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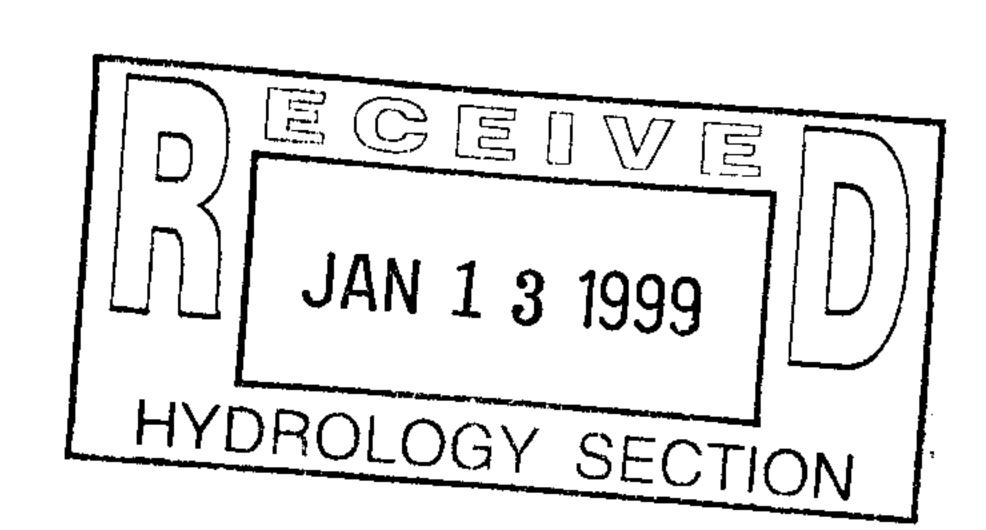
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r '<u>E</u>,l:

DRAINAGE INFORMATION

PROJECT TITLE: <u>Burger King - Wyoming</u>	ZONE ATLAS/DRNG. FILE #: F-19/D36
DRB#: 98-213 EPC #:	WORK ORDER #:
LEGAL DESCRIPTION: <u>Tract "B". Block 1. A</u>	American Heritage North Unit 3
CITY ADDRESS: <u>5315 Wyoming Blvd. NE</u>	
ENGINEERING FIRM: <u>Chavez-Grieves</u>	CONTACT: <u>Joseph DeFronzo</u>
ADDRESS: <u>5639 Jefferson NE</u>	PHONE: <u>344-4080</u>
OWNER: <u>M&M Bros. Inc.</u>	CONTACT: Tom McCollum
ADDRESS: <u>11000 Bermuda Dunes NE Alb. 871</u>	11 PHONE: 292-5744
ARCHITECT: Nims-Calvani	CONTACT: Myron Saldyt
ADDRESS: <u>525 San Pedro NE Ste. 110</u>	PHONE: 255-6400
SURVEYOR: <u>Harris Surveying</u>	CONTACT: <u>Anthony Harris</u>
ADDRESS: 2412 Monroe St. NE	PHONE: <u>889-8056</u>
CONTRACTOR: <u>Gerald Martin</u>	CONTACT: <u>Bob Cardenes</u>
ADDRESS: <u>8501 Jefferson NE</u>	PHONE: 828-1144
DRAINAGE REPORT DRAINAGE PLAN CONCEPTUAL GRADING & DRAINAGE PLAN GRADING PLAN EROSION CONTROL PLAN ENGINEER'S CERTIFICATION OTHER PRE-DESIGN MEETING: YESX_ NO	CHECK TYPE OF APPROVAL SOUGHT: SKETCH PLAT APPROVAL PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PRMT. APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL S.A.D. DRAINAGE REPORT DRAINAGE REQUIREMENTS
	OTHER
DATE SUBMITTED: <u>January 13.1999</u>	
BY: <u>Joseph DeFronzo</u>	





September 8,1998

Chris Ersham
Chavez-Grieves Engineering
5639 Jefferson NE
Albuquerque, New Mexico 87109

RE: REVISED DRAINAGE PLAN FOR BURGER KING @ WYOMING & SPAIN (F19-D36) REVISION DATED 8/5/98

Dear Ms. Ersham:

Based on the information provided on your August 6, 1998 resubmittal, the above referenced site is approved for Building Permit

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Also, a separate permit is required for construction within City R/W. A copy of this approval letter must be on hand when applying for the excavation permit.

If I can be of further assistance, please feel free to contact me at 924-3986.

C: Andrew Garcia
Arlene Portillo
File

Sincerely

Bernie J. Montoya CE
Associate Engineer





July 9,1998

Chris Ersham
Chavez-Grieves Engineering
5639 Jefferson NE
Albuquerque, New Mexico 87109

RE: REVISED DRAINAGE PLAN FOR BURGER KING @ WYOMING & SPAIN (F19-D36) REVISION DATED 6/1/98

Dear Ms. Ersham:

Based on the information provided on your June 15,1998 resubmittal, listed are some concerns that will need to be addressed prior to final approval:

- 1. Six notes from the DPM checklist for the SO19 format on the plan drawing.
- 2. Why are you ponding the run-off?
- 3. According to your plan drawing, you indicate that all the run-off will be discharged onto Heritage Court NE thru the proposed rundown. I am confused in that you identify ponding yet you have the rundown.
- 4. Your narrative indicates two eight inch pipes, where are they on the plan drawing?
- 5. I need the hydraulics for the proposed rundown, will it carry the developed run-off?
- 6. Please check your drivepads for the required one foot water block.

If I can be of further assistance, please fell free to contact me at 924-3986.

