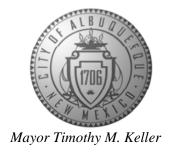
## CITY OF ALBUQUERQUE

Planning Department Brennon Williams, Director



March 3, 2020

Fred C. Arfman, P.E. Isaacson & Arfman, P.A. 128 Monroe St. N.E Albuquerque, NM 87108

**RE:** Bosque Escondido

**Grading & Drainage Plan and Drainage Report** 

Engineer's Stamp Date: 02/28/20

**Hydrology File: F14D076** 

Dear Mr. Arfman:

Based upon the information provided in your submittal received 02/28/2020, the Grading &

Drainage Plan and Drainage Report are approved for Grading Permit and for action by the DRB

on Platting.

Albuquerque Once the grading is complete, a pad certification will be required prior to release of Building

Permit. Please attach a copy of this approved plan in the construction sets for Building Permit

processing along with a copy of this letter and the pad certification approval letter.

NM 87103

www.cabq.gov

If the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and

Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, <u>jhughes@cabq.gov</u>, 924-3420) 14 days prior to any earth

disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette, P.E. CFM Senior Engineer, Hydrology

Renée C. Brissette

Planning Department

## DRAINAGE REPORT

**FOR** 

## **BOSQUE ESCONDIDO**

A 5-LOT DWELLING UNIT SINGLE-DETACHED RESIDENTIAL PRIVATE DEVELOPMENT

ALBUQUERQUE, NEW MEXICO

BY





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## **VICINITY MAP**

## FIRM MAP

- I. PROJECT INFORMATION
- II. INTRODUCTION
- III. EXISTING CONDITIONS
- IV. PROPOSED CONDITIONS
- **v.** SUMMARY & CONCLUSIONS

## **APPENDICES**

**APPENDIX A:** Drainage Basin Map

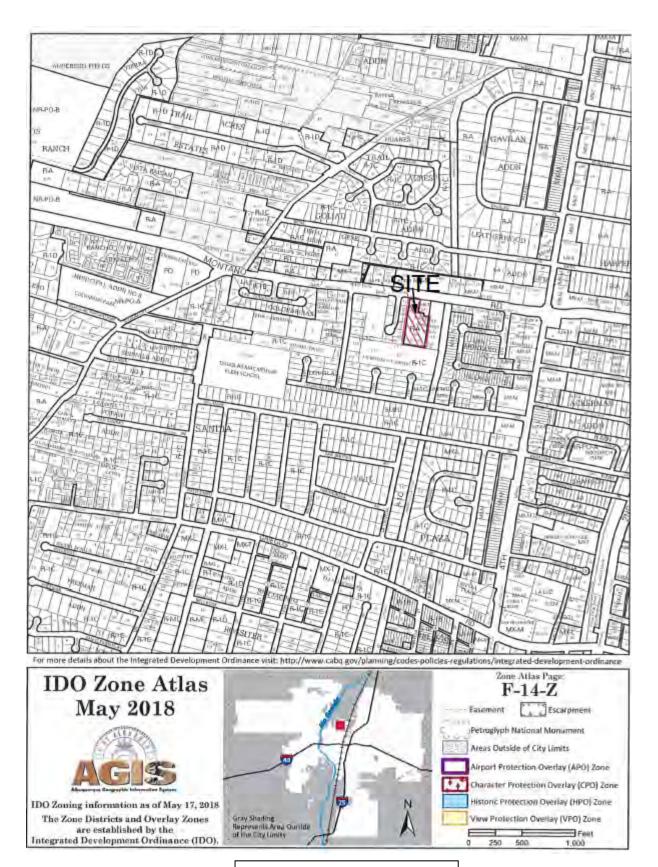
**Drainage Calculations** 

APPENDIX B: Excerpts from Drainage Report for Phase 1 of Montano Corridor

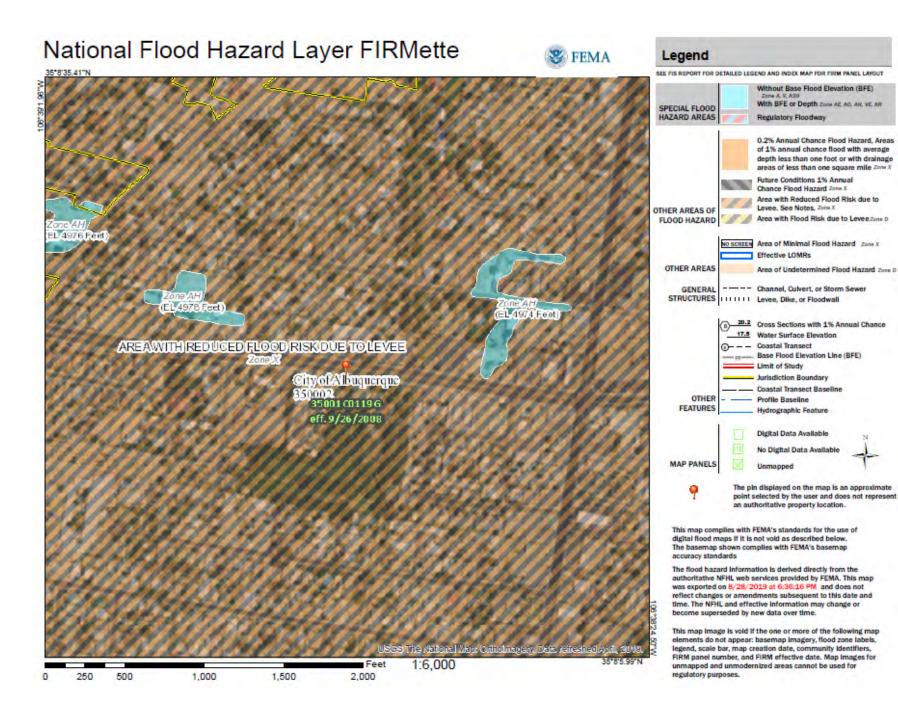
from Rio Grande to Edith by Wilson & Co.

