NOTICE TO CONTRACTOR

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2. All work detailed on these plans to be performed, except as otherwise stated or provided hereon, shall be constructed in accordance with "City of Albuquerque Standard Specifications for Public Works Construction" (1986 Edition).
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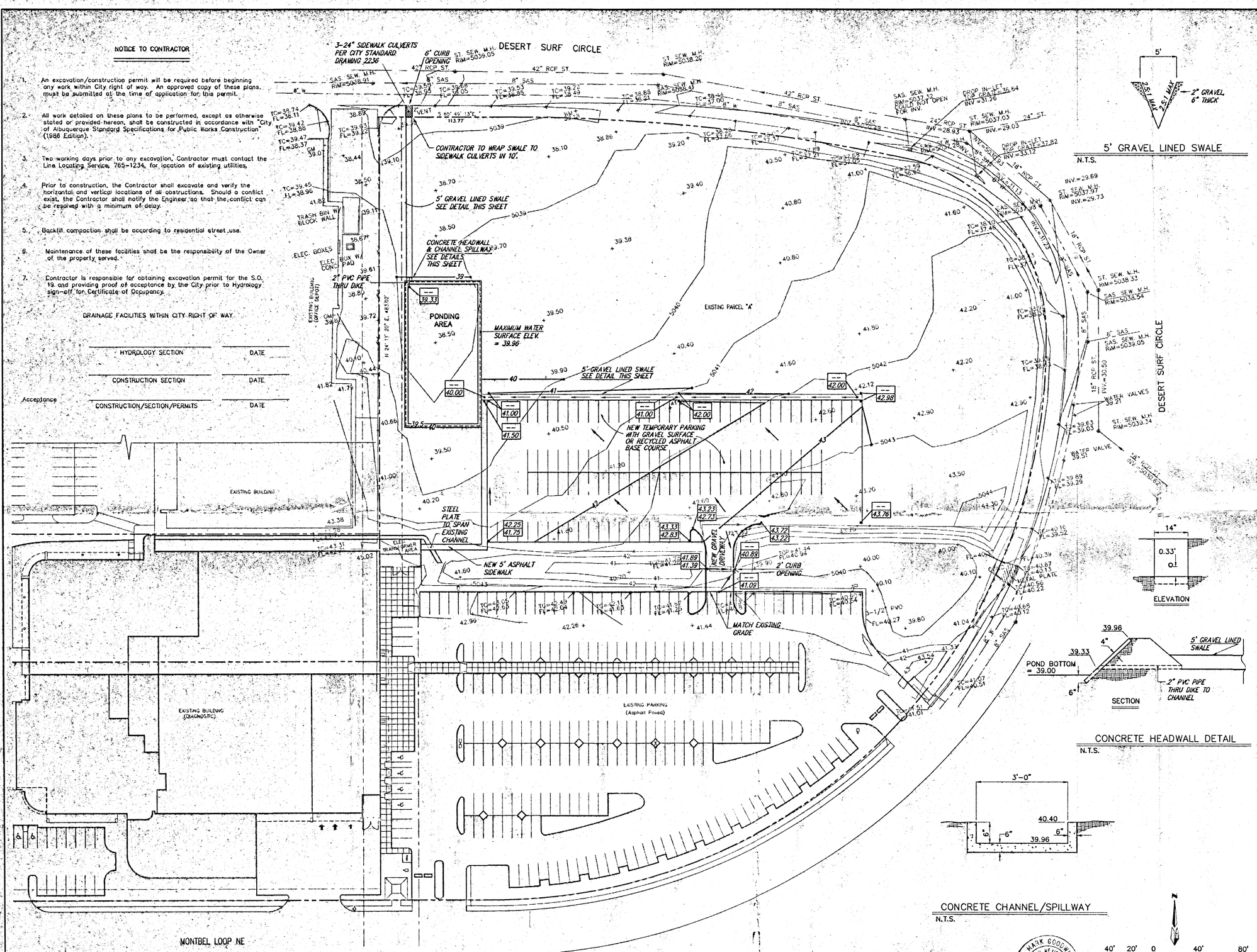
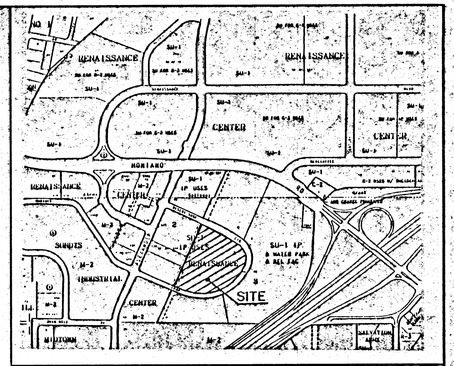
DRAINAGE FACILITIES WITHIN CITY RIGHT OF WAY

HYDROLOGY SECTION	DATE
CONSTRUCTION SECTION	DATE
Acceptance CONSTRUCTION/SECTION/PERMITS	DATE



5' GRAVEL LINED SWALE
N.T.S.



VICINITY MAP ZONE MAP: F-16

DRAINAGE CALCULATIONS

A	= 0.9556 Ac.	$Q_{allowable}$	= 0.10 cfs
$P_{(100-6)}$	= 2.35 in.	$VOLUME_{Required}$	= 3,929 Cu.Ft.
$P_{(100-24)}$	= 2.75 in.	$VOLUME_{Provided}$	= 4,295 Cu.Ft.
DT	= 0.03333 Hr.		
T_p	= 0.1333 Hr.		
Q	= 3.03 cfs		
V	= 0.0902 Ac.Ft.		

BENCHMARK

THE STATION MARK IS A STANDARD NM SHC BRASS TABLET STAMPED "STA. I-25-20", SET IN TOP OF A CONCRETE POST PROJECTING 0.1 FEET ABOVE GROUND.

