

## SO 19 SHEET KEY NOTES NOTICE TO CONTRACTOR AN EXCAVATION/ CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY INSTALL 1-24" SIDEWALK CULVERT PER STD DWG 2236 WORK WITHIN CITY RIGHT-OF-WAY. PROJECT LOCATION 2 INSTALL/1-24" CONCRETE RUNDOWN SEE SHEET, C-502 2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE INSTALL 1 PRIVATE ENTRANCE WITH PCC VALLEY GUTTER STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY WHEELCHAIR RAMP PER STD DWG 2426 OF ALBUQUERQUE INTERIM STANDARD SPECIFICATIONS FOR PUBLIC WORKS SEE SHEET 4 4 TRUNCATED DOMES AT WHEELCHAIR RAMPS O WATER SEE SHEET C-504 VALVE 3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE 5 12"/CROSSWALK STRIPING LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES. INSTALL 20' WIDE PCC DRIVE PAD PER COA STD DWG 2425 INSTALL CONCRETE VALLEY GUTTER PER COA STD DWG 2425 4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE FL71.99 INSTALL CONCRETE VALLEY GUTTER PER COA STD DWG 2421 HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONSTRUCTIONS. SHOULD A CONFLICT FL72.74 EXIST. THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN72 65 END STD/ CURB & GUTTER TRANSITION TO MEDIAN CURB & GUTTER **WATER** BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY. OVER THE LÉNGTH OF ONE STONE BLOCK 10/ DEPRESSED MEDIAN CURB & GUTTER TO MATCH TOP OF ASPHALT TC 73.62 5. BACKFILL COMPACTION SHALL BE ACCORDING TO \_\_\_\_ \_ STREET FL: 7 ELEVATION. : 73.05 : 72.**6671.83-**FL 72.95 25' CROWN TRANSITION PER COA STD/DWG 2401 6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER LOCATION MAP OF THE PROPERTY SERVED. GRAPHIC SCALE ZONE ATLAS MAP NO. F-15 WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS. FL72.74 NORTH 2nd STREET DATE APPROVALS WILSON **BUSINESS CENTER** -TC73.62 &COMPANY HYDROLOGY /( IN FEET ) FL72.95 rTC74.42 1/inch = 20 ft. FL73.75 F-15-Z FL73.45 WCEA # 06600090 A.C.E./FIELD FL73.63 SEE KEY NOTE TC 73.50 GRADING NOTES <sub>C</sub>TS73.91 FL 73.00 TC73.82 -CONSTRUCTION ACTIVITY SHALL BE LIMITED TO THE PROPERTY AND/OR FL73.32 PROJECT LIMITS. ANY DAMAGE TO ADJACENT PROPERTIES RESULTING SEE KEY NOTE 9 FROM THE CONSTRUCTION PROCESS IS THE RESPONSIBILITY OF THE \_TS74.09 CONTRACTOR. ANY COSTS INCURRED FOR REPAIRS SHALL BE THE COST OF THE CONTRACTOR. FL73.45 SEE KEY NOTE & ROJECT LOCATION EXCEPT AS PROVIDED HEREIN, GRADING SHALL BE PERFORMED TO THE ELEVATIONS, AND IN ACCORDANCE WITH THE TYPICAL SECTIONS SHOWN ON THIS PLAN. -TS74.09 **£**, · 3. THIS PLAN IS FOR THE EXCAVATION, PLACEMENT AND COMPACTION OF NATURAL EARTH MATERIALS AND THE CONSTRUCTION OF REQUIRED LTS73.91 RETAINING WALLS ONLY. /FL73.72 -TC74.25 ~TC74.42 4. PAVING AND ROADWAY GRADES SHALL BE+.05 FOOT FROM SHOWN PLAN FL73.75 FL73.92 ELEVATIONS. SOILS MAP 5. PADS SHALL NOT VARY FROM A TRUE HORIZONTAL PLANE BY MORE TH ROOF FLOW +0.1 FOOT AT ANY POINT. THIS TRUE PLANE SHALL NOT VARY FROM REFERENCE: SCS BERNALILLO COUNTY SOIL SURVEY THE SHOWN PAD ELEVATION BY +0.2 FOOT, UNLESS PERMITTED BY OWNER. SHEET NO. 21 6. CONTRACTOR SHALL ABIDE BY ALL LOCAL, STATE, AND FEDERAL REGULA-TION WHICH APPLY TO THE CONSTRUCTION OF THESE IMPROVEMENTS AND TS 73.67 5124 2nd ST NW FL72.68 GRADING OPERATIONS. SEE\_KEY NOTE 9 FL73.85 FINISH FLOOR=4974.25 THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL CONSTRUC-TION PERMITS AND INSPECTION APPROVALS NECESSARY FOR THE CON-SEE KEY STRUCTION OF THESE FACILITIES AND ALL GRADING OPERATIONS. 