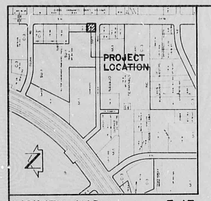
PUBLIC WORKS/HYDROLOGY

F17/D22A1



VICINITY MAP F-17

SCALE 1" = 800'

LEGEND

- 5210 — EXISTING CONTOUR
- EXISTING SPOT ELEVATION
- EXISTING FENCE
- EXISTING SWALE
- PROPOSED SWALE
- PROPOSED SPOT ELEVATION
- 10 PROPOSED CONTOUR
- PROPOSED RETAINING WALL
- PROPOSED ASPHALT PAVEMENT
- BASIN BOUNDARY
- PROPOSED CONCRETE
- PROPOSED WATER BLOCK
- RETAINING WALL

TEMPORARY BENCHMARK

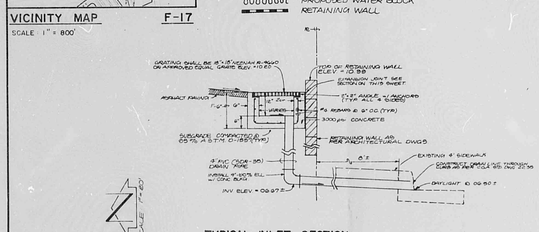
RIM OF MANHOLE IN EXISTING 6' SIDEWALK AS SHOWN
ELEVATION = 6213.59 FEET (MSL 0)

PROJECT BENCHMARK

A STANDARD NMSHTD BRASS TABLET STAMPED "NM-327" IS SET ON TOP OF CONCRETE POST SET FLUSH WITH GROUND LOCATED 41 FEET FROM THE NOSE OF THE MEDIAN IN SAN MATEO BLVD NE, 55' NORTH OF THE INTERSECTION OF OBUNA RD NE & SAN MATEO BLVD NE
ELEVATION = 6199.32 FEET (MSL 0)

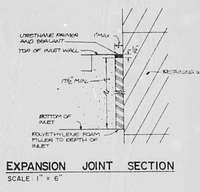
LEGAL DESCRIPTION

TRACT Y, UNCLE CLIFF'S FAMILYLAND



TYPICAL INLET SECTION

SCALE 1" = 2'-0"



EXPANSION JOINT SECTION

SCALE 1" = 6"

DRAINAGE PLAN

The following items concerning the Taco Bell Drainage Plan are contained hereon:

1. Vicinity Map
2. Grading Plan
3. Calculations

As shown by the Vicinity Map, the site is located at the northwest corner of the intersection of San Mateo Boulevard N.E. and Lincoln Road N.E. At present, the site is partially developed with minor paving. Much of the surrounding area is currently developed, thereby making this an infill site. The proposed improvements consist of a building, paving, and landscaping.

As shown by Panel 16 of the Federal Emergency Management Agency Flood Boundary and Floodway Map, dated October 14, 1983, this site does not lie within a designated Flood Hazard Zone. The site appears to contribute runoff to a flood hazard area at the end of Lincoln Road along Interstate 25. Presently, the New Mexico State Highway Department is constructing an overpass on Interstate 25 at Jefferson Street/Singer Boulevard. The new construction includes a drainage facility which will reduce the flood hazard area. As shown by the calculations, the increase in runoff by the proposed improvements is negligible. Nonetheless, a controlled discharge of runoff from the site has been implemented.

