CITY OF ALBUQUERQUE



January 23, 2012

Scott A. Eddings, P.E.

seddings@huitt-zollars.com

Huitt-Zollars, Inc.
333 Rio Rancho Blvd.
Rio Rancho, NM 87124

Re: Puente Azul Office Park, 4901 McLeod NE,

Request for Permanent C.O. - Approved

Engineer's Stamp dated: 12-07-2010, (F-17/D081)

Certification dated: 01-18-12

Dear Mr. Eddings,

Based upon the information provided in the Certification received 01-23-12, the above referenced Certification is approved for a release of a Permanent Certificate of Occupancy by Hydrology.

Hydrology is asking for an electronic copy, in .pdf format, of this certification for our

records. This certification can be e-mailed to me at: tsims@cabq.gov.

If you have any questions, you can contact me at 924-3982.

Albuquerque

NM 87103

PO Box 1293

Sincerely,

Timothy E. Sims,

Plan Checker—Hydrology Section
Development and Building Services

www.cabq.gov

C: CO Clerk—Katrina Sigala

File

DRAINAGE INFORMATION SHEET

(REV. 1/28/2003)

PROJECT TITLE:	Puente Azul Office Park (frmer midnight rodeo)	ZONE ATLAS/DRNG. F	ILE #: F-17-ZDO8
DRB #:	EPC#:	WORK OF	RDER #:
LEGAL DESCRIPTION:	Tract A Lands of Roadrunner Investor	<u> </u>	
CITY ADDRESS:	4901 MeLew) NE.	· · · · · · · · · · · · · · · · · · ·	
ENGINEERIN	G FIRM: Huitt-Zollars, Inc.	CONTACT:	Scott Eddings
AD	DRESS: 333 Rio Rancho Blvd.	PHONE:	892-5141
CITY,	STATE: Rio Rancho, NM	ZIP CODE:	87124
OWNER: Puente	e Azul, LLC	CONTACT:	Victoria Martin
AD	DRESS:	PHONE:	480-2866
CITY,	STATE:	ZIP CODE:	
ARCHITECT:	Jud Cervanak	CONTACT:	Jud Cervanak
AD	DRESS:	PHONE:	266-2066
CITY,	STATE:	ZIP CODE:	
SURVEYOR:	Huitt-Zollars, Inc.	CONTACT:	
AD	DRESS: 333 Rio Rancho Blvd	PHONE:	
CITY,	STATE: Rio Rancho, NM	ZIP CODE:	•
CONTRACTOR:		CONTACT:	
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CITY,	STATE:	ZIP CODE:	
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WAS A PRE-DESIGN ME YES X NO COPY PROVI			JAN 2 3 2012 HYDROLOGY SECTION
DATE SUBMITTED	: 1/23/12	BY: Scott Edding	'S

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or More of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5)
- 3. Drainage Report: Required for subdivisions containing more than (10) lots or constituting five (5) acres

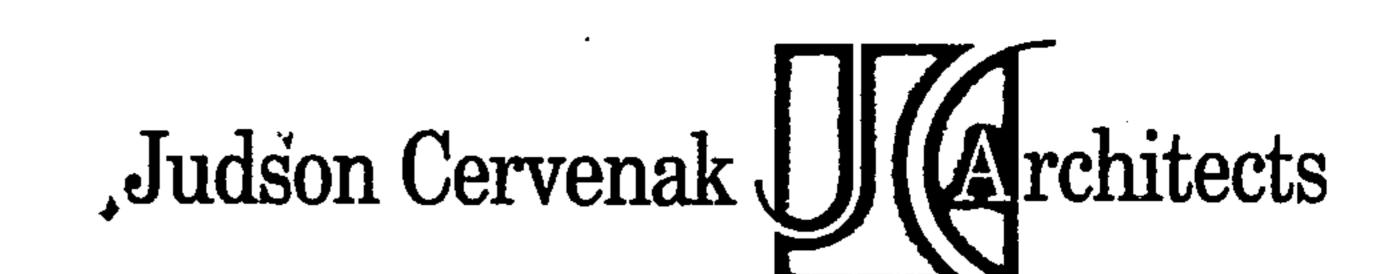
CITY OF ALBUQUERQUE Transportation Development Services Section Judson vv. Cervenak, Registered Architect Judson Cervenak Architects PO Box 23084 Santa Fe, NM 87502 Certification for Permanent Certificate of Occupancy (C.O. Gerald Martin Shop Building [F-17/D081] 4901 McCleod NE Architect's Stamp Dated 01/11/12 Dear Mr. Cervenak: Based upon the information provided in your submittal received 01-11-12, ransportation Development has no objection to the issuance of a Permanent Certificate. of Occupancy. PO Box 1293 This letter serves as a "green tag" from Transportation Development for a Permanent Certificate of Occupancy to be issued by the Building and Safety Division. If you have any questions, please contact me at (505)924-3630. www.cabq.gov Mild E. Salgado-Fernandez, P.E. Senior Traffic Engineer Development and Building Services Planning Department ------- Engineer Hydrology file CO Clerk

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (Rev. 12/2005)

PROJECT TITLE: Gerald Martin Shop Building	ZONE MAP/DRG. FILE #: F-17
DRB#: EPC#:	WORK ORDER#:
LEGAL DESCRIPTION: Lots A & B4, Lands of Roadrunr	ner Investors. Bernalillo County New Mexico
CITY ADDRESS: 4901 Mcleod NE	The state of the s
	•
ENGINEERING FIRM:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
OWNER: Gerald A. Martin, Ltd	CONTACT: Kameron Cheney – Project Mgr.
ADDRESS: 8501 Jefferson St. NE	PHONE: <u>505-828-1144</u>
CITY, STATE: Albuquerque, NM	ZIP CODE: 87113
ARCHITECT: Judson Cervenak Architects	CONTACT: <u>Judson Cervenak</u>
ADDRESS: PO Box 23084	PHONE: <u>505-983-3400</u>
CITY, STATE: Santa Fe, NM	ZIP CODE: <u>87502</u>
SURVEYOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
CONTRACTOR: Gerald Martin General Contractor	CONTACT: Kameron Cheney - Project Mgr.
ADDRESS: <u>8501 Jefferson St. NE</u>	PHONE: <u>505-828-1144</u>
CITY, STATE: Albuquerque, NM	ZIP CODE: <u>87113</u>
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CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. DEPMIT ADDROVAL
GRADING PLAN	SECTOR BLDG. PERMIT APPROVAL
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ENGINEER'S CERT (HYDROLOGY)	FOUNDATION DEPMIT ADDROVAL
CLOMR/LOMR	FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL
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X ENGINEER'S/ARCHITECT'S CERT (TCL)	CERTIFICATE OF OCCUPANCY (FERM) CERTIFICATE OF OCCUPANCY (TEMP)
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WAS A PRE-DESIGN CONFERENCE ATTENDED:	
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COPY PROVIDED	
SUBMITTED BY: SU	$1/\sqrt{1}$
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

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- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.



P.O. Box 23084
Santa Fe, NM 87502
505.983.3400
505.983.3402 fax
iud@jwcervenak.com

January 11, 2012

Krisal Metro, P.E.
Traffic Engineer, Planning Dept.
Development and Building Services
City of Albuquerque
PO Box 1293
Albuquerque, NM 87103

RE: 4901 Mcleod, NE - TCL Certification for Certificate of Occupancy

Dear Ms. Metro.

I, <u>Judson Cervenak</u>, NMRA, OF THE FIRM <u>Judson Cervenak Architects</u>, HEREBY CERTIFY THAT THIS PROJECT IS IN SUBSTANTIAL COMPLIANCE WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF APPROVED TCL DATED <u>February 18, 2011</u>. THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY <u>Judson Cervenak</u> OF THE FIRM <u>Judson Cervenak Architects</u>. I FURTHER CERTIFY THAT I HAVE PERSONALLY VISITED THE PROJECT SITE ON <u>January 10, 2012</u> AND HAVE DETERMINED BY VISUAL INSPECTION THAT THE SURVEY DATA PROVIDED IS REPRESENTATIVE OF ACTUAL SITE CONDITIONS AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR <u>Certificate of Occupancy</u>.

Indicated in red on the original TCL are as-built conditions that vary from the original design document. These are described below.

- The accessible path to the shop building, across the central parking lot was striped approximately 5'
 east of the original location.
- 2. Parking spaces in the two central parking islands were striped 9'-2" wide instead of 8'-6" wide. This results in 9 parking spaces per row instead of 10. Accordingly, 4 less spaces are provided in this island. These spaces are provided elsewhere, on the east property line, adjacent to the shop building.
- 3. Due to expansion of the dumpster enclosure, 6 parking spaces are provided north of the enclosure instead of 7 originally indicated. The omitted parking space has been provided elsewhere, on the east property line, adjacent to the shop building.
- 4. 2 motorcycle parking spaces were relocated. One on the south side of the shop building and one on the north side of the office Phase 1b building.



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JAN 1 1 2012

HYDROLOGY
SECTION



- 5. Parking spaces in the SE corner of the central parking lot, adjacent to the Phase 1b building were striped 9'-2" wide, instead of 8'-6" wide. As a result, 1 less space is provided in this bay. The omitted parking space has been provided elsewhere, on the east property line, adjacent to the shop building.
- 6. 3 parking spaces on the south wall of the shop building have been changed to Small Car spaces due to depth restrictions. These spaces are 16' deep and 8'-6" wide.

No parking deficiencies are present or required corrections necessary.

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE TRAFFIC ASPECTS OF THIS PROJECT. THOSE RELYING ON THE RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.

Judson Cervenak, AIA, LEED AP

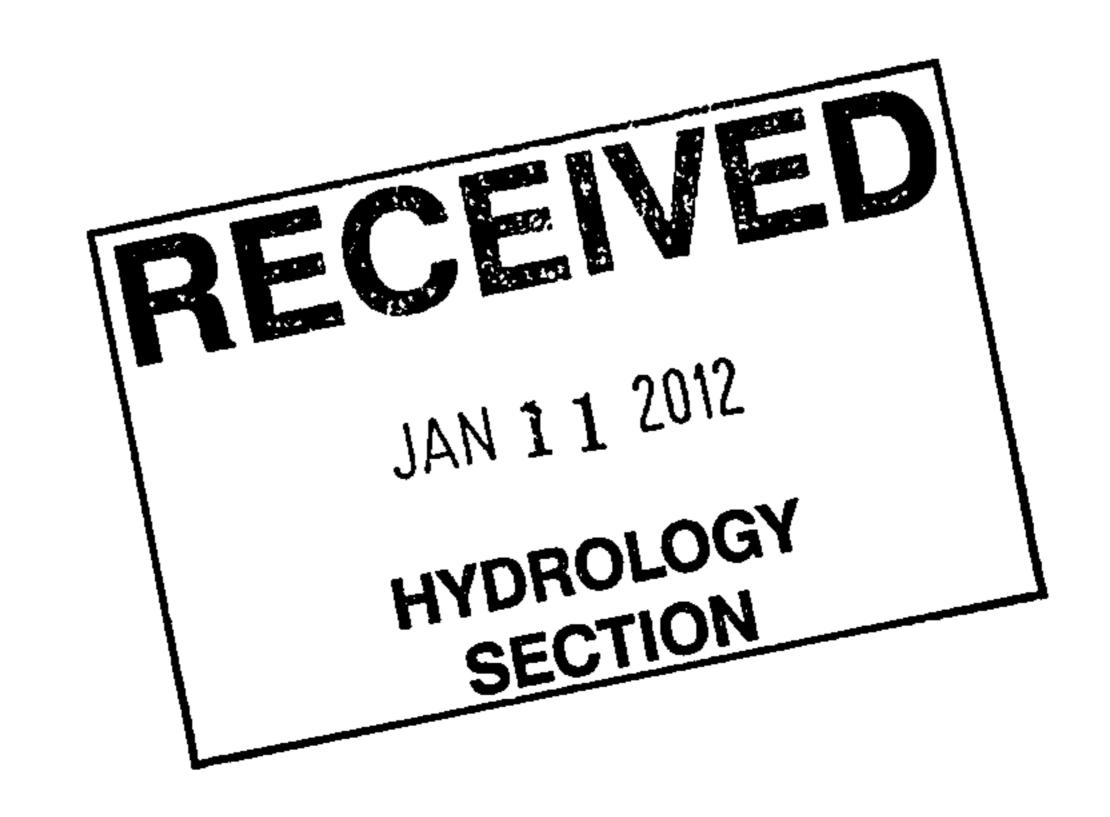
JUDSON W.
CERVENAK
NO. 2948

RED ARCHITECT

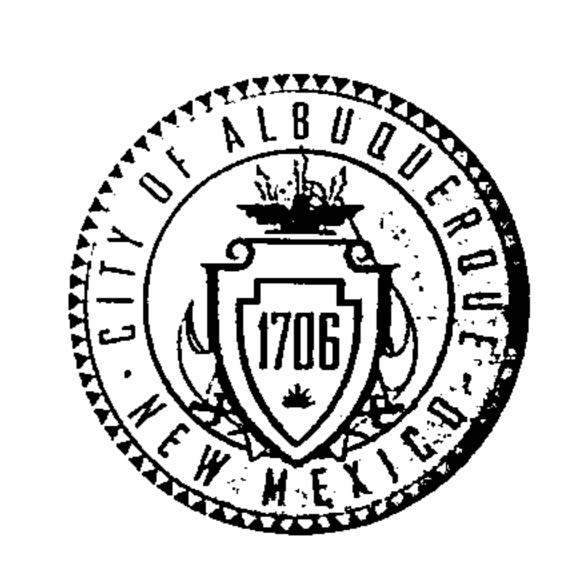
January 11, 2012

Attachments:

Drainage & Transportation Information Sheet Approved TCL



CITY OF ALBUQUERQUE



March 1, 2011

Judson Cervenak, R.A. Judson Cervenak Architects P.O. Box 23084 Santa Fe, NM 87502

Re:

Gerald Martin Shop Building, 4901 McLeod Rd NE,

Traffic Circulation Layout

Architect's Stamp dated 02-18-11 (F17-D081)

Dear Mr. Cervenak,

The TCL submittal received 02-28-11 is approved for Building Permit. The plan is stamped and signed as approved. A copy of this plan will be needed for each of the building permit plans. Please keep the original to be used for certification of the site for final C.O. for Transportation. Public infrastructure or work done within City Right-of-Way shown on these plans is for information only and is not part of approval. A separate DRC and/or other appropriate permits are required to construct these items.

