CITY OF ALBUQUERQUE

Planning Department
Alan Varela, Director

Mayor Timothy M. Keller

April 18, 2023

Ronald Bohannan, P.E.
Tierra West, LLC
5571 Midway Park Place NE
Albuquerque, NM 87109

RE: Titan - WFH
Conceptual Grading & Drainage Plan
Engineer's Stamp Date: 04/18/23
Los Pastores Master Drainage Plan Amendment Report
Engineer's Stamp Date: 04/01/23
Hydrology File: F19D013D

Dear Mr. Bohannan:

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

Based upon the information provided in your submittal received 04/10/2023, the Conceptual Grading & Drainage Plan is preliminary approved for action by the Development Facilitation Team (DFT) on Site Plan for Building Permit.

PRIOR TO BUILDING PERMIT:

1. Please submit a more detailed Grading & Drainage Plan to Hydrology for review and approval. This digital (.pdf) is emailed to PLNDRS@cabq.gov along with the Drainage Transportation Information Sheet.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Dough Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

Project Title: Titan WFH **Building Permit #**_____ **Hydrology File #**_____
DRB#_____ **EPC#**_____
Legal Description: TR A-1-E-1 / TRACT A-1- City Address OR Parcel 101906148807440507
101906148904940508

Applicant/Agent: Tierra West LLC **Contact:** VINCE CARRICA
Address: 5571 Midway Park Place NE Albuquerque, NM 87109 **Phone:** (505) 858-3100
Email: VCARRICA@TIERRAWESTLLC.COM

Applicant/Owner: Mauney Investments, LLC **Contact:** _____
Address: PO BOX 90453 Albuquerque, NM 87109 **Phone:** _____
Email: _____

TYPE OF DEVELOPMENT: X PLAT (#of lots) 2-1 RESIDENCE __DRB SITE ___ ADMIN SITE: X
RE-SUBMITTAL: _____YES _X_ NO

DEPARTMENT: _____TRANSPORTATION _X_ HYDROLOGY/DRAINAGE
Check all that apply:

TYPE OF SUBMITTAL:

___ENGINEER/ARCHITECT CERTIFICATION
___PAD CERTIFICATION
___CONCEPTUAL G&D PLAN
___GRADING PLAN
___DRAINAGE REPORT
X DRAINAGE MASTER PLAN Amendment
___FLOOD PLAN DEVELOPMENT PERMIT APP.
___ELEVATION CERTIFICATE
___CLOMR/LOMR
___TRAFFIC CIRCULATION LAYOUT (TCL) ADMINISTRATIVE
___TRAFFIC CIRCULATION LAYOUT FOR DRB APPROVAL
___TRAFFIC IMPACT STUDY (TIS)
___STREET LIGHT LAYOUT
___OTHER (SPECIFY)
___PRE-DESIGN MEETING?

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

___BUILDING PERMIT APPROVAL
___CERTIFICATE OF OCCUPANCY
___CONCEPTUAL TCL DRB APPROVAL
___PRELIMINARY PLAT APPROVAL
___SITE PLAN FOR SUB'D APPROVAL
X SITE PLAN FOR BLDG PERMIT APPROVAL
___FINAL PLAT APPROVAL
___SIA/RELEASE OF FINANCIAL GUARANTEE
___FOUNDATION PERMIT APPROVAL
___GRADING PERMIT APPROVAL
___SO-19 APPROVAL
___PAVING PERMIT APPROVAL
___GRADING PAD CERTIFICATION
___WORK ORDER APPROVAL
___CLOMR/LOMR
___FLOOD PLAN DEVELOPMENT PERMIT
___OTHER (SPECIFY)_____

DATE SUBMITTED: 04.10.2023

DRAINAGE MATER PLAN ADDENDUM

LOS PASOTRES SHOPPING CENTER

City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
APPROVED
DATE: 04/18/23
BY: Renée C. Brissette
HydroTrans # F19D013D

THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY OF ALBUQUERQUE FROM REQUIRING CORRECTION, OR ERROR OR DIMENSIONS IN PLANS, SPECIFICATIONS, OR CONSTRUCTIONS. SUCH APPROVED PLANS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.

APRIL 01, 2023

Prepared by

Tierra West, LLC

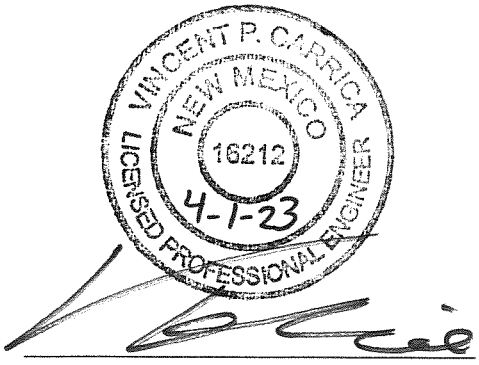
5571 Midway Park Place NE

Albuquerque, New Mexico 87109

Prepared for

Titan Development

Albuquerque, NM



Vincent P. Carrica, PE

No 16212

DRAINAGE MATER PLAN ADDENDUM

LOS PASOTRES SHOPPING CENTER

APRIL 01, 2023

Purpose

The purpose of this Drainage Master Plan addendum is to revise the existing surface detention drainage pond to an underground detention facility. The existing surface drainage pond is located in the southwest corner of Tract A-1-E-1. It accepts flow from Tracts A-1-B, A-1-C (Proposed Grocery), A-1-D-1 (Starbucks), Tracts A-1-A and A-1-E-1 (Proposed apartment complex) as well as from portions of Tracts 1 (Bank) and A-1-F (McDonald's). An outfall to the pond currently drains to the alley way west of the pond. The maximum flow rate for the pond for the design storm is currently 18.59 cfs.

Under the proposed modification to place the ponding underground, all current historic runoff from the above noted adjacent tracts along with the developed flows from the portions of the adjacent McDonald's and Bank that currently discharge to the pond will continue to be routed to the underground facility. In addition, the pond will accept developed flows from Tracts A-1-A and A-1-E-1, which are proposed to be developed as multifamily housing. The proposed underground pond will continue to discharge to the alleyway at the same general location as the existing pond outfall. The underground storage will be designed to allow for a discharge to the surface of the adjacent alley way. The maximum flow rate that will discharge from the underground pond will be 17.18 cfs for the design storm. This value is less than the currently approved maximum flow rate of 18.59 cfs, resulting in almost an 8% decrease in runoff routed down the alley way south to Montgomery Boulevard.