## **POCKET**

**Grading Plan** 



**VICINITY MAP F-14-Z** 



## I. PROJECT INFORMATION

PROPOSED LEGAL DESCRIPTION:

Bosque Escondido

EXISTING LEGAL DESCRIPTION:

Tract 6-A-2, Fourth Street Homesites Addition

ENGINEER: Isaacson & Arfman, P.A.

128 Monroe Street NE Albuquerque, NM 87108

(505) 268-8828

Attn: Åsa Nilsson-Weber

SURVEYOR: Cartesian Surveys, Inc.

(505) 896-3050 Attn: Will Plotner

DEVELOPER: Montano Family Homes, LLC

(505) 362-6824 Attn: Tim Nisly

NUMBER OF PROPOSED DWELLING UNITS: 5

TOTAL AREA: 1.7468 Ac.

FLOOD PLAIN: This property lies within Flood Zone X Shaded which is defined as an area

with reduced flood risk due to levee. As determined by FEMA and shown on Flood Insurance Rate Map dated September 26, 2008, Map No. 35001C0119G.

## II. INTRODUCTION

This site is comprised of a vacant lot located south of Montano Rd. NW, west of 4<sup>th</sup> St. and east of 9<sup>th</sup> St. and is bound on the west by a residential subdivision, on the northeast by a commercial property (beauty salon), on the southeast by a single residence and on the south by a tract with agricultural use. The site will be developed as a private development with 5 detached residential homes. The flat land grading scheme will be applied so that each lot provides ponding for the 100-year, 24-hr storm.

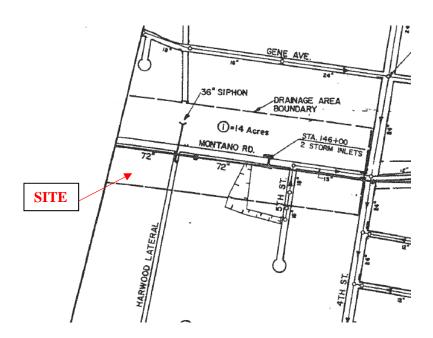
## III. EXISTING CONDITIONS

The majority of the site is undeveloped. A portion in the front has been graded with dirt access roads and the remaining consists of trees and vegetation.



AERIAL VIEW OF PROPERTY

The site is flat, and except for the graded portion at the north end that drains to Montano Rd., the majority of the site ponds on the property. Montano Rd. is a 4-lane road with a center turn lane, bike lanes and curb and gutter and sidewalks on both sides. There is a 66-inch storm drain in Montano Road that has limited capacity. Appendix B shows excerpts from the Drainage Report for Phase 1 of Montano Corridor from Rio Grande to Edith by Wilson & Co, dated August 1985. This report indicates that a portion of this site (approximately a 150' strip from Montano Rd.) is allowed to drain to the Montano Rd. storm drain. See below for an enlarged exhibit from the basin map in the Wilson & Co. report—this property is located in Basin 1.



## IV. PROPOSED CONDITIONS

The site will be developed as a private development using a flat grading scheme per DPM Chapter 22, Section 5G per the following criteria:

- The maximum percent impervious of each lot and the contributing area may not be greater than 45% (street area, drivepads, walks, roof area, patios, etc.)
- Each lot must provide retention ponding around the perimeter of each residence (outside of utility easements) to store the 100-year, 24 hour storm volume for the fully developed lot.
- Walls must be constructed along the lot lines to contain the required 100-year, 24 hour volume within the associated lot limits.

- The pad elevation shall be a minimum of one (1) foot above the 100-year 10-day stormwater surface elevation. Therefore, all pad elevations must be at elevation 4977.7 or greater.
- The flow between the front yard and back yard cannot be obstructed. Stormwater must be allowed to equalize to the same level between the front and back yards (using wall openings, swales, equilizer pipes, etc.)

This report stipulates the general criteria for ponding based on an assumed 45 percent Type D land treatment. The road and utility infrastructure will be constructed as part of the initial construction. Upon development of each lot, a grading plan will need to be submitted for building permit approval. These grading plans shall adhere to the criteria established in this report and on the grading plan.

The private road, Escondida Ln., will be graded at the same elevation throughout and be crowned so that the flows drain to each future lot. The road will be aggregate base course with sidewalks (deferred) adjacent to Lots 1-3. The grading will be contained to the roadway and across the utility easement with catch slopes tying to existing grade both sides. The site will continue to pond the roadway and undeveloped lots until the lots are developed.