X = 390,403.88
Y = 1,502,304.95
Z = 5079.46

LEGAL DESCRIPTION

PARCELS "E" AND "A", TRACT 12-B, RENAISSANCE CENTER II

NOTES

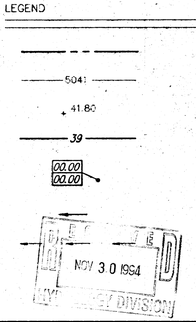
1. SITE DOES NOT LIE WITHIN A FLOOD ZONE.
2. ONE SITE CONSTRUCTION IS COMPLETE, THE OWNER SHALL ARRANGE AND BE RESPONSIBLE FOR PERIODIC MAINTENANCE OF ALL ON-SITE ORIFICE-CONTROLLED DEVICES.

LEGEND

———	PROPERTY LINE
5041	EXISTING CONTOUR
+ 41.80	EXISTING SPOT ELEVATION
39	PROPOSED CONTOUR
00.00 / 00.00	PROPOSED SPOT ELEVATION
	FLOWLINE
	SWALE

CONCRETE HEADWALL DETAIL
N.T.S.

CONCRETE CHANNEL/SPILLWAY
N.T.S.

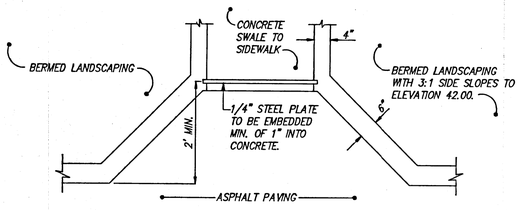


DIAGNOSTEK PARKING EXPANSION
GRADING & DRAINAGE PLAN

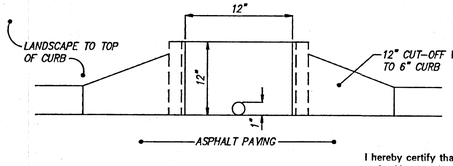
D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS & SURVEYORS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505) 345-2010

Designed: RM	Drawn By: DLD	Checked: DMG	Sheet 1 of 1	
Scale: AS SHOWN	Date: 11/94	Job: 94146		

DIAGNO/GD40/11/28/94



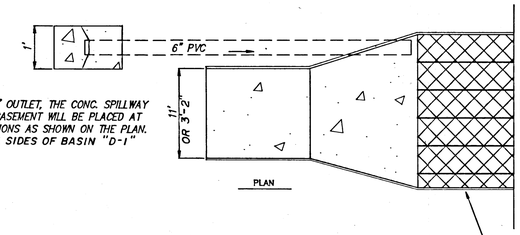
PLAN



SECTION

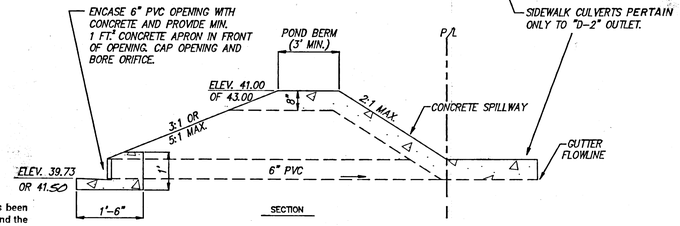
BASIN "C" OUTLET DETAIL

N.T.S.



FOR BASIN "D-1" OUTLET, THE CONC. SPILLWAY AND ORIFICE ENCASEMENT WILL BE PLACED AT SEPERATE LOCATIONS AS SHOWN ON THE PLAN. DO NOT FLARE SIDES OF BASIN "D-1" SPILLWAY.

PLAN



SECTION

SIDEWALK CULVERTS PERTAIN ONLY TO "D-2" OUTLET.

BASIN "D-1" & "D-2" OUTLET DETAIL

N.T.S.

RECORD DRAWING

I hereby certify that the information contained on this drawing has been revised in accordance with information furnished by the Contractor and the Surveyor, The Jaynes Corporation, and reflects the construction as actually accomplished. This plan as constructed is in substantial compliance with the Approved Plan.

