CITY OF ALBUQUERQUE

Hydrology Section Planning Department David S. Campbell, Director



Timothy M. Keller, Mayor

November 20, 2018

Matt Korte Adams Engineering 513 Main St Suite 300 Fort Worth, TX 76102

Re: McDonalds - 8315 Montgomery Blvd NE
Tract A-1-F of the Los Pastores Shopping Center
Grading and Drainage Plan - Engineer's Stamp dated: missing (F19D013A)

Dear Mr. Segovia,

Based on the application received 6/18/2018, the above referenced plan cannot be approved for building permit until the following comments are addressed.

- Please use the New DTIS form available at http://documents.cabq.gov/planning/DevelopmentReviewServices/DRAINAGE%20INF-0%20SHEET_ELECTRONICrev11-18.pdf when resubmitting and include the Owner's contact information. Standard resubmittal fees will apply.
- 2. An engineer's stamp, signature, and date are required on the G&D Plan.
- 3. The floor elevation of the building must be set a minimum of 1.0' above the adjacent floodplain elevation. There is a Special Flood Hazard Area in Montgomery Blvd. It is an AO (depth 1 feet) zone which may have been rounded down from 1.4' feet depth. The street flow line elevation in front of the east edge of the building 5420.7 so the minimum floor elevation should be 5423.1. Please add a note addressing the floodplain, show the floodplain on the G&D Plan, show the floor elevation on the G&D plan and indicate the direction of the roof drainage. A floodplain development permit is required for any work in the floodplain.
- 4. The alley and Montgomery Blvd. both have limited drainage and the plan increases the peak rate of runoff to both. The City engineer cannot approve any development who's increased runoff exceeds the downstream capacity per § 14-5-2-12(G) <a href="http://library.amlegal.com/nxt/gateway.dll/New%20Mexico/albuqwin/chapter14zoningplanningandbuilding/article5floodhazardanddrainagecontrol?f=templates\$fn=default.htm\$ 3.0\$vid=amlegal:albuquerque_nm_mc\$anc=JD_14-5-2-12 The Drainage Management Plan for the Los Pastores Shopping Center (Hydro# F19D013C) established an allowable discharge of 26.47cfs from the 21.18 acres draining to the pond which equates to 1.25 cfs/acre. The 0.6 acre portion of this site that currently drains to the Los Pastores Pond only contributes 0.75cfs to Montgomery after going thru the pond, but that area will contribute 2.86 cfs Montgomery with the new plan. The existing floodplain on Montgomery indicates that it does not have capacity for any increased flow from this site. This site must either drain 0.6 acres to the Los Pastores Pond, or provide an equivalent onsite pond or some combination of those two.

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

- 5. This proposed G&D Plan also shows about a 1.5 cfs increase in direct runoff to the alley from the north half of this site which is partially offset by about 0.75 cfs decrease flow from the Los Pastores Pond if 0.6 acres of this site no longer drains to the pond. The increased flow to the alley may be allowed if the applicant can demonstrate that the alley has capacity for more flow than was shown in the Drainage Management Plan Addendum for Los Pastores Shopping Center. Otherwise ponds may have to be used to mitigate the increased flow to the alley. The Drainage Management Plan and Addendum for Los Pastores Shopping Center are available thru the City's GIS Advanced Map Viewer. See the Hydrology Section web page for instructions on how to access the files.
- 6. The drainage management summary on sheet 8.1 needs to be revised to include discussion of the diversion of flow from the Los Pastores Pond and the limited capacity downstream of this site in the alley and in Montgomery. The narrative should acknowledge the Drainage Management Plan for the Los Pastores Shopping Center and discuss specifically how the proposed site will conform to that plan. This level of analysis, hydrograph development, pond routing, and detailed hydraulic analysis is more easily bound into a report than spread out on 24" x 36" sheets, but either way the calculations must be stamped and signed by a registered Professional Engineer in the state of New Mexico.
- 7. Any portion of the peak 100 year flow rate that drains into Montgomery Blvd. will have to do so through appropriately sized sidewalk culverts. Use the weir equation to size the culvert not Manning's normal depth. The typical capacity is about 1.25 cfs per foot of opening, so two 2' wide culverts should be used. The sidewalk culvert must also extend all of the way from the curb to the right of way, and the invert should be specified at that location instead of the back of sidewalk. The culverts should be moved as far west as possible to the low corner of the pond.
- 8. Additional Stormwater Quality Volume (SWQV) is required for this site. The City Council and Mayor signed a new Drainage Ordinance into effect October 24, 2018 which requires a waiver for onsite management of anything less than 100% of the SWQV. It appears that onsite management of the Stormwater Quality Volume (SWQV) may be waived if private offsite mitigation is provided by deepening the existing pond north of this site. The basis for requesting a waiver of management onsite is to be clearly demonstrated on the plan. If the site does not qualify for a waiver in accordance with § 14-5-2-6(H), then the developer may choose payment in lieu or provide the total SWQV onsite by adding below grade infiltration trenches to complete the volume. The new ordinance is available on the City Hydrology Section web page (see link in footer).
- 9. A Private Drainage Easement and Drainage Covenant is required for the offsite storm drain on Tract A-1-E. It is near other easements which need to be shown and labeled on the plan as well as any conflicting utilities. Label Tract A-1-E, Access A, and A-1-D.
- 10. A Drainage Covenant is required for the first flush ponds. The original notarized form, pond exhibits (legible on 8.5x11 paper), and recording fee (\$25, payable to City of Albuquerque) must be turned into DRC (4th, Plaza del Sol) for routing. Please contact Charlotte LaBadie (clabadie@cabq.gov, 924-3996) or Madeline Carruthers (mtafoya@cabq.gov, 924-3997) regarding the routing and recording process for covenants.

CITY OF ALBUQUERQUE

Hydrology Section Planning Department David S. Campbell, Director

Sincerely.

James D. Hughes, P.E.

Principal Engineer, Planning Dept.



Timothy M. Keller, Mayor

11. As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Curtis Cherne, PE, ccherne@cabq.gov, 924-3420) 14 days prior to any earth disturbance, Building Permit, or Work Order.

If you have any questions, you can contact me at 924-3986 or E-mail at jhughes@cabq.gov.

Development and Review Services

CC Cesar Segovia, Architect

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 10/2018)

Project Title:	Building	Permit #:	Hydrol	ogy File #:
DRB#:	EPC#:		Work (Order#:
Legal Description:				
City Address:				
Applicant:			Contact	
Address:Phone#:				
Other Contact:				
Address:				_
Phone#:				
TYPE OF DEVELOPMENT:	_ PLAT (# of lots)	RESIDENCE _	DRB SITE	ADMIN SITE
IS THIS A RESUBMITTAL? Y				
DEPARTMENT: TRAFFIC/TI		HYDROLOG	Y/DRAINAGE	
Check all that Apply:				PTANCE SOUGHT:
TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERT PAD CERTIFICATION CONCEPTUAL G & D PLAN GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT FLOODPLAIN DEVELOPMENT ELEVATION CERTIFICATE CLOMR/LOMR TRAFFIC CIRCULATION LAY TRAFFIC IMPACT STUDY (TI OTHER (SPECIFY) PRE-DESIGN MEETING?	Γ PERMIT APPLIC OUT (TCL) S)	CERTI PRELI SITE P SITE P SITE P FINAL SIA/ R FOUNI GRAD SO-19 PAVIN GRAD WORK CLOM FILOOI	PLAT APPROVALELEASE OF FINANDATION PERMIT APPLAPED APPROVAL IG PERMIT APPROVAL ING/PAD CERTIF ORDER APPROVAL	PANCY PROVAL APPROVAL PERMIT APPROVAL NCIAL GUARANTEE APPROVAL ROVAL ICATION L MENT PERMIT
DATE SUBMITTED:	By:			

FEE PAID:___



Wednesday, October 24, 2018

City of Albuquerque Development Review Services

RE: 8315 Montgomery Blvd. NE - HYDROLOGY COMMENTS

Thank you for taking the time to review Site Development plans for the McDonald's at 8315 Montgomery Blvd. NE. We have addressed your comments related to the Civil Construction Plans as follows:

Hydrology Comments:

- 1) DRB approval is required if this site includes "Major Infrastructure", and solving the adjacent alley drainage problem may require Major Infrastructure. An approved drainage design and analysis is required prior to determining if DRB approval is required. Per emails from Doug Hughes, it is expected that the developer's alley solution is nearing approval and that McDonald's plans will not need to aid in the solution. Reference Los Pastores Development plans for alley drainage.
- 2) An engineer's stamp and signature are required on the G&D Plan. If the purpose of this plan is just DRB approval of the Amended Site Plan then the sheet title must be changed to "Conceptual Grading and Drainage Plan" and it must be labeled "Not for Construction" in big bold letters. Then a separate more detailed Grading and Drainage Plan must be approved by Hydrology prior to Building Permit. Both G&D plans must be stamped by a NM Professional Engineer. During review phase, plans are submitted with the preliminary seal to help protect against someone utilizing preliminary plans for construction. Grading and Drainage plan will be sealed by a NM PE prior to building permit.
- 3) The alley drainage solution must be shown on this plan. The existing alley is graded with a 15% cross slope and does not have drainage capacity to prevent public storm water from entering the private lots west of the alley. A solution to the problem must be approved by both Hydrology and Transportation prior to approval of the G&D Plan for this development. The solution must be identified on this plan. If the improvements are to be constructed as part of a separate project they should be so noted. The engineering design and analysis of the solution may either be included on the McDonalds G&D Plan or the engineering design and analysis of the solution may be included on the separate G&D Plans for the Los Pastores Shopping Center which currently has an open Work Order to build the alley. The two plans must agree and cross reference each other. Per emails from Doug Hughes, it is expected that the developer's alley solution is nearing approval and that McDonald's plans will not need to aid in the solution. Los Pastores Shopping Center referenced on Sheet C8.1 for alley drainage.
- 4) A revision to the Los Pastores Shopping Center G&D Plan has been submitted but has not been reviewed by hydrology. It includes construction of a variable height curb, drive pads, and gravel



driveways on the private lots west of the alley. Written approval will be required from all of those lot owners and the Transportation and DRC and a Change Order must be approved before it can be approved as the solution to the alley drainage problem, so this solution may take quite a while to get approved and it may not work at all. Solutions that don't involve the neighbors involve some kind of modification to the McDonalds Site Plan, either by changing the cross slope of the alley lowering the west side of the McDonalds site or by adding a pond outfall pipe through the McDonalds site to Montgomery Blvd. Matt Corte said that he has pictures of a headwall in the old pond that indicates a piped pond outfall existed before the Los Pastores Shopping Center, and that same pipe may be incorporated into the alley drainage solution for this project. Per emails from Doug Hughes, it is expected that the developer's alley solution is nearing approval and that McDonald's plans will not need to aid in the solution. An additional solution – such as lowering the McDonald's site – is not proposed at this time.