Hydraulic Analysis

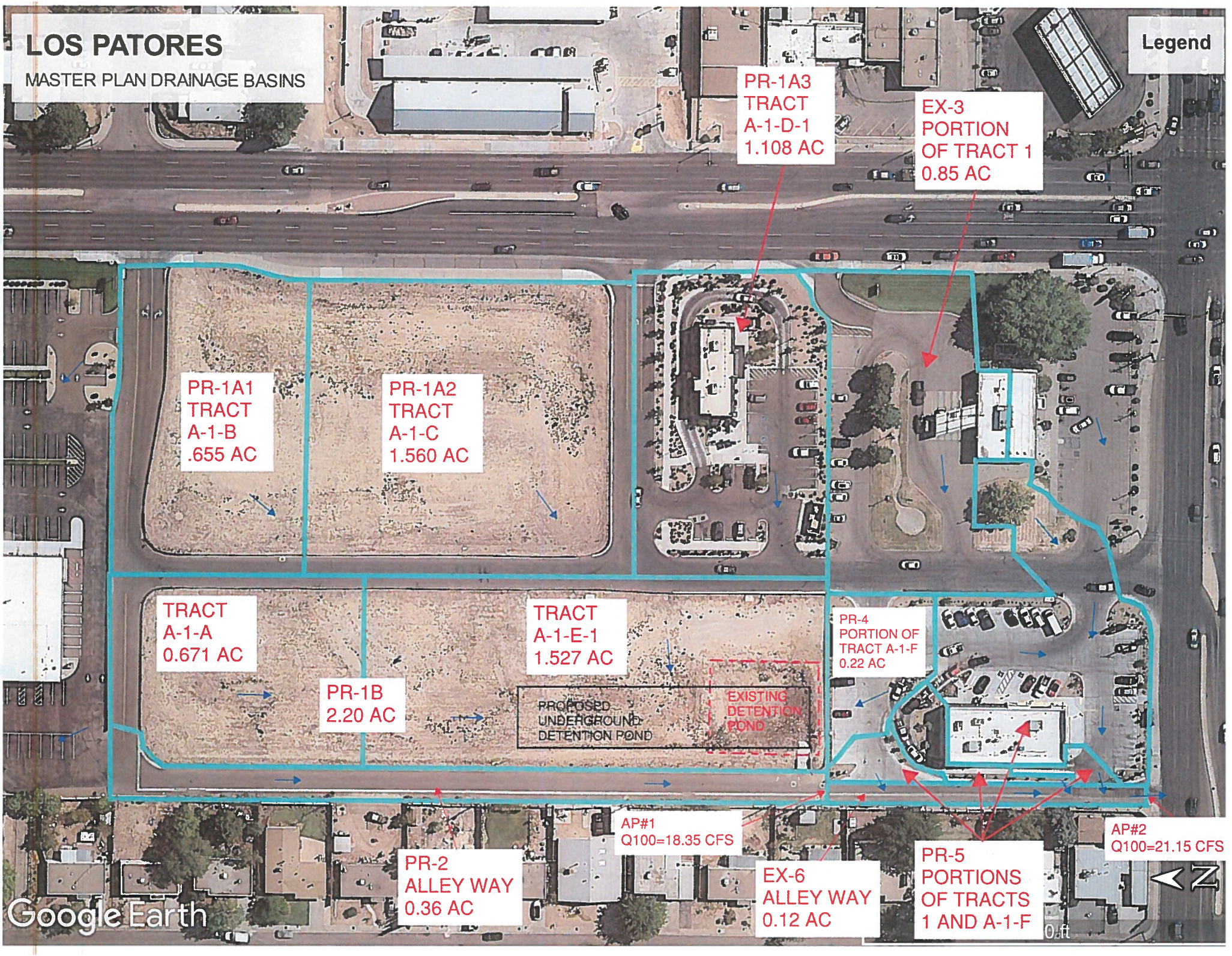
The attached Los Pastores Master Plan Drainage Basins exhibit shows the applicable drainage basins. The attached AHYMO analysis input and output files confirm the underground ponding facility proposed is sufficient to handle the runoff from the master plan area under the parameters outlined by the approved master plan where upland tracts are allowed to discharge historic flows (as of the time the master plan was approved). The underground pond will continue to retain first flush volumes from impervious areas in place at the time of the initial master plan approval and will also retain the first flush volume form the proposed multifamily development of Tracts A-1-A and A-1-E-1.

Conclusion

The Los Pastores Master Drain Plan will continue as currently approved with the exception of placing the storm drain detention pond underground. The underground pond will have sufficient capacity to handle the 100-year, 24-hour design storm and will discharge at a maximum rate of less than what is currently approved.

Vincent P. Carrica, PE

No 16212


LOS PATORES
MASTER PLAN DRAINAGE BASINS
Legend
PR-1A3
TRACT
A-1-D-1
1.108 AC
EX-3
PORTION
OF TRACT 1
0.85 AC
PR-1A1
TRACT
A-1-B
.655 AC
PR-1A2
TRACT
A-1-C
1.560 AC
TRACT
A-1-A
0.671 AC
TRACT
A-1-E-1
1.527 AC
PR-1B
2.20 AC
PROPOSED
UNDERGROUND
DETENTION POND
EXISTING
DETENTION
POND
PR-4
PORTION OF
TRACT A-1-F
0.22 AC
AP#1
Q100=18.35 CFS
AP#2
Q100=21.15 CFS
PR-2
ALLEY WAY
0.36 AC
EX-6
ALLEY WAY
0.12 AC
PR-5
PORTIONS
OF TRACTS
1 AND A-1-F
Google Earth
0 ft

Los Pastores Master Drainage Plan

Weighted E Method

Zone:
Zone 3
Developed Basins

Basin	Basin Area			Treatments								100-Year		
	Area (sf)	Area (acres)	Area (sq miles)	Treatment A		Treatment B		Treatment C		Treatment D		Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
				%	(acres)	%	(acres)	%	(acres)	%	(acres)			
PR-1A1	28,532.0	0.66	0.00102	0%	0.00	0%	0.00	85%	0.56	15%	0.10	1.314	0.072	2.21
PR-1A2	67,954.0	1.56	0.00244	0%	0.00	0%	0.00	85%	1.33	15%	0.23	1.314	0.171	5.25
PR-1A3	48,265.0	1.11	0.00173	0%	0.00	0%	0.00	85%	0.94	15%	0.17	1.314	0.121	3.73
PR-1B	95,745.0	2.20	0.00343	0%	0.00	15%	0.33	0%	0.00	85%	1.87	2.322	0.425	9.21
PR-2	15,682.0	0.36	0.00056	0%	0.00	0%	0.00	19%	0.07	81%	0.29	2.297	0.069	1.53
PR-4	9,583.0	0.22	0.00034	0%	0.00	0%	0.00	20%	0.04	80%	0.18	2.282	0.042	0.93
PR-5	37,897.0	0.87	0.00136	0%	0.00	15%	0.00	22%	0.19	78%	0.68	2.252	0.163	3.65
EX-3	37,026.0	0.85	0.00133	0%	0.00	0%	0.00	26%	0.22	74%	0.63	2.193	0.155	3.52
EX-6	5,227.0	0.12	0.00019	0%	0.00	0%	0.00	5%	0.01	95%	0.11	2.506	0.025	0.53
Total	**345,911.0**	**7.941**	**0.01241**		**0.00**		**0.330**		**3.355**		**4.256**		**1.243**	**30.57**