John M. MacKenzie 11/15/94

John M. MacKenzie NMPE#11619

NOTICE TO CONTRACTOR

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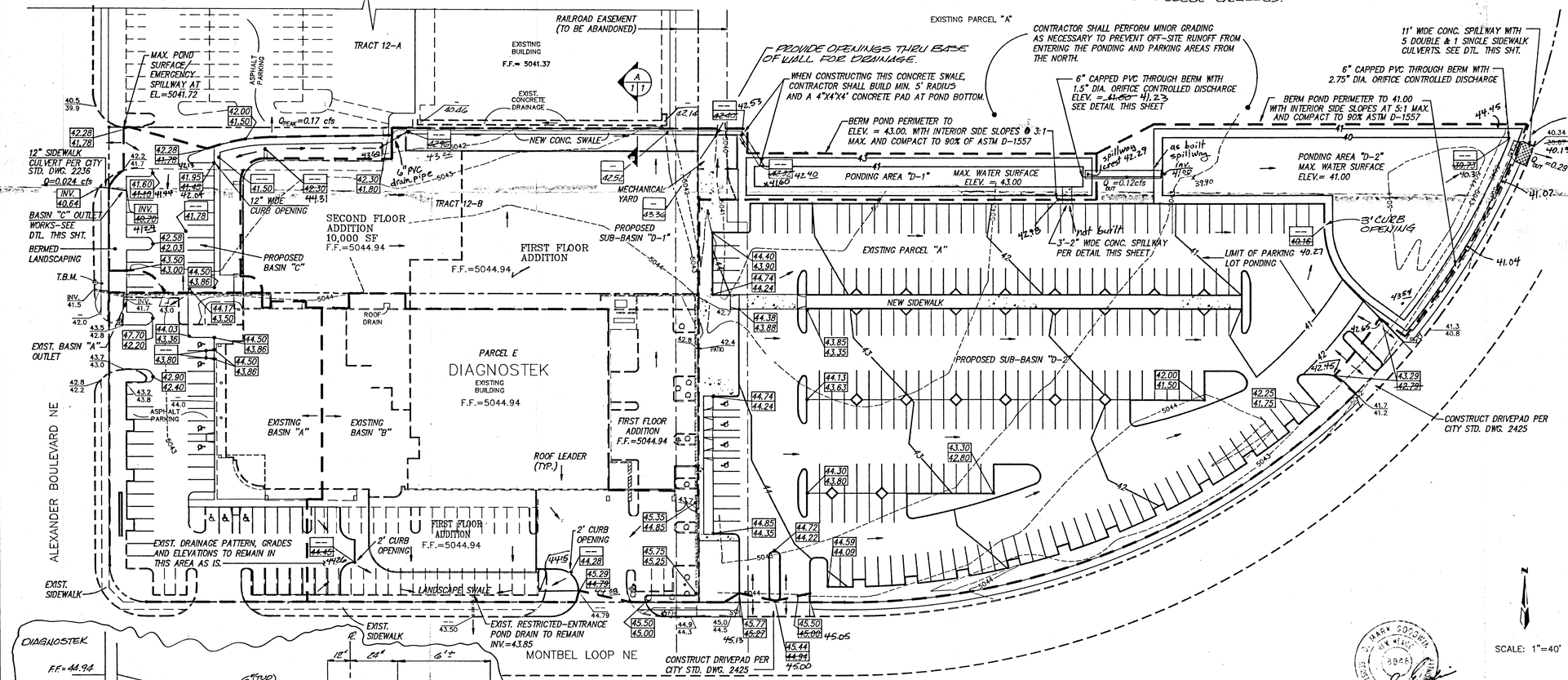
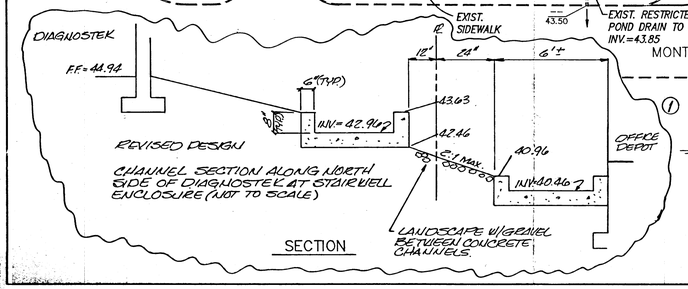
DRAINAGE FACILITIES WITHIN CITY RIGHT OF WAY

Design Approval	HYDROLOGY SECTION	DATE
Inspection Approval	CONSTRUCTION SECTION	DATE
Acceptance	CONSTRUCTION/SECTION/PERMITS	DATE

LANDSCAPING IN PONDING AREAS TO CONSIST OF FESCUE GRASSES.



SCALE: 1"=40'



SECTION

1

DRAINAGE CALCULATIONS

* BASIN A – EXISTING CONDITIONS

A= 0.93 Ac.
P(100-6)= 2.2 in. hr.
$Q_{ALLOWABLE}$ = 0.102 cfs, PEAK
NET RUNOFF VOLUME= 3268 CU.FT.

* BASIN B – EXISTING CONDITIONS

A=1.02 Ac.
P(100-6)= 2.22 in. hr.
$Q_{ALLOWABLE}$ = 0.093 cfs
NET RUNOFF VOLUME= 3268 CU.FT.

* THIS INFORMATION WAS OBTAINED FROM THE SOURCE REFERENCED IN NOTE 2 AND WAS NOT CALCULATED NOR VERIFIED.

BASIN "C" FUTURE CONDITION

A= 0.238 Ac.
P(100-6)= 2.25 in.
P(100-24)= 2.58 in.
DT= 0.05 hr.
T_P = 0.1333 hr.
$Q_{ALLOWABLE}$ = 0.024 cfs, PEAK
VOLUME REQUIRED= 1889 CU.FT.
VOLUME PROVIDED= 1928 CU.FT.

BASIN "D" FUTURE CONDITIONS

SUB-BASIN "D-1"

A= 1.164 Ac.
P(100-6)= 2.25 in.
P(100-24)= 2.58 in.
DT= 0.05 hr.
T_P = 0.1333 hr.
$Q_{ALLOWABLE}$ = 0.12 cfs
VOLUME REQUIRED= 9,235 CU.FT.
VOLUME PROVIDED= 9,312 CU.FT.

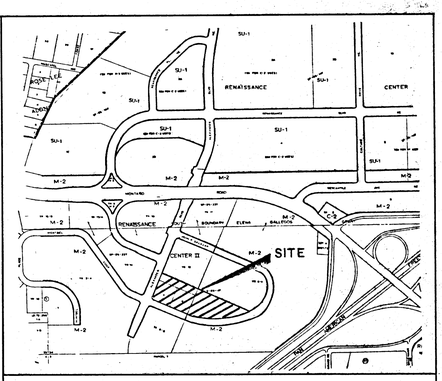
SUB-BASIN "D-2"

A= 2.903 Ac.
P(100-6)= 2.25 in.
P(100-24)= 2.58 in.
DT= 0.05 hr.
T_P = 0.1333 hr.
$Q_{ALLOWABLE}$ = 0.29 cfs
VOLUME REQUIRED= 23,035 CU.FT.
VOLUME PROVIDED= 23,072 CU.FT.