- 5) Any portion of the peak 100 year flow rate that drains into Montgomery Blvd. will have to do so through appropriately sized sidewalk culverts. No flow is allowed over the Montgomery Blvd sidewalk. Sidewalk culverts may be constructed with an SO-19 Permit if a Work Order is not required for other required infrastructure. To do so, the standard SO-19 Permit notes must be added to the G&D Plan along with hydraulic calculations, standard details, and construction notes. If a Work Order is required then the Sidewalk culverts must be constructed as part of the Work Order and an SO-19 Permit will not be allowed. Standard detail DWG 2236, notes, and hydraulic calculations have been added to comply with the SO-19 Permit requirements.
- 6) The proposed development plan for the McDonalds must either conform to the previously approved Drainage Management Plan for Tract A-1-F of the Los Pastores Shopping Center or a revised plan is required to determine new peak flow rates in the alley west of McDonalds and in Montgomery Blvd. using AHYMO instead of the simplified 100 year 6 hour runoff calculations in tables on sheets 8.0 and 8.2. If the proposed development plan is revised to conform to the previously approved Drainage Management Plan then the tables on sheets 8.0 and 8.2 must state the runoff rates and volumes associated with land treatments A, B, C, and D. Flows from the McDonald's site do not worsen the alley drainage or pond drainage (and design point flows have been added to C8.1 for reference). To conform to previously approved Drainage Management Plan, Sheets C8.0 and C8.2 show the runoff rates and volumes associated with land treatments A, B, C, and D.
- 7) The drainage summary on sheet 8.1 says basin A-1 drains into the offsite detention pond but there is not sufficient detail to see how that drainage enters the pond. Instead it appears that the drainage enters Pond 1. A typical section is required thru the north and west boundary in association with Basin A-1 and Pond 1. The sections must include horizontal and vertical dimensions to the property line and show both existing and proposed grade. Basin A-1 currently is not proposed to drain to the offsite detention pond, but it drains to Pond 1. Pond 1 overflows via depressed curb to the alley. A section of Pond 1 is shown on Sheet C8.2 and spillway calculations are shown on sheet C8.3.



- 8) Basin A-3 drains to 3 different ponds and basin A-6 drains to two different ponds. The drainage basin boundaries and areas must be revised to accurately identify the area draining to each first flush pond. The first flush requirements apply to all redeveloped impervious area on this site. Each pond should be sized for 0.26" of runoff from the impervious area draining to it. The City doesn't give credit for extra volume in a pond over and above that which is required for the impervious area draining into it. A waver may be requested for redeveloped impervious surfaces not meeting the required first flush volume. The waiver request must state the area of impervious and the first flush volume and be located next to the pond volume calculations on the G&D Plan. If the waver is granted the developer will be required to pay \$8.00/CF in lieu of constructing the required volume. Drainage areas have been revised and two ponds have been removed. Each first flush pond now collects drainage from one basin.
- 9) Details of each pond are required including typical sections, dimensions, and spot elevations. Volume calculations must be added to the plan for each pond based on the area of each contour using the conic method of volume calculations. A typical pond section is shown on Sheet C8.2. Each pond has a horizontal dimension noted on Sheet C4.0 Site Plan, and spot elevations are shown on C7.0 Grading Plan. Volume calculations are tabulated on Sheet C8.2
- 10) The term Bio Retention is inappropriate and should be replaced with the term First Flush everywhere, unless planting specifications consistent with Bio Retention are shown on the landscape plan and provided to hydrology for review and comment. Reference to 'Bio Retention' have been replaced with 'First Flush'.
- 11) Retaining walls, if any, must be clearly identified on the plan. Typical sections are required around the perimeter of the property especially at retaining walls. Existing and proposed spot elevations are required around the perimeter of the site at about 25' intervals. Spot elevations were added around the perimeter. Retaining walls are not required.
- 12) Existing Spot elevations are not legible. The minimum font size is 0.10". Dots show up instead of text around the perimeter of the site. The drainage summary on sheet 8.1 also has dots instead of text. Text heights were increased to 0.10"
- 13) Details of all drainage structures must be shown on the plan including pond spillways, curbs, curb cuts, inlets, headwalls, and sidewalk culverts. Hydraulic capacity calculations are required for all inlet, curb cuts, curbs carrying drainage, and pond spillways. Both the first flush elevation and the 100 year elevation must be identified for all ponds. Storm drains must be shown on the Grading sheet 7.0 including pipe sizes and material. Plans and details revised to show details, hydraulic calculations, and 100-yr elevations (as applicable)



- 14) HGL calculations are required for the storm drain pipes and the HGL must be shown on a profile of each pipe specifying the pipe material, slope, invert elevations, and lengths, flow rate, and velocity. Profile and HGL added for for STRM-2. Calculations tabulated on Sheet C8.2
- 15) A Private Drainage Easement and Drainage Covenant is required for the offsite storm drain on Tract A-1-E. It is near other easements which need to be shown and labeled on the plan as well as any conflicting utilities. Label Tract A-1-E, Access A, and A-1-D. Private Drainage Easement and Drainage Covenant to be provided. 10' Private drainage easement shown on plans.
- 16) A Private Facility Drainage Covenant is required for the first flush ponds. The original notarized form, pond exhibits (legible on 8.5x11 paper), and recording fee (\$25, payable to City of Albuquerque) must be turned into DRC (4th, Plaza del Sol) for routing. Please contact Charlotte LaBadie (clabadie@cabq.gov, 924-3996) or Madeline Carruthers(mtafoya@cabq.gov, 924-3997) regarding the routing and recording process for covenants.

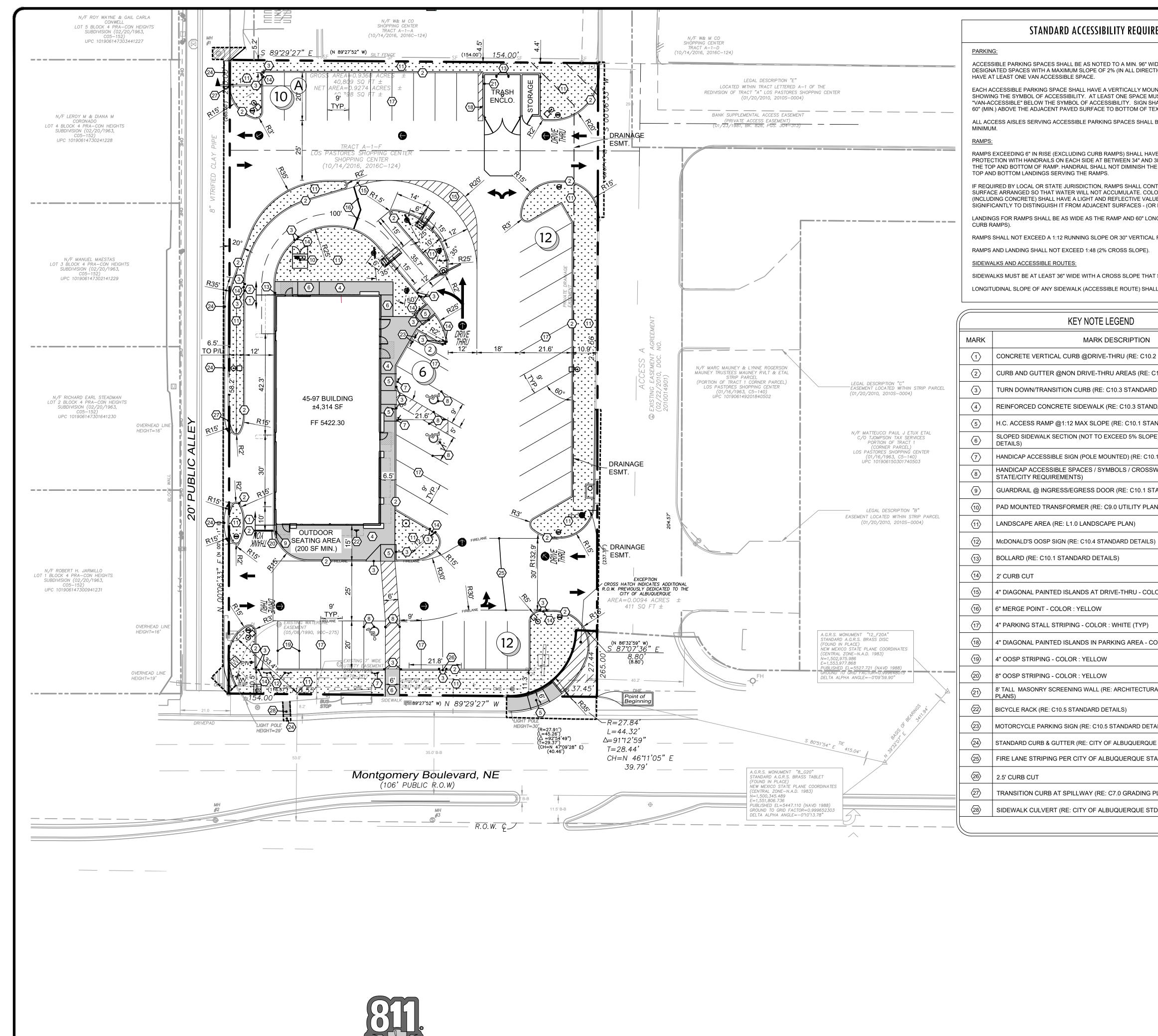
Drainage Covenant documents will be provided once grading and drainage items are approved by hydrology and ponds are set.

Please let us know if any further corrections are needed.

Sincerely,

Ben Betzold

Attachments: 1 (Updated Civil Plans)



STANDARD ACCESSIBILITY REQUIREMENTS

ACCESSIBLE PARKING SPACES SHALL BE AS NOTED TO A MIN. 96" WIDE OR A MIN. 132" WIDE FOR VAN DESIGNATED SPACES WITH A MAXIMUM SLOPE OF 2% (IN ALL DIRECTIONS). ALL BUILDINGS SHALL HAVE AT LEAST ONE VAN ACCESSIBLE SPACE.

EACH ACCESSIBLE PARKING SPACE SHALL HAVE A VERTICALLY MOUNTED (OR SUSPENDED) SIGN SHOWING THE SYMBOL OF ACCESSIBILITY. AT LEAST ONE SPACE MUST INCORPORATE "VAN-ACCESSIBLE" BELOW THE SYMBOL OF ACCESSIBILITY. SIGN SHALL BE LOCATED AS NOTED TO 60" (MIN.) ABOVE THE ADJACENT PAVED SURFACE TO BOTTOM OF TEXT.

ALL ACCESS AISLES SERVING ACCESSIBLE PARKING SPACES SHALL BE AS NOTED TO A 60" WIDE

RAMPS EXCEEDING 6" IN RISE (EXCLUDING CURB RAMPS) SHALL HAVE APPROPRIATE EDGE PROTECTION WITH HANDRAILS ON EACH SIDE AT BETWEEN 34" AND 38", AND EXTEND 12" BEYOND THE TOP AND BOTTOM OF RAMP. HANDRAIL SHALL NOT DIMINISH THE CLEAR AREA REQUIRED FOR TOP AND BOTTOM LANDINGS SERVING THE RAMPS.

IF REQUIRED BY LOCAL OR STATE JURISDICTION, RAMPS SHALL CONTAIN A TRUNCATED DOME SURFACE ARRANGED SO THAT WATER WILL NOT ACCUMULATE. COLOR OF RAMP FINISH MATERIAL (INCLUDING CONCRETE) SHALL HAVE A LIGHT AND REFLECTIVE VALUE AND MUST CONTRAST SIGNIFICANTLY TO DISTINGUISH IT FROM ADJACENT SURFACES - (OR PAINT STRIPE).

