## FEBRUARY 7, 2012

# SUPPLEMENTAL INFORMATION

#### **FOR**

# Mountain Mahogany Community School

IA Project No. 1903



The Mountain Mahogany Charter School site is a developed school property located within C.O.A. Vicinity Map F-14. The site is bound by 4<sup>th</sup> Street to the west, the Gallegos Lateral to the east and developed residential properties to the north and south. The proposed construction includes a new administration building and classroom additions with associated site walks and landscaping.

Off-site: no off-site drainage affects this property.

Flood hazard: per Bernalillo County FIRM Map #35001C0119G, dated September 26, 2008, the site is located within Floodzone 'X' (shaded) designated as 'areas of 0.2% annual chance flood; areas of 1% chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

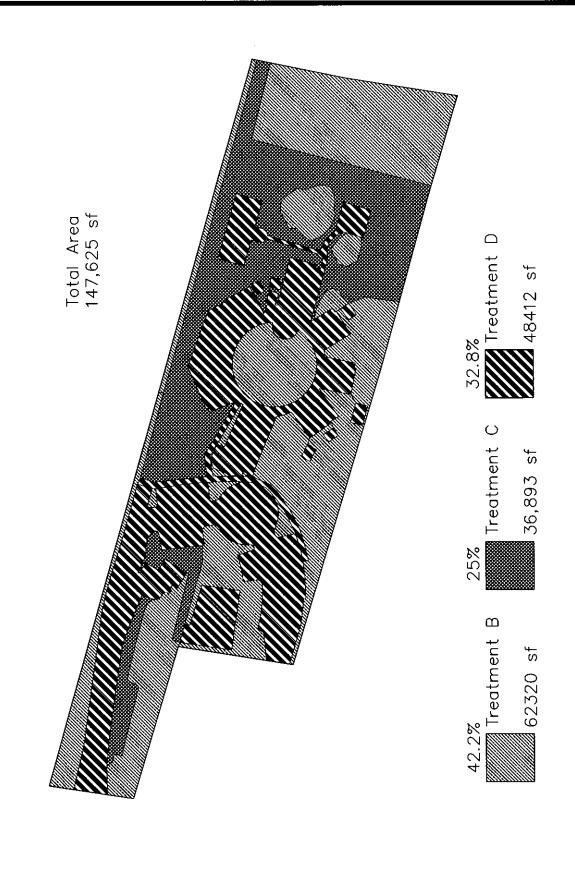
Drainage plan concept: the Mountain Mahogany Charter School site utilizes retention ponds throughout the site to store the 100-year, 10-day storm event as required. In addition, every building discharges roof drainage to large above ground rain barrels totaling over 8,000 gallons (=1,070 CF = 40 CY). Although these rain barrels do not count towards stormwater storage, they will assuredly have an impact. The proposed site demolition and new construction will not alter existing drainage patterns significantly. Site discharge will continue to be captured within localized water harvesting basins with all overflow passing to the east recreation field / retention pond.

This analysis includes calculations for the EXISTING condition, the PROPOSED condition and also provides a conceptual analysis for FUTURE additions to show that there is adequate volume available should the school seek to add additional impervious area.

In order to generate the volumes required for each of the conditions, land treatment areas were calculated (see Land Treatment Maps) and the resulting percentages were entered into the DPM calculations to obtain the volume generated for the 100-year 6-hour storm event. The 100-year 10-day volume was then calculated per the DPM to determine the required ponding volume.

Site pond depths were calculated using Civil 3D as follows. A surface was generated based on the provided topographic survey. A second surface (a level plane) was generated to represent water surface elevation and moved vertically until the required volume was achieved. See the attached Volume maps for the portions of ponding.

In summary, the existing condition provides a pond volume of at a water surface elevation of 4972.4.







ISAACSON & ARFMAN, P.A.

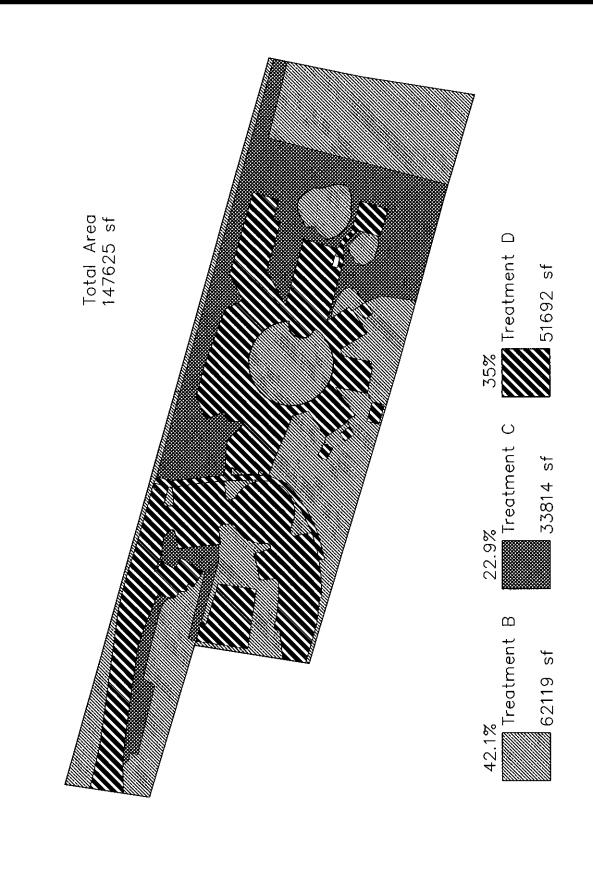
Consulting Engineering Associates
Albuquerque, New Mexico
1903 C-701-LAND AREA 4866 07,2012