D. MARK GOODWIN NEW MEXICO 9946 REGISTERED PROFESSIONAL ENGINEER

5/19/93
6/10/93

DIA\GD40



VICINITY MAP ZONE MAP: F-16-Z

BENCHMARK

THE STATION MARK IS A STANDARD NM SHC BRASS TABLET STAMPED "STA. I-25-20", SET IN TOP OF A CONCRETE POST PROJECTING 0.1 ft. ABOVE GROUND.
X=390,403.88, Y=1,502,304.95, Z=5079.46

LEGAL DESCRIPTION

PARCELS E & A, TRACT 12-B RENAISSANCE CENTER II

NOTES

1. SITE DOES NOT LIE WITHIN A FLOOD ZONE.
2. FOR DETAILS REGARDING THE AS-BUILT PONDING CONDITIONS AT THIS SITE, PLEASE REFER TO THE ORIGINAL GRADING AND DRAINAGE PLAN, SHEETS C-1 & C-2, SUBMITTED FOR THE INITIAL DIAGNOSTEK BUILDING CONSTRUCTION BY HALL ENGINEERING, INC. DATED 10/14/88.
3. ONCE SITE CONSTRUCTION IS COMPLETE, THE OWNER SHALL ARRANGE AND BE RESPONSIBLE FOR PERIODIC MAINTENANCE OF ALL ON-SITE ORIFICE-CONTROLLED DEVICES.

LEGEND

= = = = = =	EXISTING CONCRETE CURB
———	PROPERTY LINE
69	CONTOUR
69 TC / 69 FL	EXIST. SPOT ELEVATION
FF=	FINISHED FLOOR
101.61 TC FL	PROPOSED SPOT ELEVATION
→	FLOW DIRECTION
"C" / "B"	DRAINAGE BASIN

NOV 15 1994 HYDROLOGY DIVISION

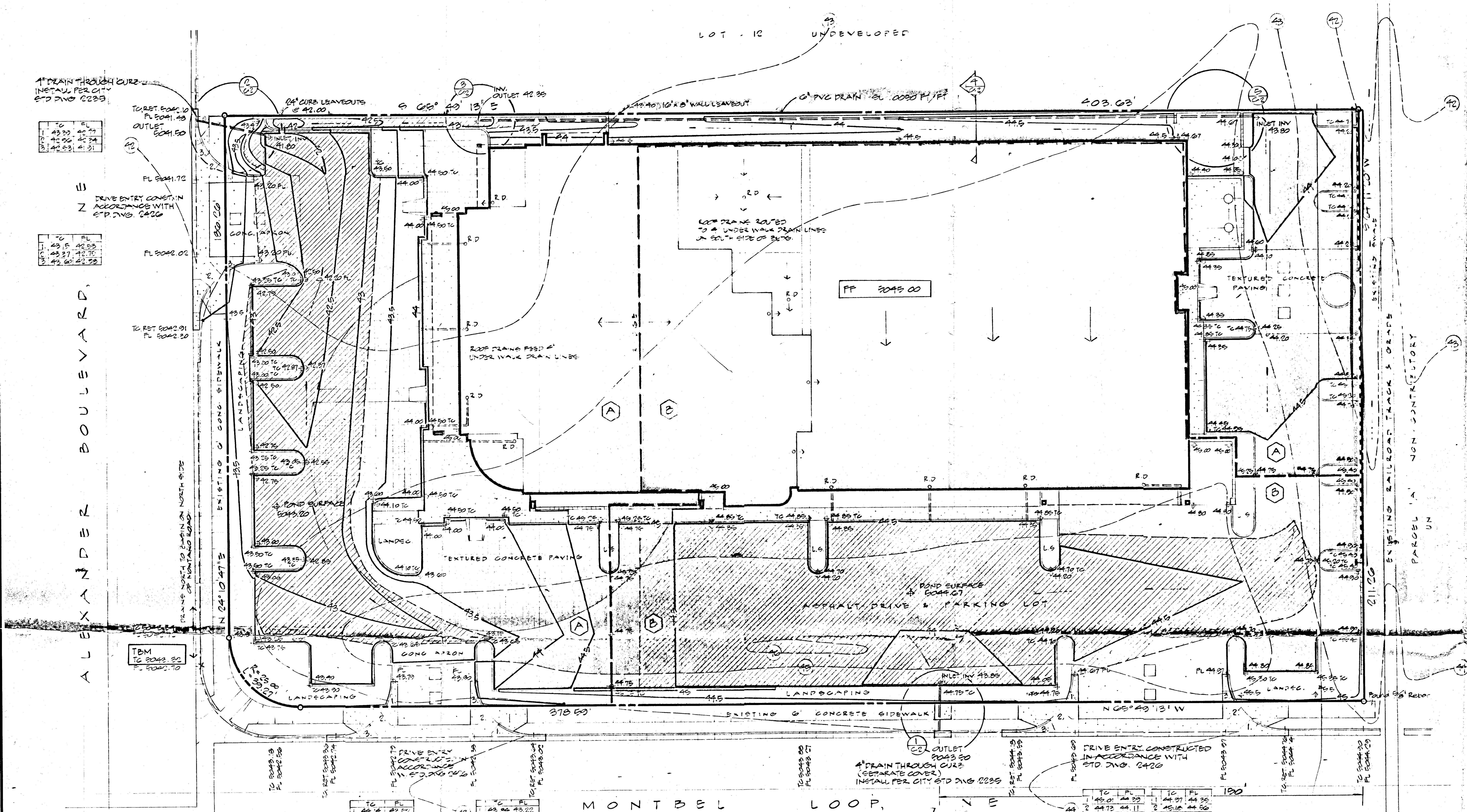
1 REVISED FOR SWALE DETAIL 7/21/93

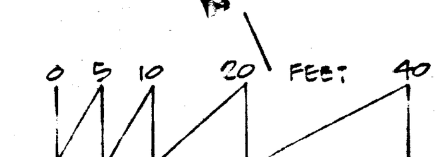
DIAGNOSTEK EXPANSION

GRADING & DRAINAGE

D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS & SURVEYORS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505) 828-2200