LANDINGS FOR RAMPS SHALL BE AS WIDE AS THE RAMP AND 60" LONG MINIMUM (36" MINIMUM FOR

RAMPS SHALL NOT EXCEED A 1:12 RUNNING SLOPE OR 30" VERTICAL RISE.

RAMPS AND LANDING SHALL NOT EXCEED 1:48 (2% CROSS SLOPE).

SIDEWALKS AND ACCESSIBLE ROUTES:

SIDEWALKS MUST BE AT LEAST 36" WIDE WITH A CROSS SLOPE THAT SHALL NOT EXCEED 1:48 (2%). LONGITUDINAL SLOPE OF ANY SIDEWALK (ACCESSIBLE ROUTE) SHALL NOT EXCEED 1:20 (5%).

KEY NOTE LEGEND

	KET NOTE LEGEND
MARK	MARK DESCRIPTION
1	CONCRETE VERTICAL CURB @DRIVE-THRU (RE: C10.2 STANDARD DETAILS)
2	CURB AND GUTTER @NON DRIVE-THRU AREAS (RE: C10.2 STANDARD DETAILS)
3	TURN DOWN/TRANSITION CURB (RE: C10.3 STANDARD DETAILS)
4	REINFORCED CONCRETE SIDEWALK (RE: C10.3 STANDARD DETAILS)
(5)	H.C. ACCESS RAMP @1:12 MAX SLOPE (RE: C10.1 STANDARD DETAILS)
6	SLOPED SIDEWALK SECTION (NOT TO EXCEED 5% SLOPE) (RE: C10.3 STANDARD DETAILS)
7	HANDICAP ACCESSIBLE SIGN (POLE MOUNTED) (RE: C10.1 STANDARD DETAILS)
8	HANDICAP ACCESSIBLE SPACES / SYMBOLS / CROSSWALK - COLOR : (PER STATE/CITY REQUIREMENTS)
9	GUARDRAIL @ INGRESS/EGRESS DOOR (RE: C10.1 STANDARD DETAILS)
(10)	PAD MOUNTED TRANSFORMER (RE: C9.0 UTILITY PLAN)
(11)	LANDSCAPE AREA (RE: L1.0 LANDSCAPE PLAN)

BOLLARD (RE: C10.1 STANDARD DETAILS)

4" DIAGONAL PAINTED ISLANDS AT DRIVE-THRU - COLOR : YELLOW 6" MERGE POINT - COLOR: YELLOW

4" PARKING STALL STRIPING - COLOR : WHITE (TYP)

4" DIAGONAL PAINTED ISLANDS IN PARKING AREA - COLOR: WHITE 4" OOSP STRIPING - COLOR : YELLOW

8' TALL MASONRY SCREENING WALL (RE: ARCHITECTURAL AND STRUCTURAL BICYCLE RACK (RE: C10.5 STANDARD DETAILS)

MOTORCYCLE PARKING SIGN (RE: C10.5 STANDARD DETAILS)

STANDARD CURB & GUTTER (RE: CITY OF ALBUQUERQUE STD. DWG. 2415A) FIRE LANE STRIPING PER CITY OF ALBUQUERQUE STANDARDS

TRANSITION CURB AT SPILLWAY (RE: C7.0 GRADING PLAN) SIDEWALK CULVERT (RE: CITY OF ALBUQUERQUE STD. DWG. 2236)

SCALE: 1" = 20'

THESE PLANS ARE SUBJECT TO REVIEW & APPROVAL BY JURISDICTIONAL ENTITIES.

OWNER INFORMATION

MCDONALD'S USA, LLC MOUNTAIN SOUTHWEST FIELD EXECUTION TEAM 511 E. JOHN CARPENTER FRWY, STE. 375 IRVING, TX 75062 (214) 533-7382 CONTACT: LEE MORRIS

HERE WE GROW AGAIN SIGN

THE GENERAL CONTRACTOR SHALL DISPLAY A "HERE WE GROW AGAIN" BANNER DURING CONSTRUCTION. THESE BANNERS ARE AVAILABLE THROUGH ALTRUA GLOBAL SOLUTIONS. CONTACT: KRISTY FIALLO - PH#: 1-800-443-6939

THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW, AGENCY APPROVAL, AND COMMENT UNDER THE AUTHORITY OF G. ROBERT ADAMS, P.E. REGISTRATION No. 15142, ON 10/24/18 THIS DOCUMENT IS NOT TO BE USED FOR

McD

CONSTRUCTION PURPOSES

8315

LEGEND

FLAG POLE

McDONALD'S EXISTING SIGN LIGHT STANDARD (15' CLEAR FROM ALL OVERHEAD

UTILITY LINES) (24" CLEAR FROM BACK OF CURB) McDONALD'S DIGITAL MENU BOARD

McDONALD'S ORDER HERE CANOPY

McDONALD'S DIGITAL PRE-BROWSE BOARD McDONALD'S DOUBLE GATEWAY

McDONALD'S DIRECTIONAL SIGN

DETECTOR LOOP (LOCATION TO BE APPROVED BY

McDONALD'S) (RE: C10.0 STANDARD DETAILS)

POWER POLE

DOMESTIC WATER METER

IRRIGATION WATER METER

EXISTING FIRE HYDRANT

GREASE INTERCEPTOR (RE: C9.0 UTILITY PLAN)

"DRIVE-THRU" WITH "CIRCLE / ARROW" - COLOR :

"THANK YOU" AT END OF PATH - COLOR: YELLOW

"CIRCLE / ARROW" - COLOR : YELLOW

ARROW PATH DIRECTION - COLOR: WHITE

THANK YOU DOUBLE DRIVE-THRU "ARROW MARKING" - COLOR:

 $(\cdot \cdot)$

LAND AREA: **CURRENT ZONING: EXISTING USE:**

BUILDING AREA (APPROXIMATE): BUILDING LOT COVERAGE:

PARKING PROVIDED:

PROPOSED USE:

4,314 SF/40,398 SF = 10.68% 1 SPACE PER 4 SEATS 64 SEATS/4 = 16 SPACES

McDONALD'S DRIVE-THRU RESTAURANT

HANDICAP PARKING PROVIDED: MOTORCYCLE PARKING REQUIRED: 1 MOTORCYCLE PARKING PROVIDED: 2 BICYCLE PARKING REQUIRED: BICYCLE PARKING PROVIDED: EXISTING IMPERVIOUS AREA: 35,001 SF PROPOSED IMPERVIOUS AREA: 33,214 SF PROPOSED PERVIOUS AREA:

SITE INFORMATION

PARKING REQUIRED:

HANDICAP PARKING REQUIRED:

40,398 SF (0.927 AC) McDONALD'S DRIVE-THRU RESTAURANT

7,184 SF (17.78%)

DATE BY DESIGNED MAR 2018 MHA DRAWN MAR 2018 MHA APR 2018 DWL CHECKED AS-BUILT

SITE PLAN

Know what's below.

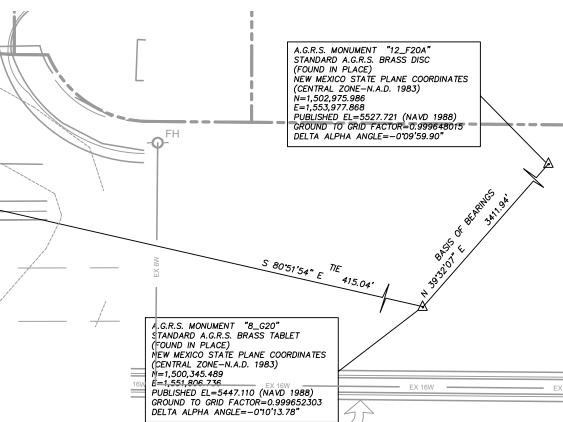
Call before you dig.

Rectangular		Highlighted	
Bottom Width (ft)	= 1.50	Depth (ft)	= 0.42
Total Depth (ft)	= 0.42	Q (cfs)	= 4.244
		Area (sqft)	= 0.63
Invert Elev (ft)	= 100.00	Velocity (ft/s)	= 6.74
Slope (%)	= 2.00	Wetted Perim (ft)	= 2.34
N-Value	= 0.013	Crit Depth, Yc (ft)	= 0.42
		Top Width (ft)	= 1.50
Calculations		EGL (ft)	= 1.13
		(GE) (ME)	

Depth (ft) - 1.00 - 0.75 **-** 0.50 0.25 0.00

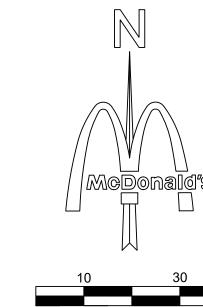
2.5

Friday, Aug 10 2018



- 3. Two working days prior to any excavation, the contractor must contact **New Mexico One Call, dial "811"** [or (505) 260-1990] for the location of existing utilities.
- 4. Prior to construction, the contractor shall excavate and verify the locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer so that the conflict can be
- resolved with a minimum amount of delay. 5. Backfill compaction shall be according to traffic/street use. 6. Maintenance of the facility shall be the responsibility of the owner of the property being served.
- 7. Work on arterial streets may be required on a 24-hour basis. 8. Contractor must contact Jason Rodriguez at 235-8016 and Construction Coordination at 924-3416 to schedule an inspection.





SCALE: 1" = 20'

THESE PLANS ARE SUBJECT TO REVIEW & APPROVAL BY JURISDICTIONAL ENTITIES.

OWNER INFORMATION MCDONALD'S USA, LLC MOUNTAIN SOUTHWEST FIELD EXECUTION TEAM 511 E. JOHN CARPENTER FRWY, STE. 375 IRVING, TX 75062 (214) 533-7382 CONTACT: LEE MORRIS

LEGEND TC = TOP OF CURB TP = TOP OF PAVEMENT FG = FINISHED GRADE FF = FINISHED FLOOR ± = MATCH EXISTING GRADE — · · · ← SWALE — — — GRADE BREAK RIDGE LINE ----- XXX ----- EXISTING CONTOUR PROPOSED CONTOUR LEVEL LANDING @ 2% MAX SLOPE IN ANY DIRECTION

BENCHMARK

BENCHMARK 1 A.G.R.S. MONUMENT "12_F20A" STANDARD A.G.R.S. BRASS DISC (FOUND IN PLACE) NEW MEXICO STATE PLANE COORDINATES (CENTRAL ZONE-N.A.D. 1983) N=1,502,975.986, E=1,553,977.868 PUBLISHED EL=5527.721 (NAVD 1988) GROUND TO GRID FACTOR= 0.999648015 DELTA ALPHA ANGLE=-0°09'59.90" BENCHMARK 1 A.G.R.S. MONUMENT "8 G20"

STANDARD A.G.R.S. BRASS TABLET (FOUND IN PLACE) NEW MEXICO STATE PLANE COORDINATES (CENTRAL ZONE-N.A.D. 1983) N=1,500,345.489, E=1,551,806.736 PUBLISHED EL=5447.110 (NAVD 1988) GROUND TO GRID FACTOR= 0.999652303 DELTA ALPHA ANGLE=-0°10'13.78"

STANDARD ACCESSIBILITY REQUIREMENTS

ACCESSIBLE PARKING SPACES SHALL BE AS NOTED TO A MIN. 96" WIDE OR A MIN. 132" WIDE FOR VAN DESIGNATED SPACES WITH A MAXIMUM SLOPE OF 2% (IN ALL DIRECTIONS). ALL BUILDINGS SHALL HAVE AT LEAST ONE VAN ACCESSIBLE SPACE.