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)
Volume = Weighted D * Total Area
Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

Project:

Chamber Model -	SC-740	
Units -	Imperial	
	250	
Number of Chambers -	220	
Voids in the stone (porosity) -	40	%
Base of Stone Elevation -	0.50	ft
Amount of Stone Above Chambers -	6	in
Amount of Stone Below Chambers -	6	in
Area of system -	7839	sf Min. Area - 7437 sf min. area

StormTech SC-740 Cumulative Storage Volumes

Height of System *(inches)*	Incremental Single Chamber *(cubic feet)*	Incremental Total Chamber *(cubic feet)*	Incremental Stone *(cubic feet)*	Incremental Ch & St *(cubic feet)*	Cumulative Chamber *(cubic feet)*	Elevation *(feet)*
42	0.00	0.00	261.30	261.30	17040.11	4.00
41	0.00	0.00	261.30	261.30	16778.81	3.92
40	0.00	0.00	261.30	261.30	16517.51	3.83
39	0.00	0.00	261.30	261.30	16256.21	3.75
38	0.00	0.00	261.30	261.30	15994.91	3.67
37	0.00	0.00	261.30	261.30	15733.61	3.58
36	0.05	12.10	256.46	268.56	15472.31	3.50
35	0.16	35.84	246.96	282.81	15203.75	3.42
34	0.28	62.03	236.49	298.52	14920.95	3.33
33	0.60	132.87	208.15	341.02	14622.43	3.25
32	0.80	176.38	190.75	367.13	14281.41	3.17
31	0.95	209.15	177.64	386.79	13914.28	3.08
30	1.07	236.39	166.74	403.14	13527.49	3.00
29	1.18	259.71	157.42	417.12	13124.36	2.92
28	1.27	278.45	149.92	428.37	12707.23	2.83
27	1.36	298.10	142.06	440.16	12278.86	2.75
26	1.45	319.90	133.34	453.24	11838.70	2.67
25	1.52	335.44	127.12	462.56	11385.46	2.58
24	1.58	348.11	122.06	470.17	10922.90	2.50
23	1.64	361.30	116.78	478.08	10452.73	2.42
22	1.70	373.89	111.74	485.64	9974.65	2.33
21	1.75	385.64	107.04	492.69	9489.01	2.25
20	1.80	396.62	102.65	499.27	8996.33	2.17
19	1.85	408.10	98.06	506.16	8497.06	2.08
18	1.89	416.48	94.71	511.19	7990.90	2.00
17	1.93	425.48	91.11	516.59	7479.71	1.92
16	1.97	434.50	87.50	522.00	6963.12	1.83
15	2.01	442.18	84.43	526.61	6441.12	1.75
14	2.04	449.90	81.34	531.24	5914.51	1.67
13	2.07	456.49	78.70	535.19	5383.27	1.58
12	2.10	463.08	76.07	539.15	4848.08	1.50
11	2.13	469.00	73.70	542.70	4308.93	1.42
10	2.15	473.85	71.76	545.61	3766.23	1.33
9	2.18	478.95	69.72	548.67	3220.62	1.25
8	2.20	483.64	67.84	551.48	2671.95	1.17
7	2.21	485.61	67.06	552.67	2120.47	1.08
6	0.00	0.00	261.30	261.30	1567.80	1.00
5	0.00	0.00	261.30	261.30	1306.50	0.92
4	0.00	0.00	261.30	261.30	1045.20	0.83
3	0.00	0.00	261.30	261.30	783.90	0.75
2	0.00	0.00	261.30	261.30	522.60	0.67
1	0.00	0.00	261.30	261.30	261.30	0.58

