13676

08/29/2016 DATE





WITH THE EPA PRIOR TO BEGINNING CONSTRUCTION.

6010-B MIDWAY PARK BLVD. NE ALBUQUERQUE, NEW MEXICO 87109 PHONE: 505.345.4250 FAX: 505.345.4254 www.highmesacg.com



BOARD OF EDUCATION ALBUQUERQUE MUNICIPAL SCHOOL **DISTRICT NO. 12**

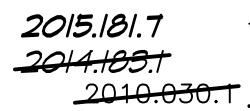
> 6400 Uptown Blvd. NE Suite 600-W Albuquerque, NM 87110

MacARTHUR ELEMENTARY SCHOOL KINDERGARTEN ADDITION

CAFETERIA ADDITION & IMPROVEMENTS RE-ROOFING

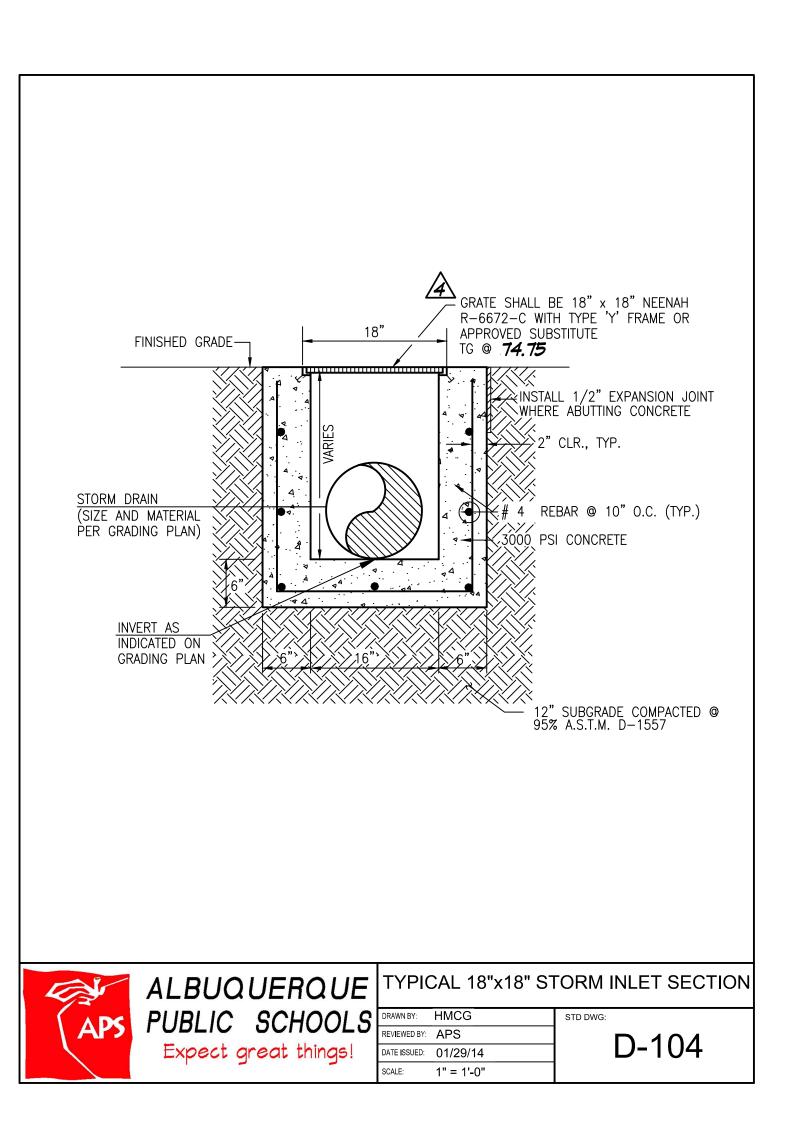
OWNER'S CONTACT: MYRON JOHNSON, ARCHITECT FACILITIES DESIGN & CONSTRUCTION 915 OAK STREET, SE ALBUQUERQUE, NM 87106 505-848-8811 4 08/16 ENG. CERTIFICATION 3 05/16 ADDED COURTYARD DRAIN 12 07/15 ENGINEER'S CERTIFICATION ▲ 01/15 NO CHANGE THIS SHEET 11/1/13 ISSUE DATE MARK DATE DESCRIPTION ISSUE INFORMATION PROJECT NO.: 0303.001.40105 COPYRIGHT WRIGHT & HAMMER ARCHITECTS 2013