The Grading Plan shows 1) existing contours at 1'0" intervals, 2) proposed grades indicated by spot elevations and contours at 1'0" intervals, 3) the limit and character of the existing improvements, and 4) the limit and character of all proposed improvements. As shown by this plan, the proposed improvements consist of a building, paving and landscaping. Flows generally flow from east to west. Offsite flows from the south are contained in Lincoln Road N.E. and flows from the east are contained in San Mateo Boulevard N.E. No offsite flows are anticipated from the north or west, hence offsite flows are not a concern. As shown by this plan, flows will be directed to the southwest corner of the site. At this corner a detention pond (875 cf) has been designed in the parking area. An inlet has also been provided at the southwest corner which will discharge into Lincoln Road at a rate of 1.0 cfs as shown in the calculations. The runoff will flow down Lincoln Road to Interstate 25 and south along Interstate 25 toward Jefferson Street/Singer Boulevard.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these calculations, the proposed improvements will result in a negligible net increase in peak runoff by 1.0 cfs. The pond volume was determined using the Average End Over Area Method and discharge from the inlet was determined using the Venturi Meter Equation.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate 21: TyB - Tijeras Complex
Hydrologic Soil Group: B
Existing Pervious CN = 70 (DPM Plate 22.2 C-2
Pasture or Range Land: fair condition)
Developed Pervious CN = 61 (DPM Plate 22.2 C-2
Open Spaces: good condition)

Time of Concentration/Time to Peak

$T_C = 0.0078\ L^{0.77}/S^{0.385}$ (Kirpich Equation)

$T_P = T_C = 10$ min.

Point Rainfall

$P_6 = 2.2$ in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: $Q = CIA$

where C varies
$I = P_6\ (6.84)\ T_C^{-0.51} = 4.65$ in/hr
$P_6 = 2.2$ in (DPM Plate 22.2D-1)
$T_C = 10$ min (minimum)
A = area, acres

SCS Method

Volume: $V = 3630(DRO)\ A$

where DRO = Direct runoff in inches
A = area, acres

Existing Condition

$A_{total} = 19,910$ sf $= 0.46$ Ac
Undeveloped area = 18,200 sf (0.91)
Paved area = 1,710 sf (0.09)
$C = 0.45$ (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = 0.45(4.65)0.46 = 1.0$ cfs
$Q_{100} = 1,710$ sf) x impervious = 9 \
Composite CN = 73 (DPM Plate 22.2 C-3)
DRO = 0.4 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)A = 680$ cf

Developed Condition

$A_{total} = 19,910$ sf $= 0.46$ Ac
Roof area = 2,720 sf (0.13)
Paved area = 16,440 sf (0.83)
Landscaped area = 730 sf (0.04)
$C = 0.92$ (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = 0.92(4.65)0.46 = 2.0$ cfs
Aimp = 19,180 sf) x impervious = 96 \
Composite CN = 96 (DPM Plate 22.2 C-3)
DRO = 1.8 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)\ A = 3,000$ cf

Comparison

$\Delta Q_{100} = 2.0 - 1.0 = 1.0$ cfs (increase)
$\Delta V_{100} = 3,000 - 680 = 2,320$ cf (increase)

Inlet Discharge

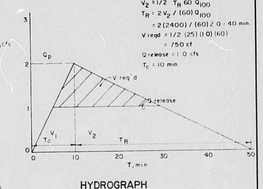
$Q = CA\sqrt{2gh}$ (Venturi Meter Equation)
Let $c = 0.75$
A = area of pipe in feet
$g = 32.2$ ft/sec^2
h = head in feet