The existing perimeter walls will remain. New perimeter walls will need to be constructed with the development of each lot to contain the 100-year, 24-hour storm.

Basin A (Tract A and the north half of the street within Lot 1) will drain approximately 0.2 cfs to Montano Rd.. The remaining basins will provide on-lot ponding for the 100-year, 24-hour storm.

The grading & drainage plan is included in the back pocket of this report.

## LAND TREATMENTS AND HYDROLOGY

Hydrology calculations assume the 45% max. impervious area allowed calculated for the developed area based on the building pad, driveway and roadway areas, and the remaining area was split between land treatments B and C.

Appendix A includes a Drainage Basin Exhibit and the 100-year, 24-hour pond volume calculations using the equations from the Drainage Design Criteria for City of Albuquerque Section 22.2, DPM, Vol 2, dated Jan., 1993.

## WATER QUALITY REQUIREMENTS

The water quality requirement will be met since the lots will have on-lot ponding. Payment in-lieu for 15 cubic feet of water quality volume may be required for Basin A which will discharge to Montano Rd.

## V. SUMMARY & CONCLUSIONS

The site will be developed with 5 detached residential homes and an aggregate base course road with a sidewalk adjacent to Lots 1-3 (sidewalk deferred until homes are constructed). Each lot shall provide the required 100-yr, 24-hr retention ponding areas.

Based on this report, it is recommended that the following improvements be constructed initially:

- Aggregate base course street with crown and utility infrastructure.
- Grading at road/utility easement edge to match existing elevations.

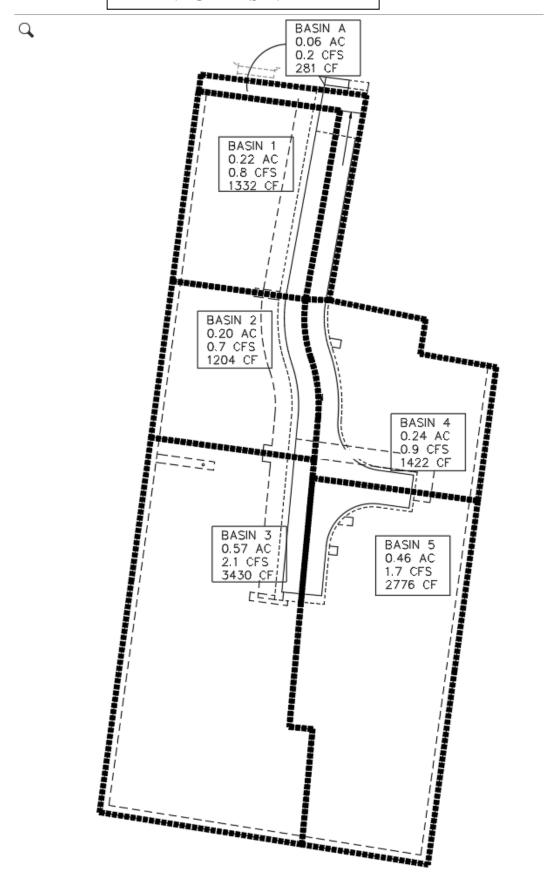
Once each lot develops, a detailed grading plan shall be submitted along with calculations for ponding volumes. The individual grading plans shall adhere to the following criteria:

- The 100-year 10-day water surface elevation will be at or below 4976.67 which would equal the edge of (deferred) sidewalk.
- The 100-year 24-hour storm (developed condition) will be at or below 4976.5.
- Perimeter walls around the lot will contain the 100-year 24 hour developed runoff.