EACH ACCESSIBLE PARKING SPACE SHALL HAVE A VERTICALLY MOUNTED (OR SUSPENDED) SIGN SHOWING THE SYMBOL OF ACCESSIBILITY. AT LEAST ONE SPACE MUST INCORPORATE "VAN-ACCESSIBLE" BELOW THE SYMBOL OF ACCESSIBILITY. SIGN SHALL BE LOCATED AS NOTED TO 60" (MIN.) ABOVE THE ADJACENT PAVED SURFACE TO BOTTOM OF TEXT.

ALL ACCESS AISLES SERVING ACCESSIBLE PARKING SPACES SHALL BE AS NOTED TO A 60" WIDE

RAMPS EXCEEDING 6" IN RISE (EXCLUDING CURB RAMPS) SHALL HAVE APPROPRIATE EDGE PROTECTION WITH HANDRAILS ON EACH SIDE AT BETWEEN 34" AND 38", AND EXTEND 12" BEYOND THE TOP AND BOTTOM OF RAMP. HANDRAIL SHALL NOT DIMINISH THE CLEAR AREA REQUIRED FOR TOP AND BOTTOM LANDINGS SERVING THE RAMPS.

IF REQUIRED BY LOCAL OR STATE JURISDICTION, RAMPS SHALL CONTAIN A TRUNCATED DOME SURFACE ARRANGED SO THAT WATER WILL NOT ACCUMULATE. COLOR OF RAMP FINISH MATERIAL (INCLUDING CONCRETE) SHALL HAVE A LIGHT AND REFLECTIVE VALUE AND MUST CONTRAST SIGNIFICANTLY TO DISTINGUISH IT FROM ADJACENT SURFACES - (OR PAINT STRIPE).

LANDINGS FOR RAMPS SHALL BE AS WIDE AS THE RAMP AND 60" LONG MINIMUM (36" MINIMUM FOR

RAMPS SHALL NOT EXCEED A 1:12 RUNNING SLOPE OR 30" VERTICAL RISE.

RAMPS AND LANDING SHALL NOT EXCEED 1:48 (2% CROSS SLOPE).

SIDEWALKS AND ACCESSIBLE ROUTES:

SIDEWALKS MUST BE AT LEAST 36" WIDE WITH A CROSS SLOPE THAT SHALL NOT EXCEED 1:48 (2%). LONGITUDINAL SLOPE OF ANY SIDEWALK (ACCESSIBLE ROUTE) SHALL NOT EXCEED 1:20 (5%).

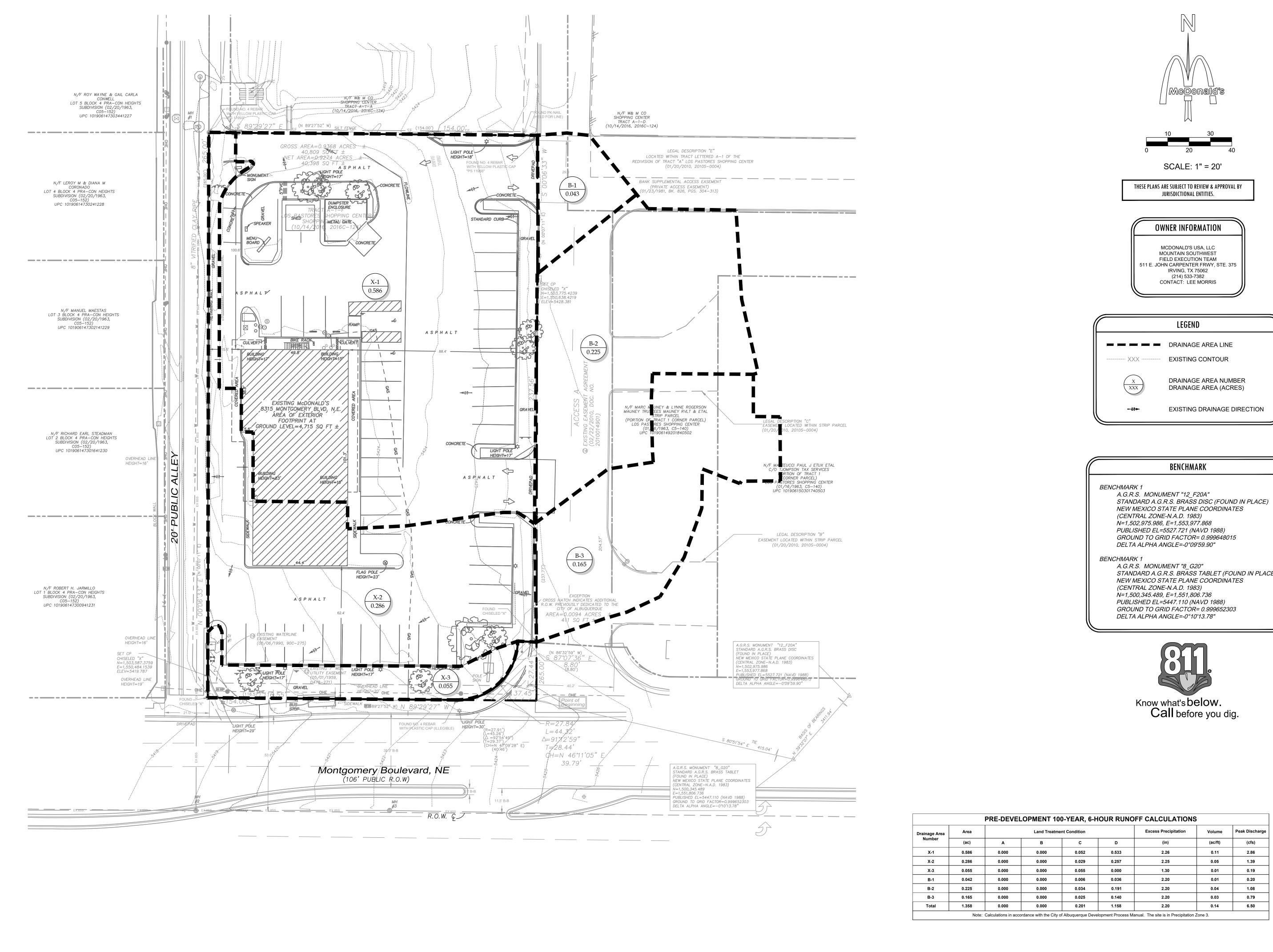
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DATE BY DESIGNED MAR 2018 HJM DRAWN MAR 2018 | HJM CHECKED 04/04/2018| DWI AS-BUILT

GRADING

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EXISTING DRAINAGE DIRECTION

STANDARD A.G.R.S. BRASS TABLET (FOUND IN PLACE)

Drainage Area	Area		Land Treatmen	nt Condition		Excess Precipitation	Volume	Peak Discharge
Number	(ac)	Α	В	С	D	(in)	(ac/ft)	(cfs)
X-1	0.586	0.000	0.000	0.052	0.533	2.26	0.11	2.86
X-2	0.286	0.000	0.000	0.029	0.257	2.25	0.05	1.39
X-3	0.055	0.000	0.000	0.055	0.000	1.30	0.01	0.19
B-1	0.042	0.000	0.000	0.006	0.036	2.20	0.01	0.20
B-2	0.225	0.000	0.000	0.034	0.191	2.20	0.04	1.08
B-3	0.165	0.000	0.000	0.025	0.140	2.20	0.03	0.79
Total	1.358	0.000	0.000	0.201	1.158	2.20	0.14	6.50
-	Note:	Calculations in acco	ordance with the City of	Albuquerque Deve	lopment Process Ma	anual. The site is in Precipitation 2	Zone 3.	

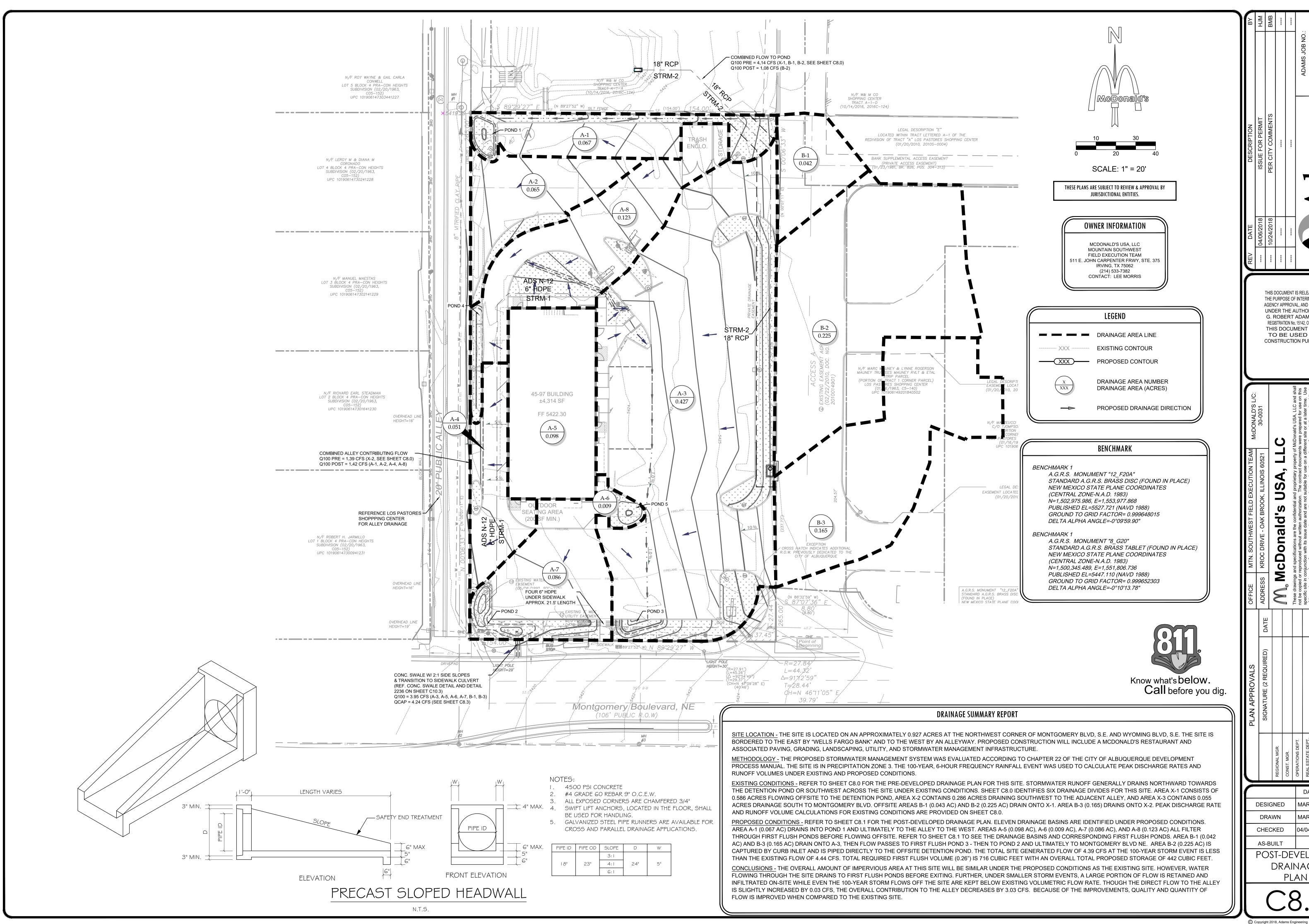
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PRE-DEVELOPED DRAINAGE PLAN

