CITY OF ALBUQUERQUE

June 3, 2015



Reza Afaghpour, PE SBS Construction and Engineering, LLC 10209 Snowflake Ct NW Albuquerque, NM 87114

Re:

17 Unit Townhouse Development

1120 Griegos Rd NW

Grading & Drainage Plan (F14D058)

Dear Mr. Afaghpour,

Based upon the information provided in your submittal received 4/28/15, the above referenced plan is approved for Site Plan for Building Permit action by the DRB. However before this plan can be accepted by the Hydrology section for building permit the following comments must be addressed,

- Provide the roof flows.
- Label turn blocks at each wall or state typical opening for drainage.
- How are the flows handled to the east of this site?
- Extend the sidewalk culvert 1 foot passed the property line.
- An ESC plan is required and accepted by Curtis Cherne before BP approval.

Albuquerque

PO Box 1293

If you have any questions, you can contact me at 924-3695 or Rudy Rael at 924-3977.

New Mexico 87103

www.cabq.gov

Sincerely,

Rita Harmon, P.E.

Senior Engineer, Hydrology

Planning Department

RR/RH

C:

email

NOTICE TO CONTRACTORS

1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY

2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE INTERIM STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION,

3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL FOR LOCATING SERVICE, 260-1990 OR "811", FOR LOCATION

4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONSTRUCTIONS. SHOULD A CONFLICT EXIST. THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.

5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE. 6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER

7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.

APPROVALS	NAME	DATE
INSPECTOR		

Lots 3 and 4, Block 2, Sandia Plaza, contains +/- 0.3519 acres and is loated at 203 Griegos Rd. N.W. See attached portion of the Vicinity Map for exact location.

The purpose of this drainage report is to present a grading and drainage solution to replace existing improvements with this new

Existing Drainage Conditions

There is undeveloped. This site is fairly flat, and it drains to Griegos Road and 12th Street N.W. No offiste runoff enters the site. Based on the FIRM Map 35001C0119G (revised September 26, 2008) the site does not fall within a 100-year floodplain.

Proposed Conditions and On-Site Drainage Management Plan The runoff generated from this site will be retained on-site. Several Ponds (A through D) are designed to hold nearly twice the volume of the 100-yr/6-day volume under the proposed conditions minus 100-yr/6-day volume under the historical conditions. Then when the ponds exceed their capacity the runoff will overflow into the parking lot and then to public street via sidewalk culverts. The allowable discharge in the Valley is 2.75 cf/acre meaning a retention volume requirement of 0.50 inches times the area (638.74 cf). The 90th Percentile/First Flush ponding requirement is 0.34 inches times the impervious area (282.32 cf). Total retention volume provided (2,052.23 cf) far exceeds the ponding requirement in the Valley (638.741 cf) and First Flush (282.32 cf).

Calculations

City of Albuquerque, Development Process Manuel, Section 22.2, Hydrology Section, was used for runoff calculations. See this plan for AHYMO input and Summary output files.

* ZONE 2

************************** 100-YEAR, 6-HR STORM (UNDER EXISITNG CONDITIONS)

START RAINFALL

TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=2.01 IN RAIN SIX=2.35 IN

RAIN DAY=2.75 IN DT=0.03333 HR

* ON-SITE COMPUTE NM HYD ID=1 HYD NO=100.0 AREA=0.000550 SQ MI PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00

TP=0.1333 HR MASS RAINFALL=-1

10-YEAR, 6-HR STORM (UNDER EXISTING CONDITIONS)

START TIME=0.0 TYPE=1 RAIN QUARTER=0.0 IN RAINFALL

RAIN ONE=1.34 IN RAIN SIX=1.57 IN RAIN DAY=1.83 IN DT=0.03333 HR

* ON-SITE COMPUTE NM HYD

ID=1 HYD NO=110.0 AREA=0.000550 SQ MI

PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1

100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS)

START TIME=0.0

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=2.01 IN RAIN SIX=2.35 IN RAIN DAY=2.75 IN DT=0.03333 HR

* ON-SITE

ID=1 HYD NO=100.1 AREA=0.000550 SQ MI COMPUTE NM HYD PER A=0.00 PER B=10.00 PER C=15.00 PER D=65.00

TP=0.1333 HR MASS RAINFALL=-1

10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS)

START RAINFALL TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=1.34 IN RAIN SIX=1.57 IN