C: Andrew Garcia
File

Sincerely

Bernie J. Montoya CE
Associate Engineer



CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT

September 8, 1998

INTEROFFICE CORRESPONDENCE

HYDROLOGY DIVISION

14

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TO:

Desiderio Salas, Street Maintenance Division

FROM:

Bernie J. Montoya CE, Engineer, PWD

SUBJECT: PRIVATE DRAINAGE FACILITIES WITHIN PUBLIC RIGHT-OF-WAY

DRAINAGE FILE NUMBER (F19-D36).

Transmitted herewith, is a copy of the approved drainage plan for the referenced project incorporating the SO #19 design.

This plan is being submitted to you for permitting and inspection. Please provide this section with a signed-off copy per the signature block upon construction and acceptance by your office.

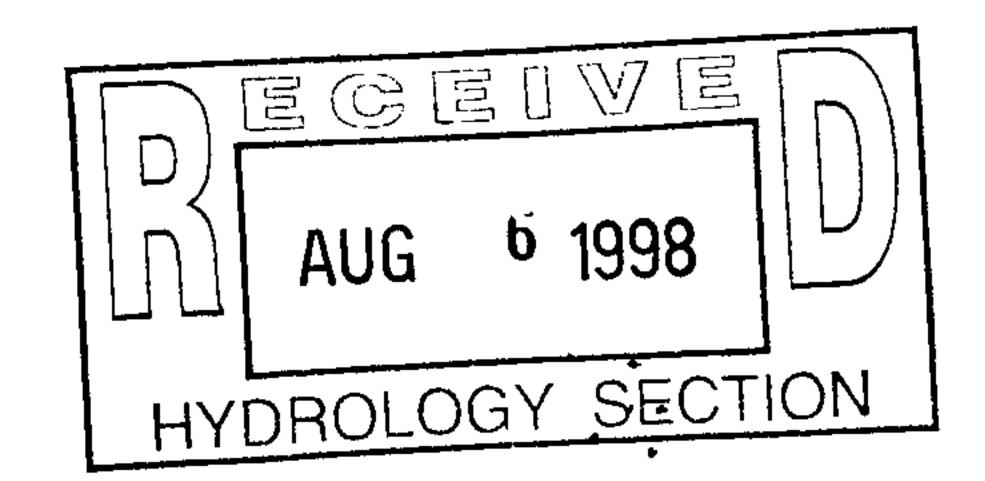
As you are aware, the signed off SO #19 is required by this office for Certificate of Occupancy release; therefore your expeditious processing of this plan would be greatly appreciated and would avoid any unnecessary delay in the release of the Certificate of Occupancy.

Thank you for your cooperation and if you should have any questions and/or comments, please feel free to call me at 924-3984.

Attachment

DUWTUWGE TULOKWWI.TON

PROJECT TITLE Burger King Wyoming	ZONE ATLAS/DRNG. FILE #: F-19-Z
DRB#: <u>98-213</u> EPC #:	
LEGAL DESCRIPTION: Tract "B", Block 1, Amo	
CITY ADDRESS: 5315 Wyoming Bud us	
ENGINEERING FIRM: Chavez-Grieves Inc.	CONTACT: Chris Ehrsom
ADDRESS: 5639 Jefferson St. N.E.	PHONE: 344-4080
OWNER: MALL Bros. Inc.	CONTACT: Mayor Son Tom Mc Gille
ADDRESS: 11000 Bermuda Dines N.E.	PHONE: 292.5744
ARCHITECT: Nims Calvani? Assoc. ALB. 87111	CONTACT: Myron Soldyt
ADDRESS: 525 San Pedro NE St. 110	PHONE: 255-6400
surveyor: Harris Surveying	CONTACT: KNTHONY HARRS
ADDRESS: 2412 Monroe St. NE	PHONE: 889-8056
CONTRACTOR: (FERALD WOWHIN	
ADDRESS: 8501 Jefferson NE.	PHONE: 828 - 1144
CHEC	K TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT	SKETCH PLAT APPROVAL
DRAINAGE PLAN	PRELIMINARY PLAT APPROVAL
CONCEPTUAL GRADING & DRAINAGE PLAN	S. DEV. PLAN FOR SUB'D. APPROVAL
GRADING PLAN	S. DEV. PLAN FOR BLDG. PRMT, APPROVAL
EROSION CONTROL PLAN	SECTOR PLAN APPROVAL
ENGINEER'S CERTIFICATION	FINAL PLAT APPROVAL
OTHER .	FOUNDATION PERMIT APPROVAL
	BUILDING PERMIT APPROVAL
PRE-DESIGN MEETING:	CERTIFICATE OF OCCUPANCY APPROVAL
YES	GRADING PERMIT APPROVAL
NO · .	PAVING PERMIT APPROVAL
COPY PROVIDED .	S.A.D. DRAINAGE REPORT
•	DRAINAGE REQUIREMENTS
	OTHER (SPECIFY)
DATE SUBMITTED:	



5639 JEFFERSON STREET NE • ALBUQUERQUE, NEW MEXICO 87109 • PHONE (505) 344-4080 • FAX (505) 343-8759

August 5, 1998

Mr. Bernie J. Montoya CE City of Albuquerque Hydrology P.O. Box 1293 Albuquerque, NM 87103

RE: DRAINAGE REPORT AND GRADING AND DRAINAGE PLAN BURGER KING @ WYOMING AND SPAIN (F19/P36) ALBUQUERQUE, NEW MEXICO

Dear Mr. Montoya:

Transmitted herewith for building permit approval, is the grading and drainage plan revised per your comments dated July 9, 1998. Your comments are addressed as follows:

- 1q. Six notes from the DPM checklist for the SO19 format on the plan drawing.
- 1a. The notes are on the plan sheet.
- 2q. Why are you ponding the run-off?
- 2a. This is what was presented on the Conceptual Grading Plan. The reason is so that the developed flows discharge into Heritage Court will be released at the same flowrate as the undeveloped through a sidewalk culvert.
- 3q. According to your plan drawing, you indicate that all the run-off will be discharged onto Heritage Court NE thru the proposed rundown. I am confused in that you identify ponding yet you have the rundown.
- 3a. We are channeling the runoff from the site to the concrete rundown then into the detention pond. From the detention pond, the run-off will be discharged to Heritage Court through a sidewalk culvert at the historical flow rate.
- 4q. Your narrative indicates two 8" pipes, which are they on the plan drawing?
- 4a. We have eliminated the pipes all together now.
- 5q. I need the hydraulics for the proposed rundown, will it carry the developed run-off?

- 5a. We have included the hydraulics for the rundown with this submittal. Yes it is capable of carrying the developed run-off.
- 6q. Please check your drive pads for the required one foot water block.
- 6a. We are matching the existing grades at the 15' access easement along the northern property lines. We will have a water block between the gas station to the south and our property.

If you have any questions or concerns, please call me.

Sincerely,

CHAVEZ-GRIEVES CONSULTING ENGINEERS, INC.

Christina A. Ehrsam, E.I.

Project Manager

CAE/cmte

Worksheet Worksheet for Rectangular Channel

Project Descript	ion
Project File	c:\haestad\fmw\burger k.fm2
Worksheet	BK-RUNDOWN
Flow Element	Rectangular Channel
Method	Hazen-Williams Formula
Solve For	Discharge

Input Data		
C Coefficient	140.0	
Channel Slope	0.05000	00 ft/ft
Depth	0.50	ft
Bottom Width	2.00	ft

Results		
Discharge	18.32	cfs
Flow Area	1.00	ft²
Wetted Perimeter	3.00	ft
Top Width	2.00	ft
Critical Depth	1.38	ft
Critical Slope	0.0040	24 ft/ft
Velocity	18.32	ft/s
Velocity Head	5.22	ft
Specific Energy	5.72	ft
Froude Number	0.00	

DRAINAGE INFORMATION SHEET

APPLICANT'S NAME: MYRO ~ SALOYT	ZONE ATLAS/DRNG. FILE #: F-/9 Z
DRB #: 98-213 EPC #: Z-97-42	WORK ORDER #:
LEGAL DESCRIPTION: LOT B, BLOCK D	MERICAN HERITAGE NORTH UNIT 3
CITY ADDRESS: 5315 WYDMING BUD	
ENGINEERING FIRM: CHAVEZ GRIEUES	CONTACT: CHAIL ERSHALL
ADDRESS: 5639 JEFFELSON N.E. ALB. M. 8710	PHONE: 344-4080
OWNER: TOM 1/2 COLLUM/BURGER KING	CONTACT: Ton Mc Colum
ADDRESS: 11000 BERMUDA DUNEZ. N.E	PHONE:
ARCHITECT: NIMS CALUANI & ASSOC.	CONTACT: MYRON SALDYT
ADDRESS: 525 SAN PEODO ME. SUITE 110	PHONE: 255-6400
SURVEYOR: TONY HARUS	CONTACT:
ADDRESS:	PHONE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
TYPE OF SUBMITTAL: X DRAINAGE REPORT DRAINAGE PLAN CONCEPTUAL GRADING & DRAINAGE PLAN GRADING PLAN EROSION CONTROL PLAN ENGINEER'S CERTIFICATION OTHER PRE-DESIGN MEETING: YES NO COPY PROVIDED	CHECK TYPE OF APPROVAL SOUGHT: SKETCH PLAT APPROVAL PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D APPROVAL S. DEV PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL S.A.D. DRAINAGE REPORT DRAINAGE REQUIREMENTS SUBDIVISION CERTIFICATION OTHER(SPECIFY)
DATE SUBMITTED: 6/16/78 BY: Myran Soldy Soldy	JUN 1 5 1998
Revised 02/98	HYDROLOGY SECTION



Martin J. Chávez, Mayor

Robert E. Gurulé, Director

April 3,1997

Joseph L. DeFranzo Chavez-Grieves 5639 Jefferson Street NE Albuquerque, New Mexico 87109

RE: DRAINAGE PLAN FOR A BURGER KING (E19-D36) ENGINEER'S STAMP DATED 3/24/97

Dear Mr. DeFranzo:

Based on the information provided on your March 26,1997, listed are some concerns that will need to be addressed prior to final approval:

- 1. Six notes and signature block from the SO19 format within the plan drawing. **no**
- 2. Close proximity of the proposed pond to the street pavement, please address.
- 3. Location and direction of the roof drains.
- 4. Identify the 100-year water surface elevation for the pond on the plan drawing.

If I can be of further assistance, please feel free to contact me at 924-3986.

C: Andrew Garcia File Sincerely

Bernie J. Montoya CE Engineering Associate

DRAINAGE INFORMATION SHEET

APPLICANT'S NAME: TOM HE COLLUM	ZONE ATLAS/DRNG. FILE #: F19 36
DRB #: EPC #:	WORK ORDER #:
LEGAL DESCRIPTION: LOT B BLOCK A	ERICAN HERITAGE NORTH UNIT 3
CITY ADDRESS:	
ENGINEERING FIRM:	CONTACT:
ADDRESS:	PHONE:
OWNER:	CONTACT:
ADDRESS:	PHONE:
ARCHITECT: NIAS CALUA~; & ASSUC	CONTACT: MYRON SALDYT
ADDRESS: 525 SAN DED RO NE SUITE 110	PHONE: 255-6400 FAX 268-6954
SURVEYOR:	CONTACT:
ADDRESS:	PHONE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
TYPE OF SUBMITTAL: DRAINAGE REPORT DRAINAGE PLAN CONCEPTUAL GRADING & DRAINAGE PLAN GRADING PLAN EROSION CONTROL PLAN ENGINEER'S CERTIFICATION OTHER PRE-DESIGN MEETING: YES NO COPY PROVIDED DIE GEWE JUN 0 2 1998 HYDROLOGY SECTION DATE SUBMITTED: O 2 98	CHECK TYPE OF APPROVAL SOUGHT: SKETCH PLAT APPROVAL PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL S.A.D. DRAINAGE REPORT DRAINAGE REQUIREMENTS SUBDIVISION CERTIFICATION OTHER (SPECIFY)
BY: Mys Soliky	

Revised 02/98

DRAINAGE INFORMATION

PROJECT TITLE: <u>Burger King</u>	ZONE ATLAS/DRNG. FILE #: F-19-Z		
DRB#: EPC #: <u>Z-9742</u>	WORK ORDER #:		
LEGAL DESCRIPTION: <u>Tract "B" in Block Numl</u>	bered One (1) of American Heritage North Unit 3		
CITY ADDRESS:			
ENGINEERING FIRM: <u>Chavez-Grieves</u>	CONTACT: <u>Jeanne Wolfenbarger</u>		
ADDRESS: <u>5639 Jefferson NE</u>	PHONE: <u>344-4080</u>		
OWNER:	CONTACT:		
ADDRESS:	PHONE :		
ARCHITECT:	CONTACT:		
ADDRESS:	PHONE :		
SURVEYOR:	CONTACT:		
ADDRESS:	PHONE :		
CONTRACTOR:	CONTACT:		
ADDRESS:	PHONE:		
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:		
X DRAINAGE REPORT	SKETCH PLAT APPROVAL		
DRAINAGE PLAN	PRELIMINARY PLAT APPROVAL		
X CONCEPTUAL GRADING & DRAINAGE PLAN	S. DEV. PLAN FOR SUB'D. APPROVAL		
GRADING PLAN	S. DEV. PLAN FOR BLDG. PRMT. APPROVAL		
EROSION CONTROL PLAN	SECTOR PLAN APPROVAL		
ENGINEER'S CERTIFICATION	FINAL PLAT APPROVAL		
OTHER	FOUNDATION PERMIT APPROVAL		
	X BUILDING PERMIT APPROVAL		
PRE-DESIGN MEETING:	CERTIFICATE OF OCCUPANCY APPROVAL		
YES	GRADING PERMIT APPROVAL		
<u>X</u> NO	PAVING PERMIT APPROVAL		
COPY PROVIDED	S.A.D. DRAINAGE REPORT		
	DRAINAGE REQUIREMENTS		
DATE SUBMITTED: March 25, 1997	OTHER		
BY: <u>Jeanne Wolfenbarger, E.I.T.</u>	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6		
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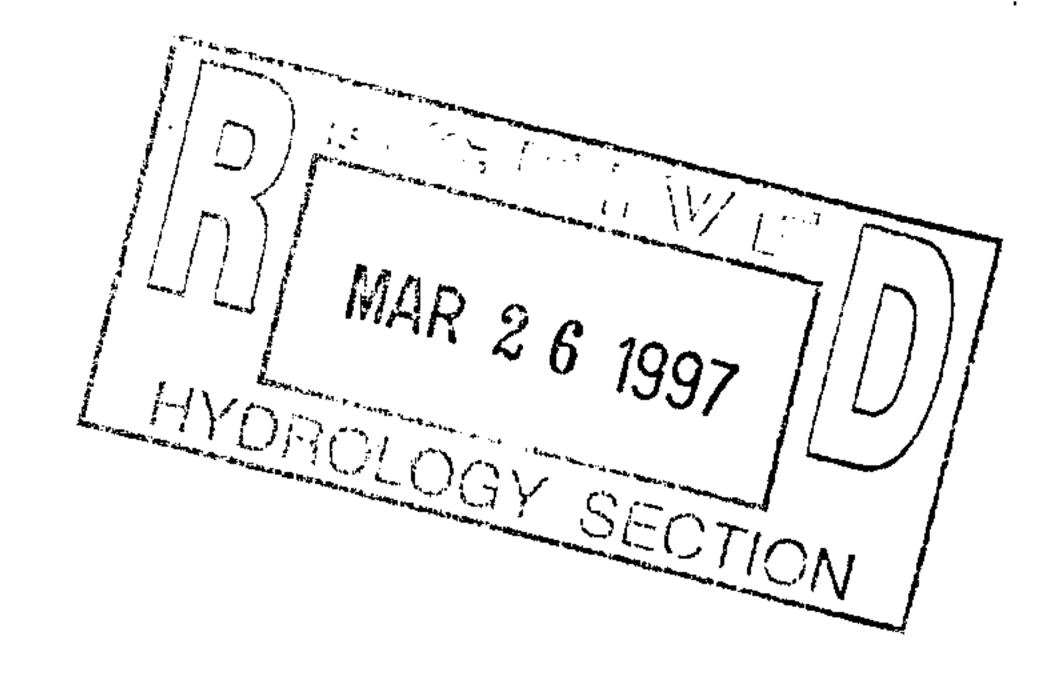
5639 JEFFERSON STREET NE · ALBUQUERQUE, NEW MEXICO 87109 · PHONE (505) 344·4080 · FAX (505) 343·8759

DRAINAGE REPORT

FOR

BURGERKING

ALBUQUERQUE, NEW MEXICO



MARCH, 1997

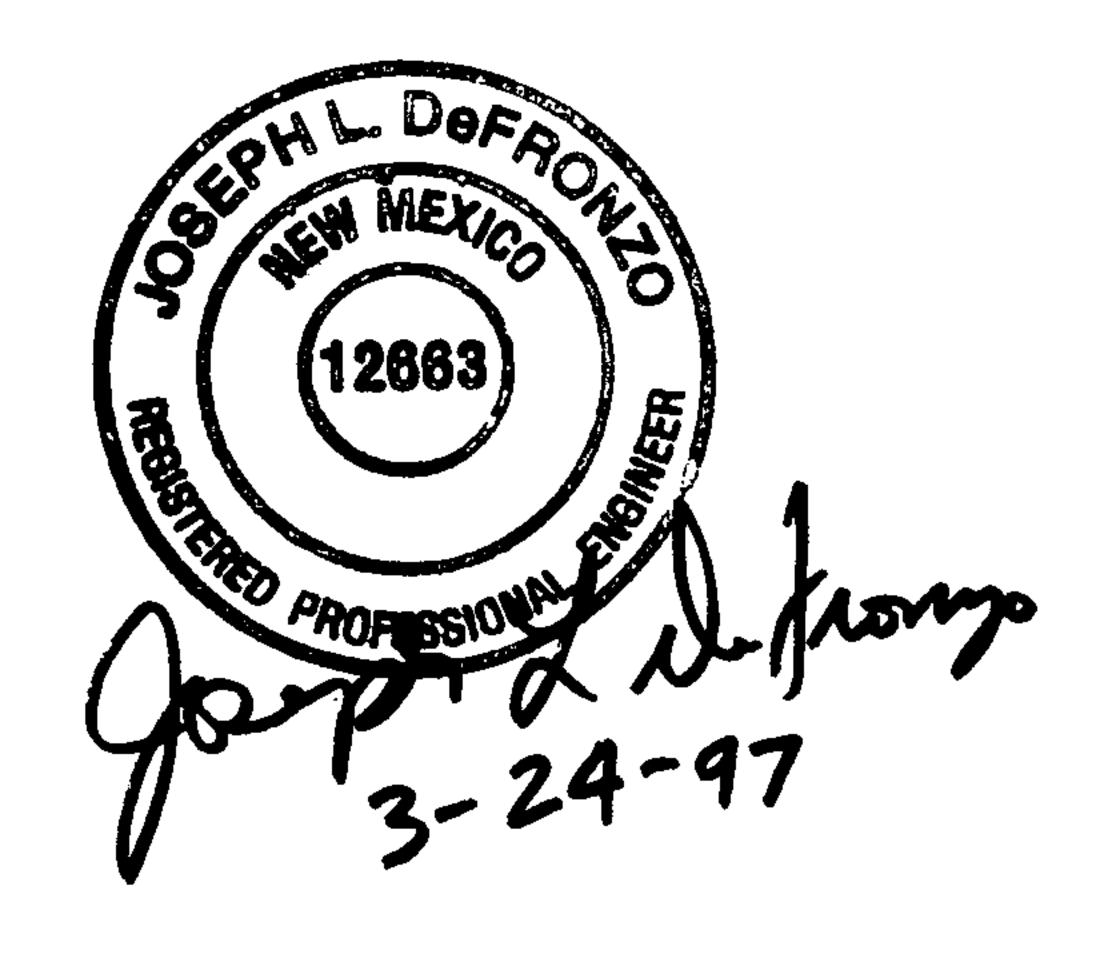
5639 JEFFERSON STREET NE · ALBUQUERQUE, NEW MEXICO 87109 · PHONE (505) 344·4080 · FAX (505) 343·8759

DRAINAGE REPORT

FOR

BURGERKING

ALBUQUERQUE, NEW MEXICO



MARCH, 1997

LOCATION

This site is located between Heritage Court and Wyoming Boulevard about a quarter of a mile north of Osuna Road.

FLOOD HAZARD ZONES

As shown by Panel 35001C0143 of the National Flood Insurance Rate Map for the City of Albuquerque, dated September 20, 1996, the site is not located within a flood plain.

EXISTING SITE CONDITIONS AND DRAINAGE PATTERN

The site currently has one building on-site with a surrounding parking lot, and a road that extends across the north side of the site from Wyoming Boulevard to Heritage Court. Surrounding development includes a gas station on the south side and an office building on the north side.

The site is not accepting any off-site runoff. Runoff generated from Wyoming Boulevard on the east side of the site is intercepted by curb and gutter and conveyed to Spain Road. Also, an existing water block on the on-site road prevents runoff from Wyoming Boulevard from entering the site.

All on-site runoff from on-site Basin A and Basin B discharges to Heritage Court by overland flow at 3-4% slopes. It is then conveyed to Spain Road by street flow. From that intersection, the runoff flows west on Spain Road and is collected by a series of catch basins on the south side of Spain Road that lead to the Bear Tributary Arroyo. As shown in the 1996 FIRM map, the Bear Tributary Arroyo has enough capacity to convey the 100-year runoff. A total existing on-site runoff of 2.97 cfs was computed (See page 4).

PROPOSED SITE CONDITIONS AND DRAINAGE PATTERN

A Burger King Restaurant with a surrounding parking lot is proposed to be built. For proposed conditions, runoff will be discharged to Heritage Court. In order to decrease the discharge to less than existing, a detention pond will be constructed on the west side of the site. The attenuated runoff will be released from the pond through two 6" pipes. These pipes will then discharge into a sidewalk culvert, releasing runoff onto Heritage Court. A total proposed runoff of 3.33 cfs (See page 4) was computed for Basin A, whereas a runoff of only 2.22 cfs will be discharged from the pond (See page 6).

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AHYMO PROGRAM (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
RUN DATE (MON/DAY/YR) = 03/12/1997
START TIME (HR:MIN:SEC) = 09:52:31
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*S********
            CHAVEZ-GRIEVES CONSULTING ENGINEERS, INC.
*S********
                     BURGER KING
  FILENAME: G:\N03\113\AHYMO.IN/OUT
*S****** 100 YEAR, 24 HOUR STORM (Section 22.2 Hydrology)
**********
*S* EXISTING RUNOFF CALCULATIONS *****
*************
**********
START
             0.00
RAINFALL
             TYPE=2 RAIN QUARTER=0.0 RAIN ONE=2.14
             RAIN SIX=2.60 RAIN DAY=3.10 DT=0.03333
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COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR. .033330 HOURS DT = END TIME = 19.964670 HOURS .0000 .0027 .0055 .0084 .0113 .0143 .0173 .0204 .0236 .0269 .0302 .0337 .0372 .0408 .0445 .0523 .0484 .0564 .0606 .0694 .0649 .0741 .0789 .0839 .0892 .0946 .1003 . 1063 .1126 .1192 .1262 .1322 . 1385 . 1597 .1452 . 1922 .2422 .3139 .4119 .5407 .7049 .9093 1.1588 1.3904 1.4871 1.5687 1.6414 1.7074 1.7683 1.8247 1.8775 1.9270 1.9735 2.0174 2.0589 2.0982 2.1354 2.1707 2.2041 2.2359 2.2661 2.2737 2.2875 2.2939 2.3001 2.3060 2.3117 2.3172 2.3226 2.3328 2.3376 2.3423 2.3470 2.3277 2.3514 2.3558 2.3601 2.3643 2.3683 2.3723 2.3762 2.3801 2.3875 2.3911 2.3947 2.3982 2.4016 2.4050 2.4083 2.4115 2.4147 2.4179 2.4210 2.4241 2.4271 2.4301 2.4330 2.4359 2.4388 2.4416 2.4444 2.4472 2.4499 2.4553 2.4579 2.4605 2.4526 2.4631 2.4656 2.4681 2.4706 2.4731 2.4755 2.4779 2.4803 2.4827 2.4850 2.4896 2.4919 2.4942 2.4964 2.4873 2.4986 2.5008 2.5030 2.5052 2.5073 2.5094 2.5115 2.5136 2.5157 2.5177 2.5198 2.5218 2.5238 2.5258 2.5277 2.5297 2.5317 2.5336 2.5355 2.5374 2.5393 2.5412 2.5430 2.5449 2.5467 2.5486 2.5504 2.5522 2.5540 2.5593 2.5610 2.5627 2.5645 2.5662 2.5679 2.5575 2.5696 2.5713 2.5729 2.5746 2.5762 2.5779 2.5795 2.5811 2.5828 2.5844 2.5860 2.5876 2.5891 2.5907 2.5923 2.5938 2.5954 2.5969 2.5984 2.6000 2.6014 2.6028 2.6042 2.6057 2.6071 2.6085 2.6099 2.6127 2.6141 2.6155 2.6169 2.6183 2.6197 2.6210 2.6224 2.6238 2.6252 2.6265 2.6279 2.6293 2.6306 2.6320 2.6333 2.6347 2.6360 2.6374 2.6387 2.6400 2.6414 2.6427 2.6440 2.6454 2.6467 2.6480 2.6493 2.6506 2.6519 2.6532 2.6546 2.6559 2.6572 2.6584 2.6597 2.6610 2.6623 2.6636 2.6649 2.6662 2.6674 2.6687 2.6700 2.6713 2.6725 2.6738 2.6751 2.6763 2.6776 2.6788 2.6801 2.6813 2.6826 2.6838 2.6850 2.6863 2.6875 2.6888 2.6900 2.6912 2.6924 2.6937 2.6949 2.6961 2.6973 2.6985 2.6997 2.7009 2.7022 2.7034 2.7046 2.7058 2.7069 2.7081 2.7093 2.7105 2.7117 2.7129 2.7141 2.7153 2.7164 2.7176 2.7188

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              2.8984 2.8993
      2.8975
                             2.9001
                                    2.9010 2.9019
2.9028
      2.9036
              2.9045
                     2.9054
                             2.9062
                                    2.9071
                                           2.9080
2.9088
      2.9097 2.9106
                     2.9114
                            2.9123 2.9131 2.9140
2.9148
      2.9157
              2.9165
                     2.9174
                             2.9182 2.9191 2.9199
2.9208 2.9216 2.9225 2.9233
                            2.9242 2.9250 2.9258
2.9267
       2.9275 2.9284 2.9292
                             2.9300
                                   2.9309 2.9317
2.9325
      2.9334 2.9342 2.9350
                             2.9358
                                    2.9367 2.9375
2.9383
      2.9391 2.9400 2.9408
                             2.9416 2.9424 2.9432
2.9441
       2.9449
              2.9457
                     2.9465
                             2.9473
                                    2.9481 2.9489
2.9497
       2.9506
             2.9514 2.9522
                             2.9530
                                   2.9538 2.9546
2.9554
      2.9562
              2.9570
                     2.9578
                             2.9586
                                   2.9594 2.9602
2.9610 2.9618 2.9626
                     2.9634
                             2.9641
                                   2.9649 2.9657
2.9665 2.9673 2.9681 2.9689
                            2.9697 2.9704 2.9712
2.9720 2.9728 2.9736 2.9743 2.9751 2.9759 2.9767
2.9775 2.9782 2.9790 2.9798 2.9805 2.9813 2.9821
2.9829 2.9836 2.9844 2.9852 2.9859 2.9867 2.9874
2.9882 2.9890 2.9897 2.9905
                            2.9912 2.9920 2.9928
2.9935 2.9943 2.9950 2.9958 2.9965 2.9973 2.9980
2.9988 2.9995 3.0003 3.0010 3.0018 3.0025 3.0033
3.0040 3.0048 3.0055 3.0062 3.0070 3.0077 3.0085
3.0092 3.0099 3.0107 3.0114 3.0121 3.0129 3.0136
3.0143 3.0151 3.0158 3.0165 3.0173
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*S COMPUTE EXISTING RUNOFF FROM ON-SITE BASIN A COMPUTE NM HYD ID=1 HYD=A DA=0.00114 SQ MI %A=30 %B=15 %C=15 %D=40 TP=0.1333 RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 1.8003 CFS UNIT VOLUME = .9933 B = 526.28 P60 = 2.1400 AREA = .000456 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

K = .139842HR TP = .133300HR K/TP RATIO = 1.049076 SHAPE CONSTANT, N = 3.365389 UNIT PEAK = 1.5926 CFS UNIT VOLUME = .9914 B = 310.37 P60 = 2.1400 AREA = .000684 SQ MI IA = .53750 INCHES INF = 1.35500 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

PRINT HYD ID=1 CODE=1

RUNOFF VOLUME = 1.61604 INCHES = .0983 ACRE-FEET
PEAK DISCHARGE RATE = 2.54 CFS AT 1.500 HOURS BASIN AREA = .0011 SQ. MI.

*S COMPUTE EXISTING RUNOFF FROM ON-SITE BASIN B
COMPUTE NM HYD
ID=2 HYD=BASIN_B DA=0.00013 SQ MI
%A=0 %B=0 %C=0 %D=100

TP=0.1333 RAINFALL=-1

K=.072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = .51325 CFS UNIT VOLUME = .9786 B = 526.28 P60 = 2.1400 AREA = .000130 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

PRINT HYD ID=2 CODE=1

HYDROGRAPH FROM AREA BASIN_B

RUNOFF VOLUME = 2.76864 INCHES = .0192 ACRE-FEET
PEAK DISCHARGE RATE = .43 CFS AT 1.500 HOURS BASIN AREA = .0001 SQ. MI.

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*S COMPUTE RUNOFF FROM PROPOSED ON-SITE BASIN A
COMPUTE NM HYD

ID=1 HYD=A DA=0.00114 SQ MI
%A=0 %B=20 %C=0 %D=80

TP=0.1333 RAINFALL=-1

K = .072649HR IP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 3.6006 CFS UNIT VOLUME = .9961 B = 526.28 P60 = 2.1400 AREA = .000912 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

K = .133656HR TP = .133300HR K/TP RATIO = 1.002670 SHAPE CONSTANT, N = 3.520804 UNIT PEAK = .55049 CFS UNIT VOLUME = .9754 B = 321.84 P60 = 2.1400 AREA = .000228 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

PRINT HYD ID=1 CODE=1

HYDROGRAPH FROM AREA A

RUNOFF VOLUME = 2.39889 INCHES = .1459 ACRE-FEET
PEAK DISCHARGE RATE = 3.33 CFS AT 1.500 HOURS BASIN AREA = .0011 SQ. MI.

*S COMPUTE RUNOFF FROM PROPOSED ON-SITE BASIN B
COMPUTE NM HYD

ID=2 HYD=BASIN_B DA=0.00013 SQ MI
%A=0 %B=0 %C=0 %D=100

TP=0.1333 RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = .51325 CFS UNIT VOLUME = .9786 B = 526.28 P60 = 2.1400 AREA = .000130 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .0333330

PRINT HYD ID=2 CODE=1

HYDROGRAPH FROM AREA BASIN_B

RUNOFF VOLUME = 2.76864 INCHES = .0192 ACRE-FEET
PEAK DISCHARGE RATE = .43 CFS AT 1.500 HOURS BASIN AREA = .0001 SQ. MI.

*S ROUTE BASIN A THROUGH THE DETENTION POND ROUTE RESERVOIR ID=3 HYD NO=BASIN_ROUTE

*

ID=3 HYD NO=BASIN_ROUTE INFLOW ID=1 CODE=10
OUTFLOW(CFS) STORAGE(AC-FT) ELEVATION(FT)

0 0

2.58 0.03440 1.30

* * * * * * * * * * * * * *

TIME (HRS) (CFS) (FEET) (AC-FT) (CFS) .00	* * * *	* * *	* * * *	* * *	* * *
(HRS) (CFS) (FEET) (AC-FT) (CFS) .00	TIME	INFLOW	ELFV	VOLUME	OUTFLOW
.00					
.33	•	, ,	((/(U / / /	(5, 5)
.67	.00	.00	.00	.000	.00
1.00 .04 .00 .000 .01 1.333 .95 .16 .004 .31 1.67 1.71 1.08 .028 2.14 2.00 .77 .57 .015 1.14 2.00 .77 .57 .015 1.14 2.33 .18 .21 .006 .42 2.67 .07 .07 .002 .14 3.00 .04 .03 .001 .06 3.33 .03 .02 .001 .04 3.67 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.67 .02 .01 .000 .03 4.67 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.50 .03 .01 <t.000< td=""> .02 5.6</t.000<>			.00	.000	.00
1.33 .95 .16 .004 .31 1.67 1.71 1.08 .028 2.14 2.00 .77 .57 .015 1.14 2.00 .77 .57 .015 1.14 2.00 .07 .07 .002 .14 2.67 .07 .07 .002 .14 3.00 .04 .03 .001 .06 3.33 .03 .02 .001 .04 3.67 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.33 .02 .01 .000 .03 4.67 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.33 .02 .01 .000 .02 5.67 .03 .01 .000 .02 5.67 .03 .01 .000 .02 7.			-00	.000	.00
1.67 1.71 1.08 .028 2.14 2.00 .77 .57 .015 1.14 2.33 .18 .21 .006 .42 2.67 .07 .07 .002 .14 3.00 .04 .03 .001 .06 3.33 .03 .02 .001 .04 3.67 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.00 .03 .01 .000 .03 4.67 .02 .01 .000 .03 4.67 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.67 .03 .01 .000 .02 5.67 .03 .01 .000 .02 5.67 .03 .01 .000 .02 7.00 .02 .01 .000 .02 7.6					.01
2.00 .77 .57 .015 1.14 2.33 .18 .21 .006 .42 2.67 .07 .07 .002 .14 3.00 .04 .03 .001 .06 3.33 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.00 .03 .01 .000 .03 4.00 .03 .01 .000 .03 4.33 .02 .01 .000 .03 4.67 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.33 .02 .01 .000 .02 5.67 .03 .01 .000 .02 5.67 .03 .01 .000 .03 6.833 .03 .01 .000 .02 7.33 .02 .01 .000 .02 7.67<					
2.33 .18 .21 .006 .42 2.67 .07 .07 .002 .14 3.00 .04 .03 .001 .06 3.33 .03 .02 .001 .04 3.67 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.00 .03 .01 .000 .03 4.67 .02 .01 .000 .03 4.67 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.67 .03 .01 .000 .02 5.67 .03 .01 <t>.000 .02 6.00 .03 .01 .000 .03 6.67 .02 .01 .000 .02 7.00 .02 .01 .000 .02 8.00<td></td><td></td><td></td><td></td><td></td></t>					
2.67 .07 .07 .002 .14 3.00 .04 .03 .001 .06 3.33 .03 .02 .001 .04 3.67 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.33 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.33 .02 .01 .000 .02 5.67 .03 .01 .000 .02 5.67 .03 .01 .000 .02 6.600 .03 .01 .000 .03 6.67 .02 .01 .000 .02 7.00 .02 .01 .000 .02 7.67 .02 .01 .000 .02 8.00 .02 .01 .000 .02 9.33 </td <td></td> <td></td> <td></td> <td></td> <td></td>					
3.00 .04 .03 .001 .06 3.33 .03 .02 .001 .04 3.67 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.33 .02 .01 .000 .03 4.67 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.33 .02 .01 .000 .02 6.00 .03 .01 .000 .02 6.00 .03 .01 .000 .02 6.67 .02 .01 .000 .02 7.00 .02 .01 .000 .02 7.67 .02 .01 .000 .02 8.00 .02 .01 .000 .02 8.67 .02 .01 .000 .02 9.33 <td></td> <td></td> <td></td> <td></td> <td></td>					
3.33 .03 .02 .001 .04 3.67 .03 .02 .000 .03 4.00 .03 .01 .000 .03 4.33 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.33 .02 .01 .000 .02 6.00 .03 .01 .000 .02 6.00 .03 .01 .000 .03 6.33 .03 .01 .000 .03 6.67 .02 .01 .000 .02 7.00 .02 .01 .000 .02 7.67 .02 .01 .000 .02 7.67 .02 .01 .000 .02 8.33 .02 .01 .000 .02 8.67 .02 .01 .000 .02 9.33 .02 .01 .000 .02 9.67 .02 .01<				_	
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4.00 .03 .01 .000 .03 4.33 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.00 .02 .01 .000 .02 5.33 .02 .01 .000 .02 6.00 .03 .01 .000 .03 6.33 .03 .01 .000 .03 6.67 .02 .01 .000 .02 7.00 .02 .01 .000 .02 7.33 .02 .01 .000 .02 7.67 .02 .01 .000 .02 7.67 .02 .01 .000 .02 8.00 .02 .01 .000 .02 8.67 .02 .01 .000 .02 8.67 .02 .01 .000 .02 9.33 .02 .01 .000 .02 9.67 .02 .01 .000 .02 10.00 .02 .01					
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PEAK DISCHARGE = 2.218 CFS - PEAK OCCURS AT HOUR 1.60
MAXIMUM WATER SURFACE ELEVATION = 1.118
MAXIMUM STORAGE = .0296 AC-FT INCREMENTAL TIME=
                                                            .033330HRS
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PRINT HYD ID=3 CODE=1

HYDROGRAPH FROM AREA BASIN_ROUTE

RUNOFF VOLUME = 2.39605 INCHES = .1457 ACRE-FEET
PEAK DISCHARGE RATE = 2.22 CFS AT 1.600 HOURS BASIN AREA = .0011 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 09:52:32