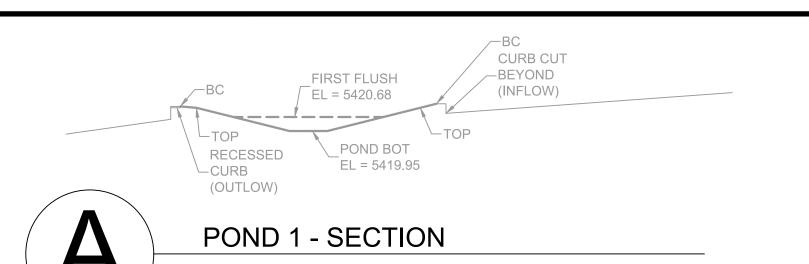


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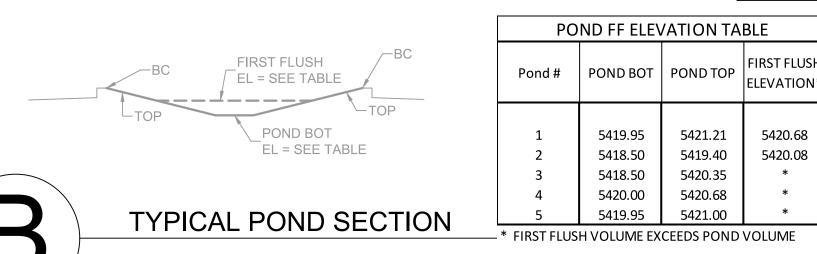
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DATE BY MAR 2018 TJR MAR 2018 | TJR 04/04/2018 DW

POST-DEVELOPED DRAINAGE



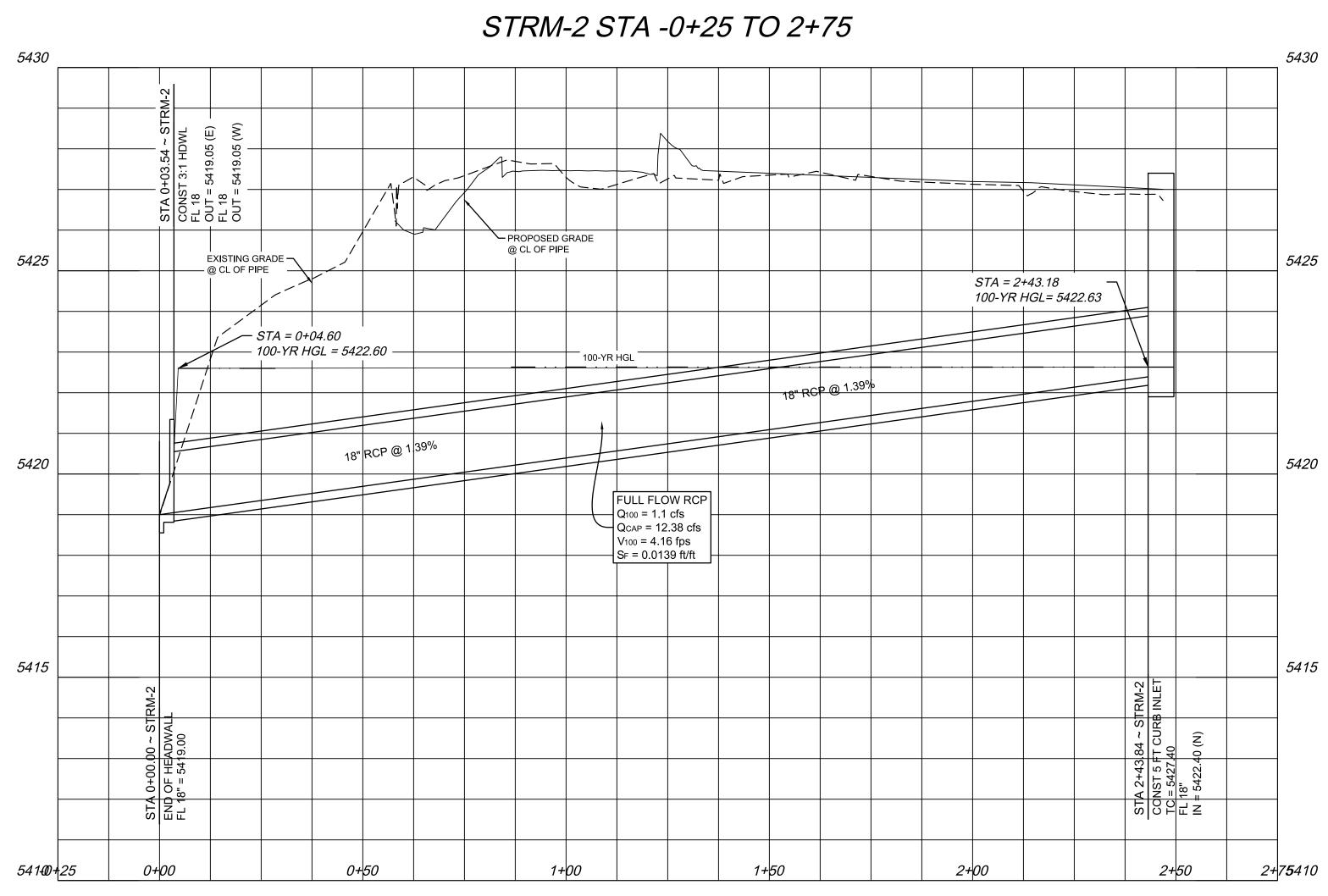
									100-YEAR	, 6-HOUR H	YDRAULIC	CALCULATI	ONS									
Line ID	Upstream Station	Downstream Station	Pipe Length	Pipe Slope	Drainage Area Designation	Time at Inlet	Time in Pipe	Cumulative Time	Peak Flow	Cumulative Peak Flow	Pipe Diameter	Friction Slope	Hydraulio	c Gradient	Velocity In	Velocity Out	V1^2/2g	V2^2/2g	Loss Coefficient	Velocity Head Loss	Upstream Invert Elev.	Downstream Invert
													Upstream	Downstream							-	
-	-	-	(ft)	(ft/ft)	-	(min)	(min)	(min)	(cfs)	(cfs)	(in)	(ft/ft)	-	-	(ft/sec)	(ft/sec)	(ft)	(ft)	"Kj"	(ft)	(ft)	(ft)
STRM-1.2	0+07.12	0+00.00	7.12	0.0828	A-5	5.00	0.16	5.16	0.1	0.1	6	0.0007	5419.48	5419.47	0.00	0.75	0.00	0.01	0.35	0.01	5418.49	5418.10
																					<u> </u>	
STRM-1.1	0+07.12	0+00.00	7.12	0.0500	A-5	5.00	0.16	5.16	0.1	0.1	6	0.0007	5419.45	5419.45	0.00	0.75	0.00	0.01	0.35	0.01	5418.49	5418.10
																					ļ	
STRM-1	1+98.86	1+08.67	90.19	0.0065	A-5	5.00	1.50	6.50	0.2	0.2	6	0.0010	5419.57	5419.47	0.00	1.00	0.00	0.02	0.35	0.02	5419.18	5419.21
STRM-1	1+08.67	0+67.90	40.77	0.0065	STRM-1.2	6.50	0.90	7.40	0.1	0.3	6	0.0006	5419.47	5419.45	1.00	0.75	0.02	0.01	0.35	0.00	5419.21	5418.94
STRM-1	0+67.90	0+00.00	67.90	0.0065	STRM-1.1	7.40	1.50	8.91	0.1	0.5	6	0.0006	5419.44	5419.40	0.75	0.75	0.01	0.01	0.35	0.01	5418.94	5418.50
STRM-2	2+43.84	0+00.00	243.84	0.0139	B-3	5.00	6.68	11.68	1.1	1.1	18	0.0001	5422.63	5422.60	0.00	0.61	0.00	0.01	0.35	0.01	5422.40	5419.00



FIRST FLUSH VOLUME CALCULA	FIRST FLUSH VOLUME CALCULATIONS							
	SF / CF	ac / ac-ft						
Total On-site Impervious Area =	33050.42	0.759						
Required Retention Volume (0.26"/acre) =	716	0.016						
Retention Volume Provided =	442	0.010						

FIRST FLUSH VOLUME CALCULATIONS								
Pond #	Drainage Areas	Impervious Area (Ac)	FF Required Volume (cf)	FF Provided Volume (cf)				
1	A-1	0.037	35	105				
2	A-5, A-7	0.167	157	131				
3	A-3	0.353	334	83				
4	A-8	0.092	87	76				
5	A-6	0.003	3	48				
Off-Site	A-2, A-4	0.107	101	-				
Total	All	0.759	716	442				

Drainage Area	Area		Land Treatme	nt Condition		Excess Precipitation	Volume	Peak Discharge
Number	(ac)	Α	В	С	D	(in)	(ac/ft)	(cfs)
A-1	0.067	0.000	0.000	0.030	0.037	1.88	0.01	0.29
A-2	0.065	0.000	0.000	0.007	0.058	2.24	0.01	0.31
A-3	0.427	0.000	0.000	0.073	0.353	2.18	0.08	2.03
A-4	0.051	0.000	0.000	0.002	0.049	2.32	0.01	0.25
A-5	0.098	0.000	0.000	0.000	0.098	2.36	0.02	0.49
A-6	0.009	0.000	0.000	0.006	0.003	1.64	0.00	0.04
A-7	0.086	0.000	0.000	0.018	0.069	2.14	0.02	0.41
A-8	0.123	0.000	0.000	0.031	0.092	2.09	0.02	0.57
B-1	0.042	0.000	0.000	0.006	0.036	2.20	0.01	0.20
B-2	0.225	0.000	0.000	0.034	0.191	2.20	0.04	1.08
B-3	0.165	0.000	0.000	0.025	0.140	2.20	0.03	0.79
Total	1.358	0.000	0.000	0.232	1.126	2.18	0.25	6.45



"FIRST	FLUSH" PO	ND 1 STAGE	-STORAGE V	OLUME
Elevation	Area	Inc. Depth	Inc. Volume	Total Volume
Lievation	(sq. ft.)	(ft.)	(cu. Ft.)	(cu. ft.)
5419.95	0.0			0.0
		0.1	0.2	
5420.00	9.6			0.2
		1.0	67.5	
5421.00	154.5			67.7
		0.2	36.9	
5421.21	198.4			104.6
"FIRST	FLUSH" PO	ND 2 STAGE	-STORAGE V	OLUME
Elevation	Area	Inc. Depth	Inc. Volume	Total Volume
	(sq. ft.)	(ft.)	(cu. Ft.)	(cu. ft.)
5418.50	27.9			0.0
		0.5	39.9	
5419.00	147.3			39.9
		0.4	90.6	
5419.40	316.6			130.5
•	'FIRST FLUSH"	POND 3 STAGE-S	TORAGE VOLUME	
Elevation	Area	Inc. Depth	Inc. Volume	Total Volume
Licvation	(sq. ft.)	(ft.)	(cu. Ft.)	(cu. ft.)
5418.50	2.2			0.0
		0.5	4.2	
5419.00	16.8			4.2
		1.0	44.9	
5420.00	80.9			49.1
		0.4	33.4	
5420.35	110.5			82.5
•	'FIRST FLUSH"	POND 4 STAGE-S	TORAGE VOLUME	
El .:	Area	Inc. Depth	Inc. Volume	Total Volume
Elevation	(sq. ft.)	(ft.)	(cu. Ft.)	(cu. ft.)
5420.00	0.0			0.0
		0.7	75.8	
5420.68	334.4			75.8