PO Box 1293

If a temporary CO is needed, a copy of the original TCL that was stamped as approved by the City will be needed. This plan must include a statement that identifies the outstanding items that need to be constructed or the items that have not been built in "substantial compliance," as well as the signed and dated stamp of a NM registered architect or engineer. Submit this TCL with a completed <u>Drainage and Transportation Information Sheet</u> to Hydrology at the Development Services Center of Plaza Del Sol Building.

Albuquerque

NM 87103

When the site is completed and a final C.O. is requested, use the original City stamped approved TCL for certification. A NM registered architect or engineer must stamp, sign, and date the certification TCL along with indicating that the development was built in "substantial compliance" with the TCL. Submit this certification TCL with a completed <u>Drainage and Transportation Information Sheet</u> to Hydrology at the Development Services Center of Plaza Del Sol Building.

www.cabq.gov

Once verification of certification is completed and approved, notification will be made to Building Safety to issue Final C.O. To confirm that a final C.O. has been issued, call Building Safety at 924-3306.

Sincerely,

Kristal D. Metro, P.E.

Traffic Engineer, Planning Dept.

Development and Building Services

C:

File

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 12/2005)

	DRB#:	Martin Strop Blog EPC#:	WORK ORDER#:
	·		TOTAL OIGHT.
	LEGAL DESCRIPTION:		
	CITY ADDRESS:		
	ENGINEERING FIRM:		CONTACT:
	ADDRESS:		PHONE:
	CITY, STATE:		ZIP CODE:
	OWNER:		CONTACT:
1	ADDRESS:	·	PHONE:
	CITY, STATE:	,	ZIP CODE:
1		Com - 11/2 1/2 1/2	
Y		EPVENAL, APCHITECT	
· \ \	ADDRESS: B	X 73084	PHONE: <u>505</u> 983-3
	CITY, STATE:	ANTA FE, NM BIG	ZIP CODE: _875_02
K	SURVEYOR:		CONTACT:
0	ADDRESS:		PHONE:
) [CITY, STATE:		ZIP CODE:
]	CONTRACTOR:		CONTACT:
	ADDRESS:		PHONE:
	CITY, STATE:		ZIP CODE:
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location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

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CITY OF ALBUQUERQUE



February 14, 2011

Judson Cervenak, R.A.
Judson Cervenak Architects
P.O. Box 23084
Santa Fe, NM 87502

Re: Gerald Martin Shop Building, 4901 McLeod Rd NE,

Traffic Circulation Layout

Architect's Stamp dated 01-31-11 (F17-D081)

Dear Mr. Cervenak,

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

Based upon the information provided in your submittal received 02-02-11, the above referenced plan cannot be approved for Building Permit until the following comments are addressed:

1. Sheet C102 does not provide a ramp detail; please revise.

2. Label the compact parking spaces by placing the words "compact" on the pavement of each space. all compact spaces compacts

3. Provide hatching across the pedestrian pathways.

4. Clarify the purpose of the ramp in the northwest corner of the site.

5. Revise the on-site ramps to utilize a flat landing area.

6. Sheet C102 has no detail numbers.

7. Clearly label the sidewalk culvert.

8. A cross access easement between tracts A and B4 must be provided... Defer to Ph 2

9. Provide a 6-foot wide, ADA accessible pedestrian pathway from McLeod to the buildings.

10. The sidewalk adjacent to Phase 1B must be built with this project. Please revise the limits of Phase 1B.

Please refer to the redlined TCL markup with comments (included). If you have any questions, you can contact me at 924-3991.

Sincerely,

Kristal D. Metro, P.E.

Traffic Engineer, Planning Dept.

Development and Building Services

C: File

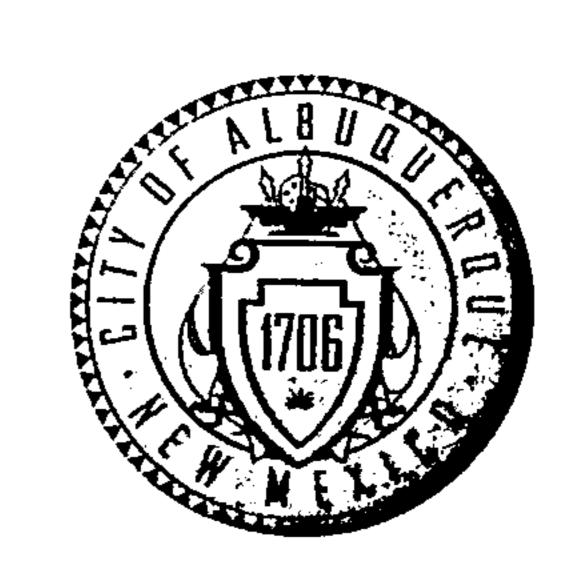
Albuquerque - Making History 1706-2006

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PROJECT TITLE: EXAL DRB#:		BUG.	ZONE MAP: #
LEGAL DESCRIPTION:	EPC#: OTS A & B 4 WCLEOD PD,	ANDS OF POATPUN NE	
ENGINEERING FIRM:ADDRESS:CITY, STATE:		C	ONTACT:HONE:P CODE:
OWNER: (FEAD) A ADDRESS: 2 CITY, STATE: 1	MAPTIN LID EFERNAL BUREPLANE, A	<u>VF</u> PI	ONTACT: HONE: 328-1144 P CODE: 37/13
CITY, STATE:	SUX 25/25	PF	ONTACT JUD (ERUFAL) HONE: 933-3400 P CODE: 97502
SURVEYOR: ADDRESS: CITY, STATE: CONTRACTOR.			ONTACT:
CONTRACTOR:ADDRESS: CITY, STATE:		PH	ONTACT:ONE:
TYPE OF SUBMITTAL: DRAINAGE REPORE DRAINAGE PLAN DRAINAGE PLAN CONCEPTUAL G & GRADING PLAN EROSION CONTRO ENGINEER'S CERT CLOMR/LOMR TRAFFIC CIRCULA ENGINEER'S CERT ENGINEER'S CERT OTHER (SPECIFY)	TION LAYOUT (ST SUBMITTAL RESUBMITTAL D PLAN (HYDROLOGY) TION LAYOUT (TCL)	PRELIMINARY PLA S. DEV. PLAN FOR S. DEV. FOR BLDG SECTOR PLAN APP FINAL PLAT APPRO FOUNDATION PER BUILDING PERMIT CERTIFICATE OF C	JARANTEE RELEASE T APPROVAL SUB'D APPROVAL PERMIT APPROVAL ROVAL MIT APPROVAL APPROVAL CCUPANCY (PERM) CCUPANCY (TEMP) APPROVAL PPROVAL ROVAL
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

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CITY OF ALBUQUERQUE



December 8, 2010

Scott Eddings, P.E. Huitt-Zollars, Inc. 333 Rio Rancho Blvd Rio Rancho, NM 87124

Re: Puenta Azul Business Park

Grading and Drainage Plan

Engineer's Stamp dated 12-7-10 (F17/D081)

Dear Mr. Eddings,

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

Based upon the information provided in your submittal received 12-8-10, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

This project requires a National Pollutant Discharge Elimination System (NPDES) permit for storm water discharge and a Topsoil Disturbance Permit since it is disturbing ¾ of an acre or more.

Hydrology is requesting that proposed landscape areas be depressed to retain/infiltrate the rain that falls on them.

Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

C: File

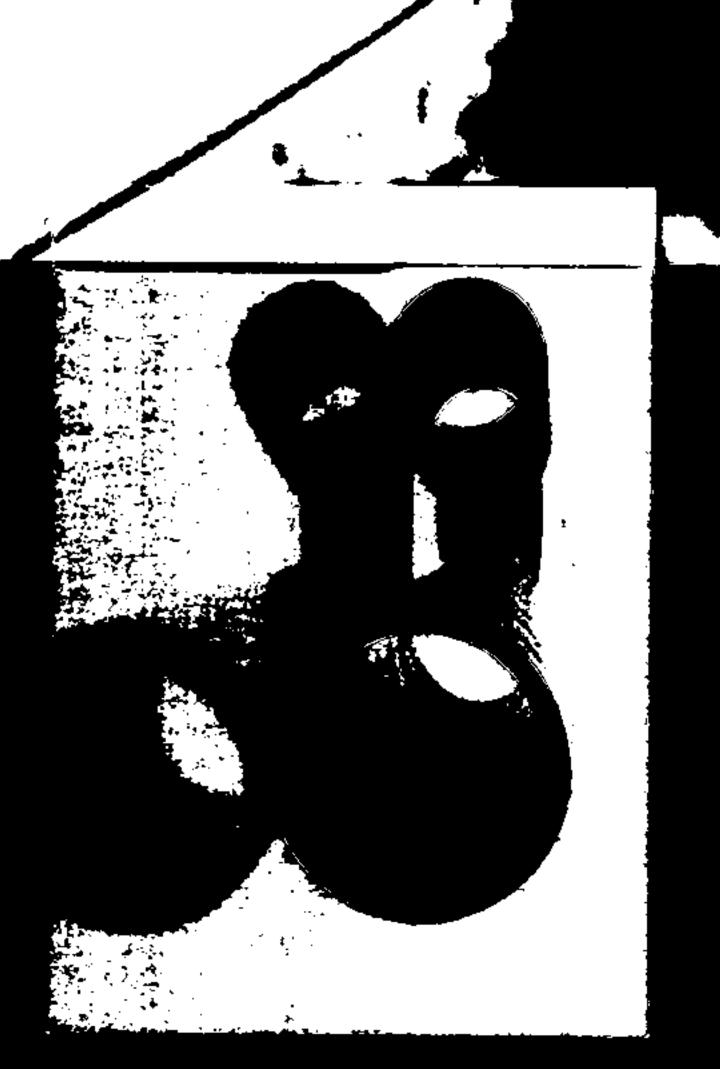
DRAINAGE INFORMATION SHEET

(REV. 1/28/2003)

PROJECT TIT	TLE: <u>Puer</u>	nte Azul Office Park (frmer midnight rodeo)	ZONE	E ATLAS/DRNG. F	TLE #: F-17-72) 0 8/
DRB #: EPC#:		EPC#:		WORK O	RDER #:
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CITY ADDRE	SS:				······································
EN	NGINEERING FIRM:	Huitt-Zollars, Inc.		CONTACT:	Scott Eddings
	ADDRESS:	333 Rio Rancho Blvd.		PHONE:	892-5141
	CITY, STATE:	Rio Rancho, NM		ZIP CODE:	87124
OWNER:	Puente Azul, LL	<u>C</u>		CONTACT:	Victoria Martin
	ADDRESS:			PHONE:	480-2866
	CITY, STATE:			ZIP CODE:	
ARCHITECT:	Jud Cerva	anak		CONTACT:	Jud Cervanak
	ADDRESS:			PHONE:	266-2066
	CITY, STATE:			ZIP CODE:	
SURVEYOR:	Huitt-Zollar	rs, Inc.		CONTACT:	
	ADDRESS:	333 Rio Rancho Blvd		PHONE:	
	CITY, STATE:	Rio Rancho, NM		ZIP CODE:	
CONTRACTO	DR:			CONTACT:	
	ADDRESS:			PHONE:	
	CITY, STATE:			ZIP CODE:	
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EF	ROSION CONTROL F	PLAN		_ FINAL PLAT	APPROVAL
EN	NGINEER'S CERTIFIC	CATION (HYDROLOGY)		_ FOUNDATIO	N PERMIT APPROVAL
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	OPY PROVIDED				
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or More of the following levels of submittal may be required based on the following:

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Albuquerque, New Mexico

Prepared By

NOVEMBER

23,

201¢

HUIII-7()11A185

ENGINEERING ARCHITECTURE CONSTRUCTION MANAGEMENT

333 Rock van ber Blad. Mr. Santes 101. Reo Ram, ho MM 8/122 (40) 1802 (5/14) Jan (40) 6802 (3) 43

FINAL DRAINAGE STUDY

PUENTA AZUL BUSINESS PARK

I, Scott A. Eddings, being first duly sworn, upon my oath state that I am a registered professional	
engineer, qualified in Civil engineering, and that the accompanying report was prepared by me or und	or
my supervision and is true and correct to the best of my knowledge and helief	U I

Registered Professional Engineer

FINAL DRAINAGE STUDY FOR PUENTA AZUL BUSINESS PARK

TABLE OF CONTENTS

<u>ltem</u>	<u>Description</u>	<u>Page</u> Number
•	Introduction	
•	Flood Hazard Zones	1
•	Related Reports	1
•	Methodology	1
•	Existing Conditions	1
•	Proposed Conditions	1
•	Conclusion	2
•	Vicinity Map	3

Appendices

Appendix A
Basin Map
Basin Hydrology

Appendix B
Open Channel Calculations

FINAL DRAINAGE STUDY FOR PUENTA AZUL BUSINESS PARK

INTRODUCTION

This drainage report addresses the storm water runoff for the proposed Puenta Azul Business Park. This report will analyze existing and proposed hydrology in an effort to show that the project complies with the City of Albuquerque Design Process Manual.

The site is an existing building with a large parking lot (Midnight Rodeo) and is located on McLeod Rd. just west of San Mateo Blvd. Please see the attached Vicinity Map.

FLOOD HAZARD ZONES

Per FEMA's Flood Insurance Rate Map (FIRM) 35001C0139G dated September 26, 2008 there is no FEMA flood zone within the site.

RELATED REPORTS

The "Drainage Report for Midnight Rodeo Paving The Parking Lot" prepared by Advanced Engineering and Consulting, LLC dated February 2001 is the drainage report for the existing site. The results of this report are discussed further in the **Existing Conditions** section below.