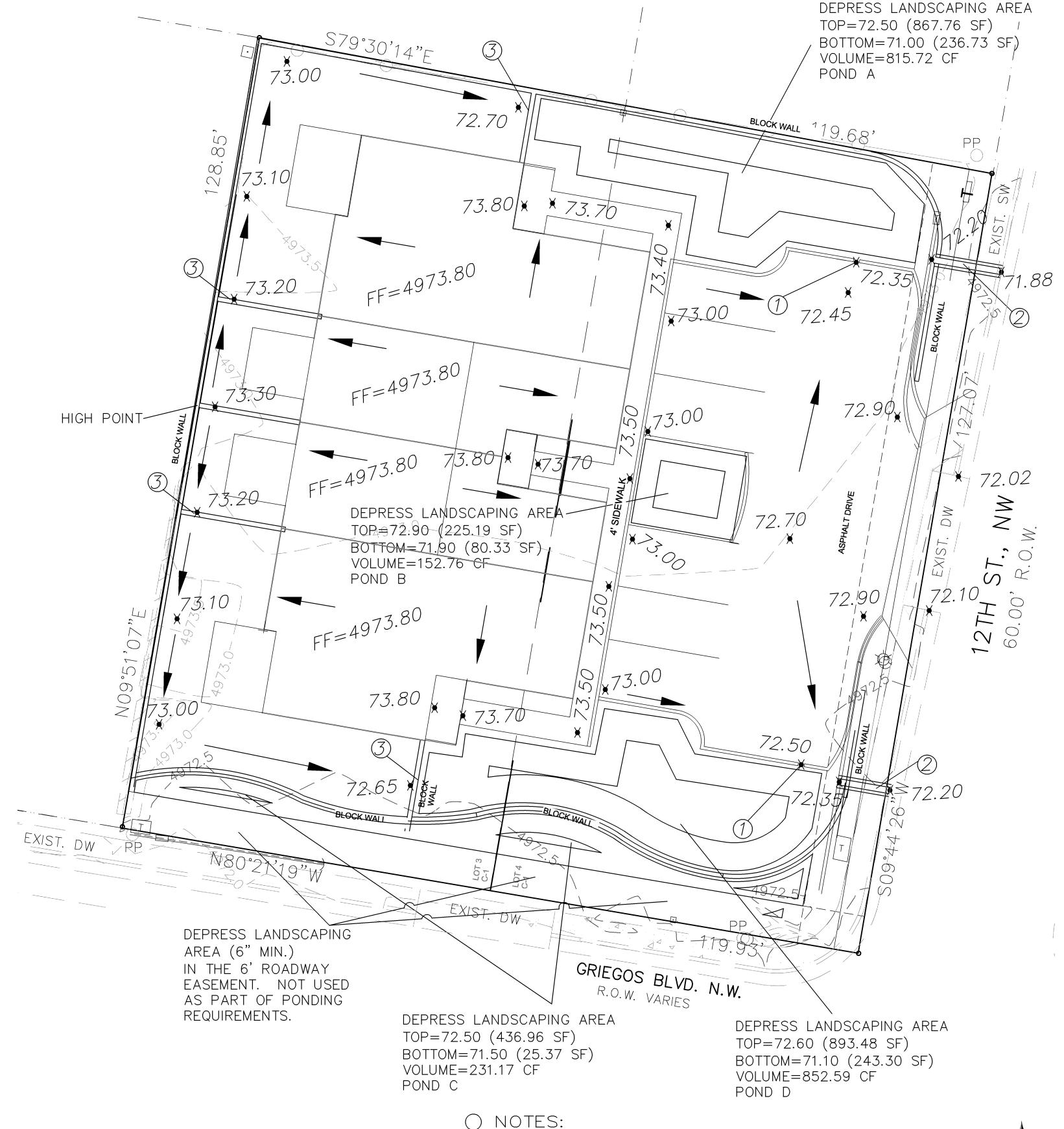
RAIN DAY=1.83 IN DT=0.03333 HR * ON-SITE

COMPUTE NM HYD ID=1 HYD NO=110.1 AREA=0.000550 SQ MI PER A=0.00 PER B=10.00 PER C=15.00 PER D=65.00

TP=0.1333 HR MASS RAINFALL=-1 ***********************************

FINISH

AHYMO PROGRAM SUMM INPUT FILE = 12th.txt	- VERSION: 1997.02d RUN DATE (MON/DAY/YR) =12/22/2014 USER NO.= AHYMO-I-9702c01000R31-AH								
	FRC DROGRAPH II FICATION NO	D ID	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES) (He	TIME TO PEAK OURS)		GE = 1 ON
START RAINFALL TYPE= 1 COMPUTE NM HYD START RAINFALL TYPE= 1	100.00 –	- 1	.00055	.81	.023	.77821	1.533	TIME= RAIN6= 2.301 PER IMP= TIME= RAIN6=	.00 2.350 00 .00 1.570
COMPUTE NM HYD START	110.00 –	. 1	.00055	.33	.008	.27828	1.533	.945 PER IMP= TIME=	
RAINFALL TYPE= 1 COMPUTE NM HYD START	100.10 –	. 1	.00055	1.48	.053	1.79770	1.500	RAIN6= 4.216 PER IMP= TIME=	2.350 72.22 .00
RAINFALL TYPE= 1 COMPUTE NM HYD FINISH	110.10 –	1	.00055	.93	.032	1.07842	1.500	RAIN6= 2.650 PER IMP=	1.570 72.22

