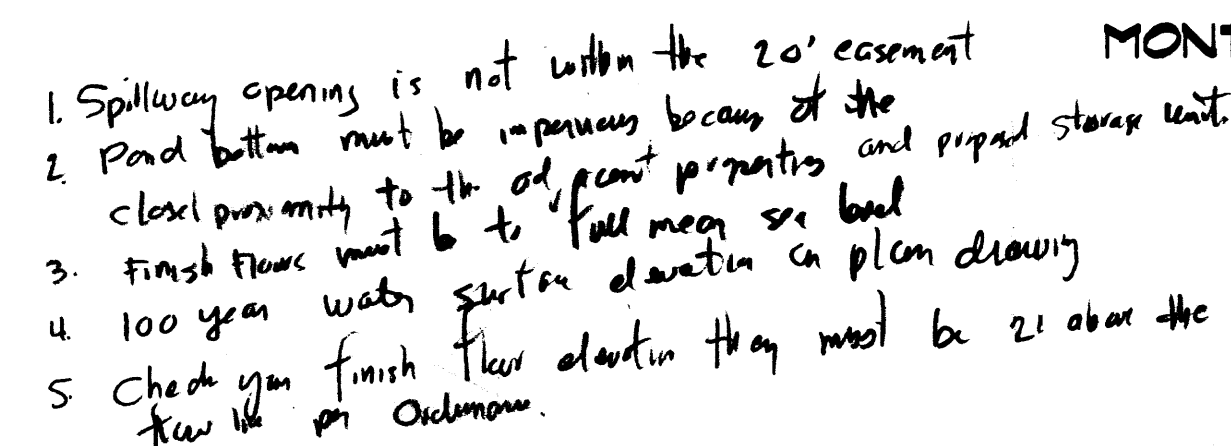
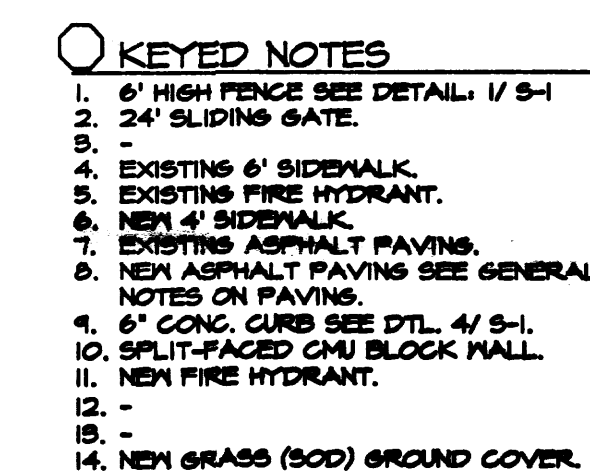




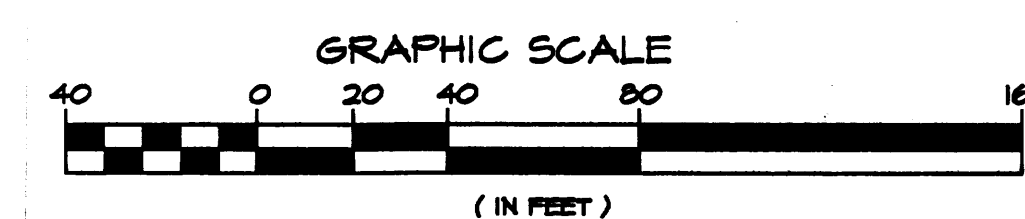
ROW 106'
PROPOSED ROW 124'

AT THE NORTHEAST CORNER OF THE
INTERSECTION OF MONTGOMERY BLVD.
AND LA BARRANCE AVE. NE AN ACS 1 3/4"
ALUMINUM DISC STAMPED "ACSEM,
15-F20", EPOXYED TO THE TOP OF THE
CONCRETE CURB AT THE ENE CURB RETURN



--- PROPERTY LINE
 -S- EXISTING CONTOUR
 50.2 EXISTING SPOT ELEVATION
 54.42FF PROPOSED SPOT ELEVATION
 - - - - - DIRECTION OF FLOW
 --- DRAINAGE BASIN BOUNDARY
 --- PROPOSED CURB
 // EXISTING 100 YR. FLOODPLAIN
 PER FIRM 550002-0017C

SCALE: 1" = 40'



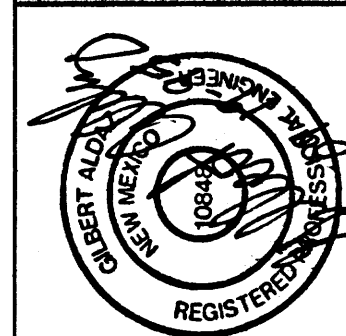
LEGAL DESCRIPTION

A CERTAIN TRACT OF LAND SITUATE WITHIN THE SOUTHWEST 1/4 OF THE SOUTHWEST 1/4 OF SECTION 32, T 11 N, R 4E, NMPM, BERNALILLO COUNTY, ALBUQUERQUE, NEW MEXICO AND BEING TRACT 11-C BLOCK 11, OF OFIMIANO J. GUTIERREZ TERRACE SUBDIVISION AS THE SAME IS SHOWN AND DESIGNATED ON THE PLAT FILED IN THE OFFICE OF THE COUNTY CLERK OF BERNALILLO COUNTY, NEW MEXICO ON JANUARY 24, 1960 IN VOLUME D2, FOLIO 91.

SEP 20 1995

GOLDEN TARGET SELF STORAGE
OWNERS: GEORGE KOURY & RODGER COX
ALBUQUERQUE, NEW MEXICO
PROJECT # 14915

REVISION DATE



Engineering and Surveying, Inc.
Civil Engineering, Land Planning

Gilbert Aldaz, P.E. & P.S.
1606 Blvd Drive NE
Albuquerque New Mexico 87112 (505) 267-1458

DATE
9-11-95

SHEET NUMBER
D-1

DRAINAGE CALCULATIONS AND PLANNING

DRAINAGE PLAN

THE FOLLOWING ITEMS CONCERNING THE GOLDEN TARGET SELF STORAGE GRADING AND DRAINAGE PLAN ARE CONTAINED HEREON:

1. VICINITY MAP
2. DRAINAGE & GRADING PLAN
3. DRAINAGE CALCULATIONS

EXISTING CONDITIONS:

AS SHOWN BY THE VICINITY MAP, THIS PARCEL IS KNOWN AS LOT 11C-1, BLOCK 11 OF THE OFIMIANO J. GUTIERREZ LOWER TERRACE ADDITION AND IS LOCATED ON THE NORTH SIDE OF MONTGOMERY BLVD. NE BETWEEN WYOMING BLVD. NE AND LA BARRANACA AVE. NE (SEE LOCATION MAP SHEET D-1). THE NORTH AND EAST SIDES OF THE PARCEL ARE BOUNDED BY THE BLOCK WALLS OF A RESIDENTIAL AREA, AND THE SOUTH SIDE BY MONTGOMERY BOULEVARD. THE WEST BOUNDARY IS ADJACENT TO THE GOLDEN CORRAL RESTAURANT AND TO THE THOMAS RESERVOIR SITE. ACCORDING TO FLOOD INSURANCE RATE MAPS (F.I.R.M.) THERE IS A FLOODPLAIN ALONG MONTGOMERY BLVD. TO THE SOUTH, PER FIRM PANEL 350002700176 DATED OCTOBER 1983.