METHODOLOGY

This drainage study is based on the procedures outlined in the Development Process Manual Section 22.2, City of Albuquerque (DPM), October 2006 revision. The precipitation zone is Zone 3. The 10-yr and 100-yr peak discharge rates were calculated using section "A.6 – Peak Discharge Rate for Small Watersheds".

EXISTING CONDITIONS

The site currently slopes from east to west and there is a swale along the west property line that conveys the discharge south to a set of 6-Sidewalk culverts at the southwest corner of the site. The sidewalk culverts route the flow on to McLeod Rd. The "Drainage Report for Midnight Rodeo Paving The Parking Lot" uses 100% D for the calculation of the onsite flow. The existing 100-yr onsite flow (22.49 cfs) is calculated in Appendix A-Basin Hydrology (Basin EX.1). There is an existing 100-yr off-site flow (14.01 cfs) discharging onto the site at the northwest corner of the site per the "Drainage Report for Midnight Rodeo Paving The Parking Lot". The existing flow reaching the existing sidewalk culverts is 36.50 cfs.

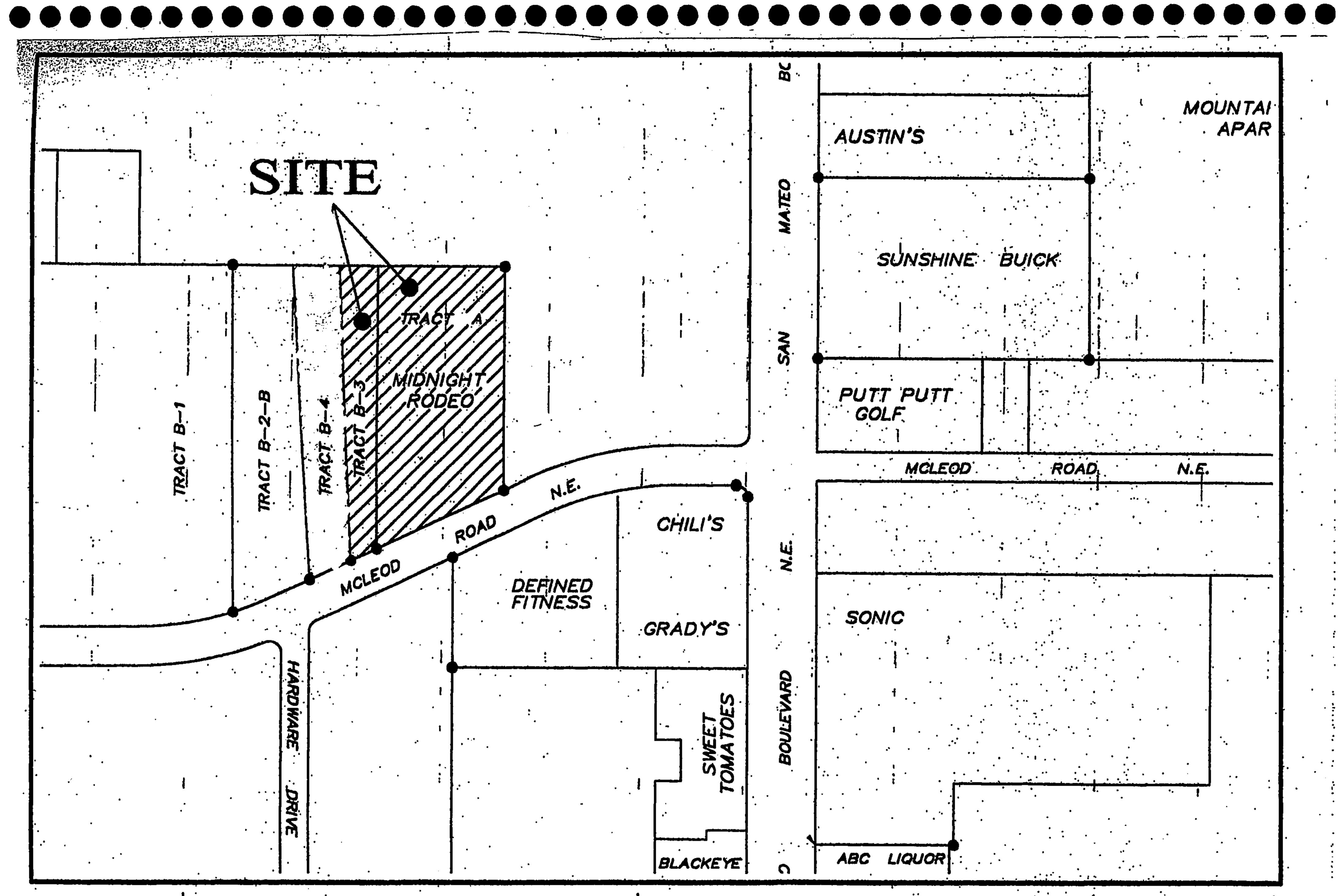
PROPOSED CONDITIONS

The proposed site plan reduces the impervious area from 100% D to 85% D with 15% C. The proposed 100-yr onsite flow (21.43 cfs) is calculated in Appendix A-Basin Hydrology (Basin 1). The off-site flow from the east shall be conveyed in an irregular open channel behind a new retaining wall. The channel has a 3' bottom, a vertical wall, a wall at 45 degrees, and a minimum depth of 1 foot. It tapers from north to south

behind the wall until it daylights into the new parking field. The channel transitions to a 5' wide 6" deep. This proposed onsite flow along with the existing off-site flow (14.01 cfs) discharges 35.44 cfs to the existing sidewalk culverts. The sidewalk culverts are not analyzed with this report since the flow is reduced to the structure.

CONCLUSION

The total flow discharging from on-site and off-site basins onto McLeod Rd. is proposed to reduce from 36.50 cfs to 35.44 cfs with this site plan. With this reduction in flow this report is in compliance with the DPM.

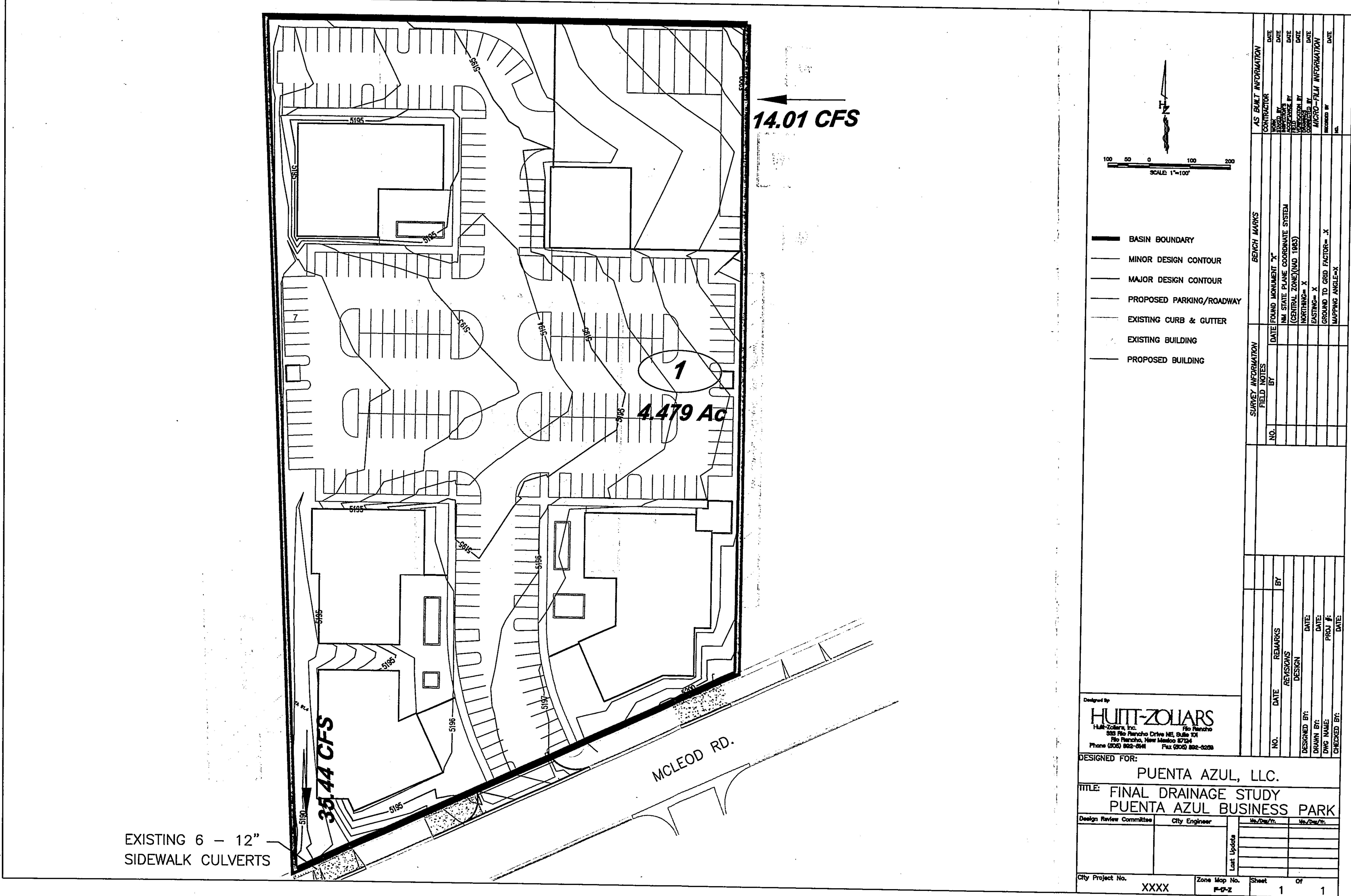


VICINITY MAP

F-17-Z

BASIN HYDROLOGY (SEE SHEET 1)

Basin	Description	Land					10-Y	'R Peak Dis	scharge (Zone 3)		100-YR Peak Discharge (Zone 3)			one 3)			
			Perc	entage		AREA		CFS/	ACRE			CFS/	ACRE		10-YR PEAK FLOW	100-YR PEAK FLOW	
	·	<u> </u>	В	C	Ď	AC	A	B	Ċ	D	Α	В	Ċ	D	ĊFS	ĊF\$	
EX.1	Existing Site	0	0	0	100	4.479	0.58	1.19	2.00	3.39	1.87	2.60	3.45	5.02	15.184	22.485	
1	Proposed Site	0	0	15	85	4.479	0.58	1.19	2.00	3.39	1.87	2.60	3.45	5.02	14.250	21.430	



Worksheet Worksheet for Irregular Channel

Project Description	}
Worksheet	channel
Flow Element	Irregular Chanı
Method	Manning's For
Solve For	Discharge
Input Data	

Input Data		_
Channel Slope	005000	ft/ft
Water Surface Elev	0.80	ft

Options	
	•

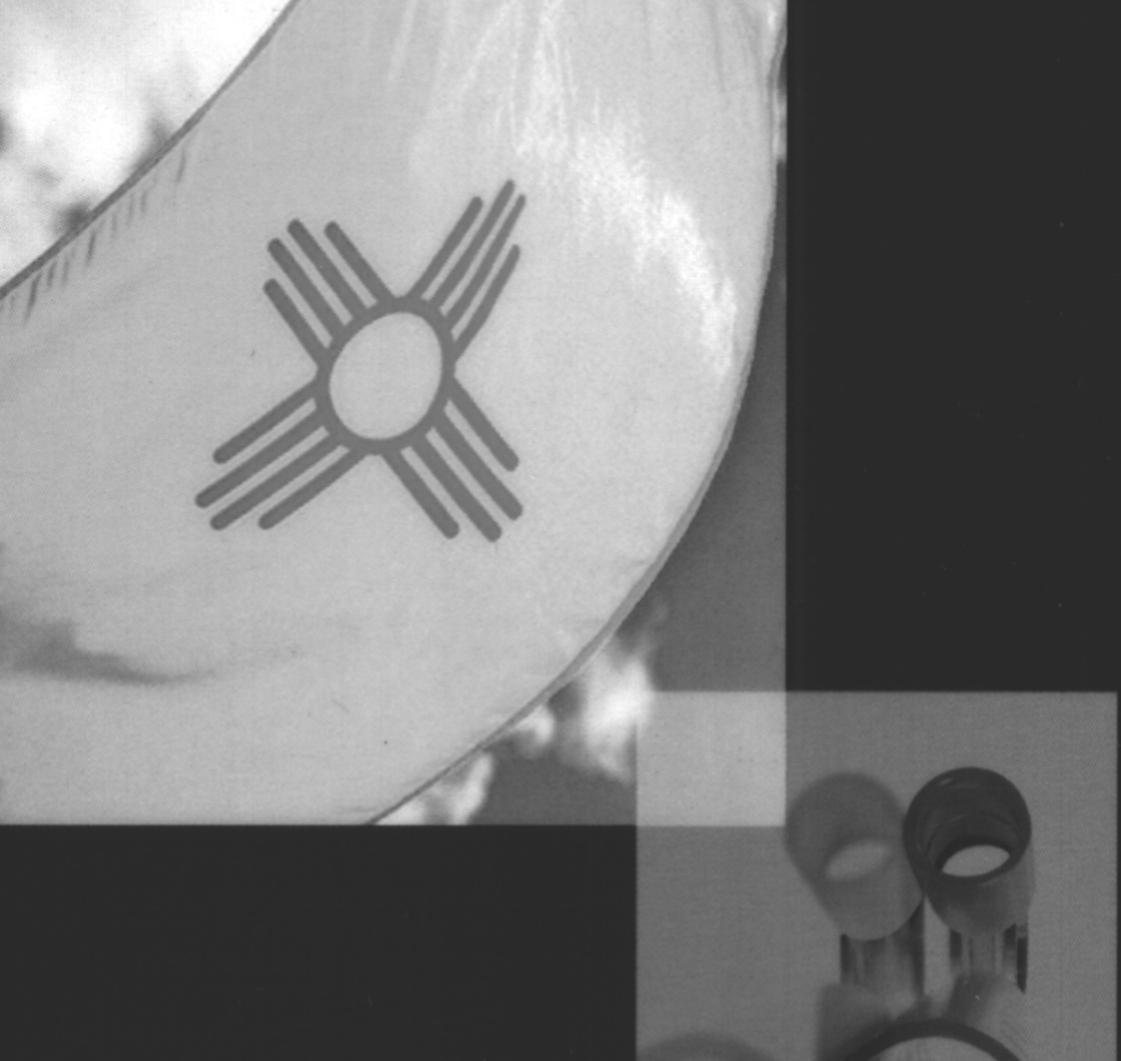
Current Roughness Methcoved Lotter's Method Open Channel Weighting wed Lotter's Method Closed Channel Weighting Horton's Method