## **APPENDIX A**

Drainage Basin Exhibit Drainage Calculations

## DRAINAGE BASIN EXHIBIT



## 2352 BOSQUE ESCONDIDO

BASIN NO. 1		DESCRIPTION	LOT 1
Area of basin flows =	9622	SF =	0.22 Ac.
The following calculati	ons are based on	Treatment %'s as shown in table to the rig	ght LAND TREATMENT
Ü		hted Excess Precipitation:	A = 0%
	Weighted E	•	B = 27%
	Sub-basin Volum		C = 28%
	V <sub>360</sub>	= 1188 CF	D = 45%
	V <sub>1440</sub>	= 1332 CF	
	Sub-basin Peak	Discharge Rate:	FIRST FLUSH VOL.
	QP360	= 0.8 cfs	123 CF
BASIN NO. 2	į.	DESCRIPTION	LOT 2
Area of basin flows =	8699	SF =	0.20 Ac.
The following calculation	ons are based on	Treatment %'s as shown in table to the rig	ght LAND TREATMENT
	Sub-basin Weig	nted Excess Precipitation:	A = 0%
	Weighted E	= 1.48 in.	B = 27%
	Sub-basin Volun	ne of Runoff:	C = 28%
	V <sub>360</sub>	= 1074 CF	D = 45%
	V <sub>1440</sub>	= 1204 CF	
	Sub-basin Peak	Discharge Rate:	FIRST FLUSH VOL.
	QP360	= 0.72 cfs	111 CF
BASIN NO. 3	1	DESCRIPTION	LOT 3
Area of basin flows =	24777	SF =	0.57 Ac.
The following calculation	ons are based on	Treatment %'s as shown in table to the rig	ght LAND TREATMENT
	Sub-basin Weig	nted Excess Precipitation:	A = 0%
	Weighted E	= 1.48 in.	B = 27%
	Sub-basin Volun		C = 28%
	V <sub>360</sub>	= 3058 CF	D = 45%
	V <sub>1440</sub>	= 3430 CF	
	Sub-basin Peak		FIRST FLUSH VOL.
	QP360	= 2.1 cfs	316 CF
BASIN NO. 4		DESCRIPTION	LOT 4
Area of basin flows =	10276		0.24 Ac.
The following calculati		Treatment %'s as shown in table to the rig	
		hted Excess Precipitation:	A = 0%
	Weighted E Sub-basin Volum		B = 27% C = 28%
		40.0	D = 45%
	V <sub>360</sub> V <sub>1440</sub>		D = 4570
	Sub-basin Peak		FIRST FLUSH VOL.
	QP360		TIKST TEOSIT VOL.
DACINI NO.	Q1 300		131 CF
BASIN NO. 5	•	= 0.9 cfs   DESCRIPTION	131 CF LOT 5
Area of basin flows =	20053	DESCRIPTION  SF =	131 CF LOT 5 0.46 Ac.
Area of basin flows =	20053	DESCRIPTION	LOT 5 0.46 Ac.
Area of basin flows =	20053 ons are based on	DESCRIPTION SF =	LOT 5 0.46 Ac.
Area of basin flows =	20053 ons are based on	DESCRIPTION  SF =  Treatment %'s as shown in table to the righted Excess Precipitation:	LOT 5  0.46 Ac.  LAND TREATMENT
Area of basin flows =	20053 ons are based on Sub-basin Weig	DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation: = 1.48 in.	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%
Area of basin flows =	20053 ons are based on Sub-basin Weig Weighted E	DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation: = 1.48 in.	LOT 5 $0.46  Ac.$ ght $A = 0\%$ $B = 27\%$
Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volun	DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation: = 1.48 in.  ne of Runoff:	LOT 5 $0.46  Ac.$ Explored Action (LAND TREATMENT) $A = 0\%$ $B = 27\%$ $C = 28\%$
Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volum V360	DESCRIPTION   SF	LOT 5 $0.46  Ac.$ Explored Action (Continuous Properties of the Continuous Properties Properties Properties of the Continuous Properties Pr
Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volun V360 V1440	DESCRIPTION   SF	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%
Area of basin flows =	20053 ons are based on Sub-basin Weig Weighted E Sub-basin Volun V1440 Sub-basin Peak	DESCRIPTION  SF =  Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.48 in.  ne of Runoff:  = 2475 CF  = 2776 CF  Discharge Rate:	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.
Area of basin flows = The following calculati	20053 ons are based on Sub-basin Weig Weighted E Sub-basin Volun V1440 Sub-basin Peak	DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation: = 1.48 in.  Description: = 2475 CF = 2776 CF  Discharge Rate: = 1.7 cfs	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.
Area of basin flows = The following calculation  BASIN NO. A Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volum V360 V1440 Sub-basin Peak QP360 2664 ons are based on Sub-based on Sub-base	DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.48 in.  De of Runoff:  = 2475 CF  = 2776 CF  Discharge Rate:  = 1.7 cfs  DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.48 in.  DESCRIPTION  SF = 1.7 cfs	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.  256 CF  0.06 Ac.  LAND TREATMENT
Area of basin flows = The following calculation  BASIN NO. A Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volum V1440 Sub-basin Peak QP360  2664 ons are based on Sub-basin Weighted E	DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.48 in.  The of Runoff:  = 2475 CF  = 2776 CF  Discharge Rate:  = 1.7 cfs  DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation:	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.  256 CF  0.06 Ac.  LAND TREATMENT  A = 0%
Area of basin flows = The following calculation  BASIN NO. A Area of basin flows =	20053 ons are based on Sub-basin Weig Weighted E Sub-basin Volun V1440 Sub-basin Peak QP360  2664 ons are based on Sub-basin Weig Weighted E	DESCRIPTION  SF =   Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.48 in.  The of Runoff:  = 2475 CF  = 2776 CF  Discharge Rate:  = 1.7 cfs  DESCRIPTION  SF =   Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.19 in.	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.  256 CF  0.06 Ac.  LAND TREATMENT  A = 0%  B = 40%
Area of basin flows = The following calculation  BASIN NO. A Area of basin flows =	20053 ons are based on Sub-basin Weig Weighted E Sub-basin Volun V360 V1440 Sub-basin Peak QP360 2664 ons are based on Sub-basin Weig Weighted E Sub-basin Volun	DESCRIPTION  SF = 1.48 in.  The of Runoff:  = 2475 CF  = 2776 CF  Discharge Rate:  = 1.7 cfs  DESCRIPTION  SF = 1.7 cfs  Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.19 in.  The of Runoff:	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.  256 CF  0.06 Ac.  LAND TREATMENT  A = 0%  B = 40%  C = 40%
Area of basin flows = The following calculation  BASIN NO. A Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volum V360 V1440 Sub-basin Peak QP360  2664 ons are based on Sub-basin Weighted E Sub-basin Volum V360	DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.48 in.  ne of Runoff:  = 2475 CF  = 2776 CF  Discharge Rate:  = 1.7 cfs  DESCRIPTION  SF = Treatment %'s as shown in table to the righted Excess Precipitation:  = 1.19 in.  ne of Runoff:  = 264 CF	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.  256 CF  0.06 Ac.  LAND TREATMENT  A = 0%  B = 40%
Area of basin flows = The following calculation  BASIN NO. A Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volum Sub-basin Peak QP360  2664 ons are based on Sub-basin Weighted E Sub-basin Volum V360 V1440 V360 V1440	DESCRIPTION   SF	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.  256 CF  0.06 Ac.  LAND TREATMENT  A = 0%  B = 40%  C = 40%  D = 20%
Area of basin flows = The following calculation  BASIN NO. A Area of basin flows =	20053 ons are based on Sub-basin Weighted E Sub-basin Volum V360 V1440 Sub-basin Peak QP360  2664 ons are based on Sub-basin Weighted E Sub-basin Volum V360	DESCRIPTION   SF	LOT 5  0.46 Ac.  LAND TREATMENT  A = 0%  B = 27%  C = 28%  D = 45%  FIRST FLUSH VOL.  256 CF  0.06 Ac.  LAND TREATMENT  A = 0%  B = 40%  C = 40%