PROPOSED CONDITIONS:

THE PROPOSED IMPROVEMENTS ARE TO CONSTRUCT SELF STORAGE UNITS. THE GRADING FOR THE SITE WILL STEP DOWN FROM EAST TO WEST, WHILE UTILIZING RETAINING WALLS TO MATCH GRADES AT BOUNDARY LINES. THE SITE WILL DRAIN FROM SOUTH TO NORTH, THEN TO DRAIN SYSTEM. THERE WILL BE AN 825 SF OFFICE LOCATED AT THE SOUTHEAST CORNER OF THIS SITE. THE FINISH FLOOR WILL BE 1.7 FT. ABOVE THE FLOWLINE OF MONTGOMERY BLVD.

OFFSITE FLOWS:

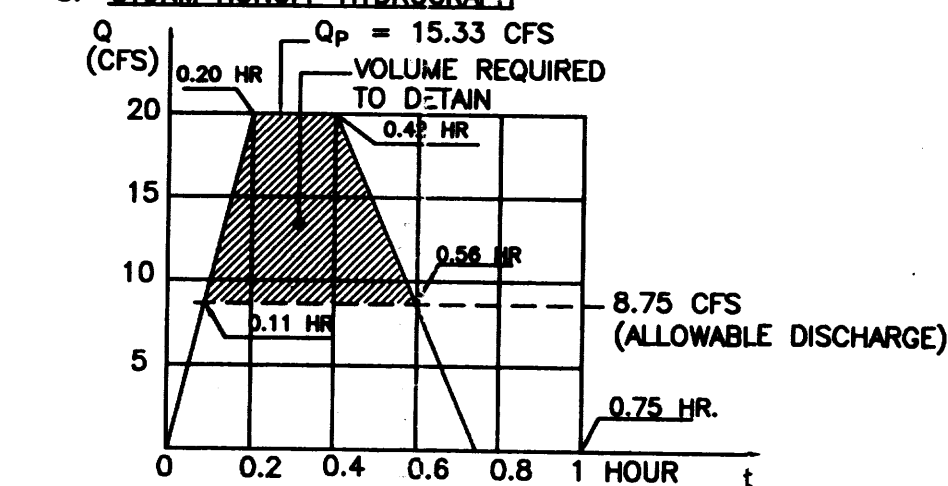
ALONG THE EAST BOUNDARY OF THIS SITE ARE ABOUT 3 BLOCK OPENINGS CONSISTING OF 2 - 5' X 5' OPENINGS (SEE PLAN). IT APPEARS THAT 50% OF THE AREA OF 3 RESIDENCES DRAIN INTO THIS SITE. THE CALCULATIONS WHICH APPEAR HEREON WILL ADDRESS ACCEPTANCE OF THIS FLOW.

THE OFFSITE PROPERTY TO THE NORTH ALSO DRAINS IN A NORTHWEST DIRECTION AND SHOULD NOT ENTER THIS PROPERTY. DOWNSTREAM CAPACITY: THE CALCULATIONS WHICH APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100 YEAR, 6 HR. RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.7, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOL. 2, DESIGN CRITERIA, DATED JAN. 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME RUNOFF. IN ORDER TO COMPLY WITH THE MASTER DRAINAGE PLAN PREPARED BY BOHANNAN-HUSTON IN 1987, CITY DRAINAGE FILE (F20-D20), THIS SITE WILL RELEASE 8.75 CFS. THIS SITE WILL REQUIRE AN ONSITE POND TO DETAIN FLOWS TO RELEASE AT 8.75 CFS. THIS FLOW WILL BE DISCHARGED INTO AN EXISTING STORM DRAIN AT WYOMING BLVD.

DRAINAGE CALCULATIONS:

1. PRECIPITATION ZONE = 3
2. DESIGN STORM = DEPTH (INCHES) AT 100 YEAR
6 HOUR = 2.80 IN
24 HOUR = 3.10 IN
3. PEAK DISCHARGE (CFS/ACRE) FOR 100 YEAR, ZONE 3
Q = 1.87 CFS/ACRE SOIL UNCOMPACTED "A"
Q = 2.80 CFS/ACRE LANDSCAPED "B"
Q = 3.45 CFS/ACRE COMPACTED SOIL "C"
Q = 5.02 CFS/ACRE IMPERVIOUS AREA "D"
FOR WATERSHEDS LESS THAN OR EQUAL TO 40 ACRES
SITE AREA = 3.24 ACRES
4. EXCESS PRECIPITATION, E (INCHES) - 6 HOUR STORM, ZONE 3
E = 0.66 INCHES SOIL UNCOMPACTED "A"
E = 0.92 INCHES LANDSCAPED "B"
E = 1.29 INCHES COMPACTED SOIL "C"
E = 2.36 INCHES IMPERVIOUS AREA "D"
5. EXISTING CONDITIONS ONSITE AND OFFSITE, PEAK FLOW AND VOLUME RUNOFF
TREATMENT AREA (ACRES)
A 3.01 (ONSITE) 3.12
B 0.14 (OFFSITE)
C - 0 -
D 0.20 (ONSITE) + 0.07 (OFFSITE)
Q (EXISTING PEAKFLOW) = (3.01 ACRES X 1.87 CFS/ACRE) + (0.14 ACRES X 2.80 CFS/ACRE) + (0.20 + 0.07 ACRES) X 5.02 CFS/ACRE = 7.4 CFS - EXISTING
V (EXISTING VOLUME) = ((3.01 ACRES X 0.66 IN) + (0.14 ACRES X 0.92 IN) + (0.20 + 0.07 ACRES) X 2.36 IN) / 12 = 0.23 ACRE FT. EXISTING
6. PROPOSED LAND TREATMENTS AND PEAK FLOW
A. BASIN A (INCLUDES OFFSITE BASIN)
TREATMENT ACRES
A - 0 -
B 0.14 (OFFSITE) 3.18
C 0.19 (ONSITE)
D 2.78 (ONSITE) + 0.07 (OFFSITE)
Q (PROPOSED PEAKFLOW) = (0.14 AC X 2.80 CFS/ACRE) + (0.19 AC X 3.45 CFS/ACRE) + (2.78 + 0.07 AC) X 5.02 CFS/ACRE = 15.33 CFS PROPOSED PEAK FLOW
7. VOLUME REQUIREMENTS
 $t_b = (2.107 * E * A_T / Q_p) - (0.25 * A_o / A_T)$
 t_b IS IN HOURS
E IS FROM TABLE A-8
WEIGHTED = $\frac{E_A A_A + E_B A_B + E_C A_C + E_D A_D}{A_A + A_B + A_C + A_D}$
 $E = \frac{0.66(0) + 0.92(0.14) + 1.29(0.19) + 2.36(2.85)}{0 + 0.14 + 0.19 + 2.85}$
 $E = 2.23$ INCHES
 $Q_p = 15.33$ CFS
 $A_o = 2.85$ ACRES
 $A_T = 3.18$ ACRES
 $t_o = 0.20$ HOURS
 $t_b = (2.107 * 2.23 * 3.18 / 15.33) - (0.25 * 2.85 / 3.18)$
 $t_b = 0.75$ HOURS
 $t_p = (0.7 * t_o) + ((1.6 - A_o / A_T) / 12)$
 $t_p = (0.7 * 0.2) + ((1.6 - (2.85 / 3.18)) / 12) = 0.20$ HRS.
DURATION OF PEAK = $0.25 * A_o / A_T$
 $= 0.25 * 2.85 / 3.18 = 0.22$ HOURS

8. STORM RUNOFF HYDROGRAPH



$$\text{VOLUME REQUIRED} = \left[\frac{((15.33 \text{ CFS} - 8.75 \text{ CFS}) * (0.20 - 0.11))}{2} + \frac{(15.33 - 8.75)(0.42 - 0.20 \text{ HR})}{2} + \frac{((15.33 - 8.75) * (0.56 - 0.42))}{2} \right] \frac{60 \text{ MIN.} \times 60 \text{ SEC.}}{\text{HOUR}} = 7,935 \text{ CF REQUIRED VOLUME}$$

9. POND VOLUME PROVIDED

GRADE (FT)	AREA (SF)	AVERAGE AREA (SF)	HEIGHT (FT)	VOLUME (CF)
40.0	0	912.5	1.0	912.5
41.0	1,825	1,825	1.0	1,825
42.0	1,825	1,825	1.0	1,825
43.0	1,825	1,825	1.0	1,825
44.0	1,825	1,825	1.0	1,825
		1,825	0.9	1,643

SPILLWAY 44.9 ELEV 1,825 8,031 CF
POND VOLUME PROVIDED = 8,031 CF > 7,935 CF REQUIRED

SPILLWAY WIDTH

$$Q = CLH^{3/2}$$

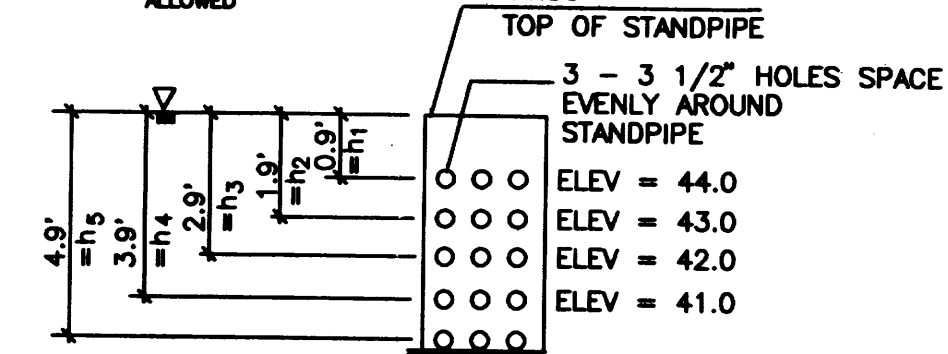
$$Q = 15.33 \text{ CFS} \quad C = 3.0 \quad H = 0.5'$$

$$L = \frac{Q}{CH^{3/2}} = \frac{15.33}{3.0(0.5)^{3/2}} = 14.45' \text{ SPILLWAY WIDTH}$$

10. ORIFICE CONTROL

$$Q = CA \sqrt{2gh}$$

$$Q_{ALLOWED} = 8.75 \text{ CFS} \quad 44.90$$



$$Q = CA \sqrt{2gh_1} + CA \sqrt{2gh_2} + CA \sqrt{2gh_3} + CA \sqrt{2gh_4}$$

$$h_1 = 0.9' \quad h_2 = 1.9' \quad h_3 = 2.9' \quad h_4 = 3.9' \quad h_5 = 4.9'$$

$$Q = 8.75 \text{ CFS} \quad C = 0.65 \quad g = 32.2$$

$$8.75 = 0.65 \times A \sqrt{2 \times 32.2 \times 0.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 1.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 2.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 3.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 4.9}$$

$$8.75 = 4.95A + 7.19A + 8.88A + 10.30A + 11.55A$$

$$8.75 + 42.87A \text{ SOLVE FOR } A = 0.20 \text{ FT}^2 = 30 \text{ IN}^2$$

$$\text{TRY } 3 - 3 \text{ 1/2 HOLES}$$

$$3 \times \pi \times (3.5)^2 = 28.9 \text{ IN}^2 \approx 30 \text{ IN}^2$$

USE 3 - 3 1/2 HOLES AT EACH ELEVATION

GOLDEN TARGET SELF STORAGE
OWNER: GEORGE KOURY & RODGER COX
ALBUQUERQUE, NEW MEXICO
PROJECT # 1445

REVISION DATE



Aldaz Engineering and Surveying, Inc.
Civil Engineering, Land Planning
& Surveying

DATE

9-11-95

SHEET NUMBER

D-2

RECEIVED
SEP 20 1995

264-96
264-696

DRAINAGE CALCULATIONS AND PLANNING

DRAINAGE PLAN

THE FOLLOWING ITEMS CONCERNING THE GOLDEN TARGET SELF STORAGE GRADING AND DRAINAGE PLAN ARE CONTAINED HEREON:

1. VICINITY MAP
2. DRAINAGE & GRADING PLAN
3. DRAINAGE CALCULATIONS

EXISTING CONDITIONS:

AS SHOWN BY THE VICINITY MAP, THIS PARCEL IS KNOWN AS LOT 11C-1, BLOCK 11 OF THE OFIMIANO J. GUTIERREZ LOWER TERRACE ADDITION AND IS LOCATED ON THE NORTH SIDE OF MONTGOMERY BLVD. NE BETWEEN WYOMING BLVD. NE AND LA BARRANACA AVE. NE (SEE LOCATION MAP SHEET D-1). THE NORTH AND EAST SIDES OF THE PARCEL ARE BOUNDED BY THE BLOCK WALLS OF A RESIDENTIAL AREA, AND THE SOUTH SIDE BY MONTGOMERY BOULEVARD. THE WEST BOUNDARY IS ADJACENT TO THE GOLDEN CORRAL RESTAURANT AND TO THE THOMAS RESERVOIR SITE.

ACCORDING TO FLOOD INSURANCE RATE MAPS (F.I.R.M.) THERE IS A FLOODPLAIN ALONG MONTGOMERY BLVD. TO THE SOUTH, PER FIRM PANEL 350002 00176 DATED OCTOBER 1983.

PROPOSED CONDITIONS:

THE PROPOSED IMPROVEMENTS ARE TO CONSTRUCT SELF STORAGE UNITS. THE GRADING FOR THE SITE WILL STEP DOWN FROM EAST TO WEST, WHILE UTILIZING RETAINING WALLS TO MATCH GRADES AT BOUNDARY LINES. THE SITE WILL DRAIN FROM SOUTH TO NORTH, THEN TO DRAIN SYSTEM. THERE WILL BE AN 825 SF OFFICE LOCATED AT THE SOUTHEAST CORNER OF THIS SITE. THE FINISH FLOOR WILL BE 1.7 FT. ABOVE THE FLOWLINE OF MONTGOMERY BLVD.

OFFSITE FLOWS:

ALONG THE EAST BOUNDARY OF THIS SITE ARE ABOUT 3 BLOCK OPENINGS CONSISTING OF 2 - 5' X 5' OPENINGS (SEE PLAN). IT APPEARS THAT 50% OF THE AREA OF 3 RESIDENCES DRAIN INTO THIS SITE. THE CALCULATIONS WHICH APPEAR HEREON WILL ADDRESS ACCEPTANCE OF THIS FLOW.

THE OFFSITE PROPERTY TO THE NORTH ALSO DRAINS IN A NORTHWEST DIRECTION AND SHOULD NOT ENTER THIS PROPERTY.

DOWNSTREAM CAPACITY:

THE CALCULATIONS WHICH APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100 YEAR, 6 HR. RAINFALL EVENT. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.7, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOL. 2, DESIGN CRITERIA, DATED JAN. 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME RUNOFF. IN ORDER TO COMPLY WITH THE MASTER DRAINAGE PLAN PREPARED BY BOHANNAN-HUSTON IN 1987, CITY DRAINAGE FILE (F20-D20), THIS SITE WILL RELEASE 8.75 CFS. THIS SITE WILL REQUIRE AN ONSITE POND TO DETAIN FLOWS TO RELEASE AT 8.75 CFS. THIS FLOW WILL BE DISCHARGED INTO AN EXISTING STORM DRAIN AT WYOMING BLVD.

DRAINAGE CALCULATIONS:

1. PRECIPITATION ZONE = 3
2. DESIGN STORM = DEPTH (INCHES) AT 100 YEAR
6 HOUR = 2.60 IN
24 HOUR = 3.10 IN
3. PEAK DISCHARGE (CFS/ACRE) FOR 100 YEAR, ZONE 3
Q = 1.87 CFS/ACRE SOIL UNCOMPACTED "A"
Q = 2.60 CFS/ACRE LANDSCAPED "B"
Q = 3.45 CFS/ACRE COMPACTED SOIL "C"
Q = 5.02 CFS/ACRE IMPERVIOUS AREA "D"
FOR WATERSHEDS LESS THAN OR EQUAL TO 40 ACRES
SITE AREA = 3.24 ACRES
4. EXCESS PRECIPITATION, E (INCHES) - 6 HOUR STORM, ZONE 3
E = 0.66 INCHES SOIL UNCOMPACTED "A"
E = 0.92 INCHES LANDSCAPED "B"
E = 1.29 INCHES COMPACTED SOIL "C"
E = 2.36 INCHES IMPERVIOUS AREA "D"

5. EXISTING CONDITIONS ONSITE AND OFFSITE, PEAK FLOW AND VOLUME RUNOFF

TREATMENT	AREA (ACRES)
A	3.01 (ONSITE)
B	0.14 (OFFSITE)
C	-
D	0.20 (ONSITE) + 0.07 (OFFSITE)

$$Q \text{ (EXISTING PEAKFLOW)} = (3.01 \text{ ACRES} \times 1.87 \text{ CFS/ACRE}) + (0.14 \text{ ACRES} \times 2.60 \text{ CFS/ACRE}) + (0.20 + 0.07 \text{ ACRES}) \times 5.02 \text{ CFS/ACRE} = 7.4 \text{ CFS} - \text{EXISTING}$$

$$V \text{ (EXISTING VOLUME)} = ((3.01 \text{ ACRE} \times 0.66 \text{ IN}) + (0.14 \text{ ACRES} \times 0.92 \text{ IN}) + (0.20 + 0.07 \text{ ACRES}) \times 2.36 \text{ IN}) / 12 = 0.23 \text{ ACRE FT. EXISTING}$$

6. PROPOSED LAND TREATMENTS AND PEAK FLOW

A. BASIN A (INCLUDES OFFSITE BASIN)

TREATMENT	ACRES
A	-0-
B	0.14 (OFFSITE)
C	0.19 (ONSITE)
D	2.78 (ONSITE) + 0.07 (OFFSITE)

$$Q \text{ (PROPOSED PEAKFLOW)} = (0.14 \text{ AC} \times 2.60 \text{ CFS/ACRE}) + (0.19 \text{ AC} \times 3.45 \text{ CFS/ACRE}) + (2.78 + 0.07 \text{ AC}) \times 5.02 \text{ CFS/ACRE} = 15.33 \text{ CFS PROPOSED PEAK FLOW}$$