Results		
Mannings Coeffic	0.013	
Elevation Range	0.00 to 1.00	
Discharge	14.54	cfs
Flow Area	2.7	ft²
Wetted Perimeter	4.84	ft
Top Width	3.78	ft
Actual Depth	0.80	ft
Critical Elevation	0.87	ft
Critical Slope	0.003816	ft/ft
Velocity	5.44	ft/s
Velocity Head	0.46	ft
Specific Energy	1.26	ft
Froude Number	1.14	
Flow Type	Supercritical	

Roughness Segments						
Start Station	End Station	Mannings Coefficient				
0+00	0+04	0.013				

Natural Channel Points							
Station (ft)	Elevation (ft)						
0+00	1.00						
0+00	0.00						
0+03	0.00						
0+04	1.00						





Office Locations

Albuquerque, New Mexico (505) 883-8114

Austin, Texas (512) 231-1119

Dallas, Texas (214) 871-3311

Denton, Texas (214) 871-3311

Denver, Colorado (303) 740-7325

El Paso, Texas (915) 587-4339

Fort Worth, Texas (817) 335-3000

Houston, Texas (281) 496-0066

Irvine, California (714) 734-5100

Ontario, California (909) 390-8400

Phoenix, Arizona (602) 952-9123

Rio Rancho, New Mexico (505) 892-5141

Seattle, Washington (206) 324-5500 Tacoma, Washington (253) 627-9131

Westlake Village, California (805) 418-1802

HINNS MRS

Worksheet Worksheet for Rectangular Channel

Project Descrip	tion	
Worksheet	Rectangul	ar Chann
Flow Element	Rectangul	ar Chann
Method	Manning's	Formula
Solve For	Discharge	
	· · · · · · · · · · · · · · · · · · ·	
Input Data		
Mannings Coef	fic 0.013	
Channel Slope	020000 ft/ft	
Depth	0.40 ft	
Bottom Width	5.00 ft	
Results		
Discharge	15.90 cfs	
Flow Area	2.0 ft ²	
Wetted Perima	5.80 ft	
Top Width	5.00 ft	
Critical Depth	0.68 ft	
Critical Slope	0.003860 ft/ft	
Velocity	7.95 ft/s	
Velocity Head	0.98 ft	
Specific Energ	1.38 ft	