Designed: JMM	Drawn: JMB	Checked: DMG	Sheet C1 of 1
Scale: 1"=40'	Date: 7/92	Job: 9250	





THIS PLAN IS INTENDED FOR THE ESTABLISHMENT OF VERTICAL CONTROL AND SURFACE ELEVATIONS OF SITEWORK. REFER TO ARCHITECTURAL SITE PLAN FOR LAYOUT DIMENSIONS

- CONTRACTOR IS RESPONSIBLE FOR ABATEMENT OF SEDIMENT RELEASES ONTO ADJOINING PRIVATE PROPERTIES AND PUBLIC STREETS DURING CONSTRUCTION

△1 • CONTRACTOR SHALL SECURE A TOPSOIL DISTURBANCE PERMIT PRIOR TO PERFORMING ANY SITE GRADING.

△2 • EROSION CONTROL PLAN APPEARS ON SHEET C-2 OF 2, CENTER

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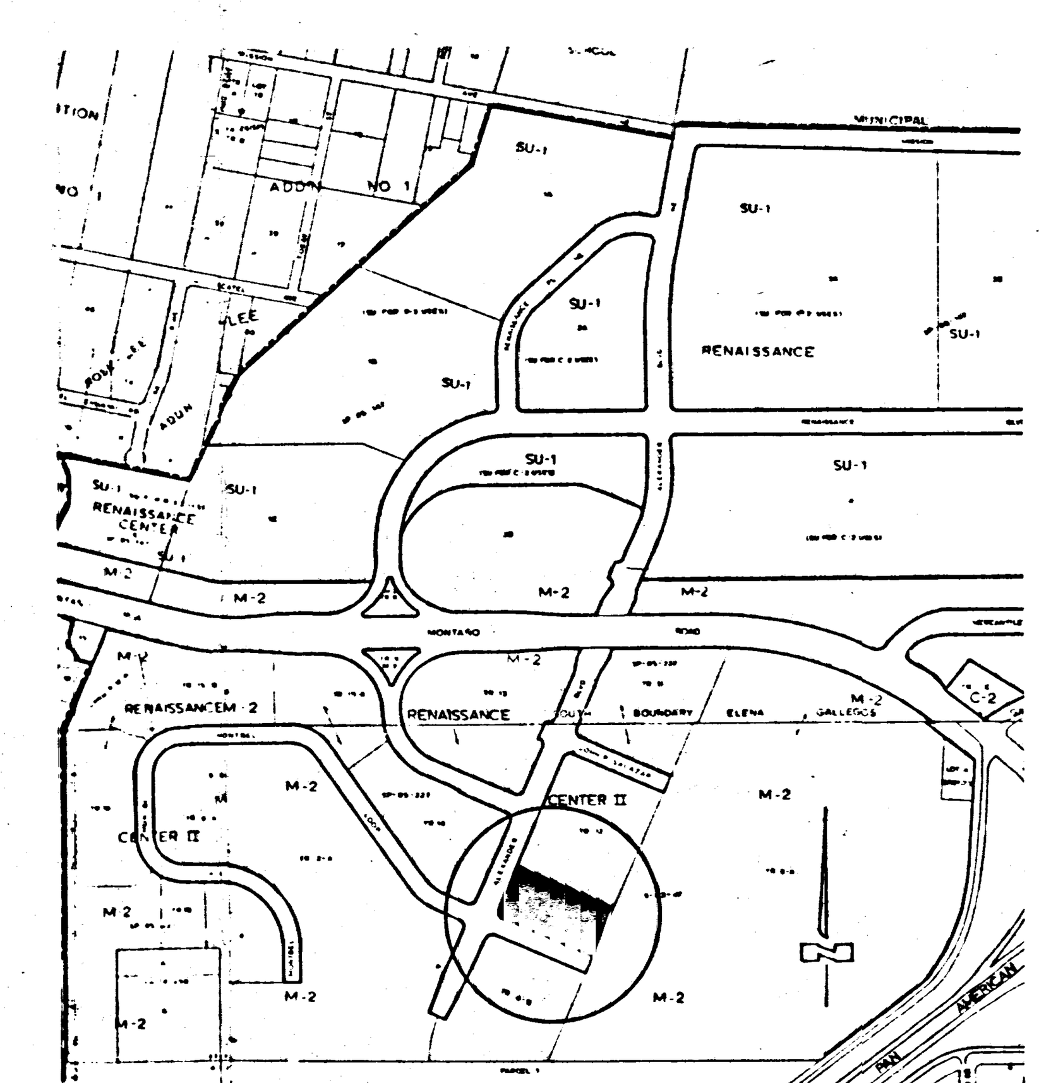
5. Backfill and compaction shall be according to ______________ street use.
6. Maintenance of this facility shall be the responsibility of the owner served.

APPROVALS	NAME	DATE
HYDROLOGY/DESIGN		
INSPECTOR/DESIGN		
CONST / ACPT.		



BENCH MARK

3-F16 ACS BRASS TABLET STAMPED "3-F16", SET IN CONCRETE POST IN MEDIAN STRIP OF MONTAÑO ROAD, NE, 0.7 MILE WEST OF I-25 INTERCHANGE

ELEV 5029.56

T.B.M.