"FIRST FLUSH" POND 5 STAGE-STORAGE VOLUME									
	Area	Inc. Depth	Inc. Volume	Total Volume					
Elevation	(sq. ft.)	(ft.)	(cu. Ft.)	(cu. ft.)					
5419.95	0.0			0.0					
		0.1	0.1						
5420.00	6.7			0.1					
		1.0	48.2						
5421.00	110.7			48.3					

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	ISSUE FOR PERMIT	04/06/2018	
	DESCRIPTION	DATE	REV
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	SIGNATURE (2 REQUIRED)	DATE	ADDRESS	KROC DRIVE - OAK BROOK, ILLINOIS 60521	30-03
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MGR.)
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				ALBUQUERQUE, NM	

	DATE	BY
DESIGNED	MAR 2018	TJR
DRAWN	MAR 2018	TJR
CHECKED	04/04/2018	DWL
AS-BUILT		

DRAINAGE CALCULATIONS

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Weir Report Weir Report Weir Report Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Friday, Aug 10 2018 Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Friday, Aug 10 2018 Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Friday, Aug 10 2018 POND 4: 0.25-FT DEEP SPILLWAY 2.5-foot Curb Cut 2-foot Curb Cut Trapezoidal Weir Highlighted Rectangular Weir Rectangular Weir Highlighted Depth (ft) Depth (ft) = Sharp Depth (ft) = 0.25Crest = Sharp = 0.50Crest = Sharp = 0.50Crest = 1.085 Bottom Length (ft) Q (cfs) = 2.355 Bottom Length (ft) = 2.00 Q (cfs) = 2.50 = 2.943 Bottom Length (ft) = 2.00 Q (cfs) = 0.25= 0.50= 1.25 = 0.50 = 1.00 = 0.75Total Depth (ft) Total Depth (ft) Total Depth (ft) Area (sqft) Area (sqft) Area (sqft) = 4.00 Velocity (ft/s) = 1.45 Velocity (ft/s) = 2.35 Velocity (ft/s) = 2.35 Side Slope (z:1) = 2.00 Top Width (ft) = 2.50 Top Width (ft) Top Width (ft) = 4.00 Calculations Calculations Weir Coeff. Cw = 3.33Weir Coeff. Cw = 3.33Calculations = 3.10 Known Depth Weir Coeff. Cw Compute by: Compute by: Known Depth Known Depth Known Depth (ft) = 0.50= 0.50Compute by: Known Depth (ft) Known Depth (ft) = 0.25POND 4: 0.25-FT DEEP SPILLWAY Depth (ft) Depth (ft) Depth (ft) 2.5-foot Curb Cut Depth (ft) Depth (ft) 2-foot Curb Cut Depth (ft) 0.50 -0.50 -0.50 — - 0.50 - 0.50 0.00 -- 0.00 0.00 -0.00 0.00 — - 0.00 ----- W.S. ----- Weir — Weir — W.S. — Weir — W.S. Length (ft) Length (ft) Length (ft) Weir Report **Channel Report Channel Report** Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Friday, Aug 10 2018 Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Friday, Aug 10 2018 Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Friday, Aug 10 2018 SIDEWALK CULVERT - 18-IN WIDE x 5-IN DEEP MIN. POND 1: 0.19-FT DEEP SPILLWAY CONC. SWALE FROM POND 2 TO S/W CULVERT Rectangular Trapezoidal Weir Highlighted Triangular Highlighted Side Slopes (z:1) = Sharp Depth (ft) = 0.19= 2.00, 2.00= 0.50 Bottom Width (ft) = 1.50 Depth (ft) = 0.42Depth (ft) Crest Total Depth (ft) Bottom Length (ft) = 2.00 Q (cfs) = 0.670 = 0.50 Q (cfs) = 5.112 Total Depth (ft) = 0.42Q (cfs) = 4.244 = 0.50 = 0.63= 0.19= 0.52Total Depth (ft) Area (sqft) Area (sqft) Area (sqft) Invert Elev (ft) = 100.00 = 10.22 = 100.00 = 6.74 = 4.00 Velocity (ft/s) = 1.28 Velocity (ft/s) Invert Elev (ft) Velocity (ft/s) Side Slope (z:1) = 2.24 Top Width (ft) = 3.52 = 5.90 Wetted Perim (ft) = 2.00 Wetted Perim (ft) = 2.34 Slope (%) Slope (%) = 0.013 Crit Depth, Yc (ft) = 0.50 = 0.013 = 0.42N-Value N-Value Crit Depth, Yc (ft) Calculations = 2.00 = 1.50 = 3.10 Weir Coeff. Cw Top Width (ft) Top Width (ft) Known Depth **Calculations** EGL (ft) = 2.13 Calculations EGL (ft) = 1.13 Compute by: Known Depth (ft) = 0.19Known Depth Known Depth Compute by: Compute by: Known Depth (ft) = 0.50 Known Depth (ft) = 0.42Depth (ft) POND 1: 0.19-FT DEEP SPILLWAY Depth (ft) Depth (ft) Elev (ft) Depth (ft) Elev (ft) Section Section 101.00 -101.00 -100.75 -100.75 -- 0.75 0.50 -- 0.50 100.50 -100.50 — 100.25 -0.25 100.25 --0.25- 0.00 0.00 -100.00 -100.00 — 0.00 - 0.00

1.5

Reach (ft)

Length (ft)

2.5

.5

1.5

Reach (ft)

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RE (2 REQUIRED)	DATE	ADDRESS	ADDRESS KROC DRIVE - OAK BROOK, ILLINOIS 60521	30-0031
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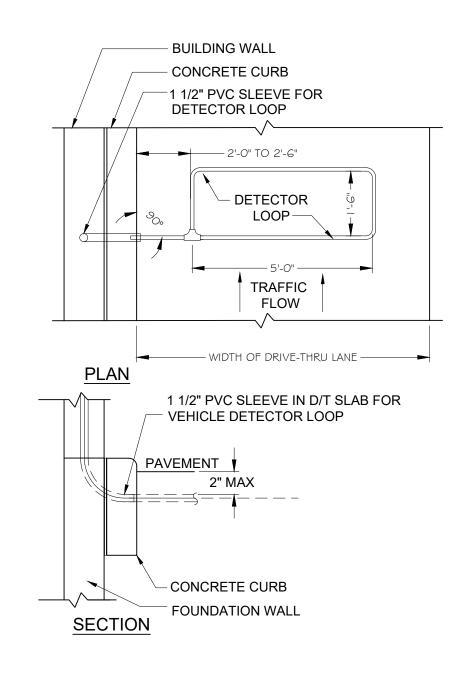
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DRAINAGE CALCULATIONS

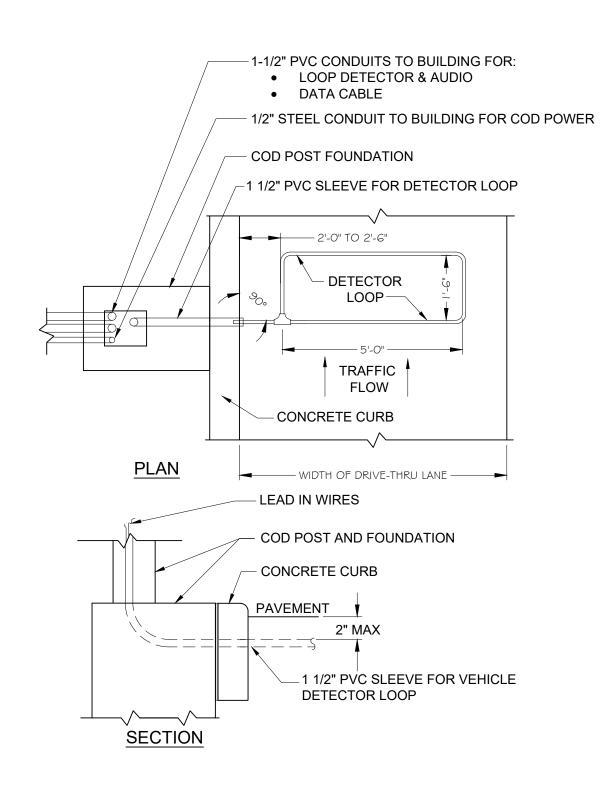
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NOTES:

- 1. VERIFY CONDUIT SIZES AND LAYOUT WITH DETECTOR LOOP MANUFACTURER.
- 2. CENTER VEHICLE DETECTOR LOOP IN DRIVE THRU LANE. INSTALL PER MANUFACTURERS RECOMMENDATIONS.
- 3. NO STEEL (REBAR OR ELECTRICAL WIRE) SHALL BE USED WITHIN 2' OF LOOP.
- 4. <u>DETECTOR LOOP MANUFACTURERS:</u> DETECTOR LOOPS MAY BE BY ONE OF THE FOLLOWINGS COMPANIES OR EQUAL. 3M: 1-800-328-0033 HME: 1-800-848-4468
- 5. DETECTOR LOOP MATERIAL: PVC TUBING 1/2" I.D. 100 PSI LOOP MADE FROM ONE LENGTH OF THIN FOURTEEN GAUGE STRANDED WIRE. LEAD-IN IS PRE-TWISTED AT FACTORY.
- 6. DETECTOR LOOP CONSTRUCTION: FORMED WITH ONE CONTINUOUS LENGTH OF PVC WITH NO SHARP CORNERS AS DETAILED. WIRE LOOPED, FORMED, & PIGTAILED AS DETAILED.