## **APPENDIX B**

Excerpts from Drainage Report for Phase 1 of Montano Corridor from Rio Grande to Edith by Wilson & Co

#### DRAINAGE REPORT

Phase 1 of the Montano Corridor From Rio Grande Boulevard to Edith

Prepared By:

WILSON & COMPANY, ENGINEERS & ARCHITECTS



AUGUST 1985 (85-517) REVISED MARCH 1986

WILSON E COMPANY EHOINEERS

#### INTRODUCTION

This report outlines the storm water drainage plan for the Montano Corridor from Rio Grande Boulevard to 2nd Street (see Phase 1 on the Location Map). This corridor is in the North Valley where the existing drainage is sheet flow with no defined drainage paths. The major outlets for drainage through this area are the Alameda Drain, the Rio Grande and, to a limited extent, the riverside drains. There are existing storm drains at the 4th Street and 2nd Street intersections. West of 5th Street there are no storm drains with the rainfall runoff ponding in many low areas and sheet flow generally to the river. Several irrigation laterals and drains cross the proposed roadway corridor. The area is currently developed east of Guadalupe, primarily residential with scattered churches and commercial buildings at the 4th Street and 2nd Street intersections. A small shopping center has been constructed in the northeast quadrant of the 4th Street intersection. The area in this phase of the project west of Guadalupe is in the process of being developed with plans for residential, multi-family and commercial development.

The drainage area to the Montano corridor is primarily a strip adjacent to the roadway with a wide sheet flow area to the northeast of Rio Grande Boulevard approximately 3,000 feet (see Figure 3-5). The City's Master Drainage Plan shows an 18-inch storm drain from a low area south of Guadalupe Trail to the Griegos Drain. The proposed drainage plan consists of a storm drain from Rio Grande Boulevard to the Alameda Drain with a pumping station at Rio Grande Boulevard to discharge the storm runoff to the Rio Grande. A tee will be provided at the Guadalupe Trail to receive a sewer draining the low area south of Montano. Approximately 130 cfs will be discharged from the Alameda Drain to the proposed system during high flows in the drain (see attached Plans and Profiles).

#### HYDROLOGY & HYDRAULICS

The design criteria, hydrologic methods and hydraulic computations were in accordance with the City of Albuquerque "Development Process Manual," with Montano a minor arterial. Calculations for the project drainage are included in the Appendix and the results summarized in the following table.

#### DRAINAGE SUMMARY

Segment Sta. to Sta.	Area No(s).	Drainage Area (Ac.)	Time of Concen- tration (Min.)	Design Flow Without Alameda Drain Flow (cfs) (100-year)	Design Flow With Alameda Drain Flow of 130 cfs (peak) 100- Year (cfs)	Storm Drain Size (Dia. _in inches)
156+00 - 164+00	0	3.0(3)*:	* 39	7.7(7)	130	60 (-)
→136+00 ~ 156+00	0-1	14.0(3)	39	15.1(8)	130	72(24)
120+00 - 136+00	0-2	15.0(3)	39	24.9(10)	130	72(24)
105+00 - 120+00	0-3	22.0(3)	39	38.6(13)	130	72(30)
76+00 - 105+00	0-4	79.0(3)	39	43.6(18)	130	72(30)
*68+00 - 76+00	5	$\frac{7.5(3)}{140.5(18)}$	9	37.8(29)	130	72 (24)

\*Segment to be added in Phase 2 of project with the Rio Grande Boulevard overpass.