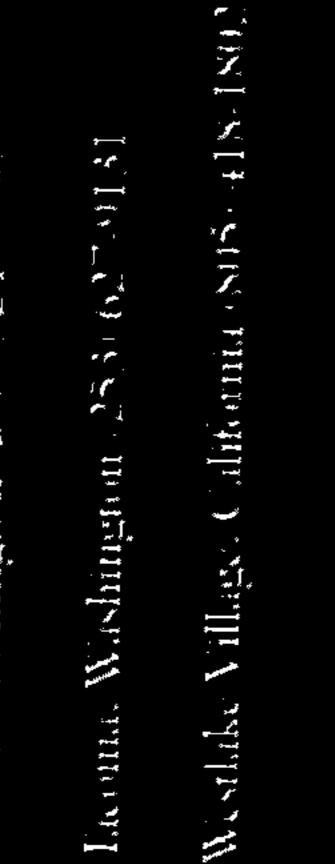
2.22

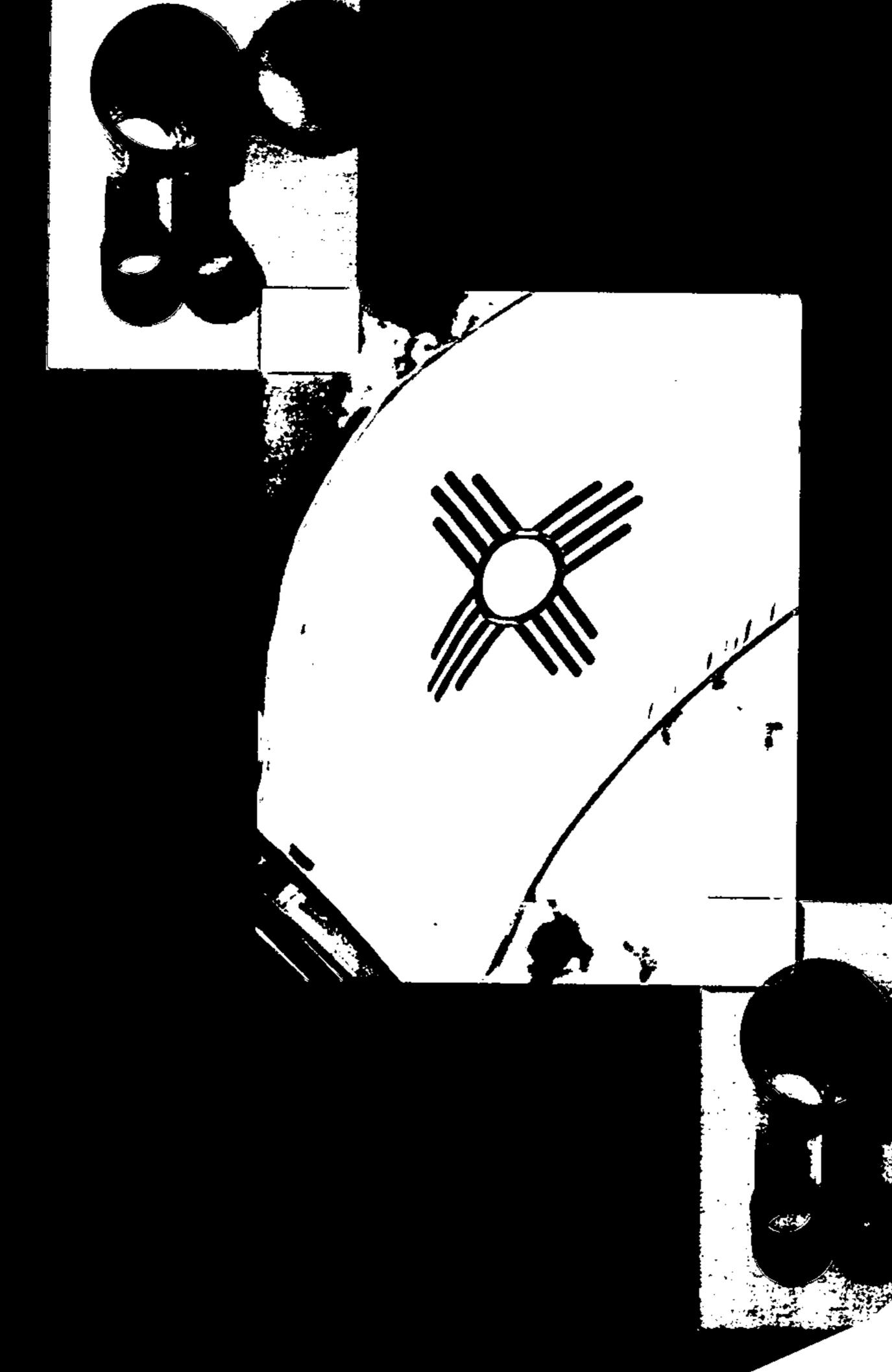
Supercritical

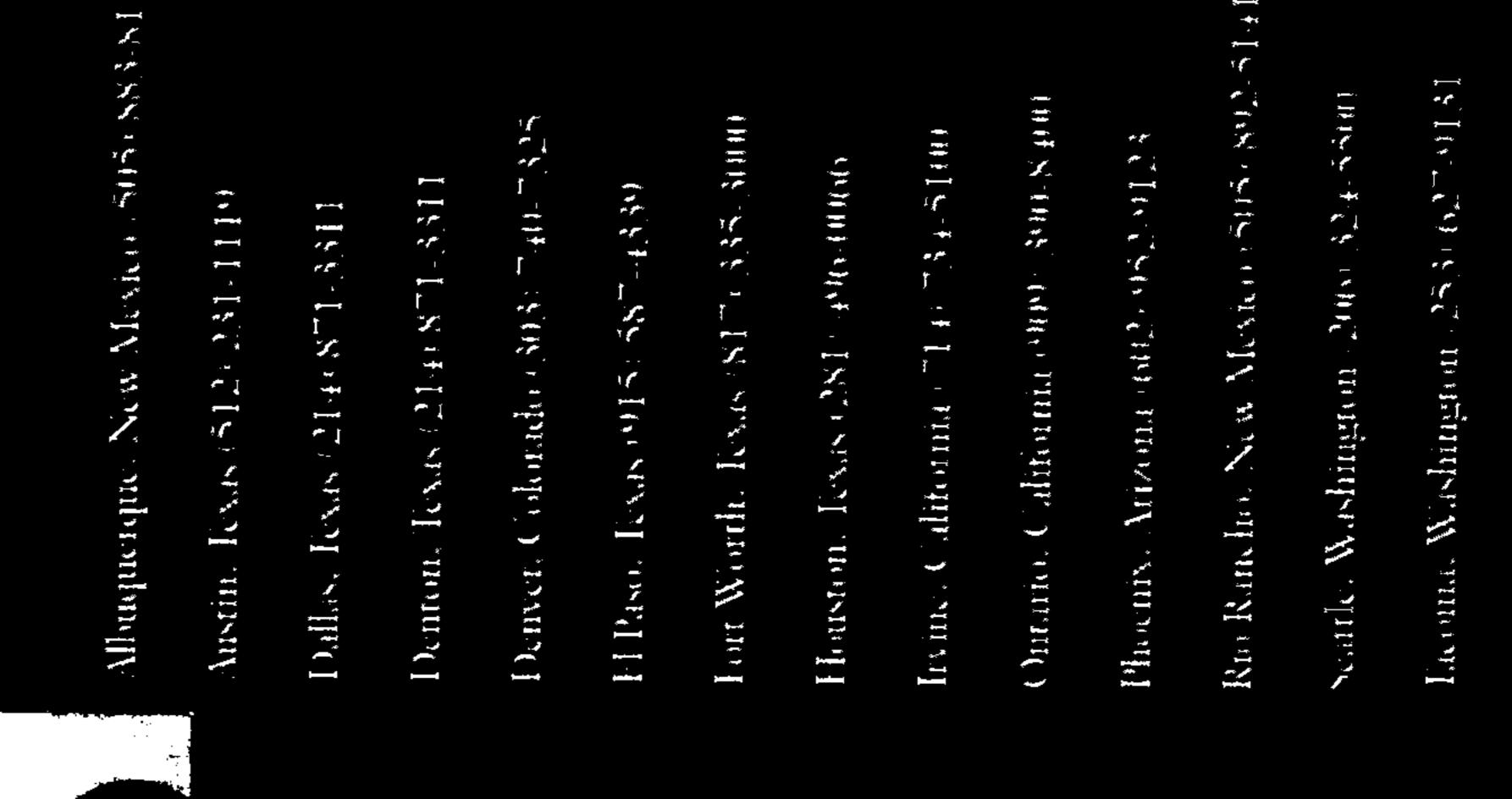
Froude Numb

Flow Type



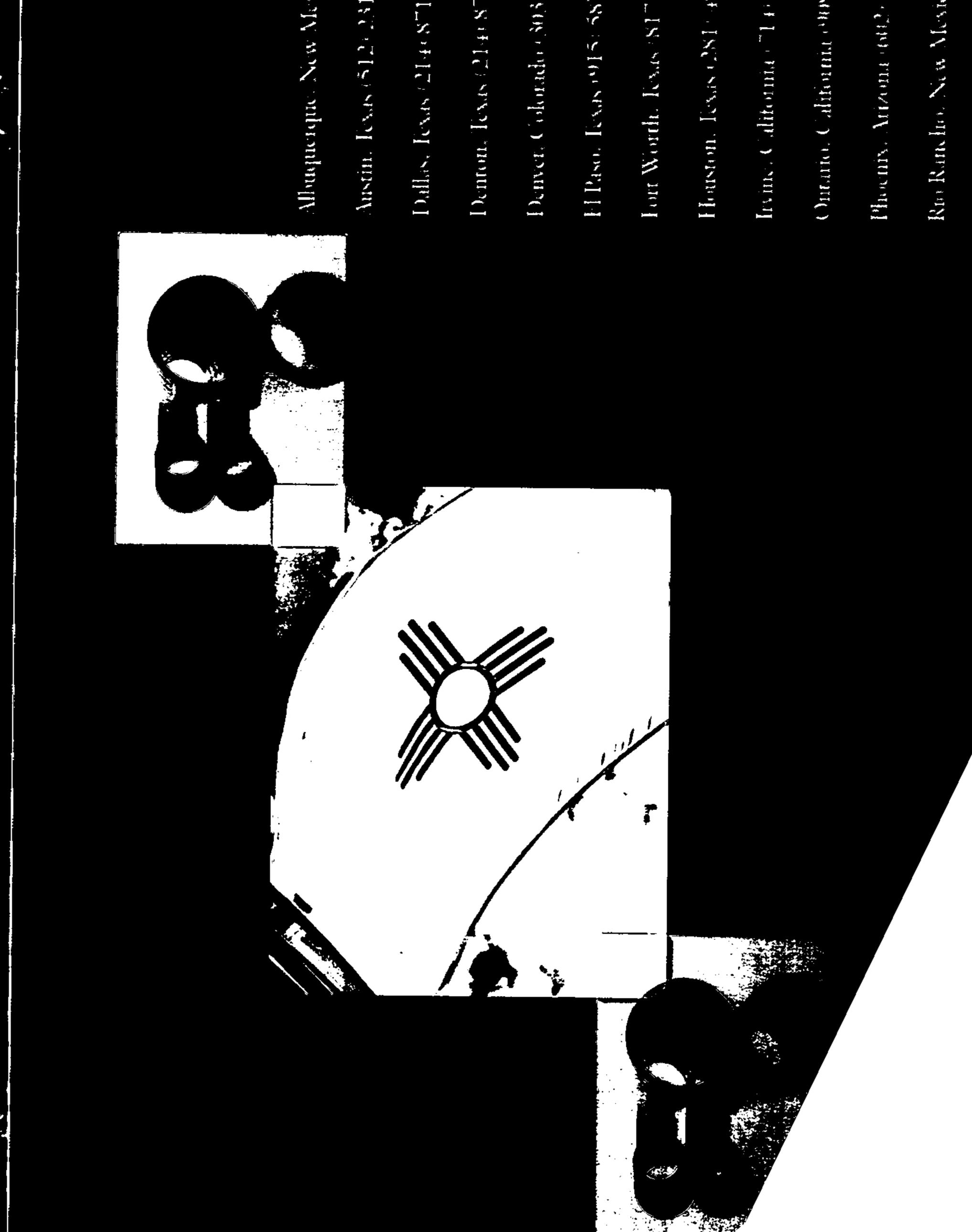




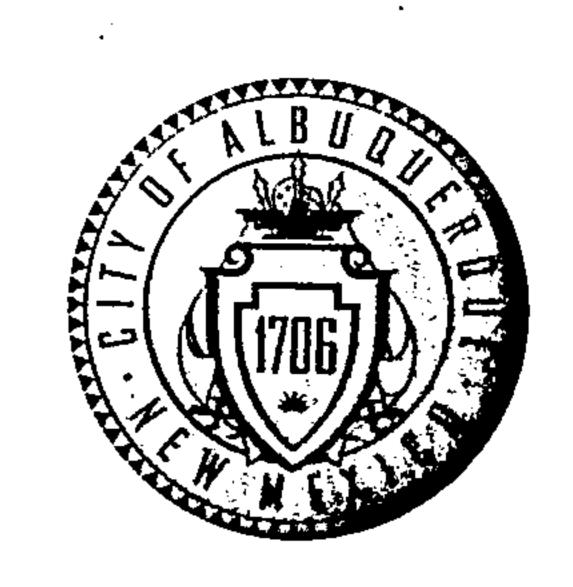








CITY OF ALBUQUERQUE



December 8, 2010

Scott Eddings, P.E. Huitt-Zollars, Inc. 333 Rio Rancho Blvd Rio Rancho, NM 87124

Re: Puenta Azul Business Park

Grading and Drainage Plan

Engineer's Stamp dated 12-7-10 (F17/D081)

Dear Mr. Eddings,

Based upon the information provided in your submittal received 12-8-10, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

This project requires a National Pollutant Discharge Elimination System (NPDES) permit for storm water discharge and a Topsoil Disturbance Permit since it is disturbing ¾ of an acre or more.

Hydrology is requesting that proposed landscape areas be depressed to retain/infiltrate the rain that falls on them.

Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

www.cabq.gov

PO Box 1293

Albuquerque

NM 87103

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

C: File

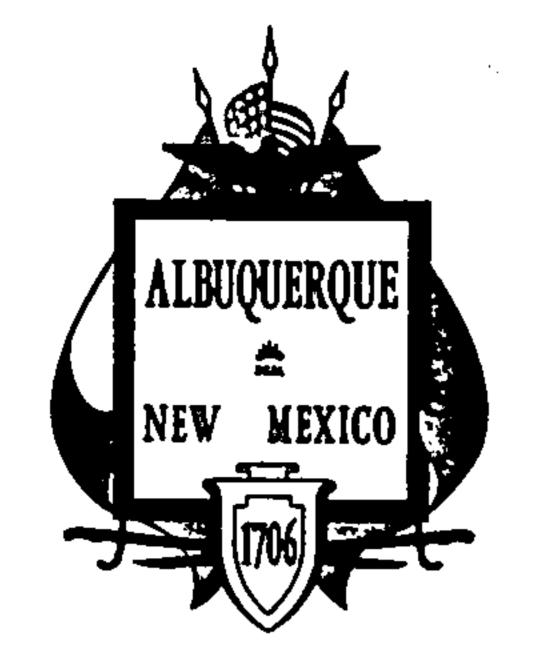
DRAINAGE INFORMATION SHEET

(REV. 1/28/2003)

PROJECT T	ITLE: Puente Azul Office Park (frmer midnight rodeo)	ZONE ATLAS/DRNG. FILE #: F-17 LOOF
DRB #:	EPC#:	WORK ORDER #:
LEGAL DES	CRIPTION:Tract A Lands of Roadrunner Investors_	
CITY ADDRE	ESS:	
Ε	NGINEERING FIRM: Huitt-Zollars, Inc.	CONTACT: Scott Eddings
	ADDRESS: 333 Rio Rancho Blvd.	PHONE: 892-5141
	CITY, STATE: Rio Rancho, NM	ZIP CODE: 87124
OWNER:	Puente Azul, LLC	CONTACT: Victoria Martin
	ADDRESS:	PHONE: 480-2866
	CITY, STATE:	ZIP CODE:
ARCHITECT	: Jud Cervanak	CONTACT: Jud Cervanak
	ADDRESS:	PHONE: 266-2066
	CITY, STATE:	ZIP CODE:
SURVEYOR	: <u>Huitt-Zollars, Inc.</u>	CONTACT:
	ADDRESS: 333 Rio Rancho Blvd	PHONE:
	CITY, STATE: Rio Rancho, NM	ZIP CODE:
CONTRACTO	OR:	CONTACT:
	ADDRESS:	PHONE:
	CITY, STATE:	ZIP CODE:
СН	IECK TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:
<u>X</u> D	PRAINAGE REPORT	SIA/FINANCIAL GUARANTEE RELEASE
D	RAINAGE PLAN 1 ST SUBMITTAL, <i>REQUIRES TCL or equal</i>	PRELIMINARY PLAT APPROVAL
D	RAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D. APPROVAL
C	ONCEPTUAL GRADING & DRAINAGE PLAN	S. DEV. PLAN FOR BLDG. PRMT. APPROVAL
X G	RADING PLAN	SECTOR PLAN APPROVAL
E	ROSION CONTROL PLAN	FINAL PLAT APPROVAL
E	NGINEER'S CERTIFICATION (HYDROLOGY)	FOUNDATION PERMIT APPROVAL
C	LOMR/LOMR	BUILDING PERMIT APPROVAL
T	RAFFIC CIRCULATION LAYOUT (TCL)	CERTIFICATE OF OCCUPANCY (PERM.)
E	NGINEER'S CERTIFICATION (TCL)	CERTIFICATE OF OCCUPANCY (TEMP.)
E	NGINEER'S CERT. (DRB APPR. SITE PLAN)	X_ GRADING PERMIT APPROVAL
	THER	PAVING PERMIT APPROVAL
		_X WORK ORDER APPROVAL
		OTHER (SPECIFY)
WAS A PRE-	-DESIGN MEETING ATTENDED:	
Y	ES	The state of the s
XN	IO	
C	OPY PROVIDED	
		NFC 2 2010
DATES	SUBMITTED: <u>12/3/10</u>	BY: Scott Eddings
		HYDROLOGY
		SECTION

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or More of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5)
- 3. Drainage Report: Required for subdivisions containing more than (10) lots or constituting five (5) acres



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

April 18, 2001

Shahab Biazar PE
Advanced Engineering and Consulting
10205 Snowflake Ct NW
Albuquerque, NM 87114

Re: Midnight Rodeo Repaving Drainage Report

Engineer's Stamp dated 4-12-01 (F17/D81)

Dear Mr. Biazar,

Based upon the information provided in your submittal dated 4-13-01, the above referenced site is approved for Paving Permit, Grading Permit and SO#19 Permit. Since you are retaining more than 18" you will need a Building Permit. You are approved for Building Permit as well.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

A separate permit is required for construction within City R/W. A copy of this approval letter must be on hand when applying for the excavation permit.

Also, prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

If you have any questions, you can contact me at 924-3986.

Sincerely,

Bradley L. Bingham, PE

Sr. Engineer Hydrology

C: Pam Lujan, CoA file



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

July 25, 2001

Shahab Biazar, P.E.
Advanced Engineering and Consulting
10205 Snowflake Court NW
Albuquerque, New Mexico 87114

RE: MIDNIGHT RODEO, REPAVING PARKING LOT

(F-17/D81)

ENGINEERS CERTIFICATION

ENGINEERS STAMP DATED MAY 9, 2001

ENGINEERS CERTIFICATION DATED 7/6/2001

Dear Mr. Biazar:

Based upon the information provided in your submittal dated 7/6/2001 for the Engineers Certification, and also based upon your submittal dated July 24, 2001 for the SO19, the above referenced project is adequate to satisfy the Grading and Drainage Certification requirements.

If you have any questions, please call me at 924-3981.

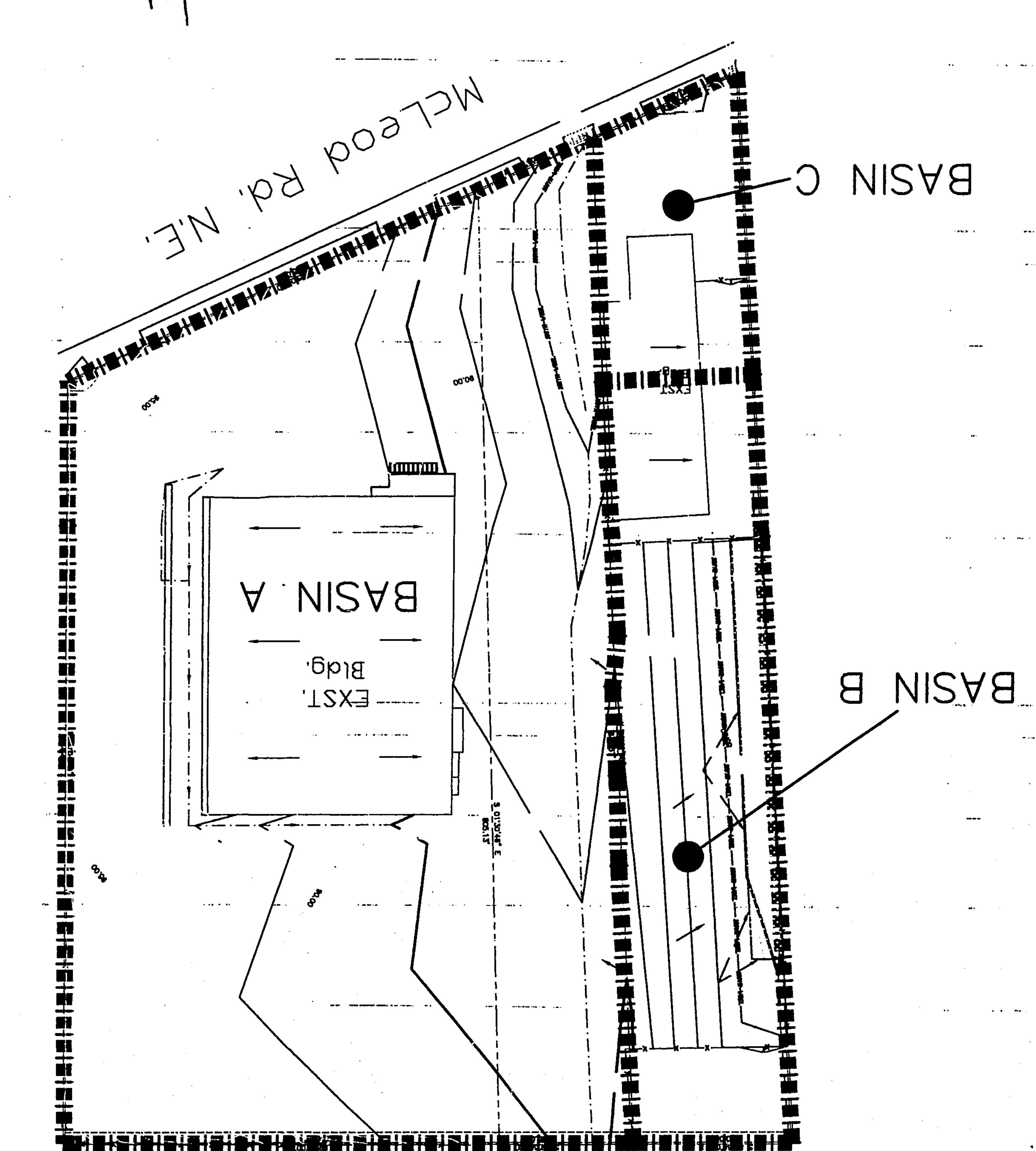
Sincerely,

Teresa A. Martin

Hydrology Plan Checker Public Works Department

BLB

C: File



IN INOXYT NISVA

RUNOFF CALCULATION RESULTS

BASIN	AREA (SF)	AREA (AC)	AREA (MI ²)
OFFSITE	121510.50	2.7895	0.004359
A	195371.83	4.4851	0.007008
\mathbf{B}	47142.37	1.0822	0.001691
C	16252.48	0.3731	0.000583

TOTAL	258766.68	8.73	0.009282
and the second s			

 $Q = 8.73 \times 4.56 = 39.81$

RUNOFF RESULTS 6 HOUR STORM

BASIN	Q-100	Q-10
,	CFS	CFS
OFFSITE	14.01	9.30
A	22.51	14.95
В	5.44	3.61
C	1.88	1.25

* ZONE 3 100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) START TYPE=1 RAIN QUARTER=0.0 IN RAINFALL RAIN ONE=2.14 IN RAIN SIX=2.60 IN RAIN DELAY=3.10 IN DT=0.03333 HR * OFFSITE BASIN COMPUTE NM HYD ID=1 HYD NO=100.0 AREA=0.004359 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1 * BASIN A * ID=2 HYD NO=101.0 AREA=0.007008 SQ MI COMPUTE NM HYD PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1 * * BASIN B * ID=5 HYD NO=102.0 AREA=0.001691 SQ MI COMPUTE NM HYD PER A=0.00 PER B=0.00 PER_C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1 * * BASIN C * ID=7 HYD NO=103.0 AREA=0.000583 SQ MI COMPUTE NM HYD PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1 ************* 10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) * START TYPE=1 RAIN QUARTER=0.0 IN RAINFALL RAIN ONE=1.43 IN RAIN SIX=1.73 IN RAIN DELAY=2.07 IN DT=0.03333 HR OFFSITE BASIN