TOP OF CURB, NNE RETURN, INTERSECTION OF MONTBEL LOOP & ALEXANDER BOULEVARD

ELEV. 5043.32

LEGEND

CONTOUR INTERVAL: EXISTING, ONE-FOOT (1.0')
PROPOSED, ONE-HALF FOOT (0.5')

(43) - - - - EXISTING CONTOUR

——43—— PROPOSED CONTOUR

· 43.0 EXISTING SPOT ELEVATION

⊕ 43.00 PROPOSED SPOT ELEVATION

PROPOSED SPOT ELEVATIONS AND CONTOURS AS SHOWN ARE TO FINISHED SURFACES. CONTRACTOR SHALL DETERMINE APPROPRIATE SUBGRADES

MASONRY SCREEN WALL

MASONRY RETAINING WALL WHERE FILL EXCEEDS 1.33 FEET

EXTENT OF STORMWATER DETENTION

DIVISION OF INTERIOR DRAINAGE BASINS

DATA

AREA OF SITE	1.9544 AC (85135 SQ FT)
SLOPE	1%
TIME OF CONCENTRATION	10 MINUTES
100-YEAR, 6-HOUR RAINFALL	2.2 INCHES
INTENSITY $(2.2 \times 6.84 \times 10^{-.51})$	4.65 IN/HR.

EXISTING ON-SITE CONDITIONS

THERE IS NO CONSTRUCTION OR OTHER EDIFICIAL IMPROVEMENTS WITHIN THE EXTERIOR BOUNDARIES. APPROXIMATELY THREE-FOURTHS OF SITE AREA DRAINS TO THE NORTHWEST. THE REMAINING PORTIONS DRAIN SOUTH TO MONTBEL BOULEVARD AND NORTH VIA RAILROAD SPUR GRADE TO AN AREA ON ADJOINING PROPERTY (LOT - 12) WHERE PONDING OCCURS.
CURB GUTTER AND SIDEWALK IMPROVEMENTS ARE IN PLACE ALONG SOUTH AND WEST PROPERTY LINES.

DIVISION OF EXISTING DISCHARGE ROUTINGS DETERMINED FROM SITE TOPOGRAPHY:

TO NORTHWEST INTO ALEXANDER BOULEVARD

AREA 1.51 ACRE
NO IMPERVIOUS COVERAGE, C = .40
$Q_{100} = .40 \times 4.65$ IN/HR $\times 1.51$ AC = 2.81 CFS, PEAK
$Q_{10} = .657\ Q_{100}$ = 1.85 CFS

TO NORTH ALONG TRACK ONTO LOT - 12

AREA .21 ACRE
$Q_{100} = .40 \times 4.65$ IN/HR $\times .21$ AC = .39 CFS, PEAK
$Q_{10} = .657\ Q_{100}$ = .26 CFS

TO SOUTH INTO MONTBEL LOOP

AREA .23 ACRE
$Q_{100} = .40 \times 4.65$ IN/HR $\times .23$ AC = .43 CFS, PEAK
$Q_{10} = .657\ Q_{100}$ = .28 CFS

TOTAL SITE PEAK RUNOFF RATES: (ALGEBRAIC SUM, Tc BEING CONSTANT)

Q_{100} = 3.63 CFS
Q_{10} = 2.39 CFS

UPSTREAM CONTRIBUTORY AREAS

NONE. SITE ISOLATED BY CURB & GUTTER ON SOUTH, WEST; BY RAILROAD GRADE ON EAST. ADJOINING PROPERTY ON NORTH IS LOWER.

PROPOSED DEVELOPMENT

COMMERCIAL WAREHOUSE/RETAIL BUILDING, PARKING AREAS, WALKS AND LANDSCAPING AS INDICATED ON PLAN. DRIVE ENTRIES WILL BE CONSTRUCTED IN ACCORDANCE WITH STD. DWG. 2426 "PRIVATE ENTRANCE".

	C =	AC.	%
PAVING	.92	1.02	52
ROOF	.95	.76	39
LANDSCAPING	.25	.17	9

LOT AS DEVELOPED WILL BE DIVIDED INTO TWO INTERIOR BASINS, EACH CONTRIBUTING TO DETENTION BASINS LOCATED IN PARKING LOTS. VOLUME OF EACH BASIN SHALL ACCOMMODATE 100% OF INCREASED RUNOFF GENERATED WITHIN CONTRIBUTORY ZONE. RELEASE WILL BE LIMITED TO 0.1 CFS PER CONTRIBUTING ACRE.

DISPOSAL ROUTING

AREA (A) WILL BE RELEASED TO ALEXANDER BOULEVARD. STREET FLOWS NORTH, UNDER MONTAÑO ROAD OVERPASS TO RENAISSANCE BLVD., THEN WEST TO A BASIN ADJACENT TO THE NORTH SIDE OF MONTAÑO RD.

AREA (B) WILL BE RELEASED TO MONTBEL LOOP. STREET FLOWS WEST TO A BASIN ADJACENT TO THE SOUTH SIDE OF MONTAÑO RD.

- VOLUMETRIC COMPUTATIONS, POND AND RELEASE DESIGNS AND DETAILS APPEAR ON SHEET D-2

EXCESS VOLUMES WILL OVERFLOW TO STREETS THROUGH DRIVEWAY ENTRANCES.