PROJECT Mountain Mahogany Private School

JOB NO. \_\_\_\_1903

BY\_BJB

DATE \_\_Feb. 3, 2012



PROPOSED CONDITIONS



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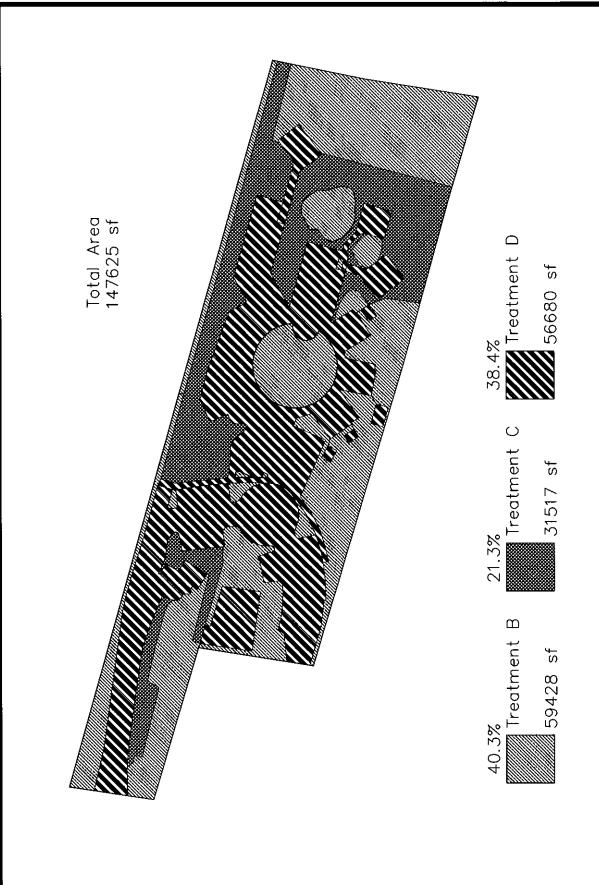
Consulting Engineering Associates
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FUTURE CONDITIONS



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The 100-year 6-hour storm event for EXISTING, PROPOSED and FUTURE impervious conditions are provided in the table below.

EXISTING CONDITION	NS	DESCRIPTION		Approximately 33%	impervious
Area of basin flows =	147624	SF	=	3.4 Ac.	
The following calculation	ns are based on	Treatment areas as shown in	table to the rig	ght LAND TRI	EATMENT
	Sub-basin Weigl	hted Excess Precipitation (see	e formula abov	/e) A =	0%
	Weighted E	= 1.31	in.	B =	42%
	Sub-basin Volun	ne of Runoff (see formula abo	ve)	C =	25%
	V360	= 16079	CF	D=	33%
	Sub-basin Peak I	Discharge Rate: (see formula	above)		
	QP	= 11.1	cfs		
PROPOSED CONDITION	ONS	DESCRIPTION		Approximately 35%	impervious
Area of basin flows =	147624	SF	=	3.4 Ac.	
The following calculation	ns are based on	Treatment areas as shown in	table to the rig	ght LAND TRI	EATMENT
	Sub-basin Weigi	hted Excess Precipitation (see	e formula abov	/e) A =	0%
	Weighted E	= 1.33	in.	$\mathbf{B} =$	42%
	Sub-basin Volun	ne of Runoff (see formula abo	ve)	C =	23%
	V360	= 16351	CF	D =	35%
	Sub-basin Peak I	Discharge Rate: (see formula	above)		
	QP	= 11.3	cfs		
FUTURE CONDITIONS	3	<b>DESCRIPTION</b>		Approximately 38%	impervious
Area of basin flows =	147624	SF	=	3.4 Ac.	
The following calculation	ns are based on	Treatment areas as shown in	table to the rig	ght LAND TRI	EATMENT
	Sub-basin Weigh	hted Excess Precipitation (see	e formula abov	/e) A =	0%
	Weighted E	= 1.37	in.	$\mathbf{B} =$	40%
	Sub-basin Volun	ne of Runoff (see formula abo	ve)	C =	21%
	V <sub>360</sub>	= 16843	CF	D =	38%
	Sub-basin Peak l	Discharge Rate: (see formula	above)		
	QР	= 11.5	cfs		