ID=1 HYD NO=110.0 AREA=0.004359 SQ MI

TP=0.1333 HR MASS RAINFALL=-1

PER A=0.00 PER B=0.00 PER-C=0.00 PER D=100.00

COMPUTE NM HYD

* BASIN A

COMPUTE NM

ID=2 HYD NO=111.0 AREA=0.007008 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00TP=0.1333 HR MASS RAINFALL=-1

BASIN B

COMPUTE NM

ID=5 HYD NO=112.0 AREA=0.001691 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1

*

* BASIN C

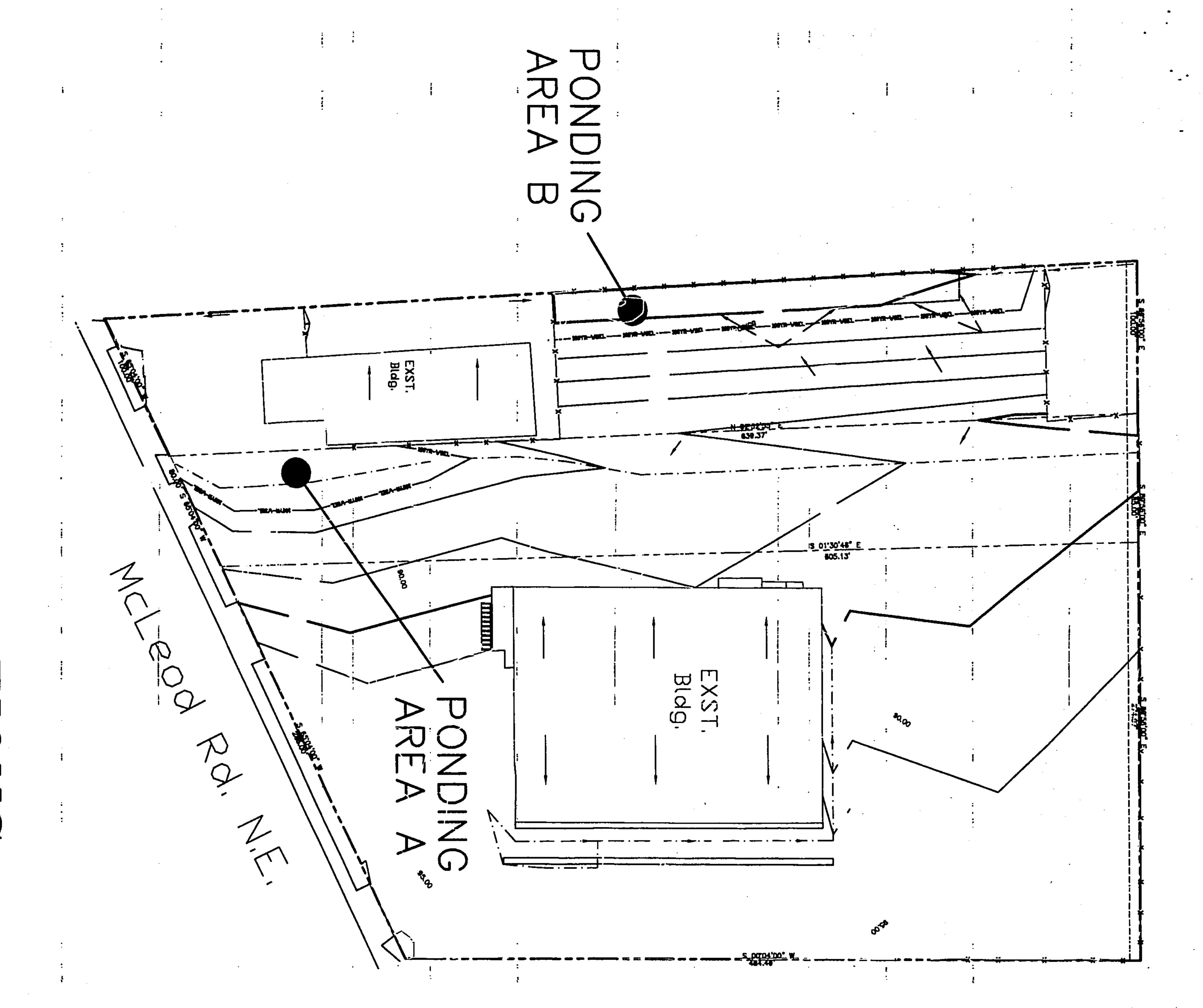
COMPUTE NM HYD

ID=7 HYD NO=113.0 AREA=0.000583 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00TP=0.1333 HR MASS RAINFALL=-1

FINISH

SUMMERY OUTPUT FILE

AHYMO PROGRAINPUT FILE	AM SUMMAR = 2035new		AHYMO_	97) -			VERSION: 19		RUN DATE SER NO.=	-	/YR) =05/0 9702c01000	09/2001 0R31-AH
			FROM	TO:		PEAK	RUNOFF	•	TIME TO	· CFS	PAGE =	= 1
		DROGRAPH	ID	ID	AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		
COMMAND	IDENTI	FICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTAT	ION
				•				•				
START				:				•			TIME=	.00
RAINFALL T	YPE=1	•					•				RAIN6=	2.600
COMPUTE NM	HYD	100.00;	_	1	.00436	14.01	.548	2.35528	1.500	5.020	PER IMP=	100.00
COMPUTE NM	HYD	101.00	_	2	.00701	22.51	.880	2.35528	1.500	5.019	PER IMP=	100.00
COMPUTE NM	HYD	102.00	_	5	.00169	5.44	.212	2.35528	1.500	5.026	PER IMP=	100.00
COMPUTE NM	HYD	103.00	_	7:	.00058	1.88	.073	2.35528	1.500		PER IMP=	
START					•	•			•		TIME=	.00
RAINFALL T	YPE=1										RAIN6=	1.730
COMPUTE NM	HYD	110.00	-	1	.00436	9.30	.348	1.49561	1.500	3.334	_	
COMPUTE NM	HYD	111.00	_	2	.00701	14.95	.559	1.49561	1.500		PER IMP=	
: COMPUTE NM	HYD	112.00	-	5	.00169	3.61	.135	1.49561	1.500		PER IMP=	
COMPUTE NM	HYD	113.00	-	7	.00058	1.25	.047	1.49561	1.500		PER IMP=	
FINISH				•	•	- · - · ·				0.000	<u> </u>	
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<u>•</u>	i †			į	+	i	i	į	ţ		į	į



VOLUME CALCULATIONS

DETENTION POND A

Ab - Bottom Of The Pond Surface Area

At - Top Of The Pond Surface Area

D - Water Depth

Dt - Total Pond Depth

C - Change In Surface Area / Water Depth

Volume =
$$Ab * D + 0.5 * C * D^2$$

$$C = (At - Ab) / Dt$$

$$Ab = 0.00^{\circ}$$
 $At = 5,393.82$
 $Dt = 1.00$
 $C = 5393.82$

ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
86.54	0.00	0.00000	0.00
86.64	0.10	0.00000	11.22
86.74	0.20	0.00062	15.87
86.84	0.30	0.00248	19.44
86.94	0.40	0.00557	22.44
87.04	0.50	0.00991	25.09
87.14	0.60	0.01548	27.49
87.24	0.70	0.02229	29.69
87.34	0.80	0.03034	31.74
87.44	0.90	0.03962	33.67
87.54	1.00	0.05015	35.49

Orifice Equation Q = CA SQRT(2gH)

C = 0.6 2-1.5' &4-2' SD/WK Culverts

Area (ft^2)= 7.37 g = 32.2

H (Ft) = Depth of water above center of orifice

Q(CFS)= Flow

VOLUME CALCULATIONS

DETENTION-POND B ------ --

Ab - Bottom Of The Pond Surface Area

At - Top Of The Pond Surface Area

D - Water Depth

Dt - Total Pond Depth

C - Change In Surface Area / Water Depth

Volume =
$$Ab * D + 0.5 * C * D^2$$

$$C = (At - Ab) / Dt$$
 $Ab = 0.00$
 $At = 13,646.12$
 $Dt = 1.00$
 $C = 13646.12$

ACTUAL	DEPTH	VOLUME	Q
ELEV.	(FT)	(AC-FT)	(CFS)
84.54	0.00	0.00000	0.00
84.64	0.10	0.00000	0.60
84.74	0.20	0.00157	0.85
84.84	0.30	0.00627	1.04
84.94	0.40	0.01410	1.20
85.04	0.50	0.02506	1.34
85.14	0.60	0.03916	1.46
85.24	0.70	0.05639	1.58
85.34	0.80	0.07675	1.69
85.44	0.90	0.10025	1.79
85.54	1.00	0.12688	1.89
85.64	1.10	0.15664	1.98
85.74	1.20	0.18953	2.07.
85.84	1.30	0.22556	2.16
86.00	1.40	0.26471	2.24

Orifice Equation Q = CA SQRT(2gH)

C = 0.6

2-6" Discharge Pipes

Area (ft^2)= 0.39

g = 32.2

H (Ft) = Depth of water above center of orifice

Q (CFS)= Flow

```
ZONE 3
         100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS)
START
                    TYPE=1 RAIN QUARTER=0.0 IN
RAINFALL
                    RAIN ONE=2.14 IN RAIN SIX=2.60 IN
                    RAIN DELAY=3.10 IN DT=0.03333 HR
* OFFSITE BASIN
*
                    ID=1 HYD NO=100.0 AREA=0.004359 SQ MI
COMPUTE NM HYD
                    PER A=0.00 PER B=0.00 PER-C=0.00 PER D=100.00
                    TP=0.1333 HR MASS RAINFALL=-1
* BASIN A
                    ID=2 HYD NO=101.0 AREA=0.007008 SQ MI
COMPUTE NM HYD
                    PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00
                    TP=0.1333 HR MASS RAINFALL=-1
                    ID=3 HYD NO=101.10 ID=1 ID=2
ADD HYD
                    ID=4 HYD NO=501.0 INFLOW ID=3 CODE=24
ROUTE RESERVOIR
                                      STORAGE (AC-FT) ELEVATION (FT)
                    OUTFLOW (CFS)
                                                        . 5186.54
                                          ___0_0000
                        0.00
                                                         5186.74
                                            0.00062
                       15.87
                                                         5186.84
                                            0.00248
                       19.44
                                                         5186.94
                                            0.00557
                       22.44
                                            0.00991
                                                         5187.04
                       25.09
                                                         5187.14
                                            0.01548
                       27.49
                                                         5187.24
                                            0.02229
                       29.69
                                                         5187.34
                                            0.03034
                       31.74
                                                         5187.44
                                            0.03962
                       33.67
                                                         5187.54
                                            0.05015
                       35.49
*
* BASIN B
                    ID=5 HYD NO=102.0 AREA=0.001691 SQ MI
COMPUTE NM HYD
                    PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00
                    TP=0.1333 HR MASS RAINFALL=-1
                    ID=6 HYD NO=502.0 INFLOW ID=5 CODE=24
ROUTE RESERVOIR
                                                       ELEVATION (FT)
                                      STORAGE (AC-FT)
                    OUTFLOW (CFS)
                                                         5184.54
                                            0.00000
                        0.00
                                           0.00157
                                                         5184.74
                        0.85
                                                        . 5184.84
                                          ---0--00-627
                        1.04
```

•			
1.20		0.01410	5184.94
1.34		0.02506	5185.04
1.46		0.03916	5185.14
1.58		0.05639	5185.24
1.69		0.07675	5185.34
1.79		0.10025	5185.44
1.89	- 197 - Mar and Andrews	0_12688	5185.54
1.98		0.15664	5185.64
2.07		0.18953	5185.74
2.16		0.22556	5185.84
2.24	•	0.26471	5186.00

* BASIN C

COMPUTE NM HYD

ID=7 HYD NO=103.0 AREA=0.000583 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1

*

FINISH

PONDING CONDITIONS SUMMERY OUTPUT FILE

AHYMO PROGRAM INPUT FILE = 3		_	AHYMO_	97) –		_	VERSION:	1997.02d	RUN DATE USER NO.=	•	/YR) =05/09/2001 9702c01000R31-AH
	: : :	DROGRAPH	FROM ID	TO ID	AREA	PEAK DISCHARGE	RUNOFF		TIME TO		PAGE = 1
COMMAND		FICATION	NO.	NO.	(SQ MI)	(CFS)	VOLUME (AC-FT		PEAK (HOURS)	PER ACRE	NOTATION
START	E= 1					_	- : :				TIME= .00
RAINFALL TYPI COMPUTE NM HY		100.00	-	1	.00436	14.01	.548	2.35528	1.500	5.020	RAIN6= 2.600 PER IMP= 100.00
COMPUTE NM HY	D .	101.00	_	2	.00701	22.51	.880	2.35528	1.500	5.019	PER IMP= 100.00
ADD HYD		101.10	1& 2	3	.01137	36.52	1.428	2.35522	1.500	5.020	. •
ROUTE RESERVO	IR :	501.00	3	4	.01137	33.67	1.428	2.35522	1.533	4.628	AC-FT=040
COMPUTE NM HY	D	102.00	_	5	.00169	5.44	.212	2.35528	1.500		PER IMP= 100.00
ROUTE RESERVO	IR	502.00	5	6	.00169	1.70	.212	2.35512	1.866		AC-FT=.079
COMPUTE NM HY	D [103.00	-	7	.00058	1.88	.073	2.35528	1.500	5.044	PER IMP= 100.00