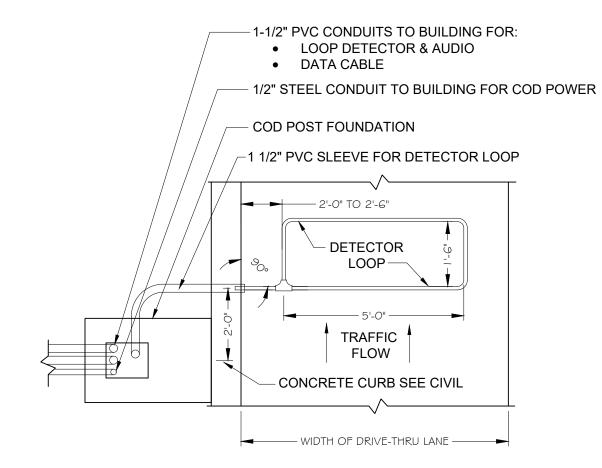


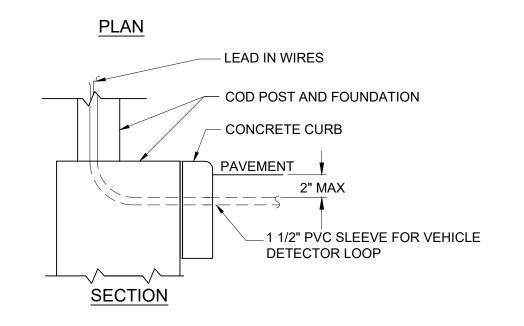
DETECTOR LOOP DETAIL AT DRIVE-THRU WINDOW NOT TO SCALE



PRIMARY LANE DETECTOR LOOP DETAIL AT C.O.D. NOT TO SCALE

NOT TO SCALE





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UNDER THE AUTHORITY OF

G. ROBERT ADAMS, P.E.

REGISTRATION No. 15142, ON 10/24/18

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DATE

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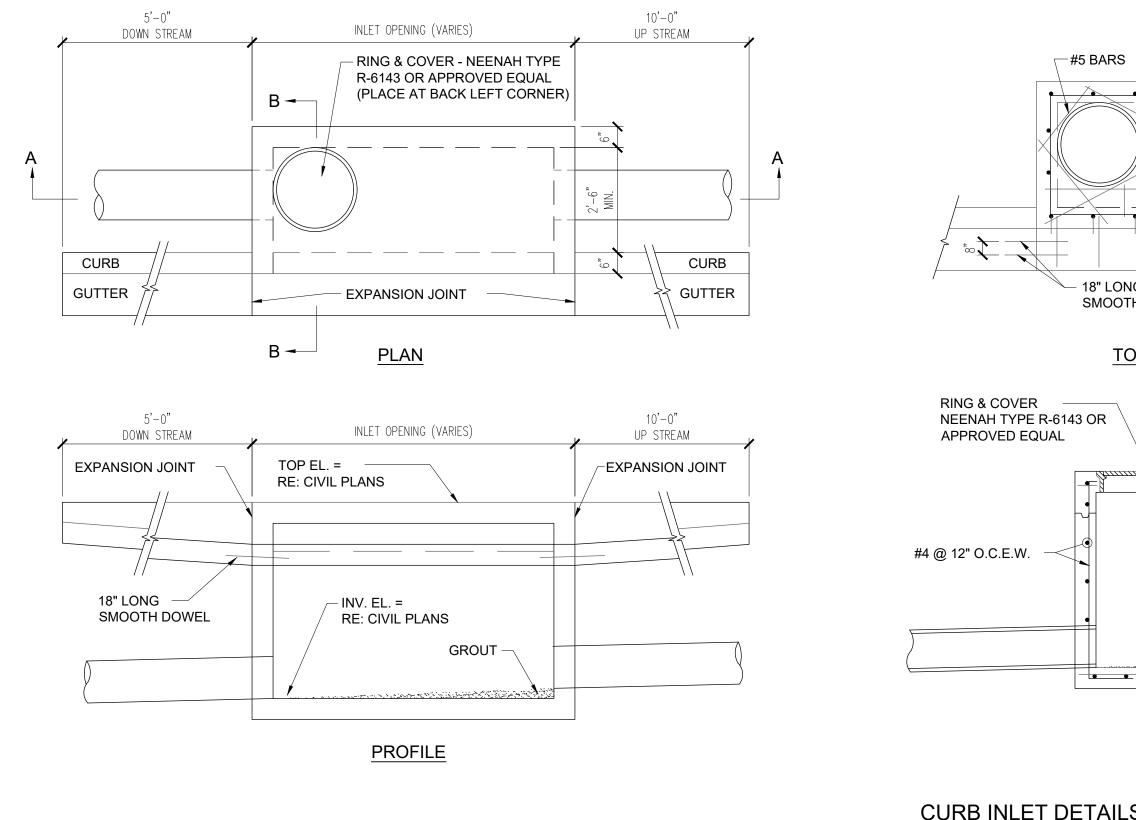
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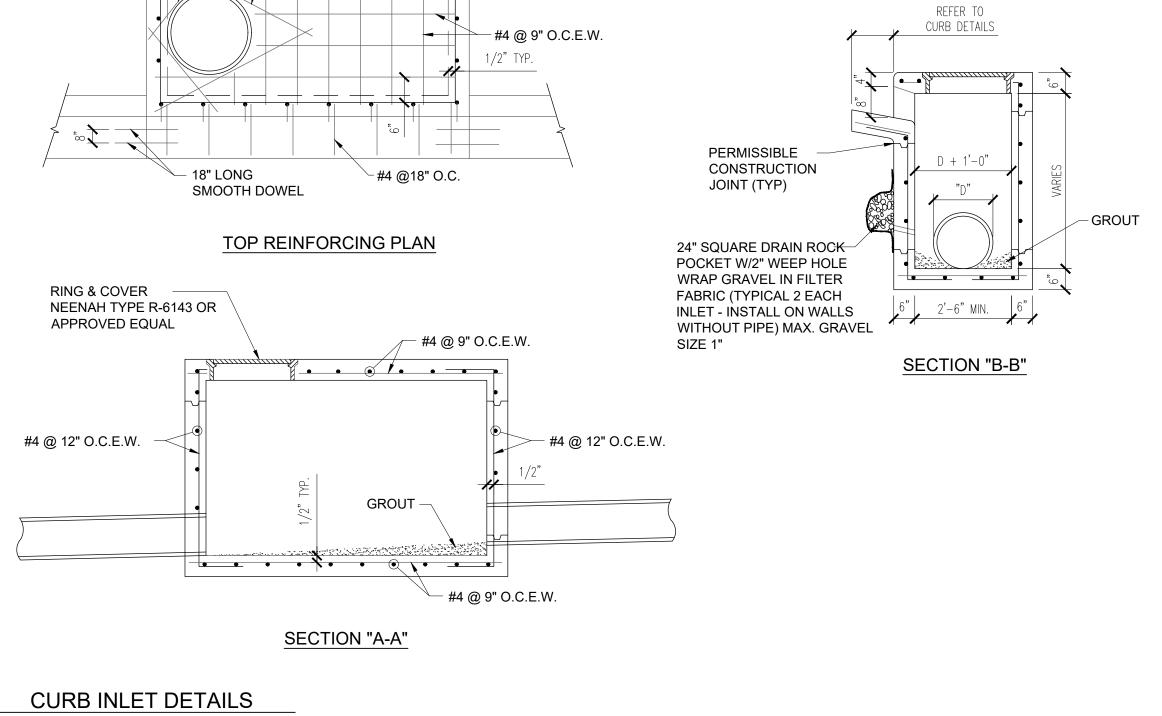
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STANDARD

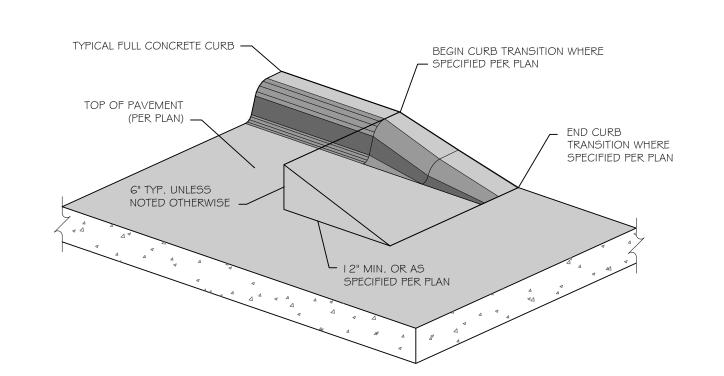
DETAILS

SECONDARY LANE DETECTOR LOOP DETAIL AT C.O.D. NOT TO SCALE



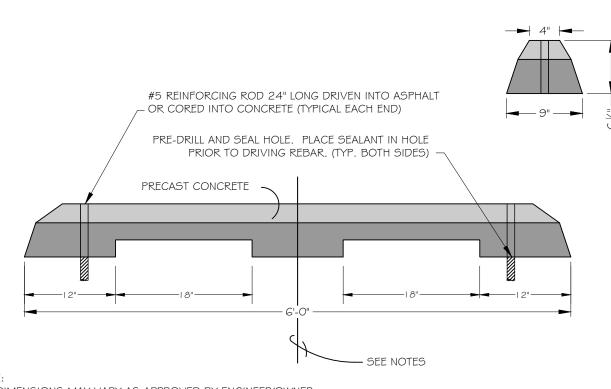


– #4 @ 12" O.C.E.W.



TURN DOWN CURB

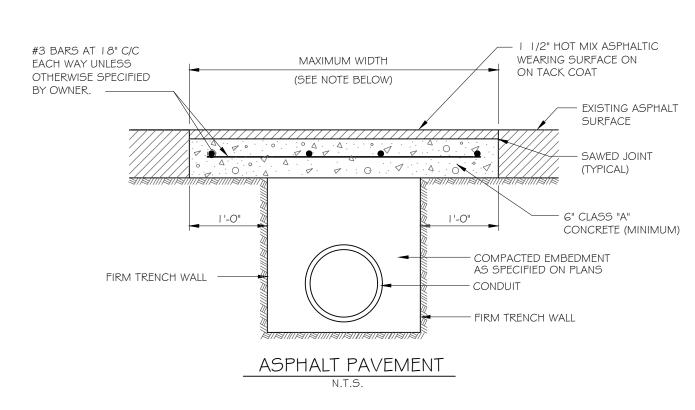
NOT TO SCALE

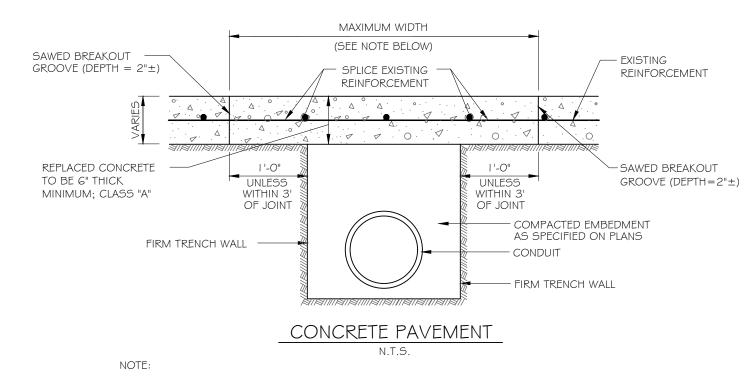


- DIMENSIONS MAY VARY AS APPROVED BY ENGINEER/OWNER
 WHEEL STOP SHALL BE CENTERED IN PARKING SPACE AS SHOWN ON SITE PLAN UNLESS NOTED OTHERWISE
- 3. WHEELSTOP SHALL BE PLACED AT 3' FROM FACE OF CURB OR EDGE OF PAVEMENT IN 90° PARKING UNLESS NOTED OTHERWISE.

WHEEL STOP

NOT TO SCALE

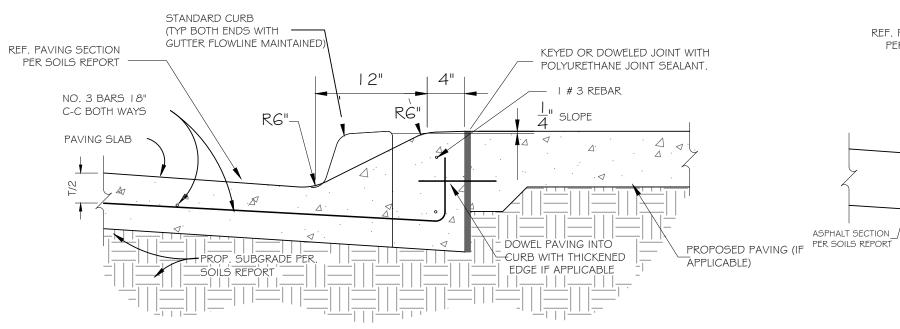




I. WHEN REMOVING CONCRETE PAVEMENT THE CONTRACTOR SHALL ENDEAVOR TO LIMIT DAMAGE TO EXISTING REINFORCEMENT SO IT MAY BE EMPLOYED IN THE REPLACEMENT OPERATION. IF ORIGINAL REINFORCEMENT IS CUT OR BROKEN, REPLACEMENT BARS OF THE SAME SIZE SHALL BE INSTALLED BY DRILLING AND DOWELING AS DIRECTED BY THE OWNER.