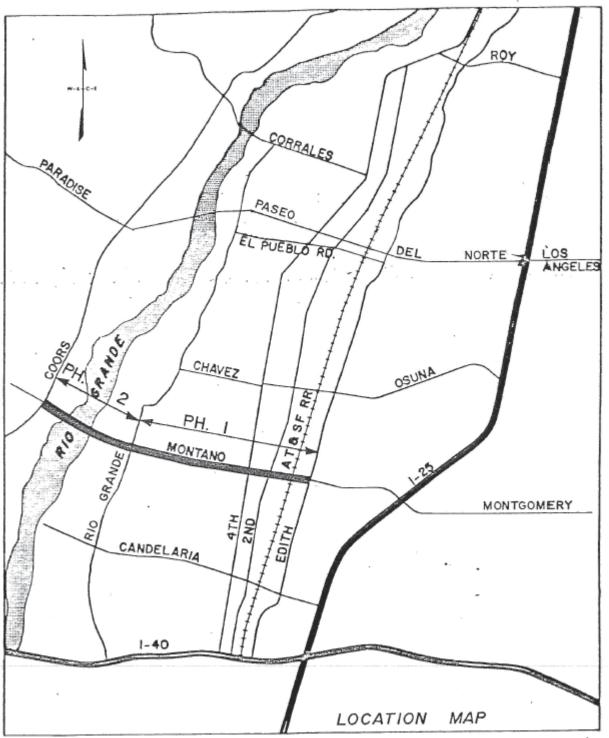
\*\*Numbers in parentheses reflect areas, flows and pipe sizes considering Roadway only.

Due to the "flat" grades and the sump conditions of the inlets, the 100-year design storm runoff maximum depth of 0.87 foot within the street right-of-way controlled the storm sewer pipe sizes.

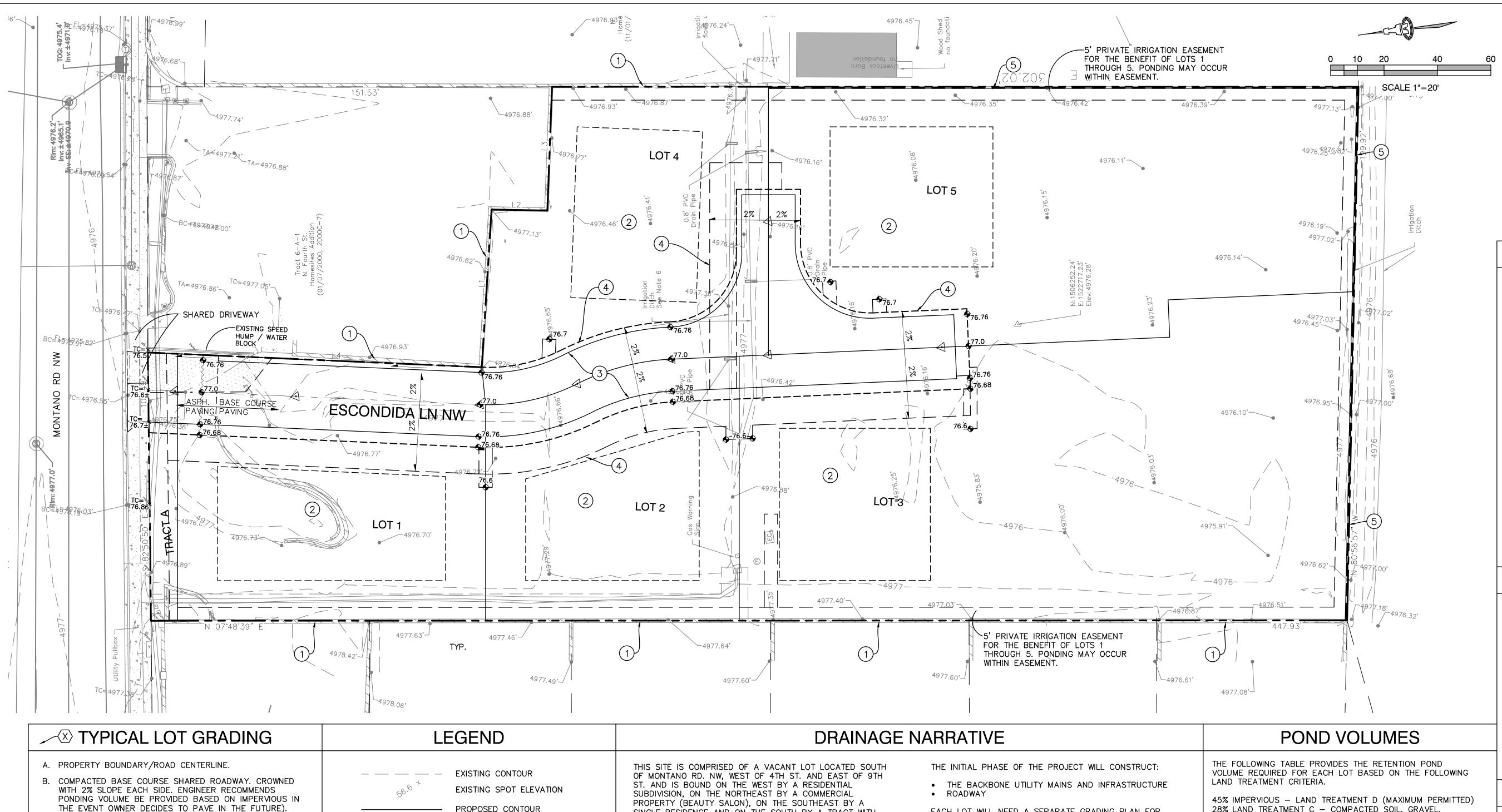
#### RECOMMENDED PLAN

The recommended plan is to provide a storm drain from Rio Grande Boulevard to the Alameda Drain with a pumping station to collect and pump the storm water from Rio Grande Boulevard to the Rio Grande. Approximately 130 cfs will be discharged from the Alameda Drain to the proposed system during high flows. The Montano grade will be "rolled" to obtain minimum grades with inlets constructed in the low points. (See the Plans and Profiles for details of the recommended plan.) The estimated construction cost for the storm drain system for Montano Corridor Phase 1 from Rio Grande to 2nd Street is \$2.96 million (see Appendix A).