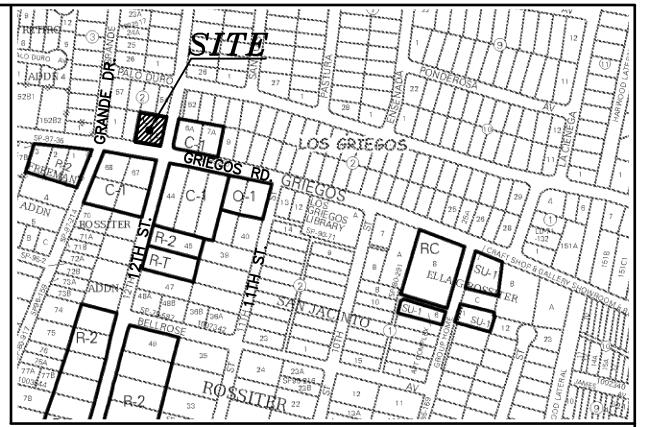


1. PROVIDE 12" CURB OPENING

- 2. 12" SIDEWALK CULRVET PER CITY STD DWG 2236 (TACK WELD PLATE AT THE BOLT)
- 3. 6" WALL OPENING (OR TURN TWO BLOCKS) BUILD #4 REBAR AT 3" ON CENTER

TOTAL POND AREA PROVIDED = POND A + B + C + D = 2,052.23 CF TOTAL PONDING VOLUME REQUIRED = VOL. PROPOSED CONDITIONS - VOL. EXISTING CONDITIONS = 0.053 - 0.023 = 0.03 AC-FT = 1,306.80 CF

TOTAL PONDING VOLUME REQUIRED (VALLEY)= 0.5 INCHES x AREA = $(0.5/12 \times 15,329.81) = 638.74$ CF TOTAL PONDING VOLUME REQUIRED (90TH PERCENTILE/FIRST FLUSH) = 0.34 IMCHES x IMPERVIOUS AREA = $(0.34/12 \times 9,964.38) = 282.32$ CF



F-14-Z

VICINITY MAP:

LEGAL DESCRIPTION: LOTS 3 AND 4, BLOCK 2, SANDIA PLAZA

CONTAINING 15,329.81 S.F. (0.3519 ACRE) **ZONING: C-1 USES**

ADDRESS:

203 GRIEGOS ROAD N.W.

GENERAL NOTES:

1: CONTOUR INTERVAL IS HALF (0.50) FOOT. 2: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE CONTROL STATION

- LSS_206, HAVING AN ELEVATION OF 4976.652 FEET ABOVE SEA LEVEL. 3: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT
- INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND / OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON-
- 4: THIS IS NOT A BOUNDARY SURVEY, BEARINGS ARE ASSUMED, DISTANCES AND FOUND PROPERTY CORNERS ARE FOR INFORMATIONAL PURPOSES ONLY.
- 5: SLOPES ARE AT 3:1 MAXIMUM

NOTICE TO CONTRACTORS

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LEGEND

---5100——— EXISTING CONTOUR (MAJOR) ---5102--- EXISTING CONTOUR (MINOR) BOUNDARY LINE

PROPOSED SPOT ELEVATION ¥ 85.46

EXISTING GRADE \times 5265.16

× 5284.43 EXISTING FLOWLINE ELEVATION

PROPOSED RETAINING WALL

TRW = 91.50

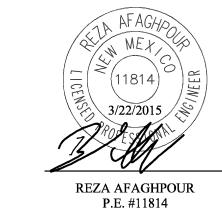
TF=88.00

BC = 89.08

TOP OF RETAINING WALL TOP OF FOOTING

BOTTOM OF CHANEL

HIGH POINT



SBS CONSTRUCTION AND ENGINEERING, LLC

> 10209 SNOWFLAKE CT., NW ALBUQUERQUE, NEW MEXICO 87114 (505)899-5570

GRAPHIC SCALE

SCALE: 1"=10'

4 UNIT TOWNHOUSE DEVELOPMENT GRADING AND DRAINAGE PLAN

DRAWING: DRAWN BY: DATE: SHEET# SH-B 201418-GR.DWG 12-22-2014