The required storage volume for the site is based on the 100-year 10-day storm event. The following tables and graphics will clearly show that this site has ponding volume available in it's existing condition to store a significant volume without impacting the surrounding properties

EXISTING CONDITIONS: 32.8% D

PROPOSED CONDITIONS: 35.0% D

MAXIMUM CONDITIONS: 38.4% D

100-year 10-day

100-year 10-day

100-year 10-day

V <sub>360</sub> (from previous calculation)		6/091
Area Treatment D (SF)		48421
Zone		2
For 10 Day Storms: Vioday = V360 + AD * (Ploday -	P360)*	- P <sub>360</sub> )*43560 SF/AC
V360	li li	16079
AD (SF)	П	48421
Zone	II	2
P10day	-	3.95
P <sub>360</sub>	Ħ	2.35
V360	=	16079
+ imp. area	11	. 6456
Total Pond Volume (V10 day)	1	22535

>
835 CV
- 11
A CE
7 535

V360 (from previous calculation)		16351
Area Treatment D (SF)		21668
Zone		2
For 10 Day Storms: Vioday = V360 + AD * (Ploday - P360)*43560 SF/AC	P360)*	43560 SF/AC
V360	II	16351
AD (SF)	H	51668
Zone	=	2
P10day	H	3.95
P <sub>360</sub>	11	2.35
V360		16351
+ inp. area	n	6889
Total Pond Volume (V10 day)	11	23240
	l	

Water Surface Elevation = 4972.4

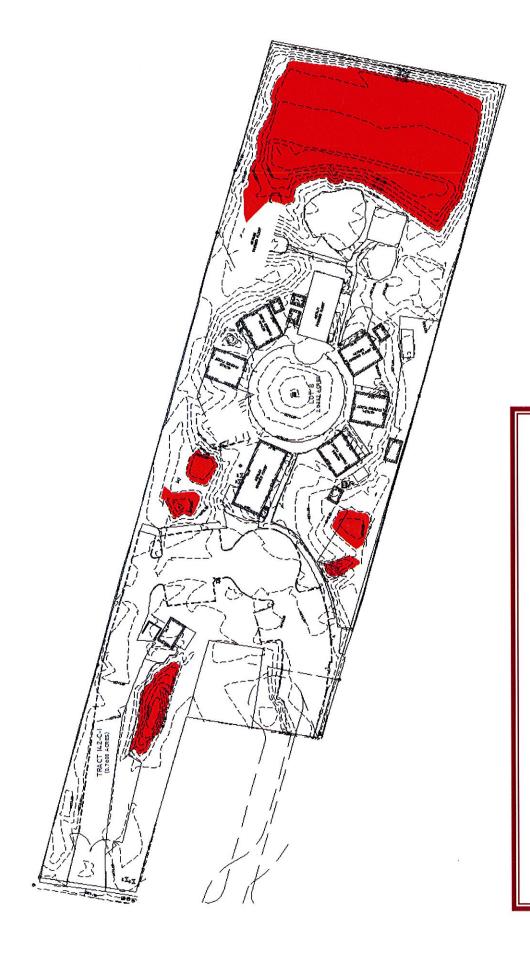
Water Surface Elevation = 4972.5

23,240 CF = 861 CY

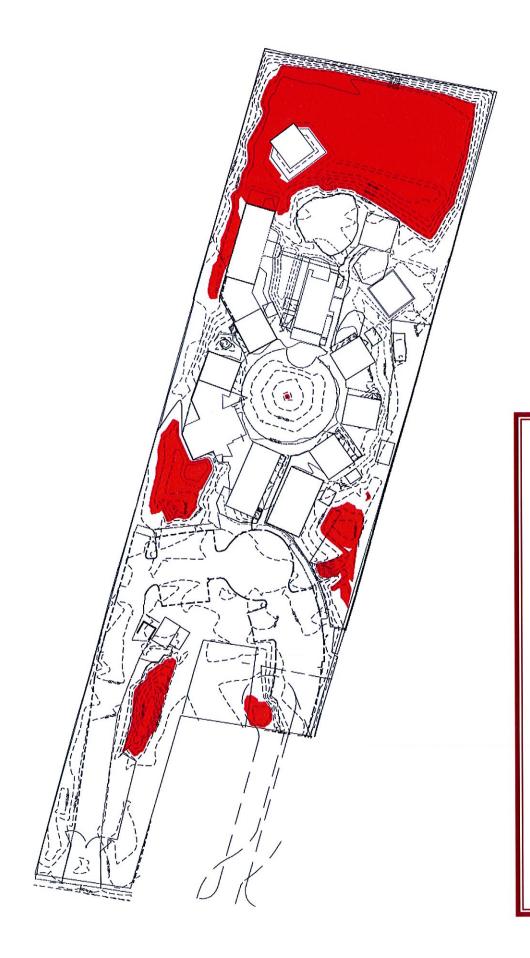
V <sub>360</sub> (from previous calculation)	16843
Area Treatment D (SF)	26688
Zone	2
For 10 Day Storms:	
Vioday = V360 + AD * (Pioday - P360)	- P <sub>360</sub> )*43560 SF/AC
V360	16843
A <sub>D</sub> (SF) =	88995
Zone	
P10day ==	3.95
$=$ $P_{360}$ $=$	2.35
V360 =	16843
+ imp. area ==	7558
Total Pond Volume (Vioday) =	24401