INPUT

```
**********************************************************************
*              WHF @ Wyoming& Mont, ABQ,NM                           *
**********************************************************************
*      100-YEAR, 24-HR STORM  (UNDER PROPOSED CONDITIONS)            *
*   W/ McD'S, STARBUCKS, BANK AND FUTURE SHOPPING CENTER SITE        *
**********************************************************************
*
START                TIME=0.0
*
*
RAINFALL             TYPE=2 RAIN QUARTER=0.0 IN
                     RAIN ONE=1.84 IN RAIN SIX=2.43 IN
                     RAIN DAY=2.84 IN DT=0.05 HR
*DEVELOPED CONDITIONS
*
*BASIN PR-1A1 (TRACT A-1-B)
*
COMPUTE NM HYD       ID=1 HYD NO=100.1 AREA=0.00102 SQ MI
                     PER A=0.00 PER B=0.0 PER C=85.0 PER D=15.0
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD            ID=1 CODE=1
*
*
*BASIN PR-1A2 (TRACT A-1-C)
*
COMPUTE NM HYD       ID=2 HYD NO=100.2 AREA=0.00244 SQ MI
                     PER A=0.00 PER B=0.0 PER C=85.0 PER D=15.0
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD            ID=2 CODE=1
*
*
*BASIN PR-1A3 (TRACT A-1-D-1)
*
COMPUTE NM HYD       ID=3 HYD NO=100.3 AREA=0.00173 SQ MI
                     PER A=0.00 PER B=0.0 PER C=85.0 PER D=15.0
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD            ID=3 CODE=1
*
*
*BASIN PR-1B (TRACTS A-1-A & A-1-E-1 WFH DEVELOPED)
*
COMPUTE NM HYD       ID=4 HYD NO=100.4 AREA=0.00343 SQ MI
                     PER A=0.00 PER B=15.0 PER C=0.0 PER D=85.0
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD            ID=4 CODE=1
*
*
*BASIN PR-2 (NORTH PORTION OF ALLEY WAY)
*
COMPUTE NM HYD       ID=5 HYD NO=100.5 AREA=0.00056 SQ MI
```

```
                    PER A=0.00 PER B=0.0 PER C=19.0 PER D=81.00
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD           ID=5 CODE=1
*
*
*BASIN PR-4 (PORTION OF TRACT A-1-F  McDONALDS)
*
COMPUTE NM HYD      ID=6 HYD NO=100.6 AREA=0.00034 SQ MI
                    PER A=0.00 PER B=0.0 PER C=20.0 PER D=80.0
                    TP=-0.1333 HR MASS RAINFALL=-1*BASIN PR-1A1 (TRACT A-1-B)
PRINT HYD           ID=6 CODE=1
*
*
*BASIN PR-5 (PORTION OF TRACT A-1-F McDONALDS)
COMPUTE NM HYD      ID=7 HYD NO=100.7 AREA=0.00136 SQ MI
                    PER A=0.00 PER B=0.0 PER C=22.0 PER D=78.0
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD           ID=7 CODE=1
*
*
*BASIN EX-3 (PORTION OF TRACT 1 BANK)
*
COMPUTE NM HYD      ID=8 HYD NO=100.8 AREA=0.00133 SQ MI
                    PER A=0.00 PER B=0.00 PER C=26.0 PER D=74.00
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD           ID=8 CODE=1
*
*
*BASIN EX-6 (SOUTH PORTION OF ALLEY)
*
COMPUTE NM HYD      ID=9 HYD NO=100.9 AREA=0.00019 SQ MI
                    PER A=0.00 PER B=0.00 PER C=5.0 PER D=95.0
                    TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD           ID=9 CODE=1
**
*
*COMBINE PR-1A1, PR-1A2, PR-1A3, PR-1B, PR-4, AND EX-3
*
ADD HYD             ID=50 HYD NO=100.21 ID=1 ID=2
ADD HYD             ID=50 HYD NO=100.21 ID=50 ID=3
ADD HYD             ID=50 HYD NO=100.21 ID=50 ID=4
ADD HYD             ID=50 HYD NO=100.21 ID=50 ID=6
ADD HYD             ID=50 HYD NO=100.21 ID=50 ID=8
*
PRINT HYD           ID=50 CODE=1
**
*ROUTE BASINS PR-1A1, PR-1A2, PR-1A3, PR-1B, PR-4, AND EX-3 THROUGH PROPOSED
UNDERGROUND DETENTION POND
ROUTE RESERVOIR     ID=55 HYD NO=200.1 INFLOW ID=50 CODE=24
                    OUTFLOW (CFS) STORAGE(AC-FT) ELEVATION(FT)
```

```
                    0.0000          0.0000          5418.15
                    0.1000          0.0060          5418.73
                    0.2000          0.0360          5419.15
                    0.3000          0.1479          5419.90
                    13.270          0.2178          5420.40
                    16.750          0.2819          5420.90
                    19.320          0.3357          5421.40
                    25.570          0.3552          5421.65
                    25.570          0.3732          5421.90
                    25.570          0.3912          5422.15
*
*
PRINT HYD           ID=55  CODE=1
*
*
*COMBINE POND OUTFLOW WITH PR-2 FOR TOTAL AT AP#1
*
ADD HYD             ID=58 HYD NO=100.22 ID=5 ID=55ADD HYD
*
PRINT HYD           ID=58 CODE=1
*
*COMBINE ALLEY FLOWS AP#1 WITH EX-6 & PR-5 FOR TOTAL AT AP#2
*
ADD HYD             ID=59 HYD NO=100.23 ID=9 ID=58
ADD HYD             ID=59 HYD NO=100.23 ID=7 ID=58
*
PRINT HYD           ID=59 CODE=1
*
FINISH
```

OUTPUT

```
      AHYMO PROGRAM (AHYMO-S4)                    - Version: S4.01a - Rel: 01a
           RUN DATE (MON/DAY/YR) = 04/09/2023
           START TIME (HR:MIN:SEC) = 12:54:25     USER NO.=
AHYMO_Temp_User:20122010
           INPUT FILE = \2022\2022030 Titan WFH\Drainage\Revised Master
Plan\master plan addendum-in.txt

    ************************************************************
    *             WHF @ Wyoming& Mont, ABQ,NM                  *
    ************************************************************
    *      100-YEAR, 24-HR STORM  (UNDER PROPOSED CONDITIONS)  *
    *   W/ McD'S, STARBUCKS, BANK AND FUTURE SHOPPING CENTER SITE  *
    ************************************************************
    *
    START               TIME=0.0
    *
    *
    RAINFALL            TYPE=2 RAIN QUARTER=0.0 IN
                        RAIN ONE=1.84 IN RAIN SIX=2.43 IN
                        RAIN DAY=2.84 IN DT=0.05 HR

                  24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE
AREAS (NM & AZ) - D1
                  DT =   0.050000 HOURS       END TIME =    24.000002 HOURS
                    0.0000  0.0039  0.0080  0.0123  0.0170  0.0218  0.0273
                    0.0350  0.0473  0.0606  0.0742  0.0891  0.1041  0.1196
                    0.1355  0.1517  0.1696  0.1883  0.2083  0.2328  0.2594
                    0.2950  0.3354  0.3853  0.4519  0.5268  0.6561  0.8571
                    1.2016  1.4438  1.6347  1.7307  1.8148  1.8752  1.9233
                    1.9652  1.9959  2.0241  2.0473  2.0677  2.0866  2.1036
                    2.1196  2.1339  2.1474  2.1606  2.1733  2.1838  2.1896
                    2.1954  2.2010  2.2063  2.2116  2.2167  2.2217  2.2267
                    2.2314  2.2360  2.2406  2.2451  2.2495  2.2537  2.2579
                    2.2620  2.2661  2.2701  2.2739  2.2777  2.2815  2.2852
                    2.2888  2.2924  2.2960  2.2995  2.3030  2.3064  2.3098
                    2.3132  2.3165  2.3198  2.3230  2.3262  2.3294  2.3325
                    2.3356  2.3387  2.3417  2.3448  2.3477  2.3507  2.3536
                    2.3565  2.3593  2.3622  2.3650  2.3678  2.3705  2.3733
                    2.3760  2.3786  2.3813  2.3839  2.3865  2.3891  2.3917
                    2.3942  2.3967  2.3993  2.4017  2.4042  2.4066  2.4090
                    2.4115  2.4138  2.4162  2.4185  2.4209  2.4232  2.4255
                    2.4277  2.4300  2.4323  2.4345  2.4367  2.4390  2.4412
                    2.4434  2.4457  2.4479  2.4501  2.4523  2.4545  2.4567
                    2.4589  2.4610  2.4632  2.4654  2.4675  2.4697  2.4718
                    2.4740  2.4761  2.4782  2.4804  2.4825  2.4846  2.4867
                    2.4888  2.4909  2.4930  2.4951  2.4971  2.4992  2.5013
                    2.5033  2.5054  2.5074  2.5094  2.5115  2.5135  2.5155
                    2.5175  2.5195  2.5215  2.5235  2.5255  2.5275  2.5295
                    2.5314  2.5334  2.5354  2.5373  2.5393  2.5412  2.5431
```

2.5451 2.5470 2.5489 2.5508 2.5527 2.5546 2.5565
2.5584 2.5602 2.5621 2.5640 2.5658 2.5677 2.5695
2.5714 2.5732 2.5750 2.5769 2.5787 2.5805 2.5823
2.5841 2.5859 2.5877 2.5894 2.5912 2.5930 2.5947
2.5965 2.5982 2.6000 2.6017 2.6034 2.6052 2.6069
2.6086 2.6103 2.6120 2.6137 2.6153 2.6170 2.6187
2.6204 2.6220 2.6237 2.6253 2.6270 2.6286 2.6302
2.6318 2.6335 2.6351 2.6367 2.6383 2.6399 2.6414
2.6430 2.6446 2.6462 2.6477 2.6493 2.6508 2.6524
2.6539 2.6554 2.6569 2.6585 2.6600 2.6615 2.6630
2.6645 2.6659 2.6674 2.6689 2.6704 2.6718 2.6733
2.6747 2.6762 2.6776 2.6790 2.6804 2.6819 2.6833
2.6847 2.6861 2.6875 2.6888 2.6902 2.6916 2.6930
2.6943 2.6957 2.6970 2.6984 2.6997 2.7010 2.7023
2.7037 2.7050 2.7063 2.7076 2.7088 2.7101 2.7114
2.7127 2.7139 2.7152 2.7165 2.7177 2.7189 2.7202
2.7214 2.7226 2.7238 2.7250 2.7262 2.7274 2.7286
2.7298 2.7310 2.7322 2.7333 2.7345 2.7356 2.7368
2.7379 2.7391 2.7402 2.7413 2.7424 2.7435 2.7446
2.7457 2.7468 2.7479 2.7490 2.7500 2.7511 2.7522
2.7532 2.7542 2.7553 2.7563 2.7573 2.7584 2.7594
2.7604 2.7614 2.7624 2.7634 2.7643 2.7653 2.7663
2.7673 2.7682 2.7692 2.7701 2.7710 2.7720 2.7729
2.7738 2.7747 2.7756 2.7765 2.7774 2.7783 2.7792
2.7801 2.7809 2.7818 2.7827 2.7835 2.7843 2.7852
2.7860 2.7868 2.7877 2.7885 2.7893 2.7901 2.7909
2.7916 2.7924 2.7932 2.7940 2.7947 2.7955 2.7962
2.7970 2.7977 2.7984 2.7992 2.7999 2.8006 2.8013
2.8020 2.8027 2.8034 2.8040 2.8047 2.8054 2.8060
2.8067 2.8073 2.8080 2.8086 2.8092 2.8099 2.8105
2.8111 2.8117 2.8123 2.8129 2.8135 2.8140 2.8146
2.8152 2.8157 2.8163 2.8168 2.8174 2.8179 2.8184
2.8189 2.8195 2.8200 2.8205 2.8210 2.8214 2.8219
2.8224 2.8229 2.8233 2.8238 2.8242 2.8247 2.8251
2.8256 2.8260 2.8264 2.8268 2.8272 2.8276 2.8280
2.8284 2.8288 2.8291 2.8295 2.8299 2.8302 2.8306
2.8309 2.8313 2.8316 2.8319 2.8322 2.8325 2.8328
2.8331 2.8334 2.8337 2.8340 2.8343 2.8345 2.8348
2.8351 2.8353 2.8355 2.8358 2.8360 2.8362 2.8364
2.8367 2.8369 2.8371 2.8372 2.8374 2.8376 2.8378
2.8379 2.8381 2.8383 2.8384 2.8385 2.8387 2.8388
2.8389 2.8390 2.8391 2.8392 2.8393 2.8394 2.8395
2.8396 2.8397 2.8397 2.8398 2.8398 2.8399 2.8399
2.8399 2.8400 2.8400 2.8400 2.8400

*DEVELOPED CONDITIONS
*
*BASIN PR-1A1 (TRACT A-1-B)
*
COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=0.00102 SQ MI

```
                    PER A=0.00 PER B=0.0 PER C=85.0 PER D=15.0
                    TP=-0.1333 HR MASS RAINFALL=-1

        K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
        UNIT PEAK =  0.60405    CFS   UNIT VOLUME =   0.9832       B =   526.28
 P60 = 1.8400
        AREA =     0.000153 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


        K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
        UNIT PEAK =   2.5310    CFS   UNIT VOLUME =   0.9970       B =   389.14
 P60 = 1.8400
        AREA =     0.000867 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=1 CODE=1

                                    PARTIAL HYDROGRAPH   100.10


       RUNOFF VOLUME =     1.31258 INCHES      =       0.0714 ACRE-FEET
       PEAK DISCHARGE RATE =       2.17 CFS  AT   1.500 HOURS   BASIN AREA =
0.0010 SQ. MI.



    *
    *
    *BASIN PR-1A2 (TRACT A-1-C)
    *
    COMPUTE NM HYD      ID=2 HYD NO=100.2 AREA=0.00244 SQ MI
                        PER A=0.00 PER B=0.0 PER C=85.0 PER D=15.0
                        TP=-0.1333 HR MASS RAINFALL=-1

        K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
        UNIT PEAK =   1.4450    CFS   UNIT VOLUME =   0.9911       B =   526.28
 P60 = 1.8400
        AREA =     0.000366 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000
```

```
         K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
         UNIT PEAK =   6.0545    CFS   UNIT VOLUME =   0.9996       B =   389.14
 P60 = 1.8400
         AREA =     0.002074 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=2 CODE=1

                                       PARTIAL HYDROGRAPH   100.20


        RUNOFF VOLUME =     1.31258 INCHES      =       0.1708 ACRE-FEET
        PEAK DISCHARGE RATE =       5.17 CFS  AT   1.500 HOURS   BASIN AREA =
0.0024 SQ. MI.



    *
    *
    *BASIN PR-1A3 (TRACT A-1-D-1)
    *
    COMPUTE NM HYD      ID=3 HYD NO=100.3 AREA=0.00173 SQ MI
                        PER A=0.00 PER B=0.0 PER C=85.0 PER D=15.0
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   1.0245    CFS   UNIT VOLUME =   0.9891       B =   526.28
 P60 = 1.8400
         AREA =     0.000260 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
         UNIT PEAK =   4.2928    CFS   UNIT VOLUME =   0.9988       B =   389.14
 P60 = 1.8400
         AREA =     0.001471 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=3 CODE=1
```

```
                                        PARTIAL HYDROGRAPH   100.30

        RUNOFF VOLUME =     1.31258 INCHES     =       0.1211 ACRE-FEET
        PEAK DISCHARGE RATE =       3.67 CFS  AT   1.500 HOURS   BASIN AREA =
0.0017 SQ. MI.



    *
    *
    *BASIN PR-1B (TRACTS A-1-A & A-1-E-1 WFH DEVELOPED)
    *
    COMPUTE NM HYD      ID=4 HYD NO=100.4 AREA=0.00343 SQ MI
                        PER A=0.00 PER B=15.0 PER C=0.0 PER D=85.0
                        TP=-0.1333 HR MASS RAINFALL=-1

        K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
        UNIT PEAK =   11.511    CFS   UNIT VOLUME =   0.9981       B =   526.28
 P60 = 1.8400
        AREA =     0.002916 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


        K =  0.131520HR    TP =  0.133300HR    K/TP RATIO =  0.986645     SHAPE
CONSTANT, N =  3.578611
        UNIT PEAK =   1.2584    CFS   UNIT VOLUME =   0.9911       B =   326.03
 P60 = 1.8400
        AREA =     0.000515 SQ MI    IA =   0.50000 INCHES    INF =   1.25000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=4 CODE=1

                                        PARTIAL HYDROGRAPH   100.40


        RUNOFF VOLUME =     2.32642 INCHES     =       0.4256 ACRE-FEET
        PEAK DISCHARGE RATE =       9.04 CFS  AT   1.500 HOURS   BASIN AREA =
0.0034 SQ. MI.



    *
    *
    *BASIN PR-2 (NORTH PORTION OF ALLEY WAY)
```

```
    *
    COMPUTE NM HYD      ID=5 HYD NO=100.5 AREA=0.00056 SQ MI
                        PER A=0.00 PER B=0.0 PER C=19.0 PER D=81.00
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   1.7908    CFS   UNIT VOLUME =   0.9928       B =   526.28
 P60 = 1.8400
         AREA =     0.000454 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
         UNIT PEAK =  0.31061    CFS   UNIT VOLUME =   0.9651       B =   389.14
 P60 = 1.8400
         AREA =     0.000106 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=5 CODE=1

                                      PARTIAL HYDROGRAPH   100.50


        RUNOFF VOLUME =     2.30143 INCHES     =       0.0687 ACRE-FEET
        PEAK DISCHARGE RATE =       1.51 CFS  AT   1.500 HOURS   BASIN AREA =
0.0006 SQ. MI.



    *
    *
    *BASIN PR-4 (PORTION OF TRACT A-1-F  McDONALDS)
    *
    COMPUTE NM HYD      ID=6 HYD NO=100.6 AREA=0.00034 SQ MI
                        PER A=0.00 PER B=0.0 PER C=20.0 PER D=80.0
                        TP=-0.1333 HR MASS RAINFALL=-1*BASIN PR-1A1 (TRACT A-1-B)

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   1.0739    CFS   UNIT VOLUME =   0.9891       B =   526.28
 P60 = 1.8400
         AREA =     0.000272 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
```

```
0.050000

        K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
        UNIT PEAK =  0.19851    CFS   UNIT VOLUME =   0.9430       B =   389.14
 P60 = 1.8400
        AREA =     0.000068 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=6 CODE=1

                                      PARTIAL HYDROGRAPH   100.60


       RUNOFF VOLUME =     2.28645 INCHES     =       0.0415 ACRE-FEET
       PEAK DISCHARGE RATE =       0.92 CFS  AT   1.500 HOURS   BASIN AREA =
0.0003 SQ. MI.



    *
    *
    *BASIN PR-5 (PORTION OF TRACT A-1-F McDONALDS)
    COMPUTE NM HYD      ID=7 HYD NO=100.7 AREA=0.00136 SQ MI
                        PER A=0.00 PER B=0.0 PER C=22.0 PER D=78.0
                        TP=-0.1333 HR MASS RAINFALL=-1

        K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
        UNIT PEAK =   4.1881    CFS   UNIT VOLUME =   0.9966       B =   526.28
 P60 = 1.8400
        AREA =     0.001061 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


        K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
        UNIT PEAK =  0.87344    CFS   UNIT VOLUME =   0.9875       B =   389.14
 P60 = 1.8400
        AREA =     0.000299 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=7 CODE=1
```

```
                                    PARTIAL HYDROGRAPH   100.70

        RUNOFF VOLUME =     2.25648 INCHES      =       0.1637 ACRE-FEET
        PEAK DISCHARGE RATE =       3.60 CFS  AT   1.500 HOURS   BASIN AREA =
0.0014 SQ. MI.



    *
    *
    *BASIN EX-3 (PORTION OF TRACT 1 BANK)
    *
    COMPUTE NM HYD      ID=8 HYD NO=100.8 AREA=0.00133 SQ MI
                        PER A=0.00 PER B=0.00 PER C=26.0 PER D=74.00
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   3.8857    CFS   UNIT VOLUME =   0.9966       B =   526.28
 P60 = 1.8400
         AREA =     0.000984 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
         UNIT PEAK =   1.0095    CFS   UNIT VOLUME =   0.9896       B =   389.14
 P60 = 1.8400
         AREA =     0.000346 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=8 CODE=1

                                    PARTIAL HYDROGRAPH   100.80


        RUNOFF VOLUME =     2.19655 INCHES      =       0.1558 ACRE-FEET
        PEAK DISCHARGE RATE =       3.48 CFS  AT   1.500 HOURS   BASIN AREA =
0.0013 SQ. MI.



    *
    *
```

```
    *BASIN EX-6 (SOUTH PORTION OF ALLEY)
    *
    COMPUTE NM HYD      ID=9 HYD NO=100.9 AREA=0.00019 SQ MI
                        PER A=0.00 PER B=0.00 PER C=5.0 PER D=95.0
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =  0.71262    CFS   UNIT VOLUME =   0.9832       B =   526.28
 P60 = 1.8400
         AREA =     0.000181 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.105528HR    TP =  0.133300HR    K/TP RATIO =  0.791661     SHAPE
CONSTANT, N =  4.530856
         UNIT PEAK =  0.27733E-01CFS   UNIT VOLUME =   0.8912       B =   389.14
 P60 = 1.8400
         AREA =     0.000010 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=9 CODE=1

                                      PARTIAL HYDROGRAPH   100.90


        RUNOFF VOLUME =     2.51119 INCHES      =       0.0254 ACRE-FEET
        PEAK DISCHARGE RATE =       0.54 CFS  AT   1.500 HOURS   BASIN AREA =
0.0002 SQ. MI.



    **
    *
    *COMBINE PR-1A1, PR-1A2, PR-1A3, PR-1B, PR-4, AND EX-3
    *
    ADD HYD                         ID=50 HYD NO=100.21 ID=1 ID=2

    ADD HYD                         ID=50 HYD NO=100.21 ID=50 ID=3

    ADD HYD                         ID=50 HYD NO=100.21 ID=50 ID=4

    ADD HYD                         ID=50 HYD NO=100.21 ID=50 ID=6

    ADD HYD                         ID=50 HYD NO=100.21 ID=50 ID=8
```

```
*
PRINT HYD           ID=50 CODE=1

                                    PARTIAL HYDROGRAPH   100.21


      RUNOFF VOLUME =     1.79614 INCHES     =       0.9857 ACRE-FEET
      PEAK DISCHARGE RATE =      24.44 CFS  AT   1.500 HOURS   BASIN AREA =
0.0103 SQ. MI.



**
*ROUTE BASINS PR-1A1, PR-1A2, PR-1A3, PR-1B, PR-4, AND EX-3 THROUGH PROPOSED UND
ROUTE RESERVOIR     ID=55 HYD NO=200.1 INFLOW ID=50 CODE=24
                        OUTFLOW (CFS) STORAGE(AC-FT) ELEVATION(FT)
                              0.0000         0.0000            5418.15

                              0.1000         0.0060            5418.73

                              0.2000         0.0360            5419.15

                              0.3000         0.1479            5419.90

                              13.270         0.2178            5420.40

                              16.750         0.2819            5420.90

                              19.320         0.3357            5421.40

                              25.570         0.3552            5421.65

                              25.570         0.3732            5421.90

                              25.570         0.3912            5422.15


  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *  *

    TIME      INFLOW    ELEV      VOLUME    OUTFLOW
    (HRS)     (CFS)     (FEET)    (AC-FT)   (CFS)

     0.00       0.00   5418.15      0.000      0.00
     1.20       2.46   5419.19      0.042      0.20
     2.40       1.04   5419.94      0.153      1.24
     3.60       0.13   5419.85      0.140      0.29
     4.80       0.13   5419.74      0.124      0.28
     6.00       0.14   5419.65      0.110      0.27
     7.20       0.14   5419.57      0.098      0.26
     8.40       0.13   5419.49      0.087      0.25
```

9.60	0.12	5419.41	0.075	0.23
10.80	0.11	5419.33	0.063	0.22
12.00	0.10	5419.26	0.052	0.21
13.20	0.09	5419.18	0.040	0.20
14.40	0.08	5419.06	0.030	0.18
15.60	0.07	5418.94	0.021	0.15
16.80	0.06	5418.84	0.014	0.13
18.00	0.05	5418.75	0.008	0.11
19.20	0.04	5418.50	0.004	0.06
20.40	0.03	5418.38	0.002	0.04
21.60	0.02	5418.31	0.002	0.03
22.80	0.01	5418.24	0.001	0.02
24.00	0.00	5418.18	0.000	0.01
25.20	0.00	5418.16	0.000	0.00

PEAK DISCHARGE = 17.175 CFS - PEAK OCCURS AT HOUR 1.60
MAXIMUM WATER SURFACE ELEVATION = 5420.982
MAXIMUM STORAGE = 0.2908 AC-FT INCREMENTAL TIME= 0.050000HRS

*
*
PRINT HYD ID=55 CODE=1

PARTIAL HYDROGRAPH 200.10

RUNOFF VOLUME = 1.79614 INCHES = 0.9857 ACRE-FEET
PEAK DISCHARGE RATE = 17.18 CFS AT 1.600 HOURS BASIN AREA = 0.0103 SQ. MI.

*
*
*COMBINE POND OUTFLOW WITH PR-2 FOR TOTAL AT AP#1
*
ADD HYD ID=58 HYD NO=100.22 ID=5 ID=55ADD HYD

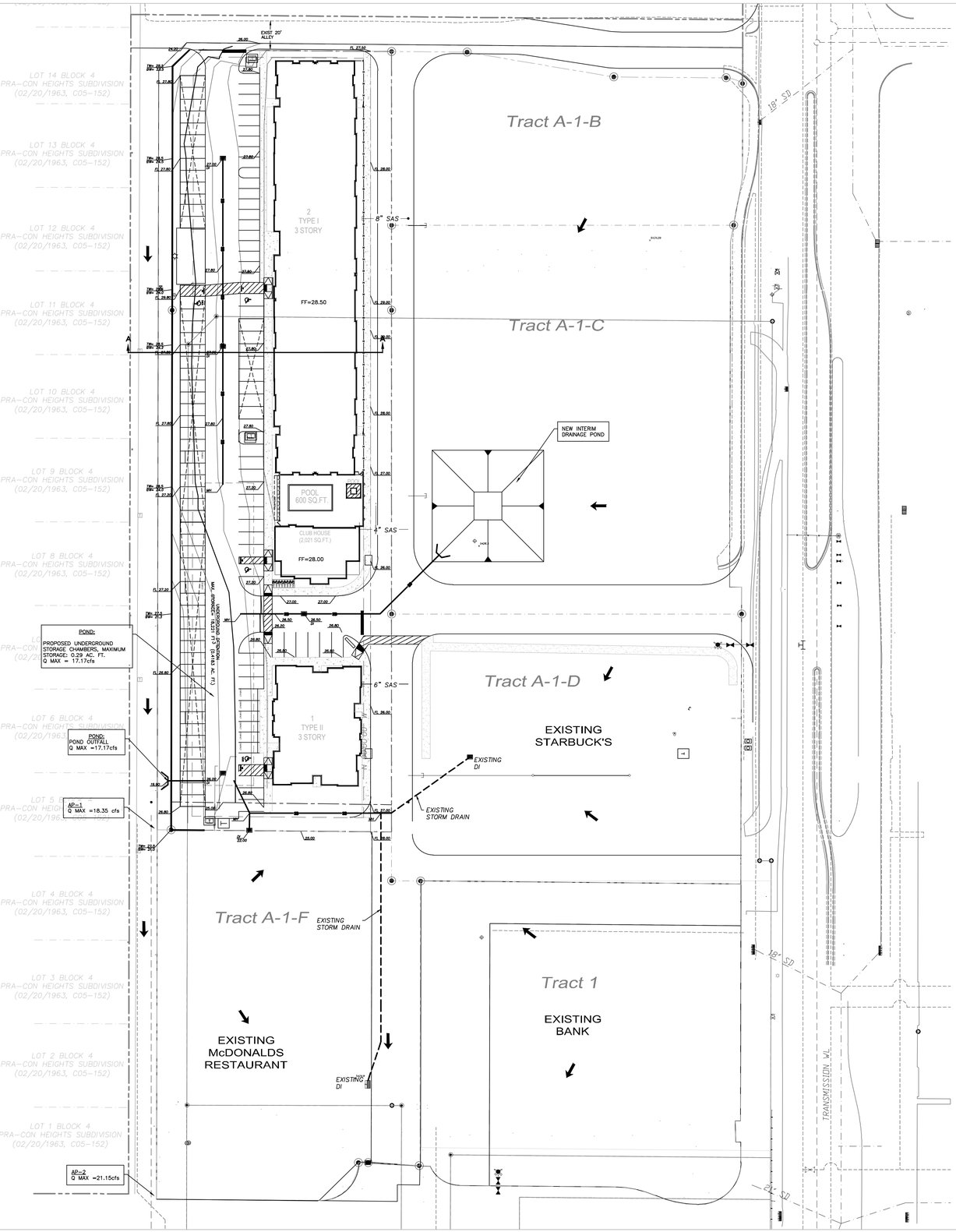
*
PRINT HYD ID=58 CODE=1

PARTIAL HYDROGRAPH 100.22

RUNOFF VOLUME = 1.82204 INCHES = 1.0543 ACRE-FEET
AP#1 PEAK DISCHARGE RATE = 18.35 CFS AT 1.600 HOURS BASIN AREA = 0.0109 SQ. MI.

*

```
*COMBINE ALLEY FLOWS AP#1 WITH EX-6 & PR-5 FOR TOTAL AT AP#2
*
ADD HYD                          ID=59 HYD NO=100.23 ID=9 ID=58

ADD HYD                          ID=59 HYD NO=100.23 ID=7 ID=58

*
PRINT HYD           ID=59 CODE=1

                                      PARTIAL HYDROGRAPH   100.23

     RUNOFF VOLUME =     1.87039 INCHES      =       1.2180 ACRE-FEET
AP#2 PEAK DISCHARGE RATE =      21.15 CFS  AT   1.600 HOURS   BASIN AREA =
 0.0122 SQ. MI.


*
FINISH

    NORMAL PROGRAM FINISH          END TIME (HR:MIN:SEC) = 12:54:25
```



Z:\2022\2022030 Titan WFH\dwg\Construction\2022030_Grading Plan.dwg Apr 18, 2023 - 3:52pm

LEGEND

- CURB & GUTTER
- BOUNDARY LINE
- EASEMENT
- SIDEWALK
- EXISTING CURB & GUTTER
- SINGLE CLEAN OUT
- DOUBLE CLEAN OUT
- EXISTING SD MANHOLE
- EXISTING SAS MANHOLE
- EXISTING FIRE HYDRANT
- EXISTING WATER METER
- EXISTING POWER POLE
- EXISTING GAS VALVE
- EXISTING OVERHEAD UTILITIES
- EXISTING GAS
- EX. 8" SAS — EXISTING SANITARY SEWER LINE
- EX. WL — EXISTING WATER LINE
- EX. RCP — EXISTING STORM SEWER LINE
- 4900 — EXISTING INDEX CONTOUR
- EXISTING CONTOUR
- T — TRANSFORMER

NOTICE TO CONTRACTORS

1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY.
2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HERON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE INTERIM STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, 1985.
3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES.
4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONNECTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE.
6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.
7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.

EROSION CONTROL NOTES:

1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL (CITY) ACCEPTANCE OF ANY PROJECT.
6. ALL SLOPES NOT STABILIZED AT THE END OF THE PROJECT SHALL BE STABILIZED IN ACCORDANCE WITH COA SPECS OR ¾" GRAVEL

N

GRAPHIC SCALE

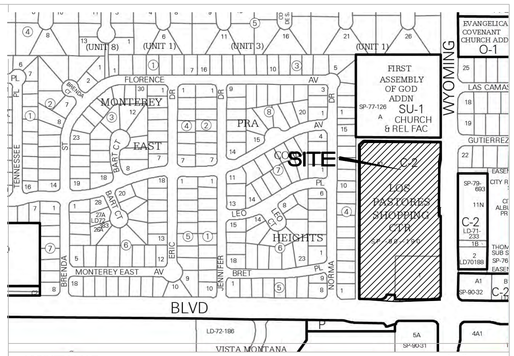
40 0 20 40

(IN FEET)
1 inch = 40 ft.

City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
PRELIMINARY APPROVED
DATE: 04/18/23
BY: Renée C. Brissette
HydroTrans # F19D013D
THESE PLANS AND/OR REPORT ARE CONCEPTUAL ONLY. MORE INFORMATION MAY BE NEEDED IN THEM AND SUBMITTED TO HYDROLOGY FOR BUILDING PERMIT APPROVAL.

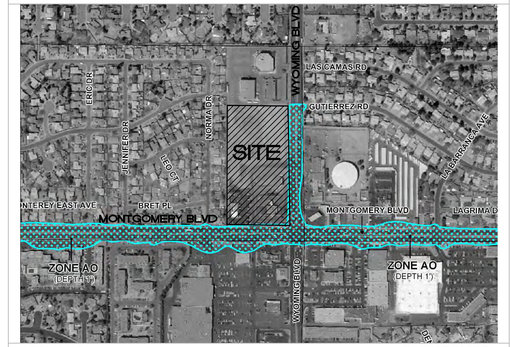
DRAINAGE CONCEPT

SITE WILL DRAIN TO A PROPOSED UNDERGROUND STORM DRAIN DETENTION POND UNDER SOUTH PARKING AREA ALONG WITH CONTROLLED RUNOFF FROM VACANT TRACTS A-1-B, A-1-C AND DEVELOPED TRACT A-1-D AND PORTIONS OF DEVELOPED TRACTS A-1-F AND TRACT 1. UNDERGROUND POND WILL DISCHARGE TO EXISTING ALLEY PAVED SURFACE AT OR BELOW HISTORIC FLOW RATE PER APPROVED LOS PASTORES MASTER DRAINAGE PLAN.

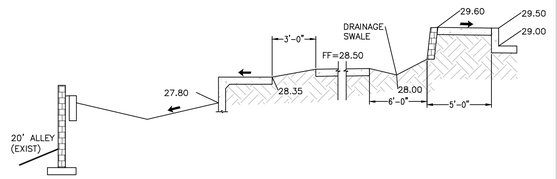


VICINITY MAP L-8

LEGAL DESCRIPTION:



FIRM MAP 35001C0143G



SECTION A-A
NTS

CAUTION

ALL EXISTING UTILITIES SHOWN WERE OBTAINED FROM RESEARCH, AS-BUILTS, SURVEYS OR INFORMATION PROVIDED BY OTHERS. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO AND INCLUDING ANY EXCAVATION, TO DETERMINE THE ACTUAL LOCATION OF UTILITIES AND OTHER IMPROVEMENTS, PRIOR TO STARTING THE WORK. ANY CHANGES FROM THIS PLAN SHALL BE COORDINATED WITH AND APPROVED BY THE ENGINEER.

DD SET: 04-18-23

ENGINEER'S SEAL: VINCENT P. CARRICA, NEW MEXICO, 16212, PROFESSIONAL ENGINEER; 04/18/2023	RENOVA AT WYOMING	DRAWN BY: RMG
	CONCEPTUAL GRADING PLAN	DATE: 04/18/2023
		2022030_GRADING PLAN
	TIERRA WEST, LLC 5571 MIDWAY PARK PLACE NE ALBUQUERQUE, NM 87109 (505) 858-3100 www.tierrawestllc.com	SHEET # GR-1
		JOB # 2022030