8. THE COST FOR REQUIRED CONSTRUCTION DUST AND EROSION CONTROL MEASURES SHALL BE INCIDENTAL TO THE PROJECT COST. 9. ALL RIM ELEVATIONS FOR SANITARY SEWER AND STORM DRAIN MANHOLES TC: 72.76 Fb: 72.34 1.00= 20.13 AC 4.5/A (7) FL72.28 BASIN 2015 AC AREA = 6.12 AC Q100 = 26.90 AC CFS -TC74.15 FL73.65 -TC73.03 FL72.36 10. BOULDERS, GREATER THAN 3' IN DIAMETER, EXCAVATED DURING GRADING ACTIVITIES SHALL BE STOCKPILED AND DISPOSED OF AT THE DISCRETION LTC73.57 FL73.07 OF THE OWNER. 11. UNLESS OTHERWISE SHOWN, DRAINAGE SWALES SHALL HAVE A MINIMUM & PRIVATE ACCESS EASEMENT 1% SLOPE IN THE DIRECTION OF FLOW. -TC73.28 FL72.61 NORTH 2<sup>ND</sup> STREET BUSINESS CENTER FL72.53 OUNTY TC73.07 FL72.40 -TC73.42 FL72.75 Master Grading & Drainage FLOOD INSURANCE MAP Drainage Report LFL72.92 REFERENCE: FLOOD INSURANCE STUDY PANEL 35001C0119E Site Location: The North 2nd Street Business Center is located at 5124 2nd Street NW just 72.10 south of the intersection of Montano Road and 2<sup>nd</sup> street. The proposed development will TC FL TC FL comprise of new buildings with site work improvements. Improvements include a parking lot, curb LEGAL DESCRIPTION 72.66 71.99 - 71.91 - 72.32 & gutter, paving, and sidewalk. ~TP73.41 2 73.08 72.41 A TRACT OF LAND SITUATE IN PROJECTED SECTION 33, TOWNSHIP 11 Methodology: Section 22.2 of City of Albuquerque DPM was followed to calculate the design NORTH, RANGE 3 EAST, NEW MEXICO PRINCIPAL MERIDIAN, BERNALILLO 3 72.50 71.83 3 73.17 72.50 volume. The charts and formulas in Part A were followed using the 100-year frequency 6-hour COUNTY, NEW MEXICO, COMPRISED OF PARCEL 2 TRACT 127B, MRGCD $\Delta = 62'31'02'$ $\Delta = 8518'29"$ rainfall as the design storm. The site is located in Zone 2 as determined from Table A-1. The MAP 32; PARCEL 3, TRACT 128B, MRGCD MAP32, PARCEL 4, TRACTS L = 44.67'L = 27.28'total storm volume was calculated as per section A.5. The peak discharge was calculated as per 129 B-2, 131 AND 131A, MRGCD MAP 32, AND A PORTION OF THE R = 25.00'R = 30.00'section A.6. VACATED GALLEGOS LATERAL (FILED AUGUST 29, 1975, BOOK MISC. 437, -TC73.84 Existing Conditions: The drainage from the site flows in an east to west direction toward 2<sup>rd</sup> PP. 740, 741). LEGEND FL73.12 Street. Basin 101 encompasses the entire site. The subject property is bounded on the north with BENCH MARK CMU wall, east by a building and fencing, and south by chainlink fence. 