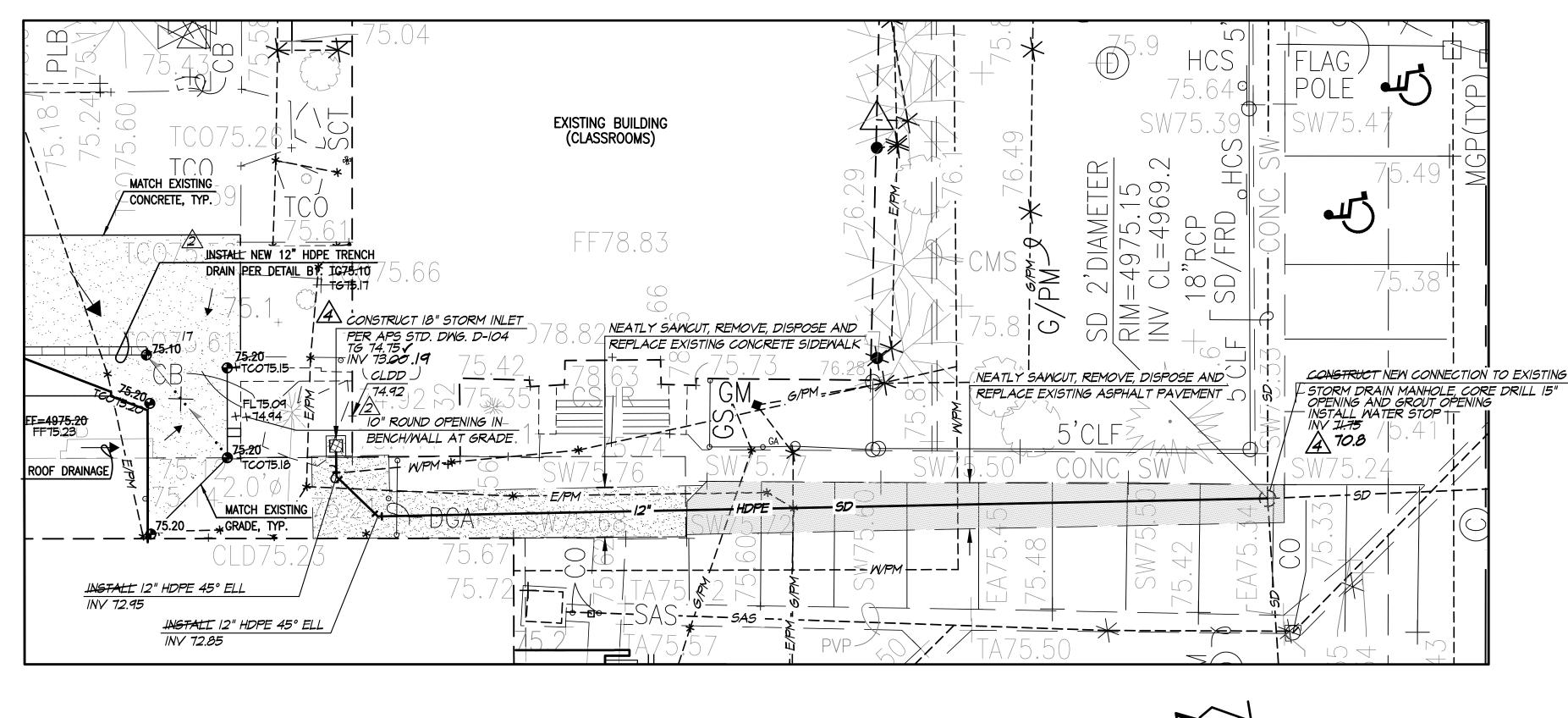
> GENERAL DRAINAGE PLAN AND CALCULATIONS

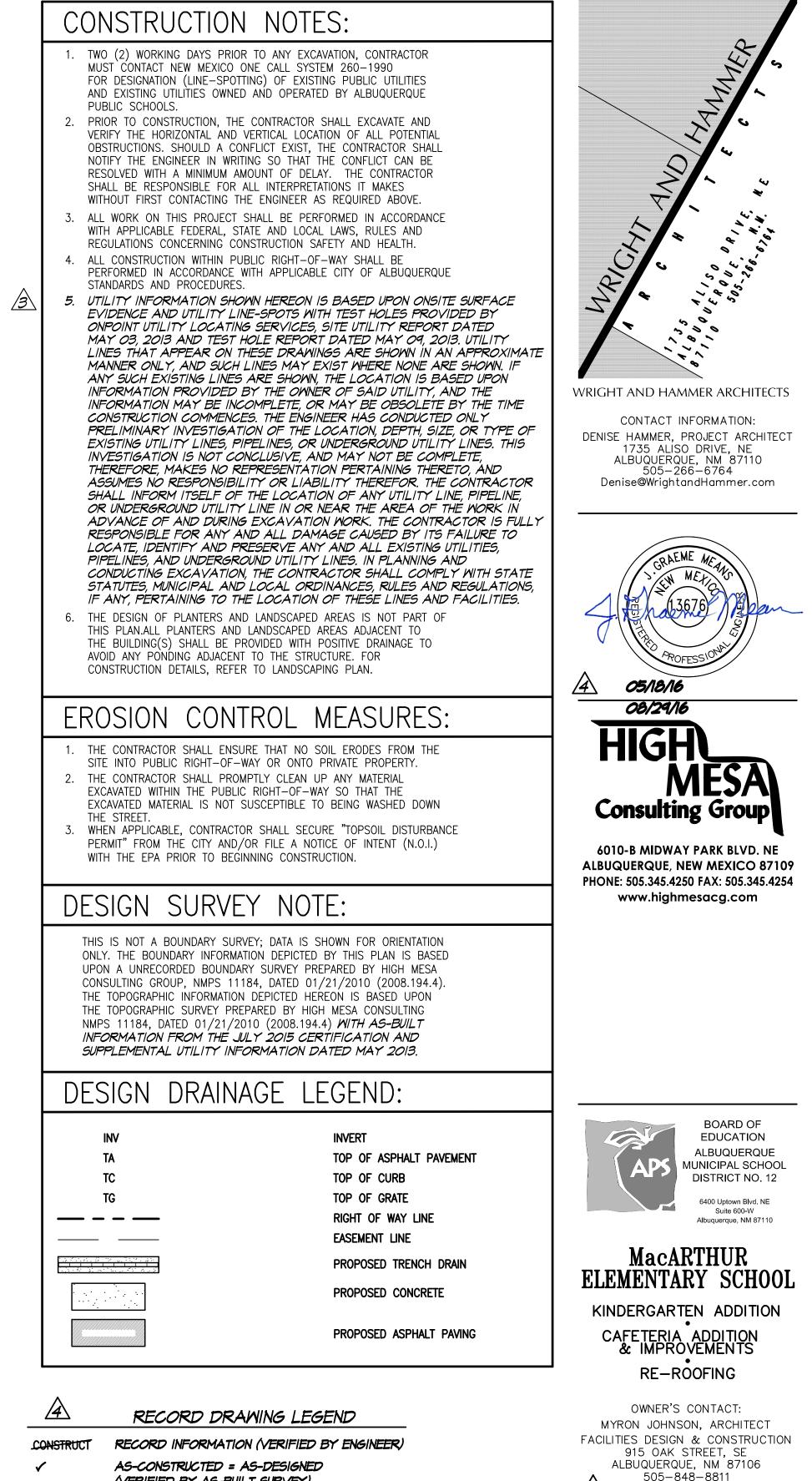


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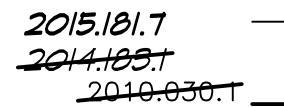




AS-CONSTRUCTED = AS-DESIGNED (VERIFIED BY AS-BUILT SURVEY) RECORD INFORMATION FROM AS-BUILT SURVEY RECORD INFORMATION FROM AS-BUILT SURVEY RECORD INFORMATION FROM AS-BUILT SURVEY **@** 28,95.**92**

,36 42"

+25.2



C-302-C

4 08/16 ENG. CERTIFICATION

11/1/13 ISSUE DATE

WRIGHT & HAMMER ARCHITECTS 2013

COURTYARD

ADDED COURTYARD

DRAIN

MARK DATE DESCRIPTION

PROJECT NO.: 0303.001.40105

ISSUE INFORMATION

COPYRIGHT

05/16 ADDED COURTYARD DRAIN

07/15 ENGINEER'S CERTIFICATION

01/15 NO CHANGE THIS SHEET



City of Albuquerque

Planning Department Development & Building Services Division DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

		Building Permit #:	City Drainage #:
DRB#:	EPC#:		Work Order#:
Legal Description:			
City Address:			
Engineering Firm:			Contact:
Address:			
Phone#:	Fax#:		E-mail:
Owner:			Contact:
Address:			
Phone#:	Fax#:		_ E-mail:
Architect:			Contact:
Address:			
Phone#:	Fax#:		_ E-mail:
Other Contact:			Contact:
Address:			
Phone#:	Fax#:		E-mail:
			F APPROVAL/ACCEPTANCE SOUGH
DEPARTMENT:	ROL	BUILDING F	F APPROVAL/ACCEPTANCE SOUGH PERMIT APPROVAL FE OF OCCUPANCY
DEPARTMENT: HYDROLOGY/ DRAINAGE TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTR	ROL	BUILDING F	PERMIT APPROVAL TE OF OCCUPANCY
DEPARTMENT: HYDROLOGY/ DRAINAGE TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTR		BUILDING F CERTIFICAT	PERMIT APPROVAL TE OF OCCUPANCY RY PLAT APPROVAL
DEPARTMENT: HYDROLOGY/ DRAINAGE TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTR TYPE OF SUBMITTAL:		BUILDING F CERTIFICAT PRELIMINA SITE PLAN	PERMIT APPROVAL TE OF OCCUPANCY
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COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ____