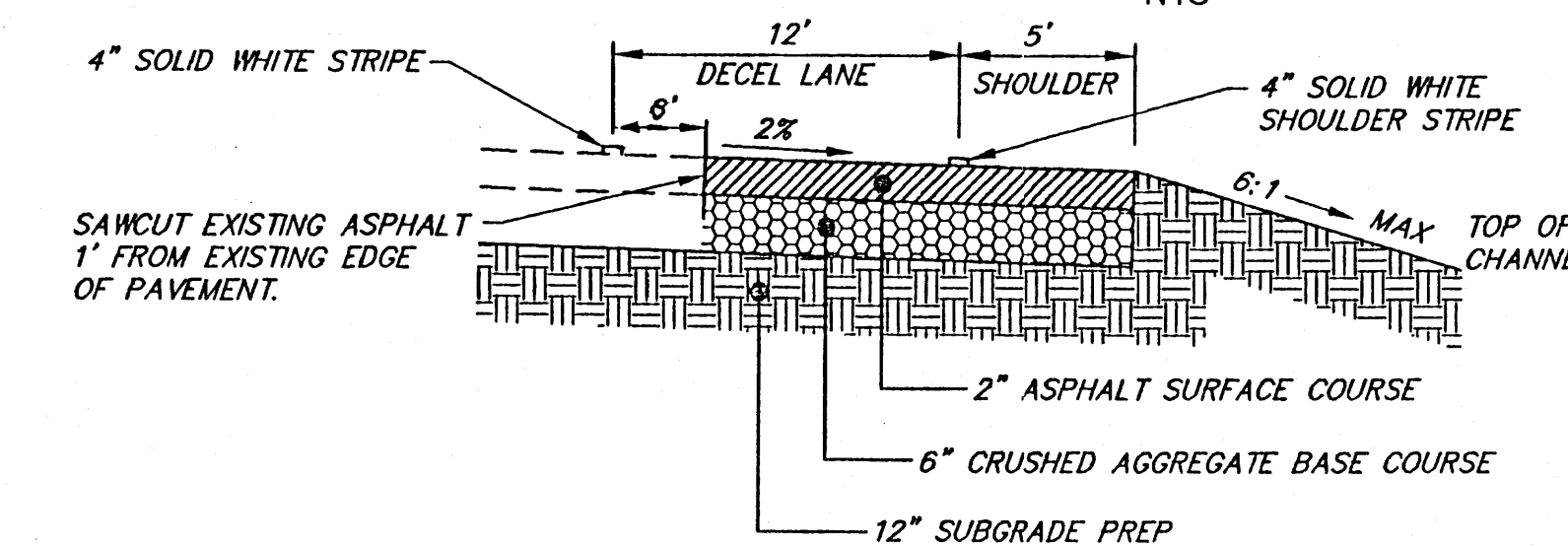
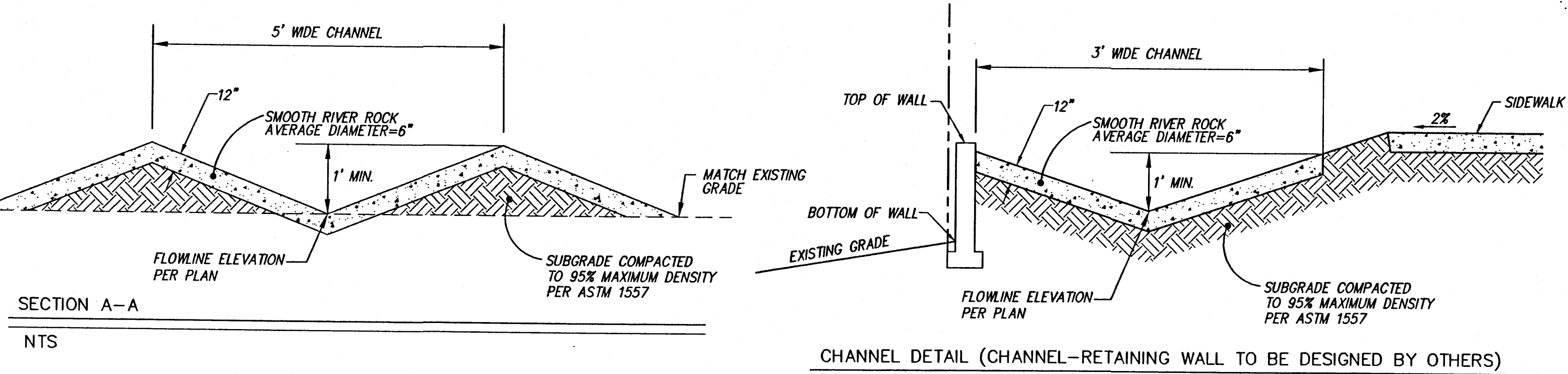
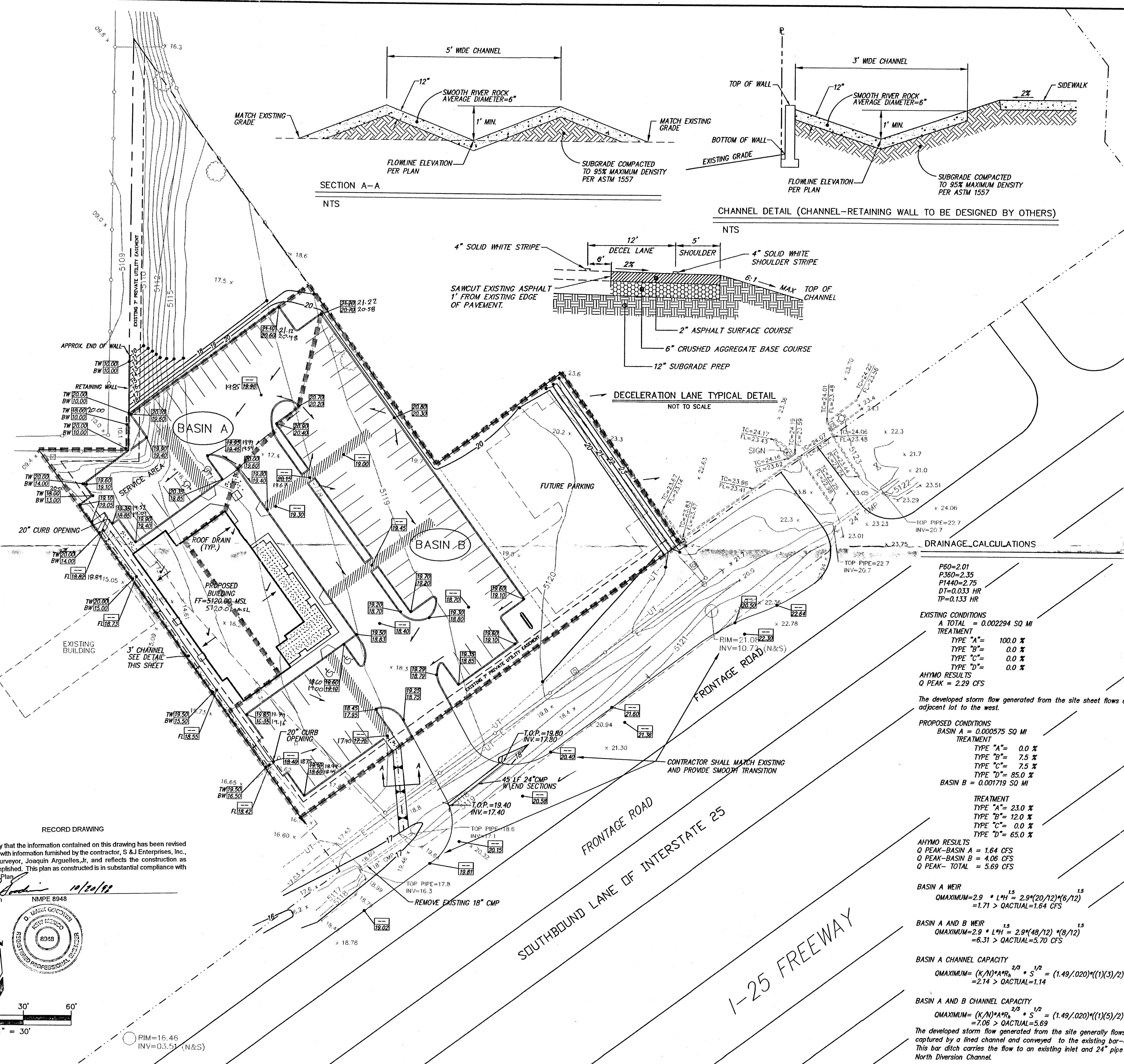




DRAINAGE CALCULATIONS

P60=2.01
P360=2.35
P1440=2.75
DT=0.033 HR
TP=0.133 HR

EXISTING CONDITIONS
A TOTAL = 0.002294 SQ MI
TREATMENT
TYPE "A" = 100.0 %
TYPE "B" = 0.0 %
TYPE "C" = 0.0 %
TYPE "D" = 0.0 %

AHYMO RESULTS
Q PEAK = 2.29 CFS

The developed storm flow generated from the site sheet flows east to west, discharge upon adjacent lot to the west.

PROPOSED CONDITIONS
BASIN A = 0.000575 SQ MI
TREATMENT
TYPE "A" = 0.0 %
TYPE "B" = 7.5 %
TYPE "C" = 7.5 %
TYPE "D" = 85.0 %
BASIN B = 0.001719 SQ MI

TREATMENT
TYPE "A" = 23.0 %
TYPE "B" = 12.0 %
TYPE "C" = 0.0 %
TYPE "D" = 65.0 %

AHYMO RESULTS
Q PEAK-BASIN A = 1.64 CFS
Q PEAK-BASIN B = 4.06 CFS
Q PEAK-TOTAL = 5.69 CFS

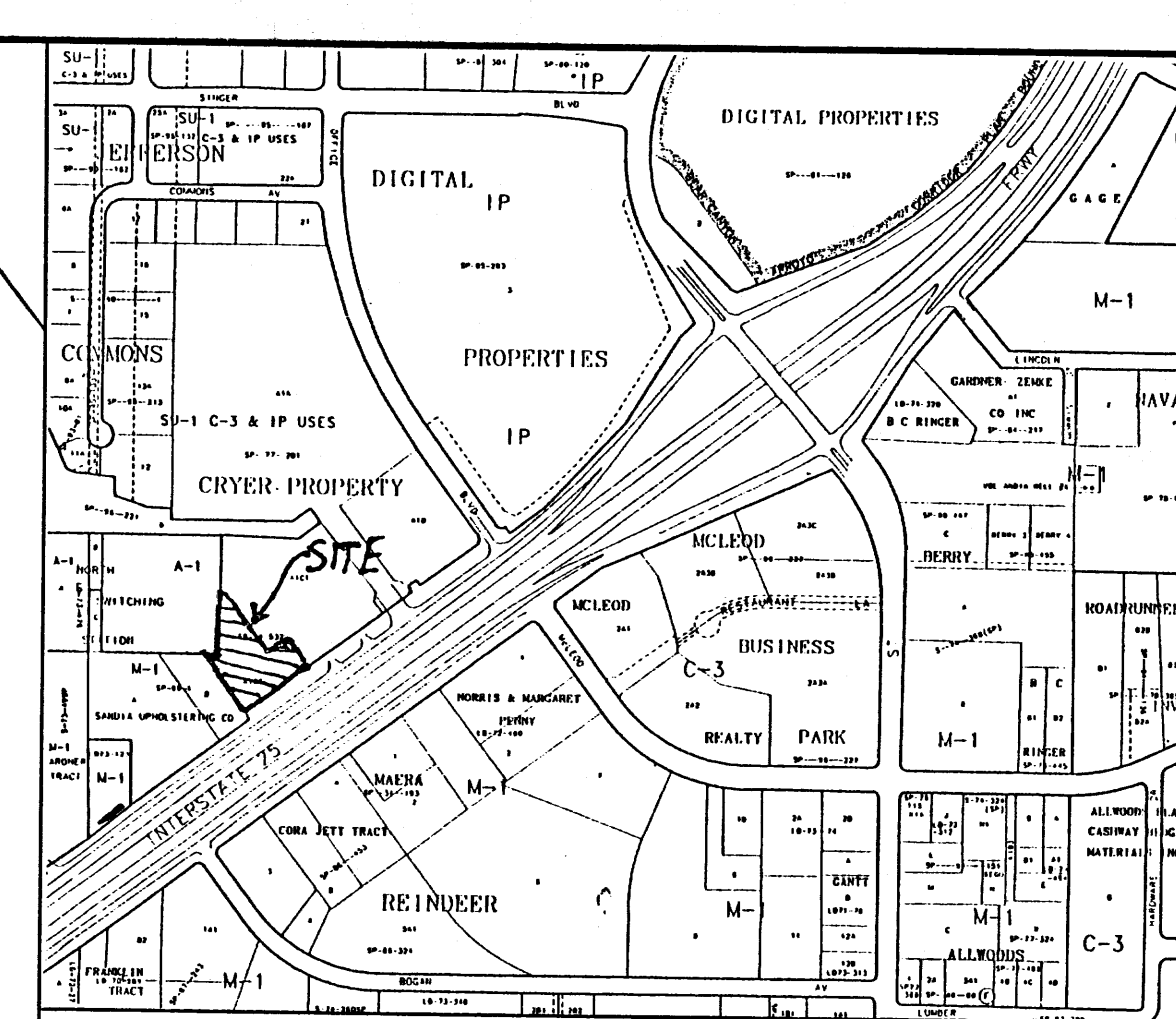
BASIN A WEIR
 $Q_{MAXIMUM} = 2.9 \cdot L^{1.5} \cdot H^{1.5} = 2.9(20/12)^{1.5}(6/12)^{1.5} = 1.71 > Q_{ACTUAL} = 1.64$ CFS

BASIN A AND B WEIR
 $Q_{MAXIMUM} = 2.9 \cdot L^{1.5} \cdot H^{1.5} = 2.9(49/12)^{1.5}(9/12)^{1.5} = 6.31 > Q_{ACTUAL} = 5.70$ CFS

BASIN A CHANNEL CAPACITY
 $Q_{MAXIMUM} = (K/N) \cdot A \cdot R^{2/3} \cdot S^{1/2} = (1.49/0.020) \cdot (11/3) \cdot (2)^{2/3} \cdot (.0025)^{1/2} = 2.14 > Q_{ACTUAL} = 1.14$

BASIN A AND B CHANNEL CAPACITY
 $Q_{MAXIMUM} = (K/N) \cdot A \cdot R^{2/3} \cdot S^{1/2} = (1.49/0.020) \cdot (11/5) \cdot (2)^{2/3} \cdot (.004)^{1/2} = 7.06 > Q_{ACTUAL} = 5.69$

The developed storm flow generated from the site generally flows east to west, where the flow is captured by a lined channel and conveyed to the existing bar-ditch along the I-25 Frontage Road. This bar ditch carries the flow to an existing inlet and 24" pipe which out falls into the AMAFCA's North Diversion Channel.



PROJECT NOTES

1. SITE DOES NOT LIE WITHIN A 100 YEAR FLOOD PLAIN.
2. OFF-SITE FLOWS DO NOT IMPACT SITE.
3. CONTRACTOR SHALL VERIFY AND PROVIDE 2' MINIMUM DEPTH OF I-25 FRONTAGE ROAD BAR DITCH BETWEEN SITE AND OUT FALL TO THE AMAFCA NORTH DIVERSION CHANNEL. THIS DITCH MUST BE A TRAPEZOID WITH 3.5' BOTTOM WIDTH WITH 4:1 SIDE SLOPE.
4. RETAINING WALL TO BE DESIGNED BY OTHERS.
5. A 2" ASPHALT MAT SHALL BE CONSTRUCTED AROUND EXISTING INLET LOCATION SOUTH AND WEST OF SIDE. MAT MUST BE CONSTRUCTED AS TO MAINTAIN 18" OF ELEVATION DIFFERENCE BETWEEN TOP OF GRATE AND AT SURROUNDING NATURAL GROUND.

A.C.S. BENCHMARK

NMHC BRASS CAP "125-18" ELEVATION=5125.70

LEGAL DESCRIPTION

TRACT A-1-C-2, WITHIN SECTION 35, TOWNSHIP 11 NORTH, RANGE 3 EAST, NMPM, CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO, OCTOBER 1997.

LEGEND

- 15-Built PROPOSED TOP OF WALL AND BOTTOM OF WALL ELEVATION
- 15-Built PROPOSED TOP OF CURB AND FLOWLINE SPOT ELEVATION
- 15-Built PROPOSED FLOWLINE SPOT ELEVATION
- PROPOSED CURB (6" UNLESS OTHERWISE NOTED)
- PROPOSED FINISH FLOOR ELEVATION
- PROPOSED FLOWLINE
- EXISTING CONTOUR
- EXISTING TOP OF CURB AND FLOWLINE
- EXISTING SPOT ELEVATION
- EXISTING CURB
- SIGN
- CHAIN LINK FENCE
- SANITARY SEWER MANHOLE
- WATER METER
- WATER VALVE
- FIRE HYDRANT
- OVERHEAD ELECTRIC LINE
- GROUND LIGHT
- BASIN B DRAINAGE BASIN LABEL
- DRAINAGE BASIN BOUNDARY

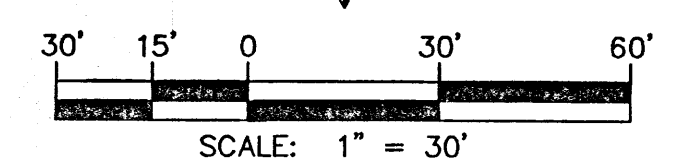
FUDDRUCKER'S GRADING AND DRAINAGE PLAN

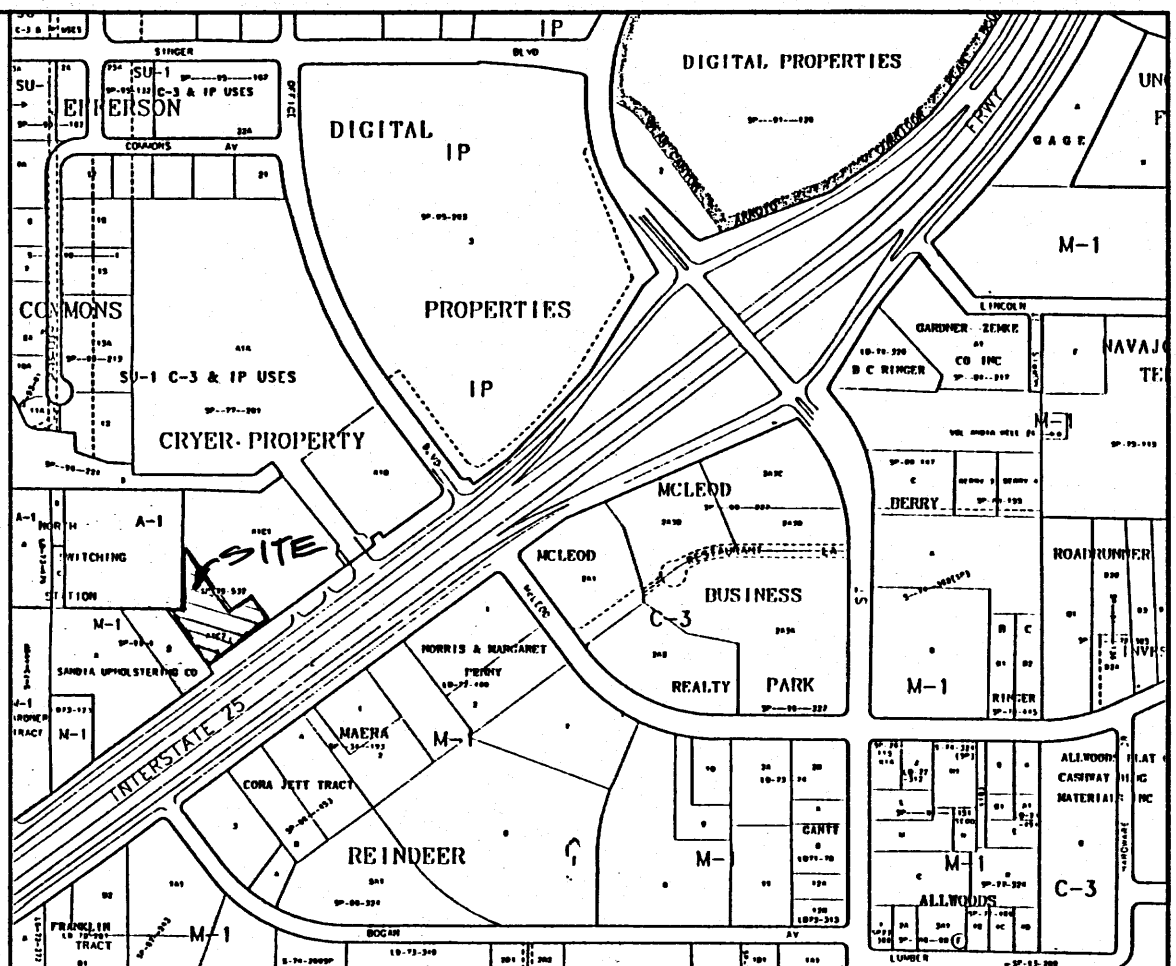
MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505)828-2200, FAX (505)797-9539

Designed: DPS Drawn: FSM, WLV Checked: DMG
Scale: 1" = 30' Date: 2-17-98 Job: 97111 Sheet 1 of 1

I hereby certify that the information contained on this drawing has been revised in accordance with information furnished by the contractor, S & J Enterprises, Inc., and by the surveyor, Joaquin Arguillas, Jr., and reflects the construction as actually accomplished. This plan as constructed is in substantial compliance with the Approved Plan.

Mark Goodwin 10/20/98
NMPE 8948





VICINITY MAP ZONE MAP: F-17-Z

ACS BENCHMARK

BENCHMARK
NIMSHC BRASS CAP
"125-18"
ELEVATION=5125.7

LEGAL DESCRIPTION

TRACT A-1-C-2, WITHIN SECTION 35,
TOWNSHIP 11 NORTH, RANGE 3 EAST, NMPM
CITY OF ALBUQUERQUE, BERNALILLO COUNTY
NEW MEXICO, OCTOBER 1997.

PROJECT NOTES

1. SITE DOES NOT LIE WITHIN A 100 YEAR FLOOD PLAIN.
2. OFF-SITE FLOWS DO NOT IMPACT SITE.

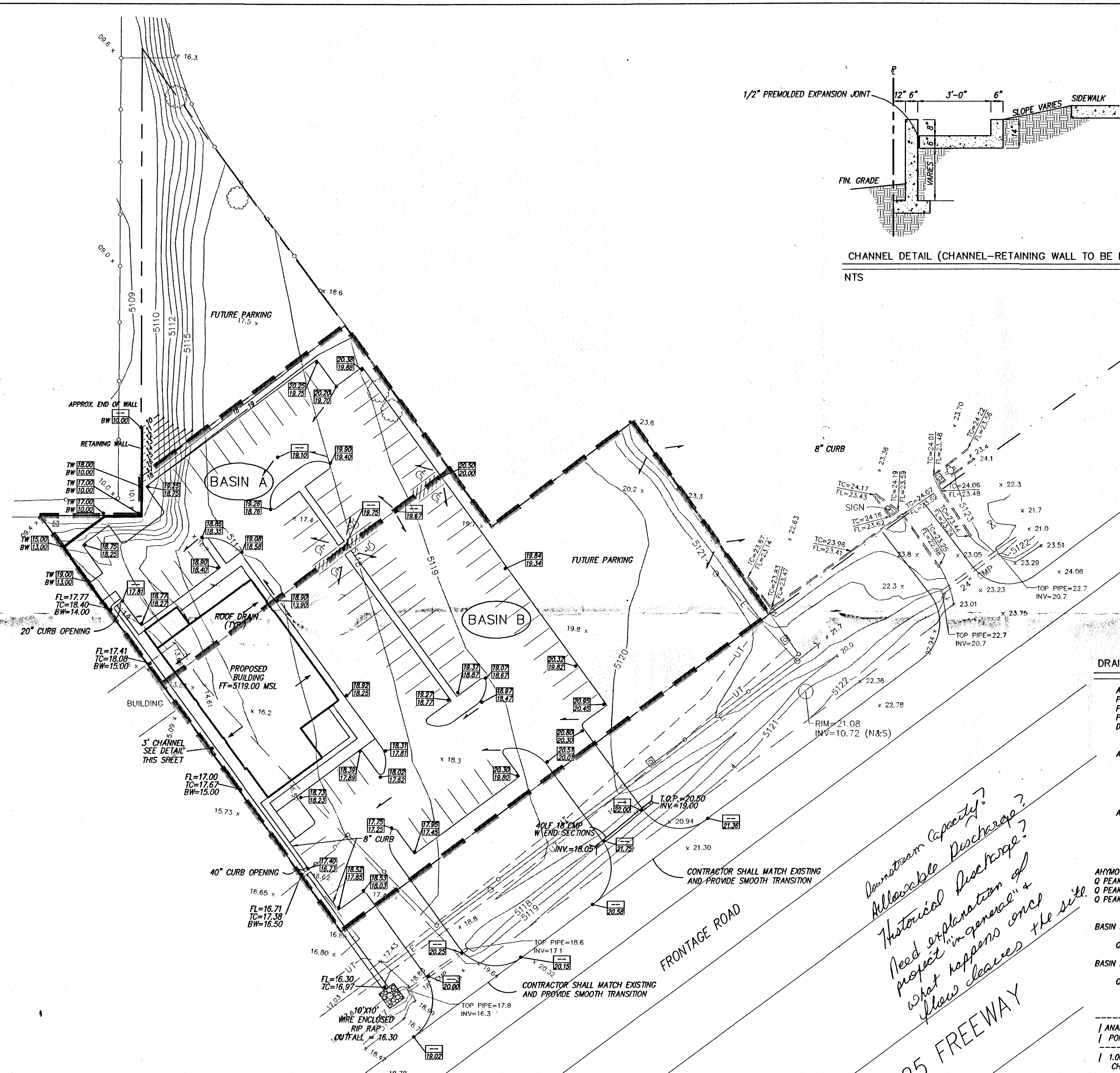
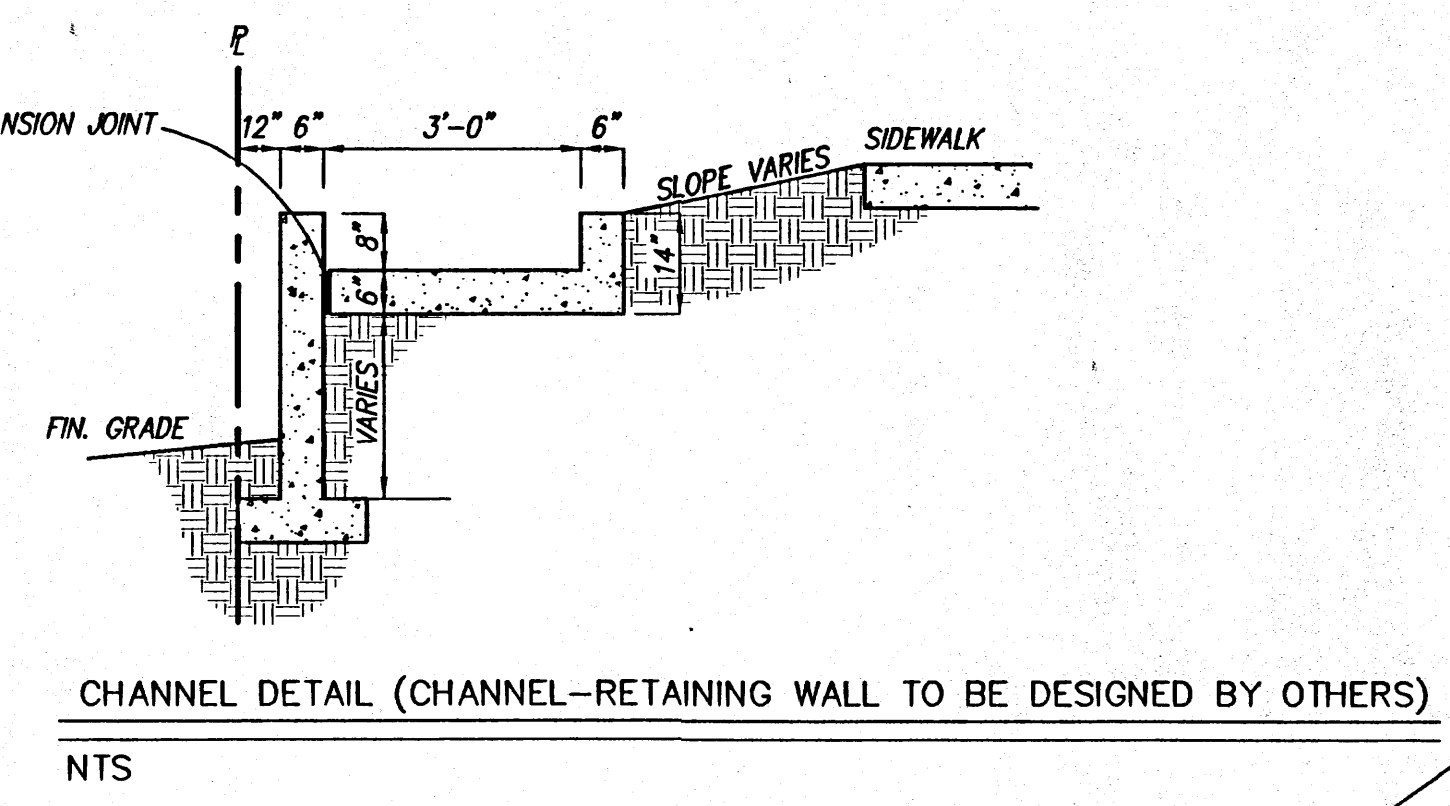
LEGEND

PROPOSED TOP OF WALL AND BOTTOM OF WALL ELEVATION
PROPOSED TOP OF CURB AND FLOWLINE SPOT ELEVATION
PROPOSED FLOWLINE SPOT ELEVATION
PROPOSED CURB
PROPOSED FINISH FLOOR ELEVATION
PROPOSED FLOWLINE
EXISTING CONTOUR
EXISTING TOP OF CURB AND FLOWLINE
EXISTING SPOT ELEVATION
EXISTING CURB
SIGN
CHAIN LINK FENCE
TREE
SANITARY SEWER MANHOLE
WATER METER
WATER VALVE
FIRE HYDRANT
OVERHEAD ELECTRIC LINE
POWER POLE
GUY WIRE
GROUND LIGHT
TELEPHONE PEDESTAL
DRAINAGE BASIN LABEL
DRAINAGE BASIN BOUNDARY

FUDDRUCKER'S
GRADING AND DRAINAGE PLAN

dmg D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505) 828-2200

Designed: DPS Drawn: FSM Checked: DMG Sheet 1 of 1
Scale: 1" = 30' Date: 10-29-97 Job: 97-111



DRAINAGE CALCULATIONS

A TOTAL = 0.002294 SQ MI
P60=2.01
P350=2.35
P1400=2.75
DT = 0.033 HR
TP = 0.133 HR

A BASIN A = 0.000575 SQ MI
TREATMENT
TYPE "A" = 0.0 %
TYPE "B" = 7.5 %
TYPE "C" = 7.5 %
TYPE "D" = 85.0 %

A BASIN B = 0.001719 SQ MI (INCL. FUTURE PARKING)
TREATMENT
TYPE "A" = 0.0 %
TYPE "B" = 7.5 %
TYPE "C" = 7.5 %
TYPE "D" = 85.0 %

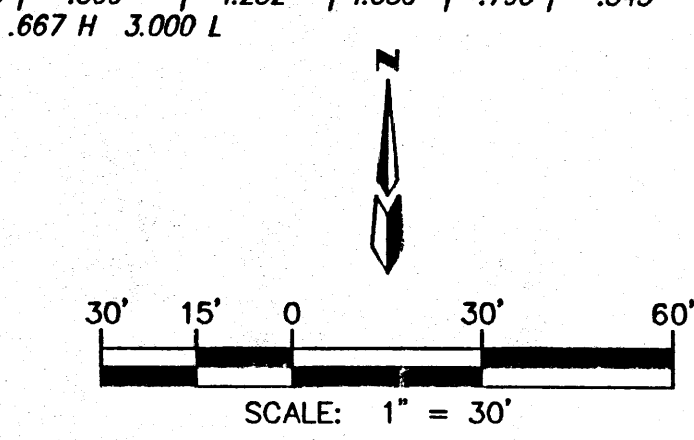
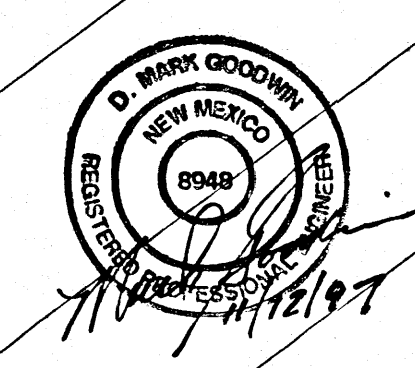
HYMO RESULTS
Q PEAK-BASIN A = 1.64 CFS
Q PEAK-BASIN B = 4.86 CFS
Q PEAK- TOTAL = 6.49 CFS

BASIN A WEIR
Q MAXIMUM=2.91 CFS
=1.71 > Q ACTUAL=1.64 CFS

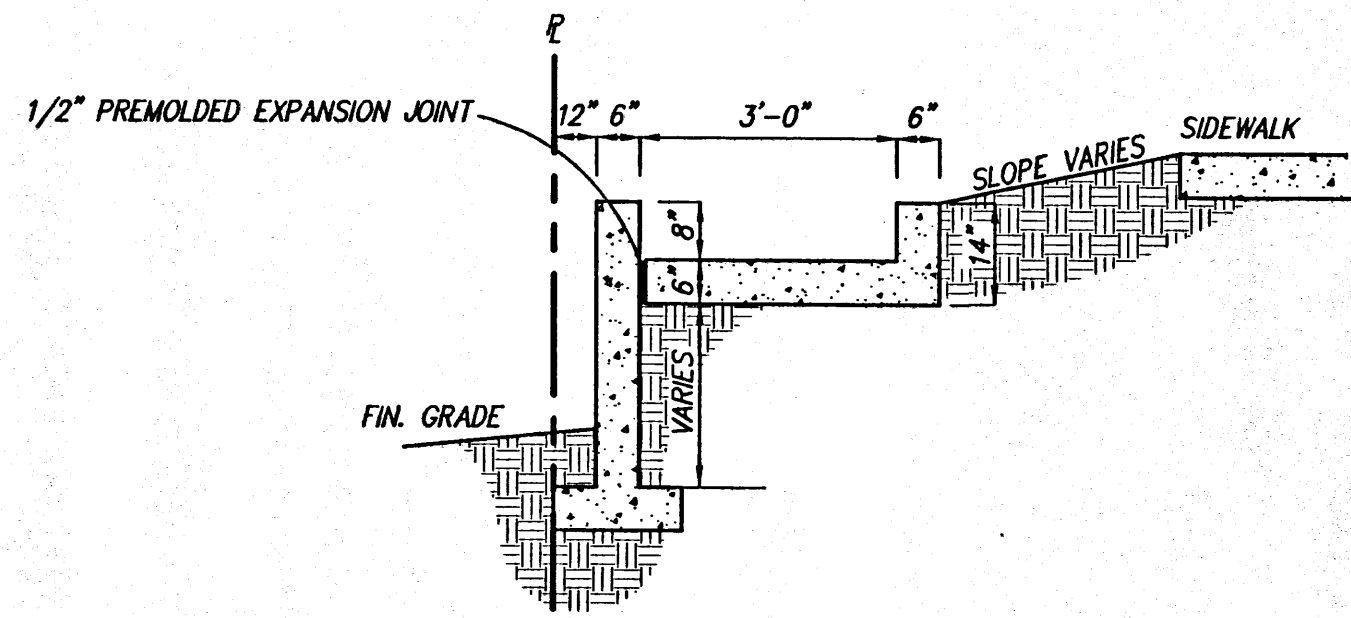
BASIN B WEIR
Q MAXIMUM=2.91 CFS
=5.26 > Q ACTUAL=4.86 CFS

SQUARE CHANNEL BASED ON 100-YR STORM
ANALYSIS | FLOW | FLOW | VELOCITY | F | E | HYD JUMP |
POINT | CFS | FT/FT | DEPTH FT | FT/S | FT | FT | FT |
1.000 | 6.49 | 1.00500 | .509 | 4.252 | 1.050 | .790 | .543 |
CHANNEL DIMENSIONS .667 H 3.000 L

*Downstream Capacity?
Allowable Discharge?
Historical Discharge?
Need explanation of
project "in general" &
what happens once
flow leaves the site.*

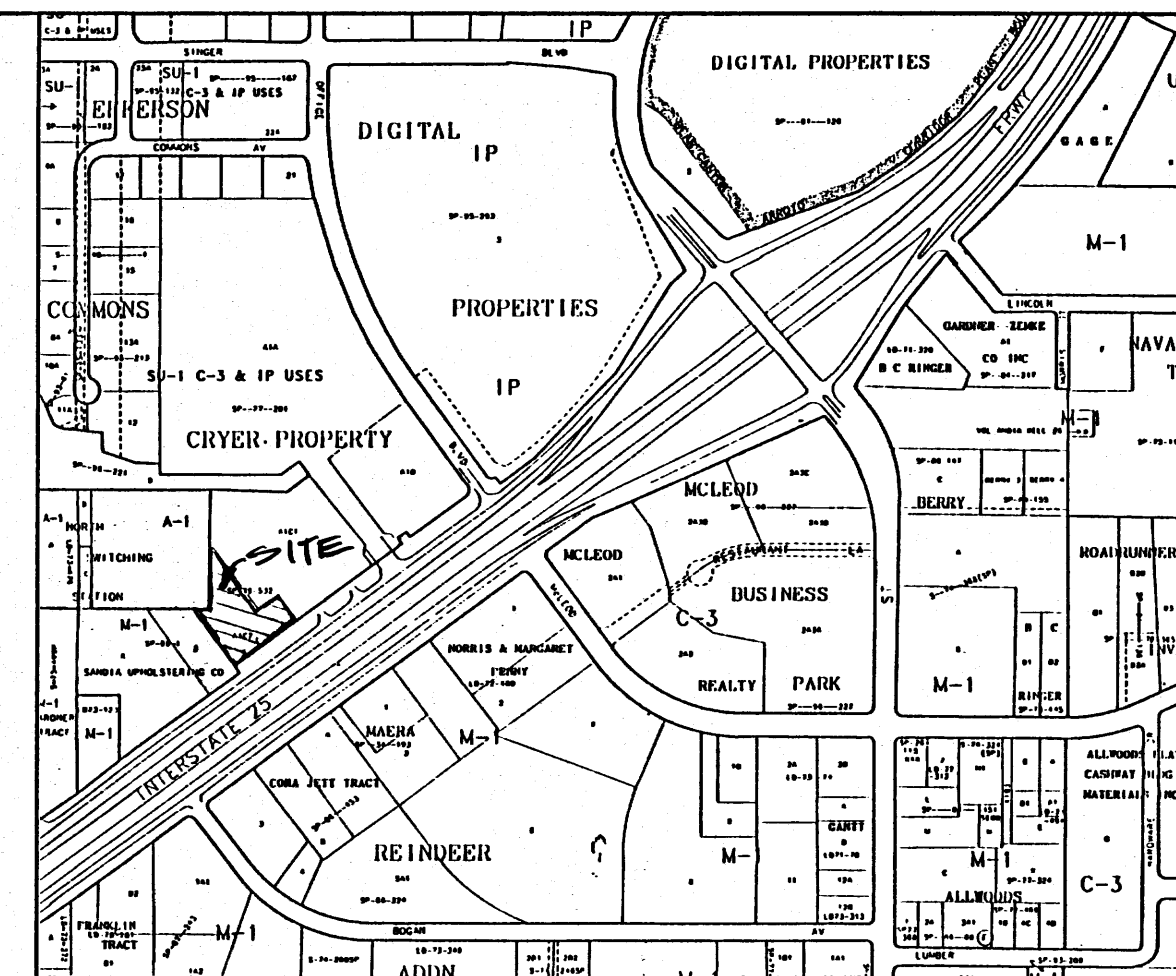


7111FUDD\7111GD30\11-11-97



CHANNEL DETAIL (CHANNEL-RETAINING WALL TO BE DESIGNED BY OTHERS)

ENTS



VICINITY MAF

ZONE MAP: F-17-Z

ACS BENCHMARK

BENCHMARK
NMSHC BRASS CAP
"125-18"
ELEVATION=5125.7

LEGAL DESCRIPTION

TRACT A-1-C-2, WITHIN SECTION 35,
TOWNSHIP 11 NORTH, RANGE 3 EAST, NMMP
CITY OF ALBUQUERQUE, BERNALILLO COUNTY
NEW MEXICO, OCTOBER 1997.

RECEIVED
NOV 12 1997
HYDROLOGY SECTION

PROJECT NOTES

- SITE DOES NOT LIE WITHIN A 100 YEAR FLOOD PLAIN.
OFF-SITE FLOWS DO NOT IMPACT SITE.

LEGEND

- | | |
|--|---|
| | PROPOSED TOP OF WALL AND BOTTOM OF WALL ELEVATION |
| | PROPOSED TOP OF CURB AND FLOWLINE SPOT ELEVATION |
| | PROPOSED FLOWLINE SPOT ELEVATION |
| | PROPOSED CURB |
| | PROPOSED FINISH FLOOR ELEVATION |
| | PROPOSED FLOWLINE |
| | EXISTING CONTOUR |
| | EXISTING TOP OF CURB AND FLOWLINE |
| | EXISTING SPOT ELEVATION |
| | EXISTING CURB |
| | SIGN |
| | CHAIN LINK FENCE |
| | TREE |
| | SANITARY SEWER MANHOLE |
| | WATER METER |
| | WATER VALVE |
| | FIRE HYDRANT |
| | OVERHEAD ELECTRIC LINE |
| | POWER POLE |
| | GUY WIRE |
| | GROUND LIGHT |
| | TELEPHONE PEDESTAL |
| | DRAINAGE BASIN LABEL |
| | DRAINAGE BASIN BOUNDARY |

(BASIN B)

DRAINAGE BASIN BOUNDARY

FUDDRUCKER'S

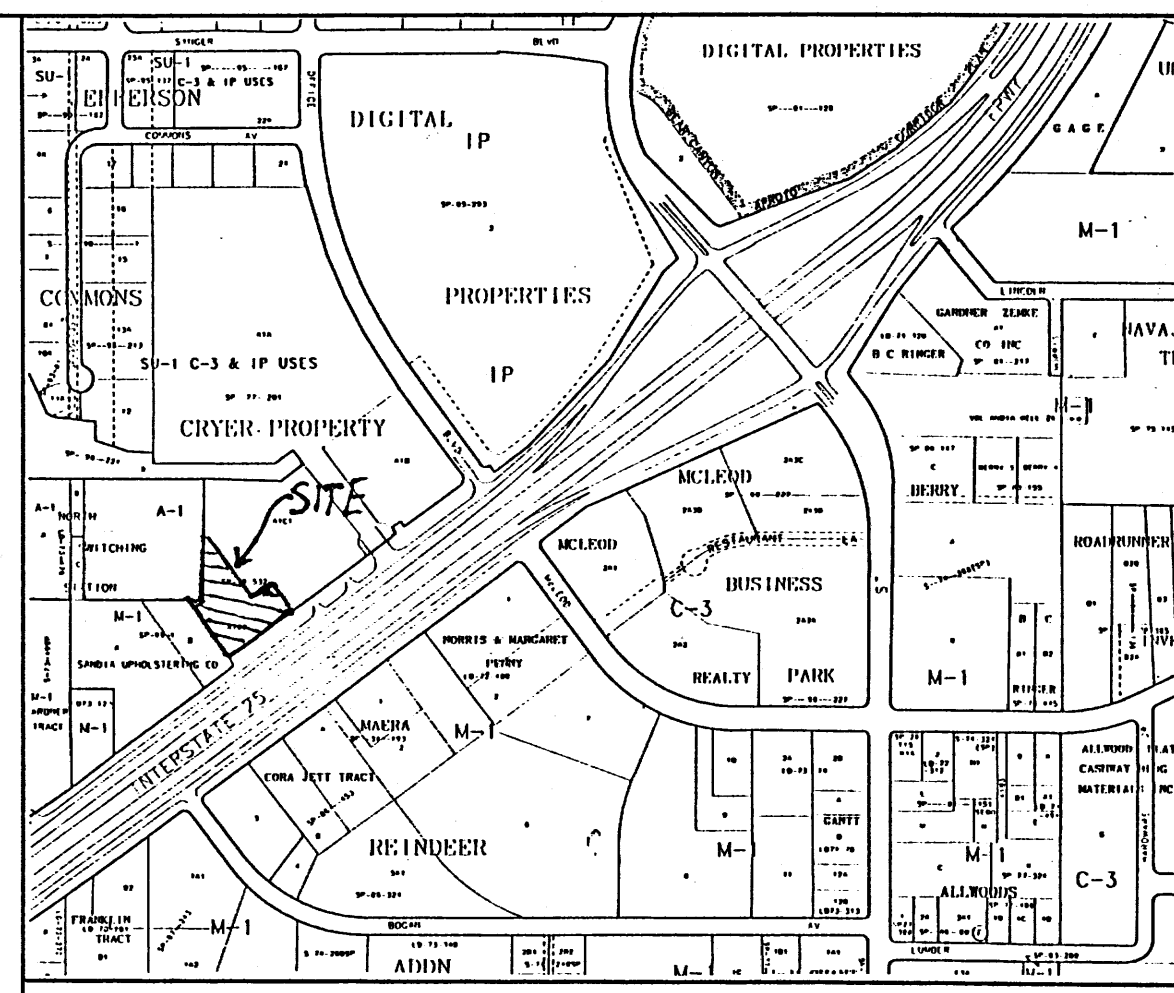
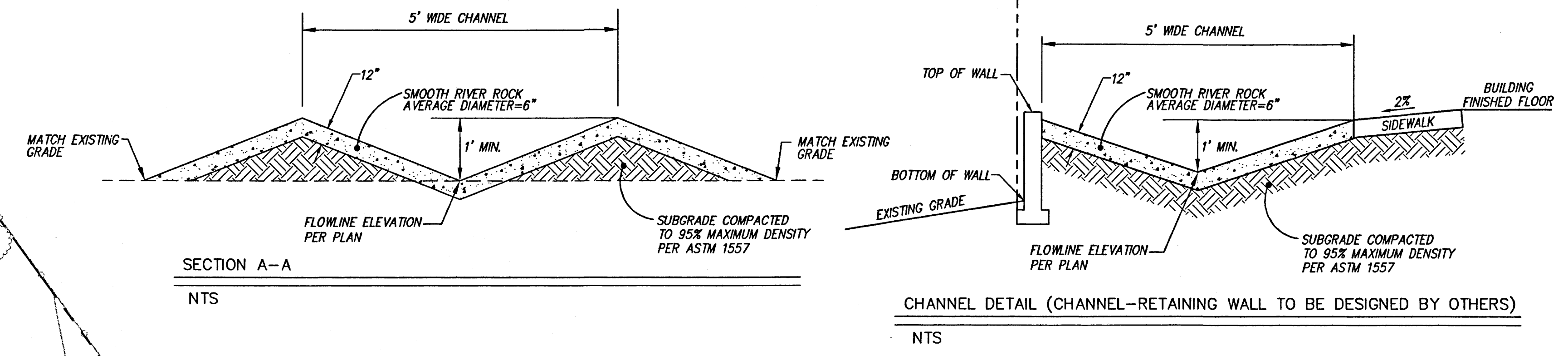
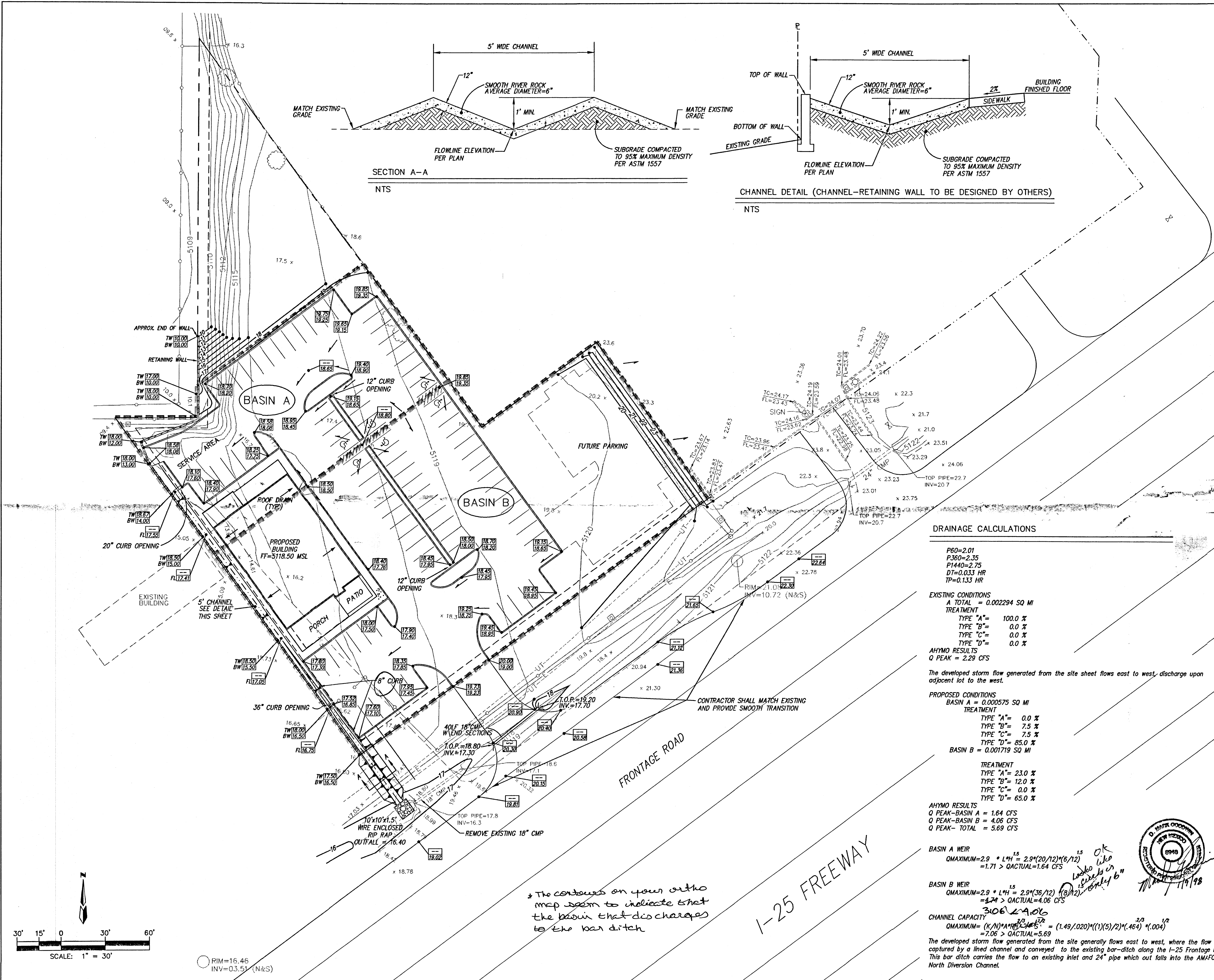
GRADING AND DRAINAGE PLAN

dmg

D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS

P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505) 828-2200

Designed: <i>DPS</i>	Drawn: <i>FSM</i>	Checked: <i>DMG</i>	Sheet 1 of 1
Scale: 1" = 30'	Date: 10-29-97	Job: 97-111	



VICINITY MAP ZONE MAP: F-17-Z

ACS BENCHMARK

NMSHC BRASS CAP "125-18" ELEVATION=5125.70

LEGAL DESCRIPTION

TRACT A-1-C-2, WITHIN SECTION 35, TOWNSHIP 11 NORTH, RANGE 3 EAST, NMPM CITY OF ALBUQUERQUE, BERNALILLO COUNTY NEW MEXICO, OCTOBER 1997.

- PROJECT NOTES
- SITE DOES NOT LIE WITHIN A 100 YEAR FLOOD PLAIN.
 - OFF-SITE FLOWS DO NOT IMPACT SITE.
 - CONTRACTOR SHALL VERIFY AND PROVIDE 2' MINIMUM DEPTH OF I-25 FRONTAGE ROAD BAR DITCH BETWEEN SITE AND OUT FALL TO THE AMAFCA NORTH DIVERSION CHANNEL.
 - RETAINING WALL TO BE DESIGNED BY OTHERS.

LEGEND

	PROPOSED TOP OF WALL AND BOTTOM OF WALL ELEVATION
	PROPOSED TOP OF CURB AND FLOWLINE SPOT ELEVATION
	PROPOSED FLOWLINE SPOT ELEVATION
	PROPOSED CURB (6' UNLESS OTHERWISE NOTED)
	PROPOSED FINISH FLOOR ELEVATION
	PROPOSED FLOWLINE
	EXISTING CONTOUR
	EXISTING TOP OF CURB AND FLOWLINE
	EXISTING SPOT ELEVATION
	EXISTING CURB
	SIGN
	CHAIN LINK FENCE
	TREE
	SANITARY SEWER MANHOLE
	WATER METER
	WATER VALVE
	FIRE HYDRANT
	OVERHEAD ELECTRIC LINE
	POWER POLE
	GUY WIRE
	GROUND LIGHT
	TELEPHONE PEDESTAL
	DRAINAGE BASIN LABEL
	DRAINAGE BASIN BOUNDARY

DRAINAGE CALCULATIONS

P60=2.01
P360=2.35
P1440=2.75
DT=0.033 HR
TP=0.133 HR

EXISTING CONDITIONS
A TOTAL = 0.002294 SQ MI
TREATMENT
TYPE "A"= 100.0 %
TYPE "B"= 0.0 %
TYPE "C"= 0.0 %
TYPE "D"= 0.0 %

AHYMO RESULTS
Q PEAK = 2.29 CFS

The developed storm flow generated from the site sheet flows east to west, discharge upon adjacent lot to the west.

PROPOSED CONDITIONS
BASIN A = 0.000575 SQ MI
TREATMENT
TYPE "A"= 0.0 %
TYPE "B"= 7.5 %
TYPE "C"= 7.5 %
TYPE "D"= 85.0 %

BASIN B = 0.001719 SQ MI
TREATMENT
TYPE "A"= 23.0 %
TYPE "B"= 12.0 %
TYPE "C"= 0.0 %
TYPE "D"= 65.0 %

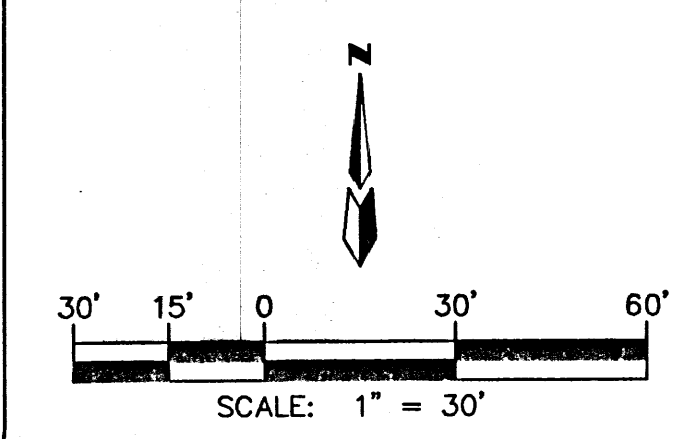
AHYMO RESULTS
Q PEAK-BASIN A = 1.64 CFS
Q PEAK-BASIN B = 4.06 CFS
Q PEAK- TOTAL = 5.69 CFS

BASIN A WEIR
 $Q_{MAXIMUM} = 2.9 \cdot L^{1.5} \cdot H^{1.5} = 2.9 \cdot (20/12)^{1.5} \cdot (6/12)^{1.5} = 1.71 > Q_{ACTUAL} = 1.64$ CFS

BASIN B WEIR
 $Q_{MAXIMUM} = 2.9 \cdot L^{1.5} \cdot H^{1.5} = 2.9 \cdot (36/12)^{1.5} \cdot (8/12)^{1.5} = 4.74 > Q_{ACTUAL} = 4.06$ CFS

CHANNEL CAPACITY
 $Q_{MAXIMUM} = (K/N) \cdot A \cdot R^{2/3} \cdot S^{1/2} = (1.49/0.020) \cdot (1/5) \cdot (2)^{2/3} \cdot (0.004)^{1/2} = 7.06 > Q_{ACTUAL} = 5.69$

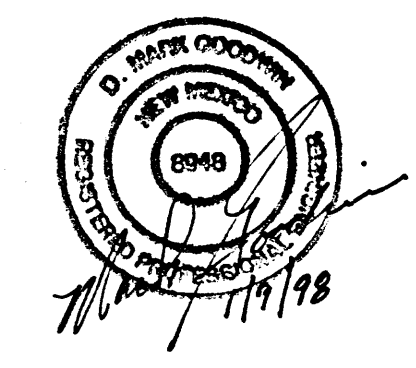
The developed storm flow generated from the site generally flows east to west, where the flow is captured by a lined channel and conveyed to the existing bar-ditch along the I-25 Frontage road. This bar ditch carries the flow to an existing inlet and 24" pipe which out falls into the AMAFCA's North Diversion Channel.



○ RIM=16.46
INV=03.51 (N&S)

* The contours on your ortho map seem to indicate that the basin that discharges to the bar ditch

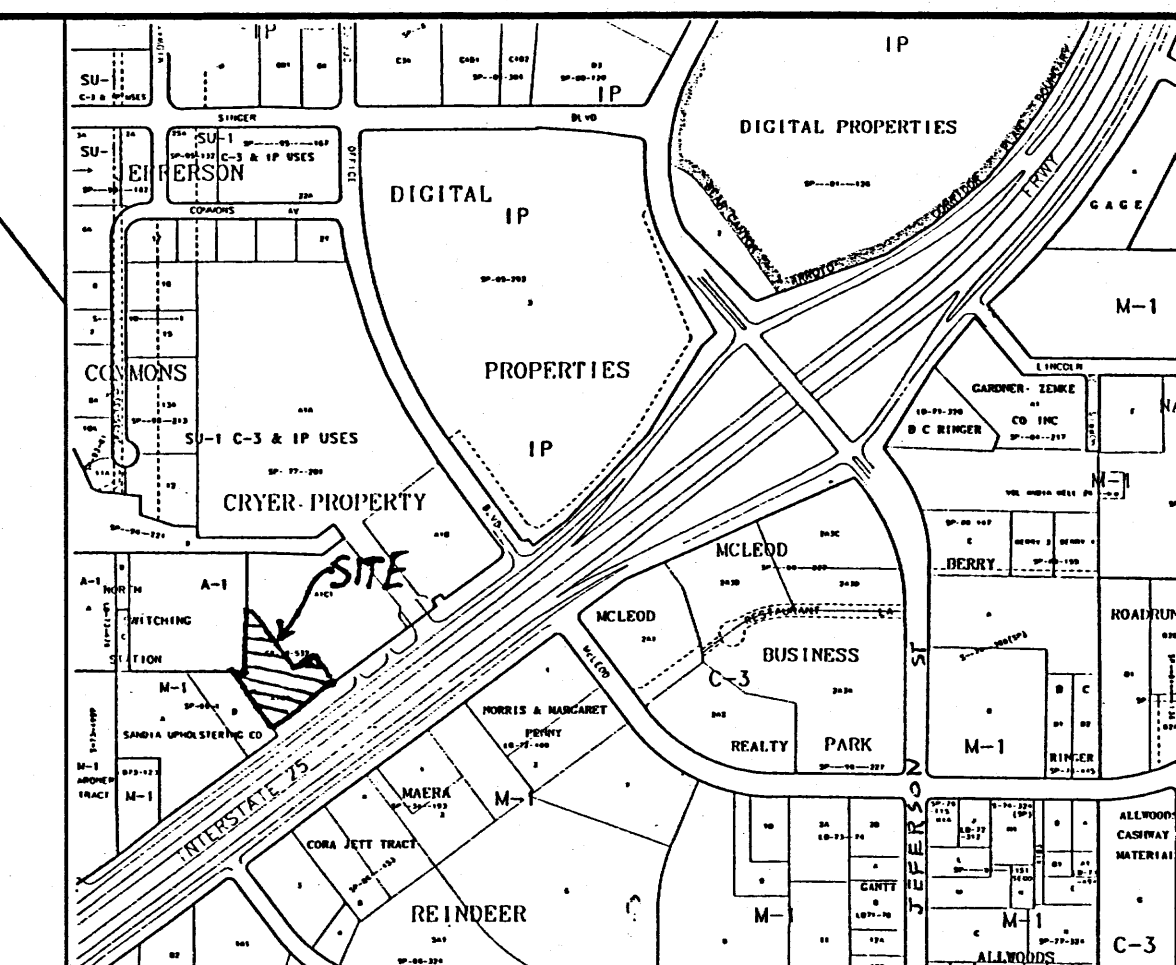
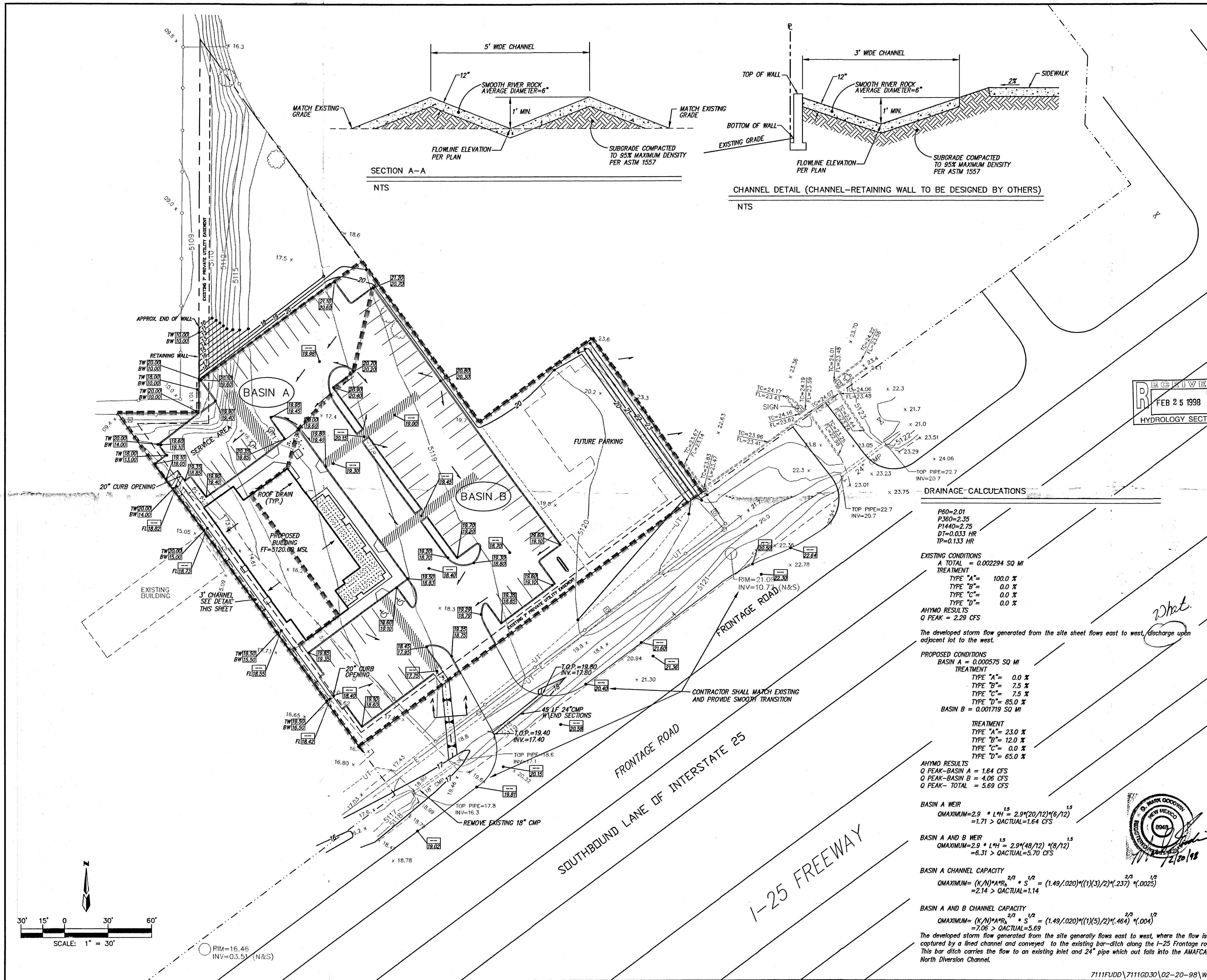
I-25 FREEWAY



FUDDRUCKER'S
GRADING AND DRAINAGE PLAN

D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505) 828-2200

Designed: DPS	Drawn: FSM, WL.V	Checked: DMG	Sheet 1 of 1
Scale: 1" = 30'	Date: 10-29-97	Job: 97-111	



VICINITY MAP ZONE MAP: F-17-Z

- PROJECT NOTES
1. SITE DOES NOT LIE WITHIN A 100 YEAR FLOOD PLAIN.
 2. OFF-SITE FLOWS DO NOT IMPACT SITE.
 3. CONTRACTOR SHALL VERIFY AND PROVIDE 2' MINIMUM DEPTH OF I-25 FRONTAGE ROAD BAR DITCH BETWEEN SITE AND OUT FALL TO THE AMAFCA NORTH DIVERSION CHANNEL.
 4. RETAINING WALL TO BE DESIGNED BY OTHERS.

A. C. S BENCHMARK
NMSHC BRASS CAP "125-18" ELEVATION=5125.70

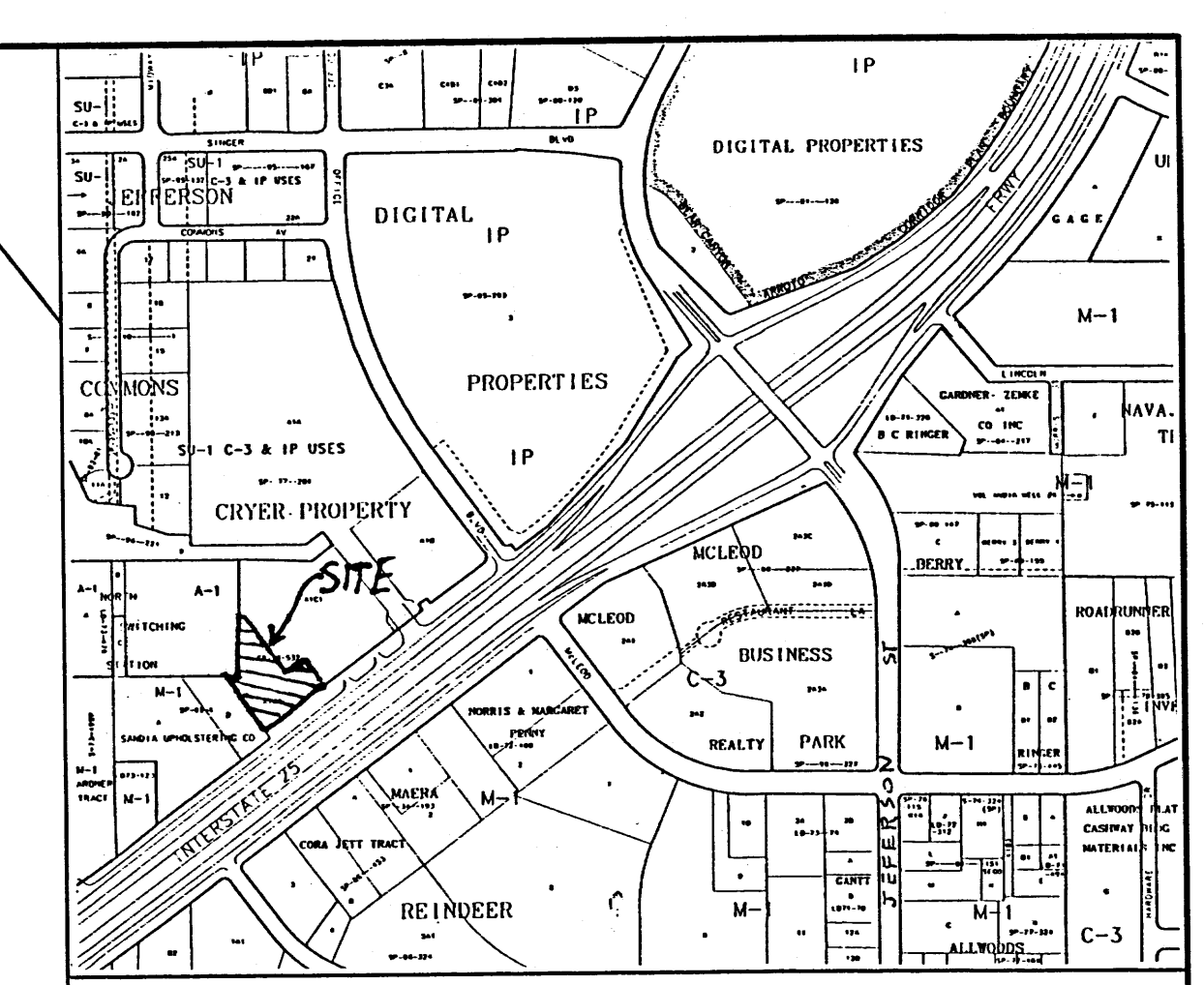
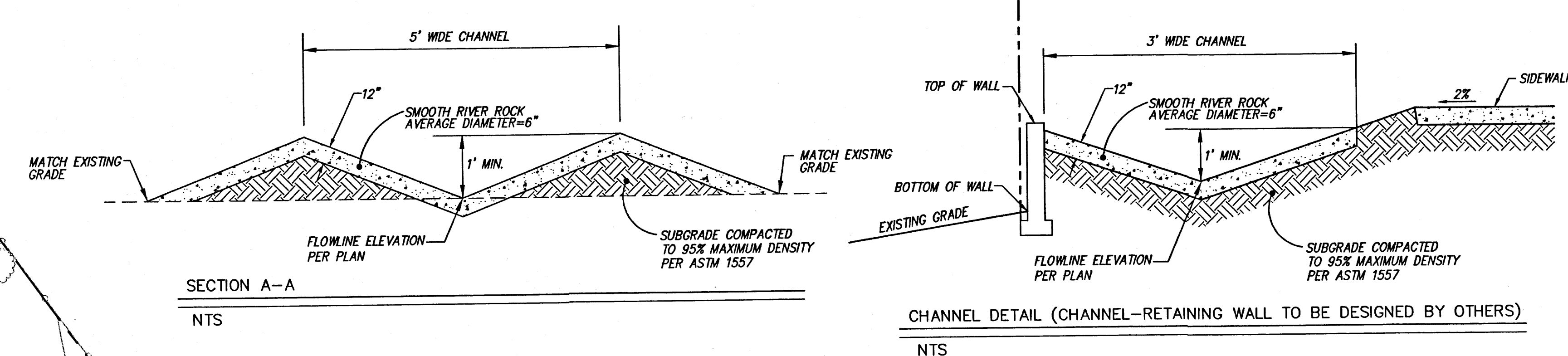
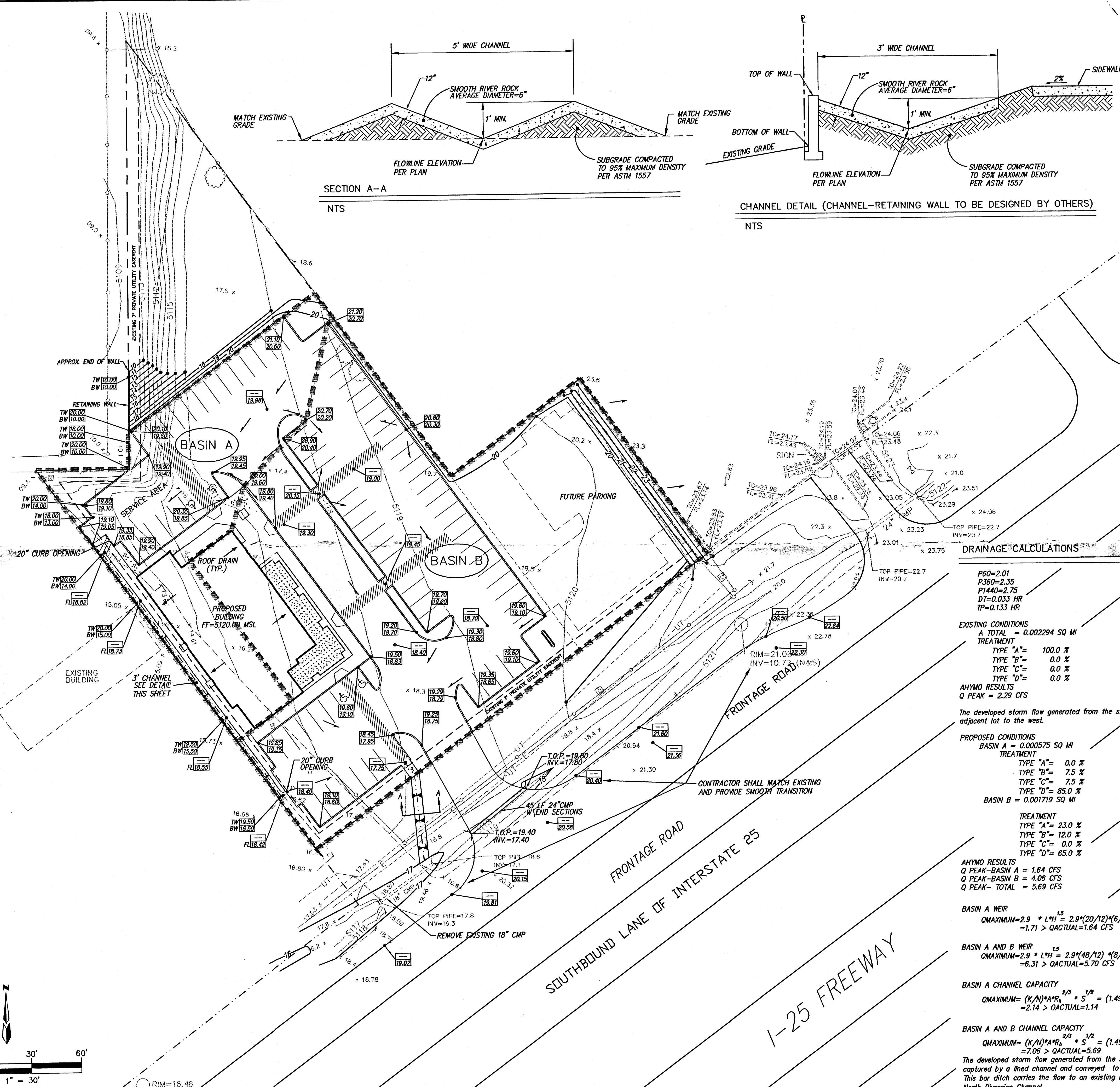
LEGAL DESCRIPTION
TRACT A-1-C-2, WITHIN SECTION 35,
TOWNSHIP 11 NORTH, RANGE 3 EAST, NMPM
CITY OF ALBUQUERQUE, BERNALILLO COUNTY
NEW MEXICO, OCTOBER 1997.

LEGEND

- PROPOSED TOP OF WALL AND BOTTOM OF WALL ELEVATION
- PROPOSED TOP OF CURB AND FLOWLINE SPOT ELEVATION
- PROPOSED FLOWLINE SPOT ELEVATION
- PROPOSED CURB (6" UNLESS OTHERWISE NOTED)
- PROPOSED WALL
- PROPOSED FINISH FLOOR ELEVATION
- PROPOSED FLOWLINE
- EXISTING CONTOUR
- EXISTING TOP OF CURB AND FLOWLINE
- EXISTING SPOT ELEVATION
- EXISTING CURB
- SIGN
- CHAIN LINK FENCE
- TREE
- SANITARY SEWER MANHOLE
- WATER METER
- WATER VALVE
- FIRE HYDRANT
- OVERHEAD ELECTRIC LINE
- POWER POLE
- GUY WIRE
- GROUND LIGHT
- TELEPHONE PEDESTAL
- DRAINAGE BASIN LABEL
- DRAINAGE BASIN BOUNDARY

FUDDRUCKER'S
GRADING AND DRAINAGE PLANS

MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505)828-2200, FAX (505)797-9539



VICINITY MAP ZONE MAP: F-17-Z

PROJECT NOTES

1. SITE DOES NOT LIE WITHIN A 100 YEAR FLOOD PLAIN.
2. OFF-SITE FLOWS DO NOT IMPACT SITE.
3. CONTRACTOR SHALL VERIFY AND PROVIDE 2' MINIMUM DEPTH OF I-25 FRONTAGE ROAD BAR DITCH BETWEEN SITE AND OUT FALL TO THE AMAFCA NORTH DIVERSION CHANNEL.
4. RETAINING WALL TO BE DESIGNED BY OTHERS.

A.C.S BENCHMARK

NMSHC BRASS CAP "125-18" ELEVATION=5125.70

LEGAL DESCRIPTION

TRACT A-1-C-2, WITHIN SECTION 35, TOWNSHIP 11 NORTH, RANGE 3 EAST, NMPM CITY OF ALBUQUERQUE, BERNALILLO COUNTY NEW MEXICO, OCTOBER 1997.

LEGEND

- PROPOSED TOP OF WALL AND BOTTOM OF WALL ELEVATION
- PROPOSED TOP OF CURB AND FLOWLINE SPOT ELEVATION
- PROPOSED FLOWLINE SPOT ELEVATION
- PROPOSED CURB (6" UNLESS OTHERWISE NOTED)
- PROPOSED WALL
- FF=5349.50 M.S.L. PROPOSED FINISH FLOOR ELEVATION
- PROPOSED FLOWLINE
- EXISTING CONTOUR
- EXISTING TOP OF CURB AND FLOWLINE
- EXISTING SPOT ELEVATION
- EXISTING CURB
- SIGN
- CHAIN LINK FENCE
- TREE
- SANITARY SEWER MANHOLE
- WATER METER
- WATER VALVE
- FIRE HYDRANT
- OVERHEAD ELECTRIC LINE
- POWER POLE
- GUY WIRE
- GROUND LIGHT
- TELEPHONE PEDESTAL
- BASIN B DRAINAGE BASIN LABEL
- DRAINAGE BASIN BOUNDARY

DRAINAGE CALCULATIONS

P60=2.01
P360=2.35
P1440=2.75
DT=0.033 HR
TP=0.133 HR

EXISTING CONDITIONS
A TOTAL = 0.002294 SQ MI
TREATMENT
TYPE "A" = 100.0 %
TYPE "B" = 0.0 %
TYPE "C" = 0.0 %
TYPE "D" = 0.0 %

AHYMO RESULTS
Q PEAK = 2.29 CFS

The developed storm flow generated from the site sheet flows east to west, discharge upon adjacent lot to the west.

PROPOSED CONDITIONS
BASIN A = 0.000575 SQ MI
TREATMENT
TYPE "A" = 0.0 %
TYPE "B" = 7.5 %
TYPE "C" = 7.5 %
TYPE "D" = 85.0 %

BASIN B = 0.001719 SQ MI
TREATMENT
TYPE "A" = 23.0 %
TYPE "B" = 12.0 %
TYPE "C" = 0.0 %
TYPE "D" = 65.0 %

AHYMO RESULTS
Q PEAK-BASIN A = 1.64 CFS
Q PEAK-BASIN B = 4.06 CFS
Q PEAK- TOTAL = 5.69 CFS

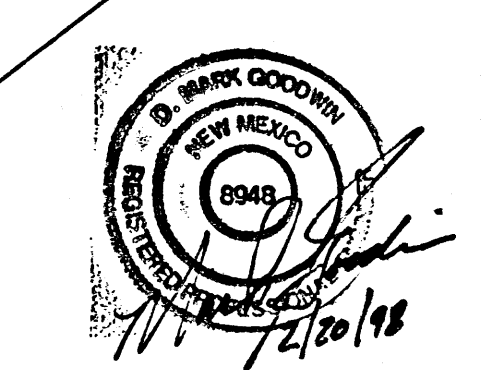
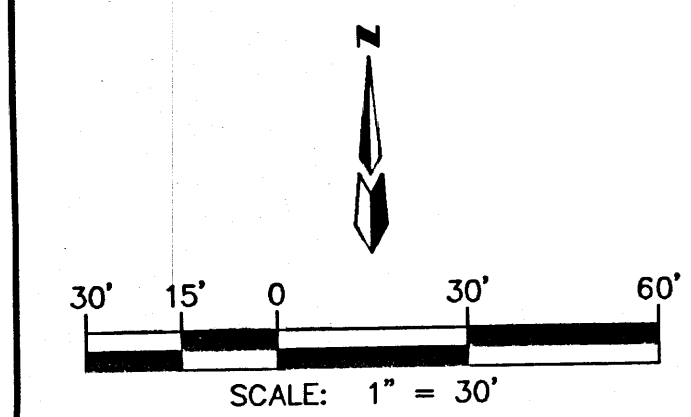
BASIN A WEIR
 $Q_{MAXIMUM} = 2.9 \cdot L^1 \cdot H^1.5 = 2.9 \cdot (20/12)^1 \cdot (6/12)^1.5 = 1.71 > Q_{ACTUAL} = 1.64$ CFS

BASIN A AND B WEIR
 $Q_{MAXIMUM} = 2.9 \cdot L^1 \cdot H^1.5 = 2.9 \cdot (48/12)^1 \cdot (8/12)^1.5 = 6.31 > Q_{ACTUAL} = 5.70$ CFS

BASIN A CHANNEL CAPACITY
 $Q_{MAXIMUM} = (K/N) \cdot A \cdot R^{2/3} \cdot S^{1/2} = (1.49/0.020) \cdot ((1/3)/2) \cdot (.237)^{2/3} \cdot (.0025)^{1/2} = 2.14 > Q_{ACTUAL} = 1.14$

BASIN A AND B CHANNEL CAPACITY
 $Q_{MAXIMUM} = (K/N) \cdot A \cdot R^{2/3} \cdot S^{1/2} = (1.49/0.020) \cdot ((1/5)/2) \cdot (.464)^{2/3} \cdot (.004)^{1/2} = 7.06 > Q_{ACTUAL} = 5.69$

The developed storm flow generated from the site generally flows east to west, where the flow is captured by a lined channel and conveyed to the existing bar-ditch along the I-25 Frontage road. This bar ditch carries the flow to an existing inlet and 24" pipe which out falls into the AMAFCA's North Diversion Channel.



FUDDRUCKER'S GRADING AND DRAINAGE PLAN

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Designed: DPS	Drawn: FSM, WLW	Checked: DMG	Sheet 1 of 1
Scale: 1" = 30'	Date: 2-17-98	Job: 97111	

7111FUDD\7111GD30\02-20-98\WLW