DESCRIPTION OF PROPERTY

PARCEL "E"
RENAISSANCE CENTER III
ALBUQUERQUE, NEW MEXICO
ALEXANDER BOULEVARD, NE

DIAGNOS-TEK

RECEIVED DEC 16 1988 HYDROLOGY SECTION

APPROVED FOR DRAINAGE
12 JAN 89
DATE
G.S. Reeder P.E. CE/HYD.
SIGNATURE TITLE
ADVISE DRANAGE INSPECTOR WHEN GRADING EXECUTED

REGISTERED PROFESSIONAL ENGINEER NEW MEXICO NO. 7918
10-14-88

OCTOBER 1988

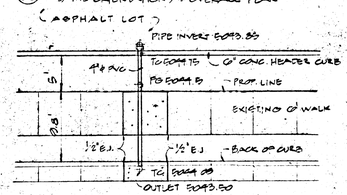
△1 REVISION, TOPSOIL DISTURBANCE PERMIT 11-11-88
△2 REVISION, EROSION CONTROL PLAN 11-11-88

SHEET C-1 OF 2

HALL ENGINEERING COMPANY INC.
ENGINEERING • SURVEYING • PLANNING • CONSTRUCTION
6840 2ND ST., N.W., SUITE 306 • ALBUQUERQUE, NEW MEXICO 87107
PHONE: (505) 345-1064

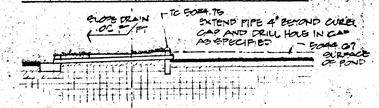
(1/C-2) OUTLET DETAILS POND (A)

SAME ORIENTATION AS OVERALL PLAN



REMOVE & REPLACE ONE STONE EACH OF CURB AND WALK. INSTALL DRAIN LINE THROUGH CURB IN ACCORDANCE WITH CITY STD. DWG. 2235.

PLAN VIEW 1" = 10'



SECTION 1" = 10'

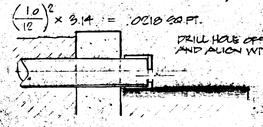
ALLOWABLE DISCHARGE, AREA (A) = .102 CFS. PEAK

ORIFICE AREA = $\frac{.102}{C_d\sqrt{2gh}} = \frac{.102}{.62\sqrt{2(32.16\ FT/SEC^2)\times .82\ FT}}$

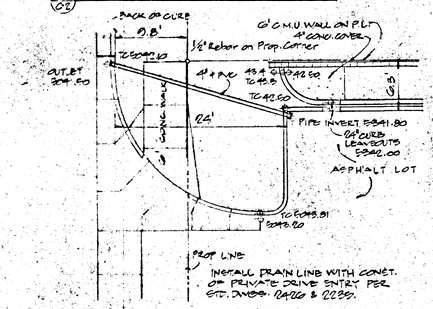
= .022 SQ. FT.

DRILL ONE 2½ INCH HOLE IN CAP

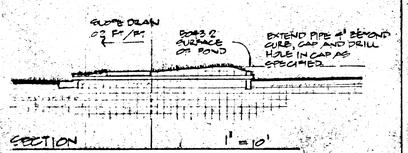
$\left(\frac{1.0}{12}\right)^2 \times 3.14 = .0218$ SQ FT.



(2/C-2) OUTLET DETAILS POND (B)



PLAN VIEW 1" = 10'



SECTION 1" = 10'

ALLOWABLE DISCHARGE, AREA (B) = .003 CFS. PEAK

ORIFICE AREA = $\frac{.003}{C_d\sqrt{2gh}} = \frac{.003}{.62\sqrt{2(32.16\ FT/SEC^2)\times 1.4\ FT}}$

= .016 SQ. FT.

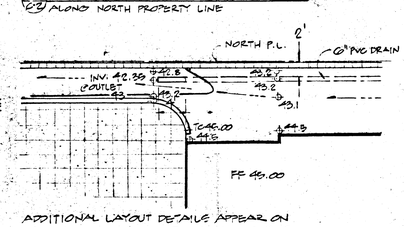
DRILL ONE 1⅜ INCH HOLE IN CAP (1.375")

$\left(\frac{1.07}{12}\right)^2 \times 3.14 = .011$ SQ. FT.

(+ SIMILAR TO DETAIL AT LEFT)

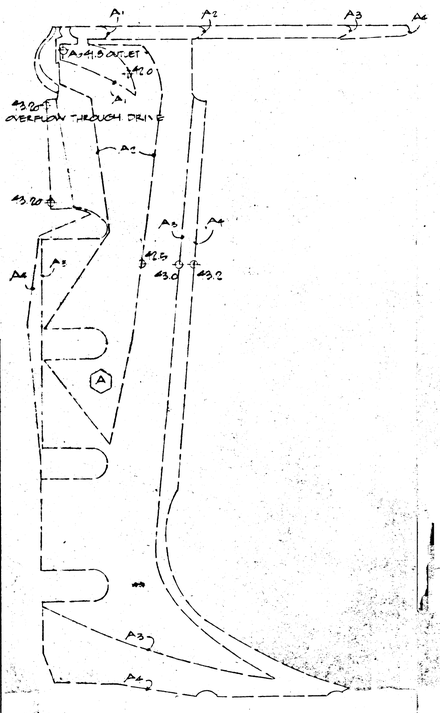
(3/C-2) OUTLET, DRAIN PIPE

ALONG NORTH PROPERTY LINE



ADDITIONAL LAYOUT DETAILS APPEAR ON WEST BUILDING ENTRY PLAN, SHEET A-2

GROUND COVER IN SWALES AS SPECIFIED IN LANDSCAPING PLAN.



DRAINAGE AREA (A)

.93 ACRE

COVERAGE	C =	AC	% OF CONTRIBUTORY AREA
PAVING	.92	.61	66
ROOF	.95	.20	22
LANDSCAPING	.25	.12	13

$C_{AVG} = (.92\times.66)+(.95\times.22)+(.25\times.13) = .84$

REQUIRED POND VOLUME

DEV RUNOFF VOL. = $\frac{.84\times 2.2\ IN\times .93\times 43560}{12}$ = 6023 CU FT

- LESS -

NATURAL RUNOFF VOL. = $\frac{.40\times 2.2\ IN\times .93\times 43560}{12}$ = 2795 CU FT

3228 CU FT

VOLUME OF POND (A)

$A_0 - A_1$ (A_0 @ 5041.8, A_1 @ 5042.0 = 100 FT.²)

$\frac{100\ FT^2}{3}\times .2\ FT$ = 7 CU FT

$A_1 - A_2$ (A_2 @ 5042.5 = 1800 FT.²)