24,401 CF = 904 CY

Water Surface Elevation = 4972.7

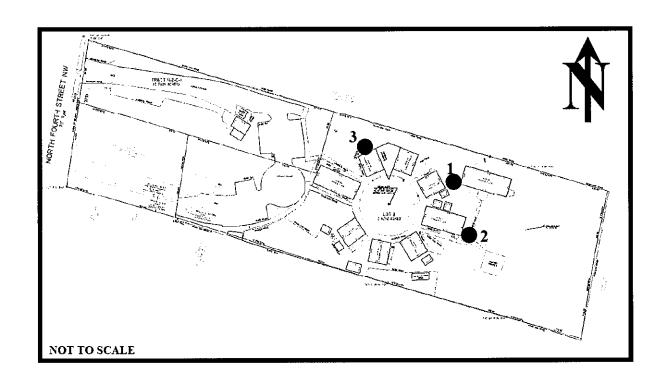


Existing Conditions (32.8% impervious): Required 100-year 10-day volume = 22,535 CF = 835 CY Water Surface Elevation = 4972.4



Future Conditions (38.4% impervious): Required 100-year 10-day volume = 24,401 cf = 904 cy Water Surface Elevation = 4972.7 In conclusion, the Mountain Mahogany Charter School site, based on existing topographic survey information, has volume available to store the 100-year 10-day storm event for the proposed condition as well as future additions shown. All of the existing and proposed buildings are elevated at least 2.0' above the future condition water surface elevation of 4972.7.

It is important to note that the future condition water surface elevation of 4972.7 does not take into account the existing french drains and proposed percolation pits which will improve the infiltration rate of the ponded waters through the shallow clay lenses to the sand strata below. See the following pages for excerpts from the soils report.



SITE PLAN

TEST HOLE LOCATION

#### **LOG OF TEST HOLE NO.: 1**

Project: 5014 4th St NW, Albuquerque, NM

Date Drilled: 1.13.2012

Drilling Method: Hand Auger

Surface Elevation: Not Available

Depth to Groundwater: Not Encountered

Bottom of Hole: Test Hole 1: 10 ft Test Hole 2: 10 ft

Depth	N-Value	Sample	Unified		Dry Density	Moisture
(feet)	(blows/ft)	Туре	Class.	Description	(pcf)	Content (%)
		В	SM	SAND, silty, fine to medium grained, moist, brown		12.5
2		В	CL	CLAY, sandy, fine grained, moist, dark-brown		28.5
			lelelelelele			6.7
5-		В	SM:	SAND, slightly slity, fine to coarse grained, moist, light-brown to gray		
				- lenses of very moist, gray, smelly clay		
		В		- dry sand at 8 ft		4.0
10				Bottom of Hole at 10 Feet		

### **LOG OF TEST HOLE NO.: 2**

	В	CL	CLAY with lenses of sand	20.0
2	•			24.2
	В			24.3
5	В	<b>-</b>  ::::::::::::::::::::::::::::::::::::	SAND, slightly silty, fine to medium grained, medium moist, light-brown	3.1
	В	10000000000	SAND, fine to medium grained, medium moist, light- brown, trace caliche	3.1
10			Bottom of Hole at 10 Feet	

## **LOG OF TEST HOLE NO.: 3**

Project:	5014 4th St NW, Albuquerque, NM
Date Drilled:	1.13.2012
Drilling Method:	Hand Auger
Surface Elevation:	Not Available
Depth to Groundwaters	Not Encountered
Bottom of Hole:	5 ft

Depth	N-Value	Sample	Unified		Dry Density	Moisture
(feet)	(blows/ft)	Туре	Class.	Description	(pcf)	Content (%)
		В	-CH-	CLAY with lenses of sand		23.9
<del>-2-</del>						
_				SAND, fine to medium grained, slightly moist, light- brown		
5				Bottom of Hole at 5 Feet		
	•					
	1					
10-						