7. VOLUME REQUIREMENTS

$$t_b = (2.107 \cdot E \cdot A_T / Q_P) - (0.25 \cdot A_D / A_T)$$

t_b IS IN HOURS

E IS FROM TABLE A-8

$$\text{WEIGHTED } E = \frac{E_A A_A + E_B A_B + E_C A_C + E_D A_D}{A_A + A_B + A_C + A_D}$$

$$E = \frac{0.66(0) + 0.92(0.14) + 1.29(0.19) + 2.36(2.85)}{0 + 0.14 + 0.19 + 2.85}$$

$$E = 2.23 \text{ INCHES}$$

$$Q_P = 15.33 \text{ CFS}$$

$$A_D = 2.85 \text{ ACRES}$$

$$A_T = 3.18 \text{ ACRES}$$

$$t_c = 0.20 \text{ HOURS}$$

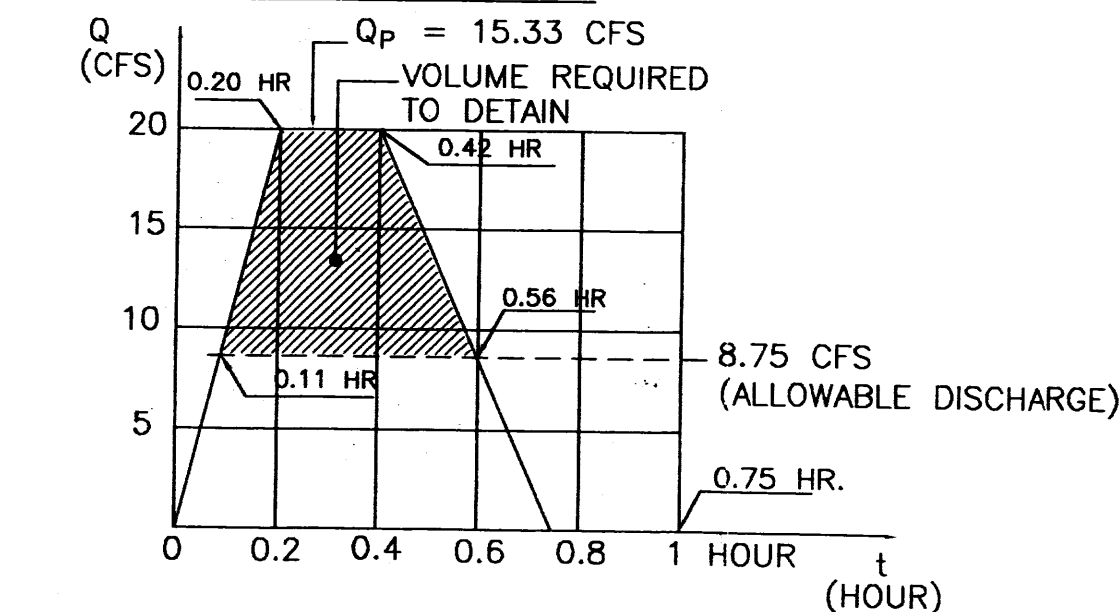
$$t_b = (2.107 \cdot 2.23 \cdot 3.18 / 15.33) - (0.25 \cdot 2.85 / 3.18) = 0.75 \text{ HOURS}$$

$$t_p = (0.7 \times t_c) + ((1.6 - A_D / A_T) / 12)$$

$$t_p = (0.7 \times 0.2) + ((1.6 - (2.85 / 3.18)) / 12) = 0.20 \text{ HRS.}$$

$$\text{DURATION OF PEAK} = 0.25 \cdot A_D / A_T = 0.25 \cdot 2.85 / 3.18 = 0.22 \text{ HOURS}$$

8. STORM RUNOFF HYDROGRAPH



$$\text{VOLUME REQUIRED} = [((15.33 \text{ CFS} - 8.75 \text{ CFS}) \cdot (0.20 - 0.11)) / 2 + ((15.33 - 8.75) \cdot (0.42 - 0.20 \text{ HR})) + ((15.33 - 8.75) \cdot (0.56 - 0.42)) / 2] \cdot \frac{60 \text{ MIN.}}{\text{HOUR}} \cdot \frac{60 \text{ SEC}}{\text{MIN.}} = 7,935 \text{ CF REQUIRED VOLUME}$$

9. POND VOLUME PROVIDED

GRADE (FT)	AREA (SF)	AVERAGE AREA (SF)	HEIGHT (FT)	VOLUME (CF)
40.0	0	912.5	1.0	912.5
41.0	1,825	1,825	1.0	1,825
42.0	1,825	1,825	1.0	1,825
43.0	1,825	1,825	1.0	1,825
44.0	1,825	1,825	1.0	1,825
		1,825	0.9	1,643

$$\text{POND VOLUME PROVIDED} = 8,031 \text{ CF} > 7,935 \text{ CF REQUIRED}$$

SPILLWAY DEPTH

$$Q = CLH^{3/2}$$

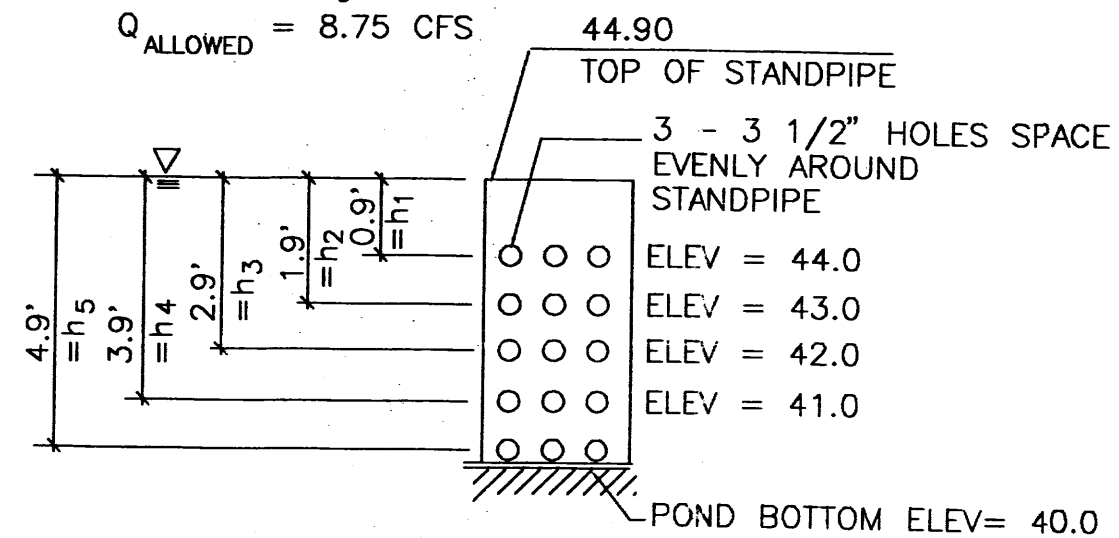
$$Q = 15.33 \text{ CFS} \quad C = 3.0 \quad L = 9.0' (\text{SPILLWAY WIDTH BASED ON EASEMENT WIDTH})$$

$$H = \left(\frac{Q}{CL} \right)^{2/3} = \left(\frac{15.33}{3.0 \times 9.0} \right)^{2/3} = 0.68'$$