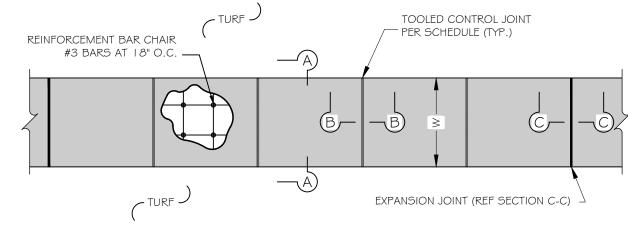
PAVEMENT CUT REMOVAL & REPLACEMENT

NOT TO SCALE



MONOLITHIC MOUNTABLE CURB

NOT TO SCALE



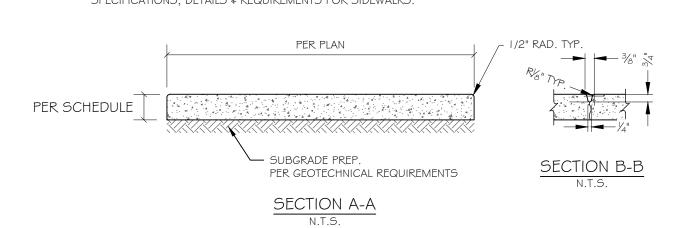
L	EXPANSION JOINT: JOINT SPACING (TYP.)	S SHALL BE LO	DCATED EVER	RY 5 PANELS — L ——■		- L	>
EXPANSION JOIN	NT (REF SECTION C	} }	· []		**********		

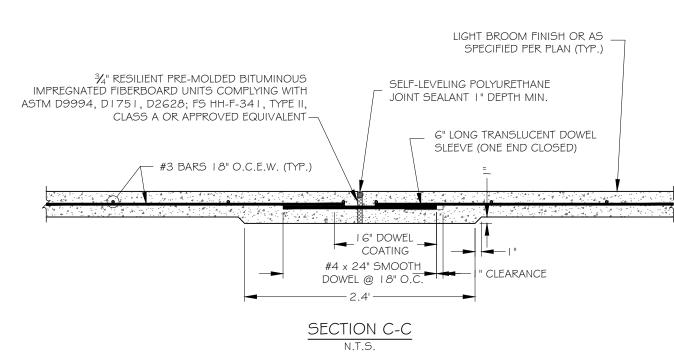
- SIDEWALK NOTES
- SAND CUSHION WILL NOT BE PERMITTED UNDER SIDEWALKS OR OTHER PAVEMENT.

 DOWEL WITH #4 BARS AT 18" C-C WHEN CONNECTING TO EXISTING SIDEWALKS, DRIVEWAYS, CURBS AND GUTTER.

 CONCRETE STRENGTH FOR SIDEWALKS SHALL BE A MINIMUM OF 3,600 PSI @ 28 DAYS OR AS SPECIFIED BY
- THE AUTHORITY HAVING JURISDICTION, WHICHEVER IS GREATER.

 4. ALL JOINTS LOCATED WITHIN LARGE AREAS OF CONCRETE FLATWORK (PLAZA AREAS OR PAVEMENT BETWEEN
- 4. ALL JOINTS LOCATED WITHIN LARGE AREAS OF CONCRETE FLATWORK (PLAZA AREAS OR PAVEMENT BETWEE BUILDINGS) SHALL BE SEALED WITH POLYURETHANE JOINT SEALANT PER JOINT DETAILS THIS SHEET.
- CONTRACTOR SHALL SUBMIT FULL-SIZE SCALEABLE PLAZA AREAS JOINT LAYOUT FOR APPROVAL.
 JOINT SEALANT IS NOT REQUIRED ON SIDEWALKS LOCATED IN TURF AREAS.
- 7. ALL SIDEWALKS LOCATED WITHIN THE PUBLIC R.O.W. SHALL CONFORM TO THE GOVERNING AUTHORITY'S SPECIFICATIONS, DETAILS & REQUIREMENTS FOR SIDEWALKS.





NOTES:

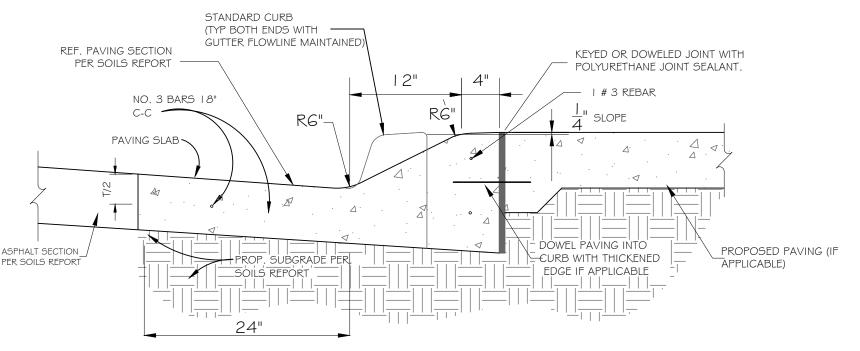
I. SLEEVES FOR DOWELS SHALL HAVE AN INSIDE DIAMETER OF ⅓₆" GREATER THAN THE DIAMETER OF THE DOWELS AND SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO USE.

2. DOWEL COATING SHALL BE ASPHALTIC COATING.

<u> </u>	SIDEWALK PANEL SCHEDULE						
SIDEWALK WIDTH = W (FT.)	SIDEWALK THICKNESS = T (IN.)	JOINT SPACING = L (FT.)					
4	4	4					
5	4	5					
6	4	5					
7	5	7					
8	5	8					
9	5	9					
10	5	10					

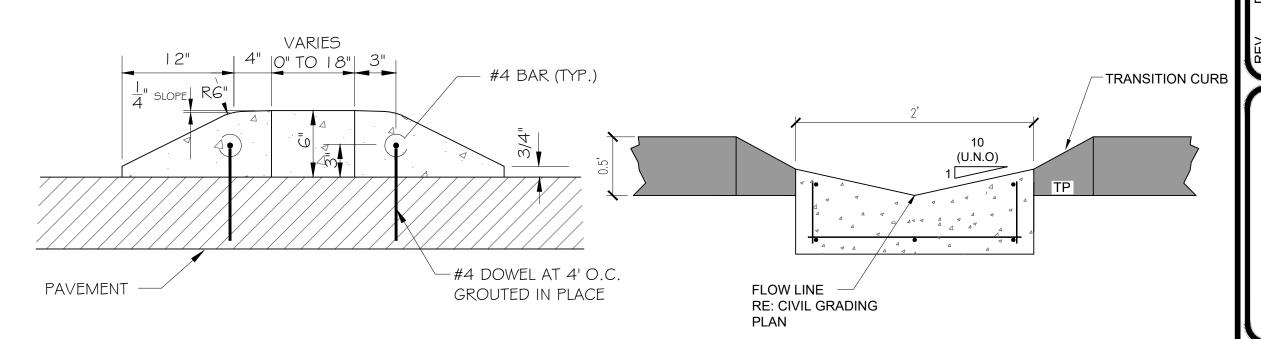
LANDSCAPE SIDEWALK

NOT TO SCALE



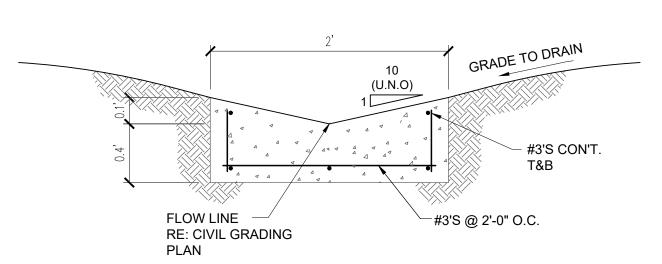
SEPARATE MOUNTABLE CURB & GUTTER

NOT TO SCALE



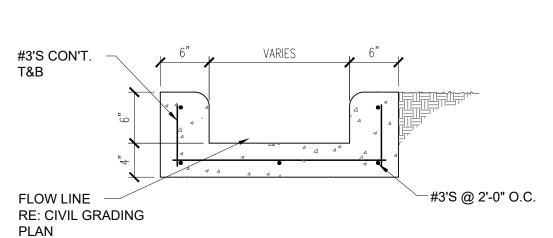
MOUNTABLE CURB RAISED ISLAND

NOT TO SCALE



V-NOTCH CONCRETE SWALE IN GRASS AREA

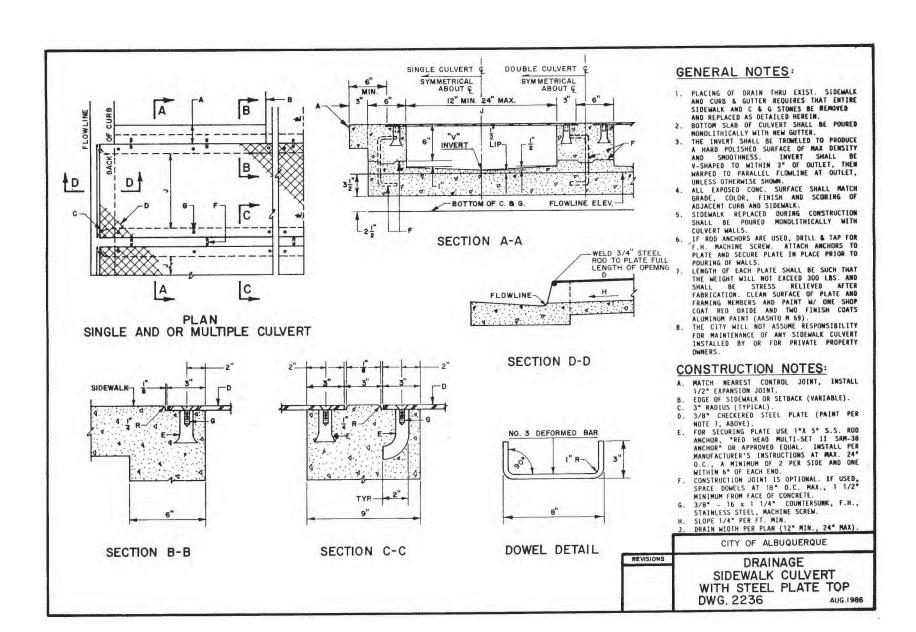
NOT TO SCALE



V-NOTCH CONCRETE SWALE & TRANSITION CURB

NOT TO SCALE

NOT TO SCALE



THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW, AGENCY APPROVAL, AND COMMENT UNDER THE AUTHORITY OF G. ROBERT ADAMS, P.E. REGISTRATION No. 15142, ON 10/24/18 THIS DOCUMENT IS NOT TO BE USED FOR CONSTRUCTION PURPOSES

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DESIGNED	MAR 2018	МНА				
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STANDARD DETAILS

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