FINISH

PONDING CONDITIONS OUTPUT_FILE

1997.02d - Version: AHYMO PROGRAM (AHYMO 97) (MON/DAY/YR) = 05/09/2001START TIME (HR:MIN:SEC) = 16:10:09USER NO. = AHYMO-I-9702c01000R31-AH INPUT FILE = 2035PDB* ZONE 3 6-HR STORM (UNDER PROPOSED CONDITIONS) START TYPE=1 RAIN QUARTER=0.0 IN RAINFALL RAIN ONE=2.14 IN RAIN SIX=2.60 IN RAIN DELAY=3.10 IN DT=0.03333 HR COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR. 5.999400 HOURS .033330 HOURS END TIME = DT =.0084 .0113 .0143 .01730027-.0000 .0055 .0302 .0337 .0372 .0408 .0269 .0204 .0236 .0694 .0564 .0606 .0649 .0523 .0445 .0484 .1063 .0892 .1003 .0839 .0946 .0741 .0789 .1597 .1322 .1385 .1452 .1262 .1126 .1192 .7049 --- : 9093 .2422 .3139 .4119 75407 .1922 1.7683 1.7074 1.5687 1.6414 1.3904 1.4871 1.1588 2.0174 2.0589 2.0982 1.9270 1.9735 1.8247 1.8775 2.2359 2.2737 2.2807 2.2041 2.2661 2.1354 2.1707 2.3172 2.3226 2.3001 2.3060 2.3117 2.2875 2.2939 2.3558 2.3514 2.3277 2.3328 2.3376 2.3423 2.3470 2.3838 2.3723 2.3801 2.3683 2.3762 2.3601 2.3643 2.4050 2.4083 2.3982 2.4016 2.3947 2.3875 2.3911 2.4301 2.4210 2.4241 2.4271 2.4147 2.4179 2.4115 2.4499 2.4472 2.4388 2.4416 2.4444 2.4330 2.4359 2.4605 2.4631 2.4656 2.4681 2.4526 2.4553 2.4579 2.4779 2.4803 2.4755 2.4731 2.4942 2.4964 2.4986 - 2.50082.4896 2.4919 2.4873 2.5157 2.5094 2.5115 2.5136 2.5073 2.5052 2.5030 2.5297 2.5218 2.5238 2.5258 2.5277 2.5177 2.5198 2.5412 2.5374 2.5393 2.5336 2.5355 2.5317 2.5557 2.5504 2.5522 2.5540 2.5486 2.5449 2.5467 2.5627 2.5645 2.5662 2.5679 2.5610 2.5593 2.5575 2.5795 2.5762 2.5779 2.5729 2.5746 2.5713 2.5696 2.5891 2.5907 2.5876 2.5844 2.5860 2.5811 2.5828 2.6000 2.5969 2.5984 2.5938 2.5954 2.5923 * OFFSITE BASIN ID=1 HYD NO=100.0 AREA=0.004359 SQ MI COMPUTE NM HYD PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1 SHAPE CONSTANT, N = 7.106420.545000 .133300HR K/TP RATIO =TP =.072649HR B = P60 = 2.1400526.28 · CFS 17.210 .04000 INCHES PER HOUR .10000 INCHES INF =.004359 SQ MI IA =AREA = RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330 * BASIN A ID=2 HYD NO=101.0 AREA=0.007008 SQ MI COMPUTE NM HYD

PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00

TP=0.1333 HR MASS RAINFALL=-1

SHAPE CONSTANT, N = 7.106420.545000 .133300HR K/TP RATIO = .072649HR P60 = 2.1400526.28 .9990 UNIT VOLUME = 27.668 CFS .04000 INCHES PER HOUR .10000 INCHES $INF = \cdots$ IA = I.007008 SQ MI AREA =RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

ID=3 HYD NO=101.10 ID=1 ID=2 ADD HYD ID=4 HYD NO=501.0 INFLOW ID=3 CODE=24 ROUTE RESERVOIR STORAGE (AC-FT) ELEVATION (FT) OUTFLOW (CFS) 5186.54 0.00000 0.00 5186.74 0.00062 15.87 0.00248----5186.84 19.44 0.00557 5186.94 22.44 0.00991 ---~5187*:*04 25.09 0.01548 5187.14 27.49 5187.24 29.69 0.02229 5187.34 0.03034 31.74 5187.44 0.03962 33.67 5187.54 0.05015 35.49 OUTFLOW VOLUME ELEV TIME INFLOW (CFS) (AC-FT) (CFS) (FEET) (HRS) .00 .000 5186.54 .00 .00 .00 .000 5186.54 .80 .00 .025 30.38 5187.27 24.99 1.60

1.76 1.69 5186.56 .000 2.40 .000 . 47 5186.55 .45 3.20 .33 .000 .32 5186.54 4.00 .30 .000 5186.54 .30 4.80 .31 .000 5186.54 5.60 .31 .03 .000 5186.54 6.40 .03 PEAK OCCURS AT HOUR -- 1--53-33.667 CFS -

PEAK DISCHARGE =

5187.440 MAXIMUM WATER SURFACE ELEVATION =

INCREMENTAL TIME= .033330HRS .0396 AC-FT MAXIMUM STORAGE =

* BASIN B

COMPUTE NM HYD

ID=5 HYD NO=102.0 AREA=0.001691 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 6.6762 CFS UNIT VOLUME = .9976 B = 526.28 P60 = 2.1400.001691 SQ MI IA = .100000 INCHES INF = .04000 INCHES PER HOURAREA =RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

.*	•		•				
ROUTE	RESERVOIR	ID=6 HYD NO=502.0 INFLOW ID=5 CODE=24					
		OUTFLOW (CFS)	STORAGE (AC-FT)	ELEVATION (FT)			
		0.00	0.0000	5184.54			
		0.85	0.00157	5184.74			
		1.04	0.00627	5184.84			
		1.20	0.01410	5184.94			
- 1889	•	1.34	0.02506	5185.04			
		1.46	0.03916	5185.14			
		1.58	0.05639	5185.24			
		1.69	0.07675	5185.34			
		1.79	0.10025	5185.44			
4 = 44		1.89	0.12688	5185. 54 ·			
		1.98	0.15664	5185.64			
		2.07	0.18953	5185.74			
		2.16	0.22556	5185.84			
		2.24	0.26471	5186.00			
			*** * * ***	AND AND THE PARTY OF THE PARTY			

·		OUTFLOW	VOLUME	ELEV	INFLOW	TIME
		(CFS)	(AC-FT)	(FEET)	(CFS)	(HRS)
•		.00	.000	5184.54	.00	.00
		.00	.000	5184.54	.00	.80
		1.62	.063	5185.27	3.72	1.60
•		1.50	.046	5185.18	.25	2.40
		.07	.000	5184.56	.07	3.20
		.05	.000	5184.55	.05	4.00
		.04	.000	5184.55	.04	4.80
		.05	.000	5184.55	.05	5.60
					.00	
1.87	HOUR	OCCURS AT	CFS - PEAK	1.701	IARGE =	EAK DISCH
		5.351	N =518	ELEVATIO	TER SURFACE	AW MUMIXA
IME= .033330HRS	ITAL TI	INCREMEN	3 AC-FT	.079	ORAGE =	AXIMUM ST

* BASIN C

COMPUTE NM HYD

ID=7 HYD NO=103.0 AREA=0.000583 SQ MI PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00 TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT VOLUME = .9941 B = 526.28 P60 = 2.1400UNIT PEAK = 2.3017 CFS .000583 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR ---AREA =RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

FINISH

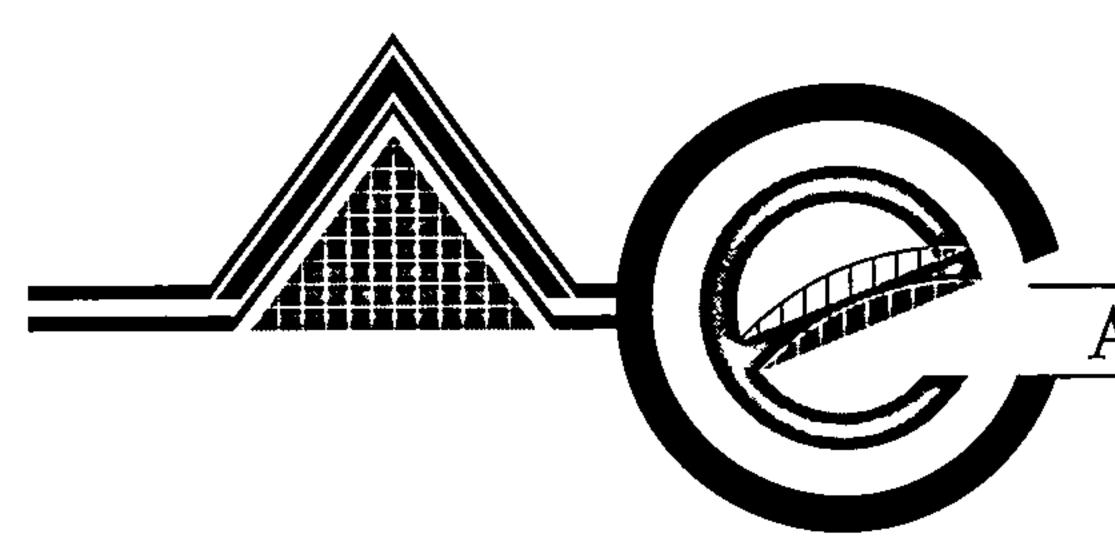
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NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 16:10:09

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ADVANCED ENGINEERING and CONSULTING, LLC

July 6, 2001

Consulting
Design
Development
Management
Inspection

Mr. John Murray, P.E. City of Albuquerque 600 Second Street NW Albuquerque, New Mexico 87102

Re:

Final Certification of Grades for Midnight Rodeo (F17/D81)

(4901 McLeod Road NE)

Dear Mr. Murray:

Enclosed please find one copy of the as-built Grading Plan for the above mentioned site. The site is repaved, and all the storm drain structures have been constructed. The grades are built according to the grading & drainage plan approved with engineering stamp date May 9, 2001. The runoff drains to the sidewalk culverts as designed.

Should you have any questions, please do not hesitate to contact our office.

Sincerely

Shahab Biazar, P.E.

GRANT OF DRAINAGE EASEMENT

Enchantment Carpet Company, Inc., a New Mexico Corporation, hereinafter referred to as grantor, is the owner in fee simple of a parcel of land located at 4815 McLeod Rd. NE, Albuquerque, New Mexico 87109-2108, and described as:

Tract B-3 of the Replat of Tract B-1 through B-4, Lands of Roadrunner Investments, within the Elena Gallegos Grant, Bernalillo County, New Mexico, as shown on the said plat, filed in the office of the County Clerk of Bernalillo County, New Mexico on June 21, 1978 in Plat Book C13, page 130.

B&S Investors, LLC, hereinafter referred to as grantee, is the owner in fee simple of a parcel of land located at 4901 McLood Rd. NE, Albuquerque, New Mexico 87109-2108, and described as:

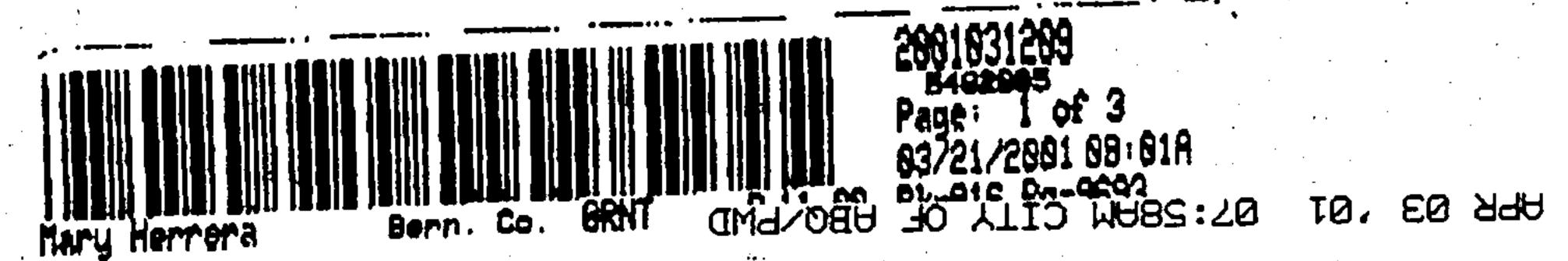
Tract B-4 of the Plat of Tracts B-1 through B-4, Lands of Road Runner Investors, within Elena Gallegos Grant, Bernalillo County, New Mexico, as the same as shown and designated on the plat thereof, filed in the office of the County Clerk of Bernalillo County, New Mexico, on June 21, 1978; and

Tract A, Lands of Road Runner Investors, within Elena Gallegos Grant, Bernalillo County, New Mexico, as the same as shown and designated on the plat thereof, filed in the office of the County Clerk of Bernalillo County, New Mexico, on June 16, 1976.

The grantor hereby grants and conveys to the grantee a drainage easement as described and illustrated in the attached Exhibit 1 to this Grant of Drainage Easement. This Grant of Drainage Easement is in exchange for adequate consideration, the sufficiency and receipt of which is hereby acknowledged by the grantor.

This easement is hereby granted to the grantee, its heirs assigns, successors, executors,

Page 1 of 2



administrators, tenants, and licensees, and is for the benefit of, and appurtenant to, the said land of grantee.

In witness, whereof, the grantor has hereto set his hand and seal on BIGO)

Donald Chavez, President (Enchantment Carpet Co., Inc.