Detention ponding to control the discharges was not feasible for this project due to: 1) lack of suitable right-of-way, 2) the lack of relief, and 3) the depressed roadway for Montano under Rio Grande Boulevard. Any alternate using detention ponding would require pumps to lift the storm water runoff to the pond. However, approximately five acre-feet of storage in the 72"/60" storm drain was used in the pump station hydraulics to reduce the peak flow for the pump station. The flow was reduced from 130 cfs to 95 cfs see Appendix E for calculations.







LOT #	LOT AREA (SF)	100YR 24HR VOL.	POND AREA (1.5' DEEP)
1	9,622	1,332 CF	888 SF
2	8,699	1,204 CF	803 SF
3	24,776	3,430 CF	2,287 SF
4	10,276	1,422 CF	948 SF
5	20,053	2,776 CF	1,851 SF

F PROPERTY OWNER DESIRES TO REDUCE POND DEPTH OR VOLUME BASED ON A DIFFERENT LAND TREATMENT RATIO, UPDATE CALCULATIONS AND SUBMIT WITH THE INDIVIDUAL LOT GRADING & DRAINAGE PLAN.

## WALL NOTES

4. TRANSITION AT MAX. 6:1 TO EXISTING GRADES (TYPICAL).

5. FUTURE PERIMETER WALLS REQUIRED CONCURRENT WITH THE DEVELOPMENT OF LOTS 3 AND 5. WALL SHALL CONTAIN THE 100-YEAR 24-HOUR DEVELOPED RUNOFF.

**ZONE ATLAS F-14-Z** 

PROJECT DATA

FLOOD ZONE: BASED UPON SCALING, THIS PROPERTY LIES

NO. 35001C0119G.

ÅSA NILSSON-WEBER

ISAACSON & ARFMAN, P.A.

PHONE: (505) 268-8828

CARTESIAN SURVEYS INC.

PHONE: (505) 896-3050

N=1520286.421\*, E=1505417.495\*, G-G=0.999682452, DA=-0013'52.53" ELEVATION, IN FEET (NAVD 1988) =

**KEYED NOTES** 

4975.078' (US SURVEY FEET)

2. APPROXIMATE LOCATION OF FUTURE BUILDING PAD (BY OTHERS). SEE TYPICAL LOT GRADING PLAN AND NOTES FOR INDIVIDUAL LOT GRADING/DRAINAGE REQUIREMENTS.

RETENTION PONDS ON EACH LOT MUST RETAIN THE

3. GRADE CROWNED ROAD AT 2% CROSS SLOPE. EXTEND

TO BE 6" COMPACTED AGGREGATE BASE COURSE.

NOTES AND POND VOLUME TABLE THIS SHEET.

100-YEAR 24-HOUR STORM. SEE POND CONSTRUCTION

GRADING ACROSS ALL UTILITY EASEMENTS. NOTE: ROAD

BENCHMARK: ACS MONUMENT "DOUGLAS" NAD 1983

1. EXISTING PERIMETER WALL TO REMAIN.

128 MONROE ST NE, ABQ. NM 87108

P.O. BOX 44414, RIO RANCHO, N.M. 87174

TRACT 6-A-2, FOURTH STREET

HOMESITES ADDITION

WITHIN SHADED FLOOD ZONE X WHICH IS DEFINED AS AN AREA WITH REDUCED FLOOD RISK DUE TO LEVEE. AS DETERMINED BY F.E.M.A. AND SHOWN ON FLOOD INSURANCE RATE MAP DATE SEPTEMBER 26, 2008, MAP

**LEGAL DESCRIPTION:** 

ENGINEER:

(CENTRAL ZONE)

SITE AREA: 1.7468 ACRES

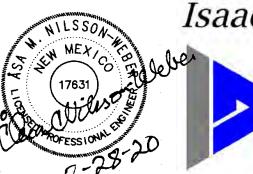
SURVEYOR: WILL PLOTNER

A. PERIMETER WALLS SHALL REMAIN.

B. FUTURE RETAINING WALL DESIGN(S) SHALL BE BY STRUCTURAL

## KEYED EASEMENT NOTES

A. DRAINAGE EASEMENT GRANTED TO THE CITY OF ALBUQUERQUE BY FINAL PLAT.



Isaacson & Arfman, Inc. Civil Engineering Consultants

> 128 Monroe Street NE Albuquerque, NM 87108 505-268-8828 | www.iacivil.com

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**BOSQUE ESCONDIDO** 

MONTANO FAMILY HOMES, LLC

GRADING & DRAINAGE PLAN

te:	No.	Revision:	Date:	Job No.
01/2020				2352
awn By:				00.404
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# PROPOSED CONTOUR

**59.20** 

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4978.6

C. SIDEWALK (DEFERRED). NO PONDING SHALL OCCUR WITHIN

ROADWAY AND UTILITY EASEMENTS.