$Q_{release} = 0.75(0.0873)\sqrt{2(32.3)(10.70 - 6.6)}$
$= 1.0$ cfs

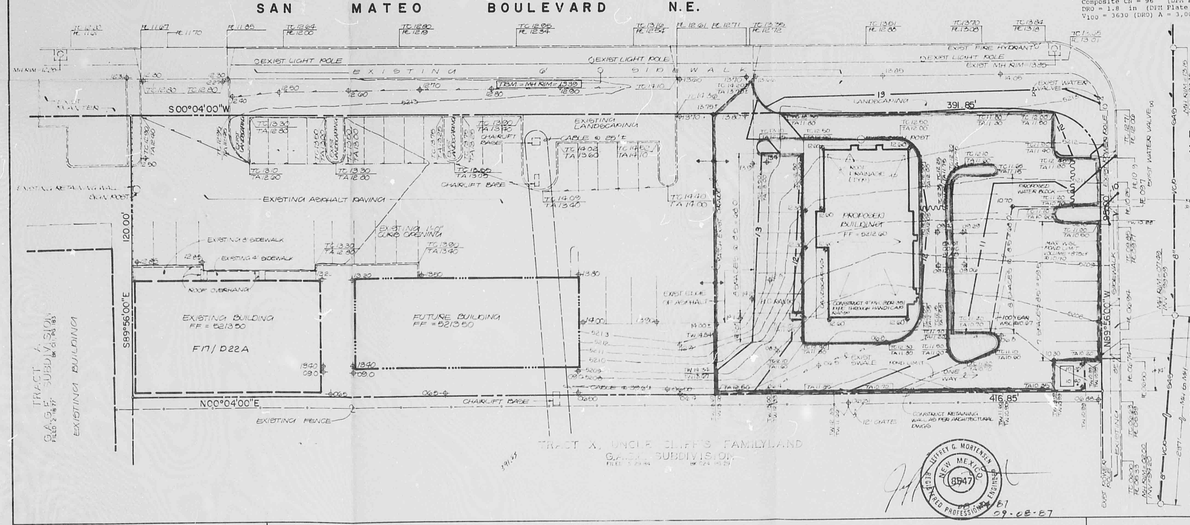
Pond Volume

$V = 1/2(A_{top} + A_{bot})(Elev_{top} - Elev_{bot})$
$= 1/2(3900 + 0)(10.70 - 10.20)$
$= 875$ cf

$Q_P = Q_{100} = 2.0$ cfs
$V_{100} = 3,000$ cf
$V_1 = 1/2\ (10)(60)(2.0) + 600$ cf
$V_2 = V_{100} - V_1 = 2400$ cf
$V_2 = 1/2\ T_R\ 60\ Q_{100}$
$T_R = 2\ V_2 / (60\ Q_{100})$
$= 2(2400)/(60)(2.0) = 40$ min
$V req'd = 1/2\ (25)(10)(60)$
$= 750$ cf
$Q release = 1.0$ cfs
$T_c = 10$ min



HYDROGRAPH



EROSION CONTROL MEASURES

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
3. THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.

CONSTRUCTION NOTES

1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTACTOR MUST CONTACT LINE LOCATING SERVICE 765-1234, FOR LOCATION OF EXISTING UTILITIES.
2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS UNDERTAKEN NO FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM itself OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES, IN PLANNING AND CONDUCTING EXCAVATION. THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
6. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY. AN APPROVED COPY OF THESE PLANS MUST BE SUBMITTED AT THE TIME OF APPLICATION FOR THIS PERMIT.
7. BACKFILL COMPACTION SHALL BE ACCORDING TO ARTERIAL STREET USE.
8. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.

RECEIVED SEP 08 1987 CITY OF ALBUQUERQUE HYDROLOGY SECTION

APPROVALS	NAME	DATE
A.C.E./DESIGN	B. Montoya	9/9/87
INSPECTOR		
A.C.E./FIELD		

BOHANNAN-HUSTON
8 DALLAS N.E. • ALBUQUERQUE • NEW MEXICO 87108
ENGINEERS

GRADING AND DRAINAGE PLAN
TACO BELL

DESIGN BY P.M.L.
DRAWN BY R.A.R.
APPROVED BY J.G.M.

No.	Date	By	Revision
1	9/87	P.M.L.	ADDED ROOF DRAINAGE

JOB NO. 30078
DATE 08/87
SHEET 1 OF 1

RI 25 DATE 3-17-92 OP EOP

CITY OF ALBUQUERQUE

This microimage is certified to be a complete and accurate copy of the original as it appears in the files of the Public Works Department and was created in the normal course of business.

The photographic process used meets the Basic Microfilm Standards of the National Micrographics Association (MSI/ANSI-1977)

Chas. T. Hildebrand 11/5/92
DIRECTOR

Roberta Duran
NOTARY PUBLIC

OFFICIAL SEAL
Roberta Duran
NOTARY PUBLIC
STATE OF NEW MEXICO
My Commission Expires 07/23/92

PLAN : NOT TWICE ONCE ON LIGHT ONCE ON DARK

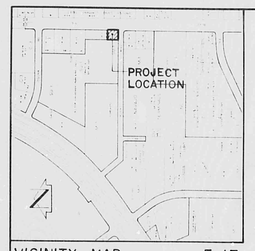
CITY OF ALBUQUERQUE
THIS MICROIMAGE IS THE BEST POSSIBLE REPRODUCTION DUE TO THE POOR QUALITY OF THE ORIGINAL DOCUMENT.

PLANS/PLATS ON FILE

FILE DESC: F17/D22A1

PLANS/PLATS 1

E 30 D 24 C 20 B 16 A 12 CITY OF ALBUQUERQUE C D F



VICINITY MAP F-17

SCALE 1" = 800'

LEGEND

- 820 — EXISTING CONTOUR
- EXISTING SPOT ELEVATION
- EXISTING FENCE
- EXISTING SWALE
- PROPOSED SWALE
- PROPOSED SPOT ELEVATION
- 10 — PROPOSED CONTOUR
- PROPOSED RETAINING WALL
- PROPOSED ASPHALT PAVEMENT
- BASIN BOUNDARY
- PROPOSED CONCRETE
- PROPOSED WATER BLOCK
- RETAINING WALL

TEMPORARY BENCHMARK

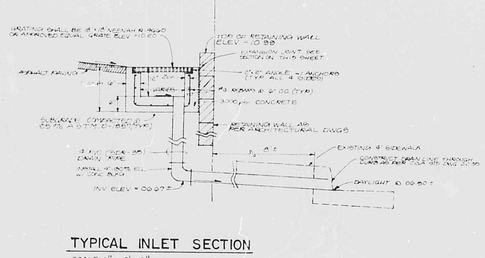
RIM OF MANHOLE IN EXISTING 6' SIDEWALK AS SHOWN
ELEVATION = 6218.89 FEET (MSL0)

PROJECT BENCHMARK

A STANDARD N.M.S.H.D. BRASS TABLET STAMPED "NM 507" SET IN TOP OF CONCRETE KOBE SET FLUSH WITH GROUND LOCATED 4' FEET FROM THE NOSE OF THE MEDIAN IN SAN MATEO BLVD. N.E., 150' NORTH OF THE INTERSECTION OF OSUNA RD. N.E. & SAN MATEO BLVD. N.E.
ELEVATION = 6198.38 FEET (MSL0)

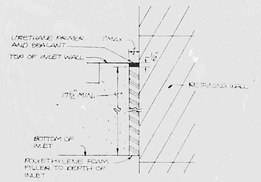
LEGAL DESCRIPTION

TRACT Y, UNCLE CLIFF'S FAMILYLAND



TYPICAL INLET SECTION

SCALE 1" = 2'-0"



EXPANSION JOINT SECTION

SCALE 1" = 6"

DRAINAGE PLAN

The following items concerning the Taco Bell Drainage Plan are contained hereon:

1. Vicinity Map
2. Grading Plan
3. Calculations

As shown by the Vicinity Map, the site is located at the northwest corner of the intersection of San Mateo Boulevard N.E. and Lincoln Road N.E. At present, the site is partially developed with minor paving. Much of the surrounding area is currently developed, thereby making this an infill site. The proposed improvements consist of a building, paving, and landscaping.

As shown by Panel 16 of the Federal Emergency Management Agency Flood Boundary and Floodway Map, dated October 14, 1983, this site does not lie within a designated Flood Hazard Zone. The site appears to contribute runoff to a flood hazard area at the end of Lincoln Road along Interstate 25. Presently, the New Mexico State Highway Department is constructing an overpass on Interstate 25 at Jefferson Street/Singer Boulevard. The new construction includes a drainage facility which will reduce the flood hazard area. As shown by the calculations, the increase in runoff by the proposed improvements is negligible. Nonetheless, a controlled discharge of runoff from the site has been implemented.

The Grading Plan shows 1) existing contours at 1'0" intervals, 2) proposed grades indicated by spot elevations and contours at 1'0" intervals, 3) the limit and character of the existing improvements, and 4) the limit and character of all proposed improvements. As shown by this plan, the proposed improvements consist of a building, paving and landscaping. Flows generally flow from east to west. Offsite flows from the south are contained in Lincoln Road N.E. and flows from the east are contained in San Mateo Boulevard N.E. No offsite flows are anticipated from the north or west, hence offsite flows are not a concern. As shown by this plan, flows will be directed to the southwest corner of the site. At this corner a detention pond (875 cf) has been designed in the parking area. An inlet has also been provided at the southwest corner which will discharge into Lincoln Road at a rate of 1.0 cfs as shown in the calculations. The runoff will flow down Lincoln Road to Interstate 25 and south along Interstate 25 toward Jefferson Street/Singer Boulevard.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these calculations, the proposed improvements will result in a negligible net increase in peak runoff by 1.0 cfs. The pond volume was determined using the Average End Over Area Method and discharge from the inlet was determined using the Venturi Meter Equation.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate 21: TpD - Tijeras Complex
Hydrologic Soil Group: B
Existing Previous CN = 79 (DPM Plate 22.2 C-2
Pasture or Range Land: fair condition)
Developed Pervious CN = 61 (DPM Plate 22.2 C-2
Open Spaces: good condition)

Time of Concentration/Time to Peak

$T_c = 0.0078\ L^{0.77}/S^{0.385}$ (Kirpich Equation)

$T_p = T_c = 10$ min.

Point Rainfall

$P_6 = 2.2$ in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: $Q = CIA$

where C varies

$I = P_6\ (6.84)\ T_c^{-0.4} = 4.65$ in/hr
$P_6 = 2.2$ in (DPM Plate 22.2D-1)
$T_c = 10$ min (minimum)
A = area, acres

SCS Method

Volume: $V = 3630\,(DRO)\,A$

Where DRO = Direct runoff in inches
A = area, acres

Existing Condition

$A_{total} = 19,910$ sf $= 0.46$ Ac
Undeveloped area = 18,200 sf (0.91)
Paved area = 1,710 sf (0.09)
$C = 0.45$ (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = 0.45(4.65)0.46 = 1.0$ cfs
$A_{imp} = 1,710$ sf; % Impervious = 9 %
Composite CN = 73 (DPM Plate 22.2 C-3)
DRO = 0.4 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)A = 680$ cf

Developed Condition

$A_{total} = 19,910$ sf $= 0.46$ Ac
Roof area = 2,720 sf (0.13)
Paved area = 16,460 sf (0.83)
Landscaped area = 730 sf (0.04)
$C = 0.92$ (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = 0.92(4.65)0.46 = 2.0$ cfs
$A_{imp} = 19,180$ sf; % Impervious = 96 %
Composite CN = 96 (DPM Plate 22.2 C-3)
DRO = 1.8 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)\ A = 3,000$ cf

Comparison

$\Delta Q_{100} = 2.0 - 1.0 = 1.0$ cfs (Increase)
$\Delta V_{100} = 3,000 - 680 = 2,320$ cf (Increase)

Inlet Discharge

$Q = CA\sqrt{2gh}$ (Venturi Meter Equation)
Let $C = 0.75$
A = area of pipe in feet
$g = 32.2$ ft/sec^2
h = head in feet

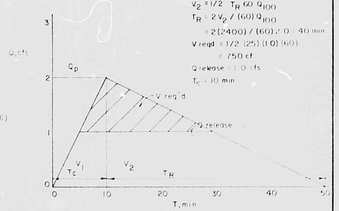
$Q_{release} = 0.75(0.0873) \times 2(32.2)(10.78 - 6.6)$
$= 1.0$ cfs

Pond Volume

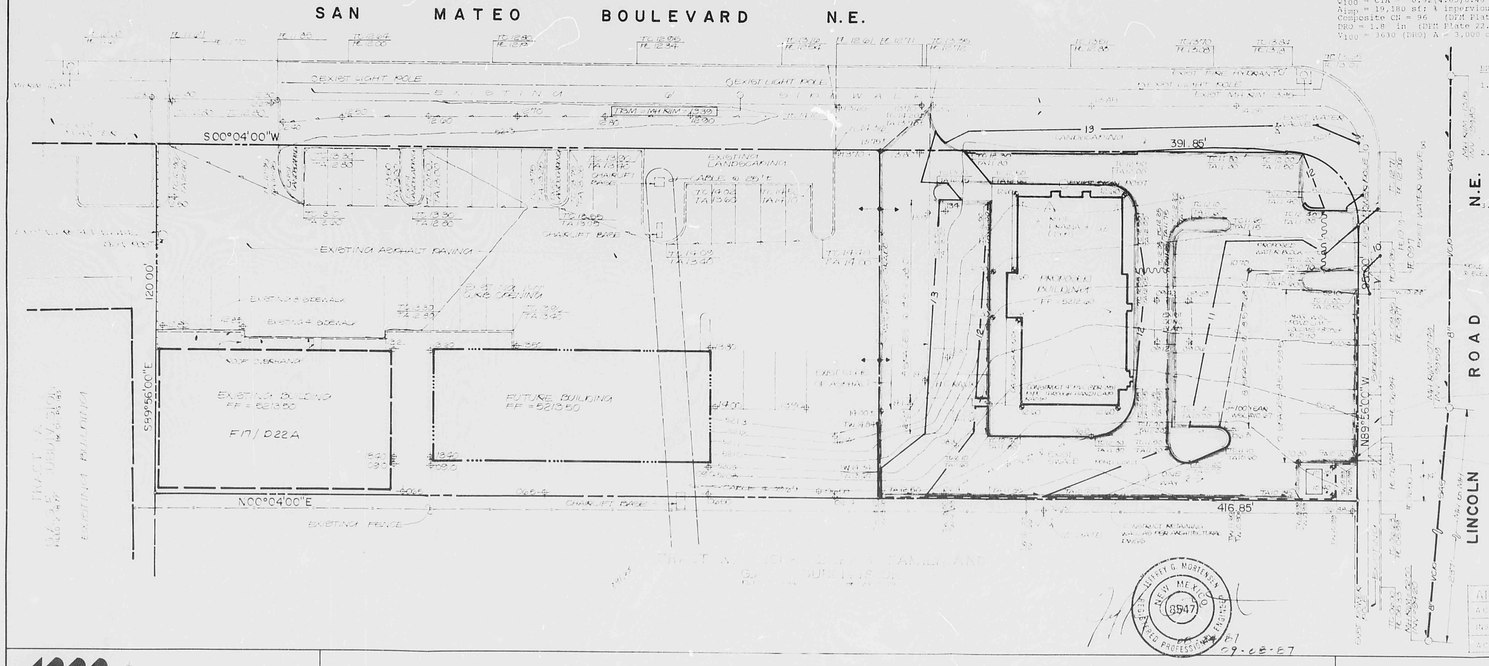
$V = 1/2(A_{top} + A_{bot})(Elev_{top} - Elev_{bot})$
$= 1/2(1600 + 0)(10.78 - 10.00)$
$= 875$ cf

$Q_p = Q_{100} = 2.0$ cfs
$V_{100} = 3,000$ cf
$V_1 = 1/2\ (10)(60)(2.0) = 600$ cf
$V_2 = V_{100} - V_1 = 2400$ cf
$V_2 = 1/2\ T_R\ 60\ Q_{100}$
$T_R = 2V_2/(60)Q_{100}$
$= 2(2400)/(60)2.0 = 40$ min
$V_{req'd} = 1/2\ (25)(10)(60)$
$= 750$ cf

$Q_{release} = 1.0$ cfs
$T_c = 10$ min



HYDROGRAPH



EROSION CONTROL MEASURES

1. THE CONTRACTOR SHALL PROVIDE THAT NO MUD IS TRACKED ONTO PUBLIC PAVEMENT FROM THE SITE. THIS MAY INCLUDE RIGHT-OF-WAY OR OWNER PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP FUGITIVE DUST DOWN.

2. THE CONTRACTOR SHALL PERIODICALLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT PERMITTED TO BEING WASHED DOWN THE STREET.

3. THE CONTRACTOR SHALL SECURE "EFFECTIVE DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.

CONSTRUCTION NOTES:

1. THE CONTRACTOR SHALL CONTACT THE ONE CALL SYSTEM, INC. (842-1225) FOR THE CITY AND PUBLIC RIGHT-OF-WAY OR OWNER PRIVATE PROPERTY FOR LOCATING EXISTING UTILITIES.

2. PRIOR TO CONSTRUCTION, THE CONTRACTOR MUST EXCAVATE AND VERIFY HORIZONTAL AND VERTICAL LOCATION OF ALL UTILITIES. IF CONSTRUCTION CONFLICTS EXISTS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.

3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.

4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.

5. IF ANY UTILITY LINES, EXCAVATIONS, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THE DRAWINGS, THEY ARE PLACED AS ACCURATELY AS MANNER ONLY, AND SUCH LINES MAY TAKE WHICH NOTE AND BEARD, IF ANY, CONTAINED ON THE ARE SHOWN. THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OR DATA UTILITY, AND THE CONTRACTOR MUST ASSUME INCOMPLETE, OR MAY BE OMITTED BY THE NON-CONTRACTOR CONTRACTOR, AND ASSUME THE UNDERLAYER NO FULL VERIFICATION HAS BEEN MADE LOCATION, DEPTH, SIZE, TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. PARTS OR REPRESENTATION REPAIRING THERETO, AND ACCEPTS NO RESPONSIBILITY OR LIABILITY HEREON. THE CONTRACTOR SHALL INFORM PUBLIC OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF ANY DIGGING EXCAVATION OCCUR. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY HIS FAILURE TO LOCATE, IDENTIFY AND PROTECT ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE AND FEDERAL STATUTES AND LOCAL ORDINANCES, RULES AND REGULATIONS IF ANY, REGARDING TO THE LOCATION OF PIPELINES AND FACILITIES.

6. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY. AN APPROVED COPY OF THESE PLANS MUST BE SUBMITTED AS THE TIME OF APPLICATION FOR THIS PERMIT.

7. BACKFILL COMPACTION SHALL BE ACCORDING TO ARTERIAL STREET USE.

8. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.

APPROVALS	NAME	DATE
A.C.E. FOREMAN	R. Montoya	9/1/87
INSPECTOR		
A.C.E. ENGINEER		

GRADING AND DRAINAGE PLAN
TACO BELL

DESIGN BY	F.M.L.
DRAWN BY	R.A.R.
APPROVED BY	J.G.M.

No.	Date	By	Revision
1	9/87	F.M.L.	ADDED EXP. DRAINAGE

JOB NO.	30078
DATE	08/87
SHEET	1 OF 1

PLAN SHOT TWICE
ONCE ON LIGHT
ONCE ON DARK

CITY OF ALBUQUERQUE
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R# 35 DATE 3-19-02 OP EP

CITY OF ALBUQUERQUE

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07/02/02

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