ACKNOWLEDGMENT

State of New Mexico

County of Bernalillo

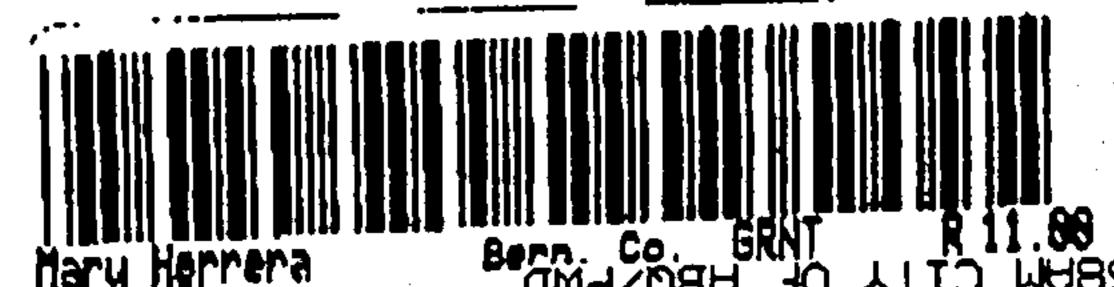
Before me the undersigned, a Notary Public, in and for said County and State, on this 16th day of March, 2001, personally appeared Donald Chavez to me known to be the identical person who executed the within and foregoing instrument and acknowledged to me that he executed the same as his free and voluntary act and deed for the uses and purposes therein set forth.

Given under my hand and seal the day and year last above written.

Commission expires: 3

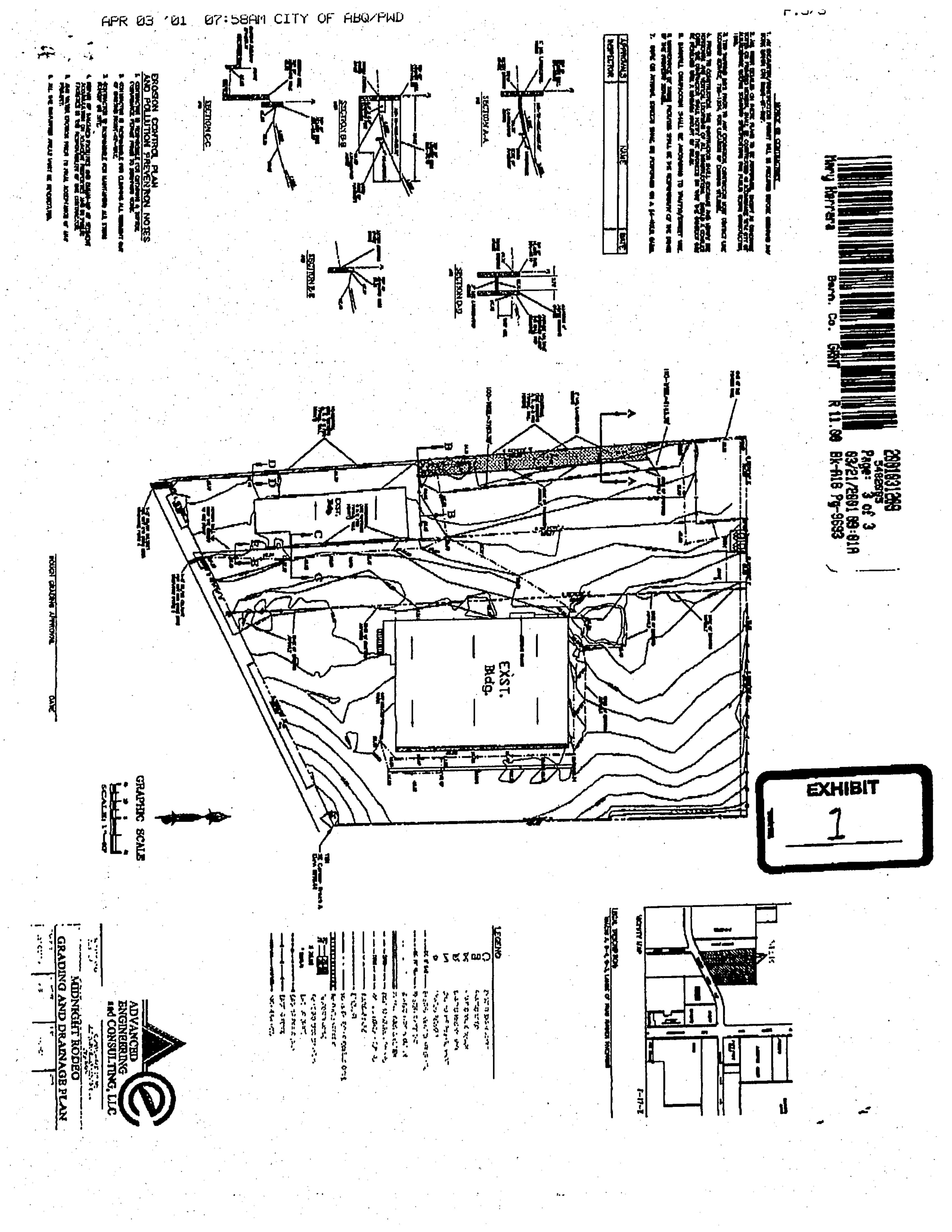
10-02

Page 2 of 2



2801831289 Page: 2 of 3 88-816 Pg-9693

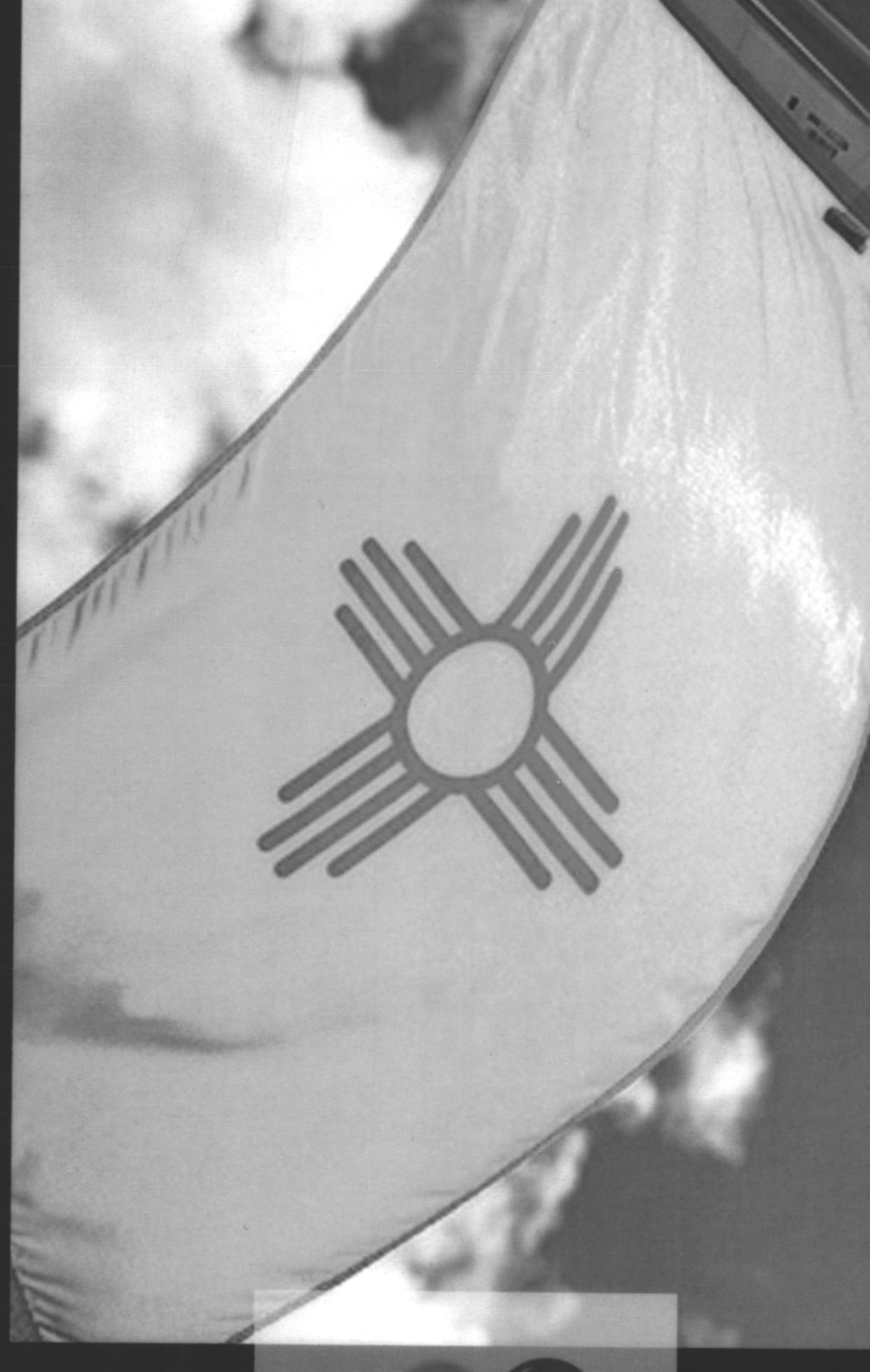
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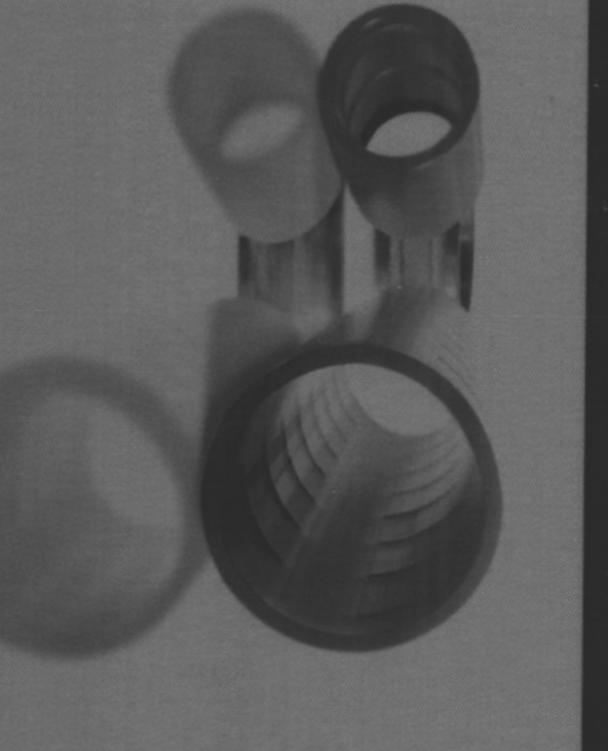


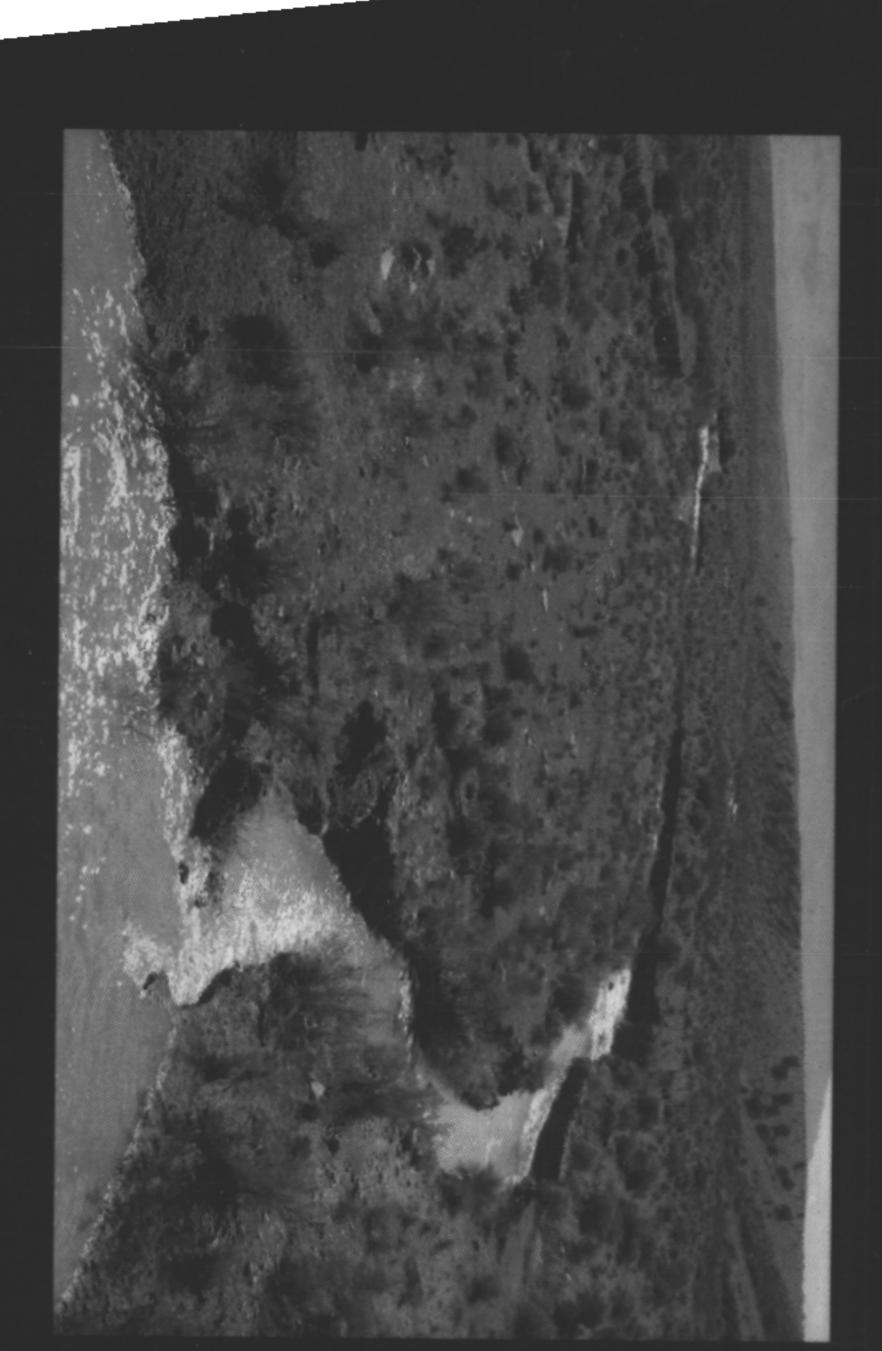
Prepared For

PUENTA AZUL,









Prepared Ву

NOVEMBER

2010

333 Rio Rancho Blvd. NE Suite 101, Rio Rancho NM 87 (505) 892-5141, Fax (505) 892-3259 www.huitt-zollars.com