10. ORIFICE CONTROL

$$Q = CA \sqrt{2gh}$$

$$Q_{\text{ALLOWED}} = 8.75 \text{ CFS}$$



$$Q = CA \sqrt{2gh_1} + CA \sqrt{2gh_2} + CA \sqrt{2gh_3} + CA \sqrt{2gh_4} + CA \sqrt{2gh_5}$$

$$h_1 = 0.9' \quad h_2 = 1.9' \quad h_3 = 2.9' \quad h_4 = 3.9' \quad h_5 = 4.9'$$

$$Q = 8.75 \text{ CFS} \quad C = 0.65 \quad g = 32.2$$

$$8.75 = 0.65 \times A \sqrt{2 \times 32.2 \times 0.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 1.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 2.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 3.9} + 0.65 \times A \sqrt{2 \times 32.2 \times 4.9}$$

$$8.75 = 4.95A + 7.19A + 8.88A + 10.30A + 11.55A$$

$$8.75 + 42.87A \text{ SOLVE FOR } A = 0.20 \text{ FT}^2 = 30 \text{ IN}^2$$

$$\text{TRY } 3 - 3 \frac{1}{2} \text{ HOLES}$$

$$\frac{3 \times \pi \times (3.5)^2}{4} = 28.9 \text{ IN}^2 \approx 30 \text{ IN}^2$$