E. DRIVE CONNECTION TO PAD. SLOPE TO DRAIN

F. AVAILABLE POND AREA OUTSIDE OF ALL EASEMENTS

G. EXISTING PERIMETER WALL. MAX. 3:1 TRANSITION FROM

H. SIDE YARD WALLS SHALL BE CONSTRUCTED TO CONTAIN

(2 PER LOT) TO IMPROVE INFILTRATION OF PONDED

STORMWATER. LOCATE MIN. 10' FROM BUILDING. SEE

K. MAXIMUM WATER SURFACE ELEVATION = 4976.5;

N. RUNOFF FROM LOT AND  $\frac{1}{2}$  ADJACENT ROADWAY IS

L. MAXIMUM DEPTH = 18"; REQUIRED POND VOLUME IS

M. DRAINAGE COVENANT FOR EACH POND IS REQUIRED TO

PONDING VOLUMES AT A TOP OF WALL=76.5 MIN. WALL

ENGINEER RECOMMENDS PERCOLATION PITS BE INSTALLED

J. OWNER'S OPTION: TRANSITIONS AND EXTENTS OF PONDING

BASED ON 100-YEAR 24-HOUR WSEL = 4976.5. ALL SITE

PONDS MUST BE ABLE TO EQUILIZE WITHIN THE PROPERTY.

D. PAD GRADE = 4977.7 (MINIMUM).

WALL TO BOTTOM OF POND.

DESIGN BY OTHERS.

DETAIL SHEET CG-501.

ENSURE COMPLIANCE.

CAPTURED WITHIN LOT PÕNDING.

ACHIEVED.

MAY VARY AS LONG AS:

PROPOSED SPOT ELEVATION

PROPOSED PAD GRADE

107

F

-4977-/-/-

CALCULATIONS: • THE STREET MUST DRAIN INTO THE ADJACENT LOTS. • THE 100-YEAR 10-DAY WATER SURFACE ELEVATION

HOMES.

WILL BE AT OR BELOW 4976.67 WHICH WOULD EQUAL THE EDGE OF (DEFERRED) SIDEWALK. • THE 100-YEAR 24-HOUR STORM (DEVELOPED CONDITION) WILL BE AT OR BELOW 4976.5.

SINGLE RESIDENCE AND ON THE SOUTH BY A TRACT WITH

PRIVATE DEVELOPMENT WITH 5 DETACHED RESIDENTIAL

SCHEME PER DPM CHAPTER 22, SECTION 5.G. PER

AGRICULTURAL USE. THE SITE WILL BE RE-DEVELOPED AS A

THE SUBDIVISION WILL BE DEVELOPED WITH A FLAT GRADING

THE PRIVATE COMPACTED BASE COURSE ROADWAY HIGH POINT WILL BE 4" ABOVE THE 100-YEAR 10-DAY STORMWATER SURFACE ELEVATION (4976.67 + 0.33 = 4977 = HIGH POINT.

 A PERMANENT PERIMETER WALL AROUND THE DEVELOPMENT WILL CONTAIN THE 100-YEAR 24 HOUR DEVELOPED RUNOFF.

EACH LOT WILL NEED A SEPARATE GRADING PLAN FOR BUILDING PERMIT APPROVAL.

THE CONTRIBUTING AREA MAY NOT BE GREATER THAN

PATIOS, ETC.) EACH LOT MUST PROVIDE RETENTION PONDING AROUND THE PERIMETER OF EACH RESIDENCE (OUTSIDE OF UTILITY EASEMENTS) TO STORE THE 100-YEAR 24 HOUR

STORM VOLUME FOR THE FULLY DEVELOPED LOT. WALLS MUST BE CONSTRUCTED ALONG THE LOT LINES TO CONTAIN THE REQUIRED VOLUME WITHIN THE

• THE PAD ELEVATION SHALL BE A MINIMUM OF ONE (1) FOOT ABOVE THE 100-YEAR 10-DAY STORMWATER SURFACE ELEVATION. THEREFORE, ALL PAD ELEVATIONS

 THE FLOW BETWEEN THE FRONT YARD AND BACK YARD CANNOT BE OBSTRUCTED. STORMWATER MUST BE ALLOWED TO EQUALIZE TO THE SAME LEVEL BETWEEN THE FRONT AND BACK YARDS (USING WALL OPENINGS, SWALES, EQUILIZER PIPES, ETC.)

## THE LOTS WILL BE DEVELOPED INDIVIDUALLY WITH A FLAT

RADING	SC	HEME	PER	DPM	CHAP	IFK	22,	2FC	HON	5.6	
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# 45% (STREET AREA, DRIVEPADS, WALKS, ROOF AREA,

ASSOCIATED LOT LIMITS.

MUST BE AT ELEVATION 4977.7 OR GREATER.

A SMALL BASIN AT THE NORTH END OF THE SITE WILL DRAIN 0.2 CFS TO MONTANO RD.

45%	IMPER	VIOUS -	– LAND	) TI	REATMEN	IT D	(MAXI	MUM	PERMI	ITED
28%	LAND	TREATM	MENT C	_	COMPAC	CTED	SOIL,	GRA'	VEL.	
27%	LAND	TREATM	MENT B	_	LAWNS,	NATI	IVE ĞF	RASSE	ES.	