2<sup>nd</sup> street bounds the ACS STATION 8-F15 1 34" ALUMINUM DISK EPOXIED west side of the subject area. Off-site flows are cut off by existing fencing or natural ground ridges FLOWLINE WATER VALVE TO THE TOP OF A CONCRETE BASE OF A along the existing property lines and therefore do not enter the site. METAL FENCE WATER METER GALVANIZED STEEL ELECTRIC TRANSMISSION LINE Existing volumetric runoff and peak discharge quantities are as shown below: POLE, APPROXIMATELY 10' NORTH OF THE FIRE HYDRANT CHAIN LINK FENCE NORTHWEST OF THIS PROPERTY. ELEVATION Table 1 – Existing Conditions SANITARY SEWER MANHOLE WIRE OR WOOD FENCE 4973.196 (NGVD 1988) Treatment $Q = 0.375 \left( \frac{1}{nS} \right) d^{8/3} S_{C}^{1/2} =$ Basin Area (ac) A (%) B (%) C (%) D (%) V<sub>360</sub>(ac-ft) Q<sub>p</sub>(cfs) WATER METER CONCRETE BLOCK WALL CITY OF ALBUQUERQUE 101 6.12 0 0 90 10 0.627 20.17 POWER POLE PUBLIC WORKS DEPARTMENT 0.627 20.17 EXISTING CONTOURS © 1 FT. INTERVALS Table 1 - Provides a breakdown of existing volumetric runoff and peak discharge **ENGINEERING GROUP** So=6.25% ANCHOR ANCH INDEX CONTOURS • 5 FT. INTERVALS of the site. Sr=0.5% NORTH 2ND STREET LIGHT POLE WILSON **BUSINESS CENTER** Proposed Conditions: The proposed improvements maintained the same flow direction as the STEEL TRANSMISSION LINE PYLON PROPOSED 3" ASPHALT &COMPANY existing. The proposed basin remained the same as the existing. Basin 201 will continue to drain 4900 LANG AVE CABLE TELEVISION RISER into Second Street. Volumetric run-off will primary surface flow onto the private access road along Sx=0.2% & DRAINAGE GRADING ALBUQUERQUE, NEW MEXICO the south property line and discharge onto 2<sup>nd</sup> street. The private access road street capacity is BASIN (01) EXISTING BASIN DESIGNATION 87110 **(505) 348–4000** ---- OVERHEAD POWER LINE approximately 52.59 cfs. Calculations are provided on figure 1. A portion of the surface flow will be conveyed to 2<sup>nd</sup> street via the north driveway entrance. The north driveway access road is (505) 348-4072 Fax PLAN PROPOSED BASIN DESIGNATION approximately 46.46 cfs. Calculations are provided in figure 1. The proposed runoff from the www.wilsonco.com SPOT ELEVATION entire site is 26.86 which is well below the capacities of both the access road and north driveway. UNLESS OTHERWISE INDICATED, SPOT NO. DATE REMARKS ELEVATIONS ARE TO NATURAL GRADE EXISTING BASIN BOUNDARY Proposed volumetric runoff and peak discharge quantities are as shown below: EDGE OF CONCRETE Table 2 - Proposed Conditions PROPOSED BASIN BOUNDARY Figure 1.- Private Access Road Capacity TOP OF CURB 16205) ROOF DRAIN FLOW DIRECTION Basin Area (ac) A (%) B (%) C (%) D (%) V<sub>360</sub>(ac-ft) Q<sub>100</sub>(cfs) MJI WCEA NO. 06600090 DATE APR 2007 DESIGN 201 6.12 10 0 0 90 1.001 26.86 Conclusion: In conclusion, comparing existing conditions to proposed development, there will be an increase of approximately 6.69 cfs or 33% more drainage into Second Street. The increase in SWALE/DIRECTION OF SURFACE FLOW STAFF PROJECT NO. SHEET NO. Total 6.12 10 90 1.001 26.86 ORAWN peak discharge will not significantly impact the flow into Second Street. Equally important, street C - 103Table 2 - Provides a breakdown of proposed volumetric runoff and peak discharge capacities are adequate for storm run-off conveyance. PROPERTY LINE of the site.

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