USE 3 - 3 1/2" HOLES AT EACH ELEVATION

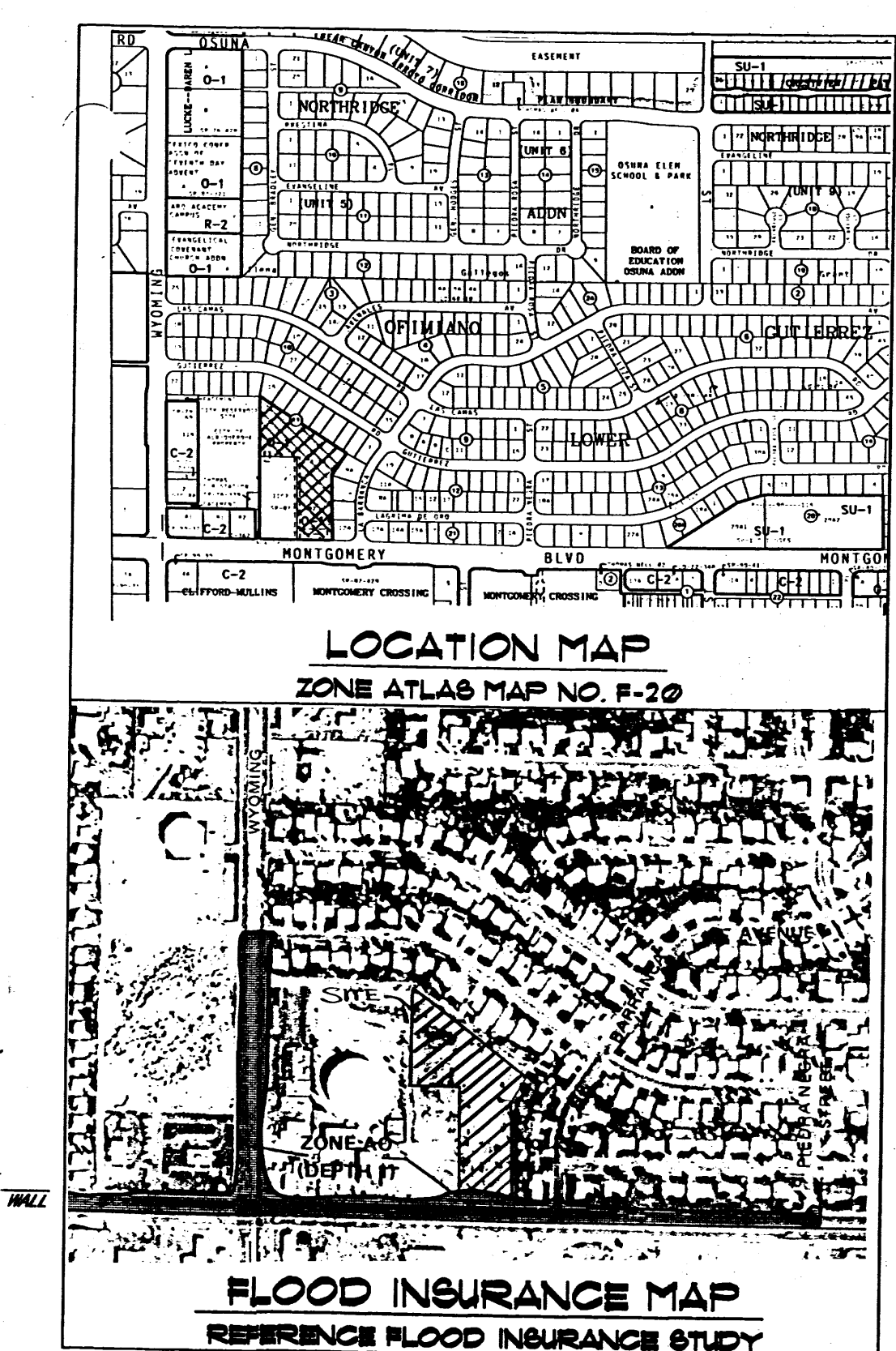
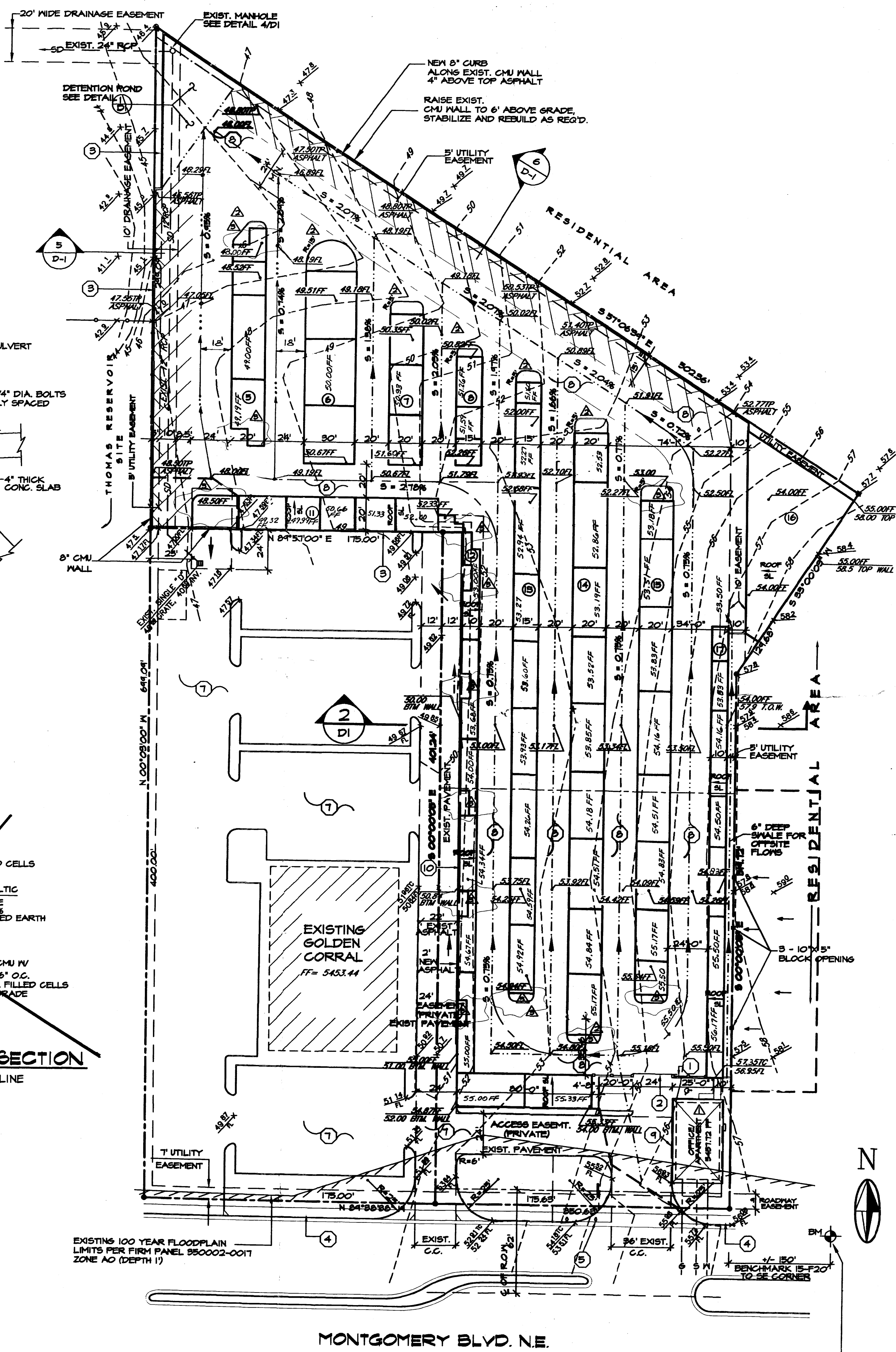
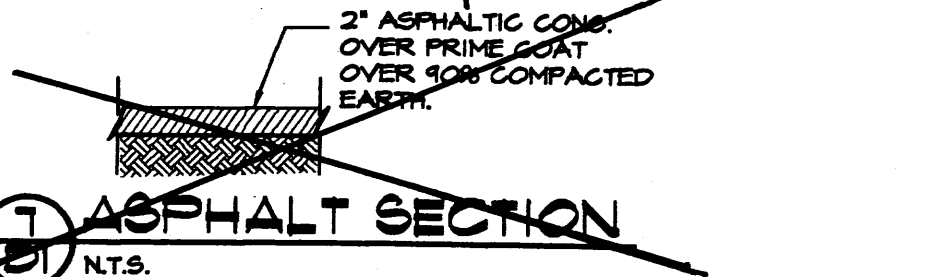