$\frac{\sqrt{1800\times 100}+1800+100}{3}\times .5\ FT$ = 387 CU FT

$A_2 - A_3$ (A_3 @ 5043.0 = 5600 FT.²)

$\frac{\sqrt{5600\times 1800}+5600+1800}{3}\times .5\ FT$ = 1760 CU FT

$A_3 - A_4$ (A_4 @ 5043.2 = 6300 FT.²)

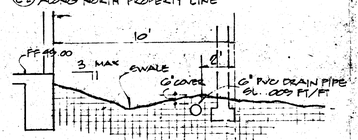
$\frac{\sqrt{6300\times 5600}+6300+5600}{3}\times .2\ FT$ = 1238 CU FT

TOTAL AVAILABLE VOL. = 3392 CU FT

(4/C-2) GRADE, PIPE SECTION

1" = 1'-0"

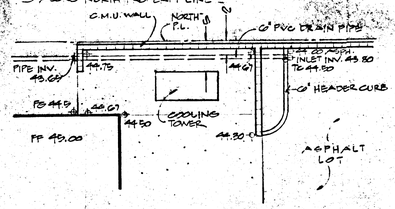
ALONG NORTH PROPERTY LINE



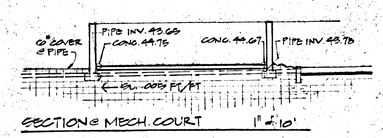
INSTALL CLEANOUTS IN DRAIN PIPE @ 100'-0"

(5/C-2) INLET, DRAIN PIPE

ALONG NORTH PROPERTY LINE

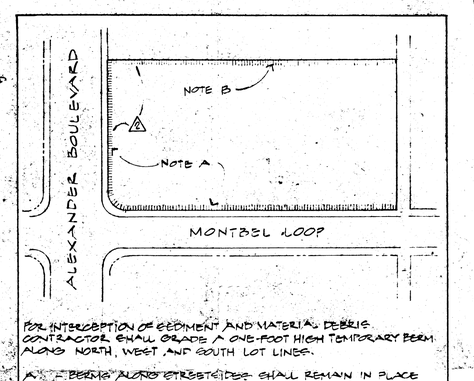


PLAN VIEW 1" = 10'



SECTION @ MECH. COURT 1" = 10'

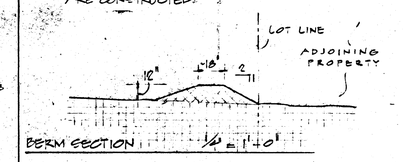
ADDITIONAL LAYOUT DETAILS APPEAR ON MECHANICAL/LOUNGE COURT PLAN, SHEET A-2



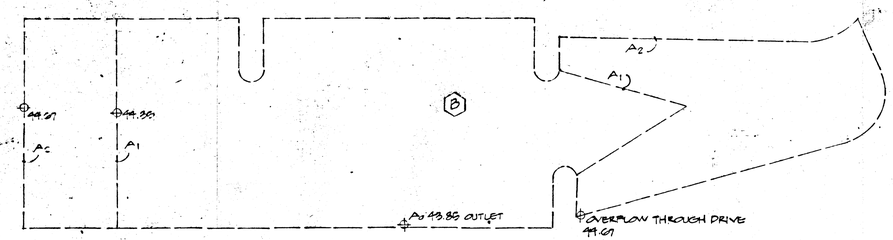
FOR INTERCEPTION OF SEDIMENT AND MATERIAL DEBRIS CONTRACTOR SHALL GRADE A ONE-FOOT HIGH TEMPORARY BERM ALONG NORTH, WEST, AND SOUTH LOT LINES.

A - BERMS ALONG STREETSIDE SHALL REMAIN IN PLACE UNTIL CURBS AND DRIVE ENTRANCES ARE COMPLETE.

B - BERM ALONG NORTH LOT LINE SHALL REMAIN IN PLACE UNTIL FACILITIES FOR DRAINAGE TO BASIN "A" ARE CONSTRUCTED.



BERM SECTION 1" = 1'-0"



DRAINAGE AREA (B)

1.02 ACRE

COVERAGE	C =	AC	% OF CONTRIBUTORY AREA
PAVING	.92	.41	40
ROOF	.95	.56	55
LANDSCAPING	.25	.03	5

$C_{AVG} = (.92\times.40)+(.95\times.55)+(.25\times.05) = .90$

REQUIRED POND VOLUME

DEV. RUNOFF VOL. = $\frac{.90\times 2.2\ IN\times 1.02\times 43560}{12}$ = 7331 CU FT.

- LESS -

NATURAL RUNOFF VOL. = $\frac{.40\times 2.2\ IN\times 1.02\times 43560}{12}$ = 3258 CU FT.

4073 CU FT.

$A_0 - A_1$ (A_0 @ 5043.85, A_1 @ 5044.35 = 7800 FT.²)

$\frac{7800}{3}\times .5\ FT$ = 1260 CU FT.

$A_1 - A_2$ (A_2 @ 5044.67 = 19100 FT.²)

$\frac{\sqrt{7800\times 13100}+7800+13100}{3}\times .3$ = 3067 CU FT.

TOTAL AVAILABLE VOL. = 4327

RECEIVED
DEC 16 1988
HYDROLOGY SECTION

APPROVED FOR DRAINAGE
12 JAN 89
DATE
G.S. Reeder, Jr. P.E. CE/HYDROLOGY
SIGNATURE TITLE

ADVISE DRAINAGE INSPECTOR WHEN GRADING EXECUTED

RICHARD U. HALL NEW MEXICO REGISTERED PROFESSIONAL ENGINEER
10-14-88

⚠ REVISED 11-11-88 ADDED EROSION CONTROL

SHEET C-2 OF 2

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