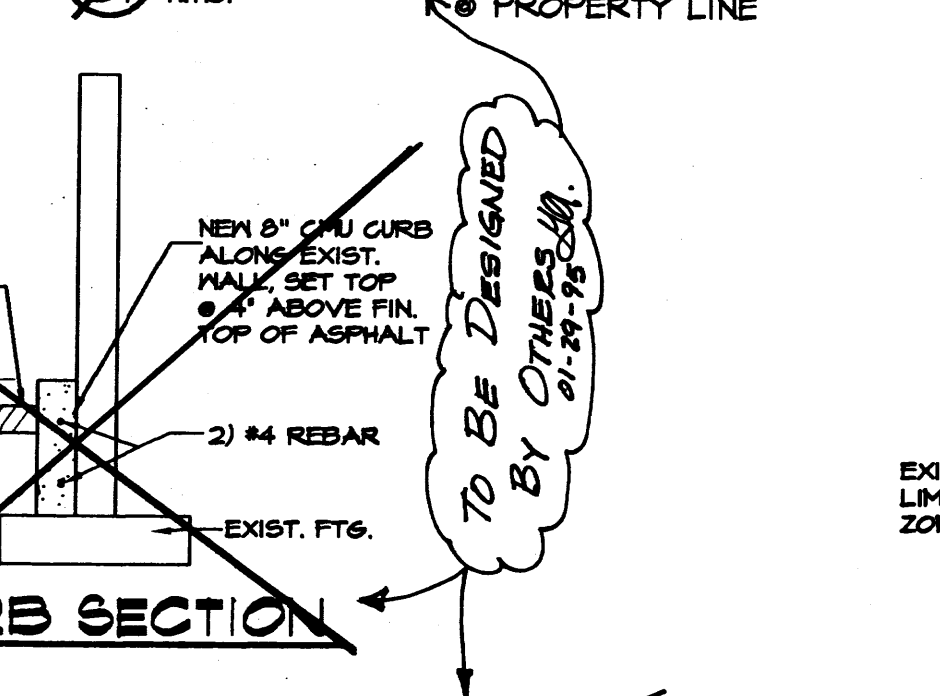
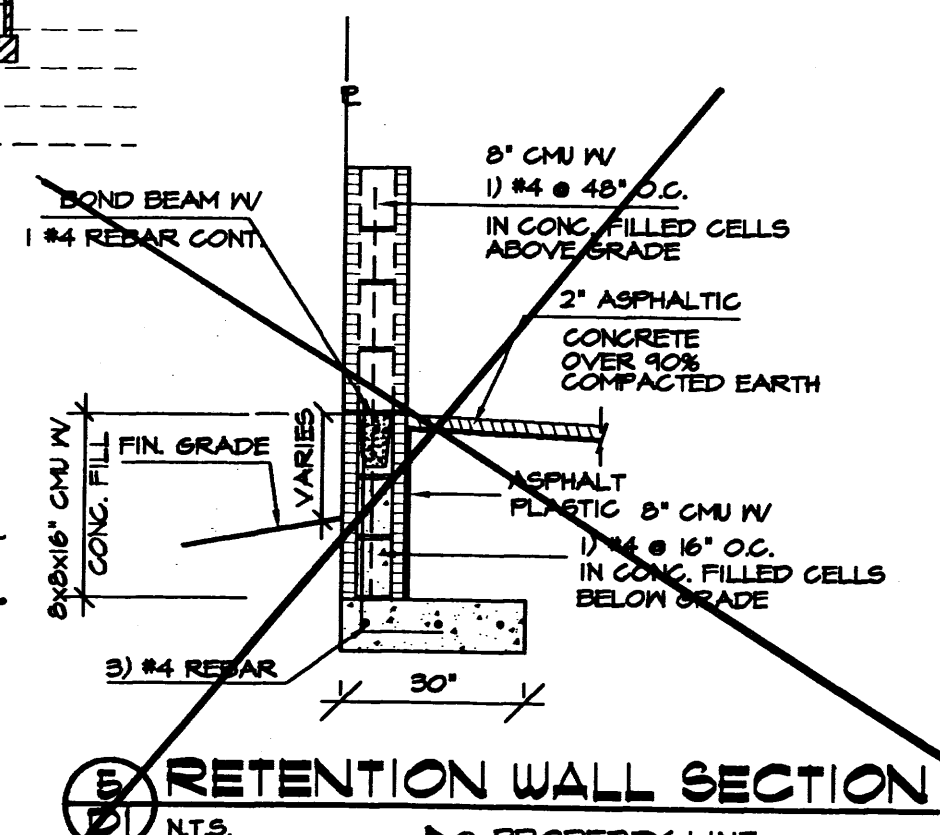
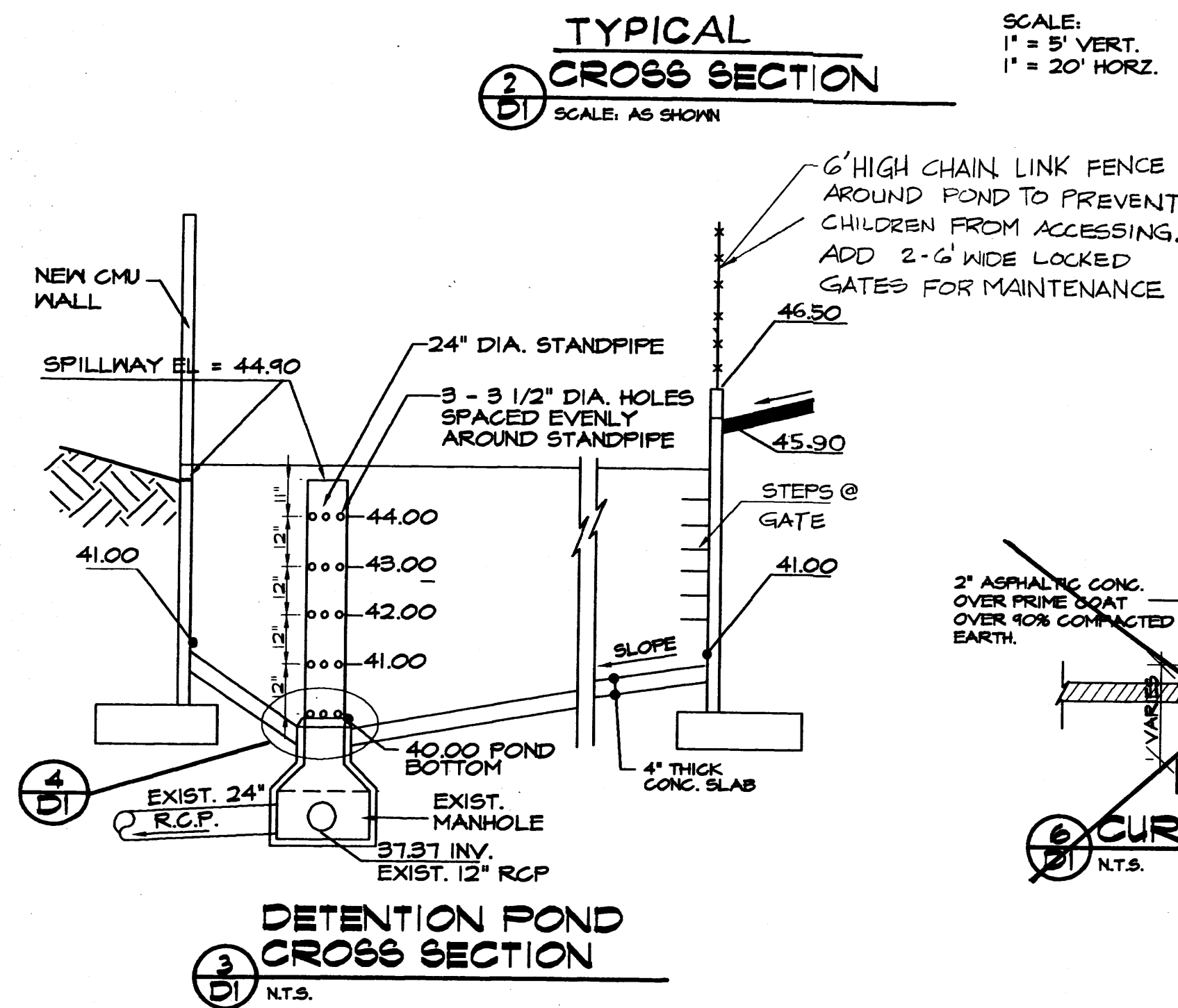
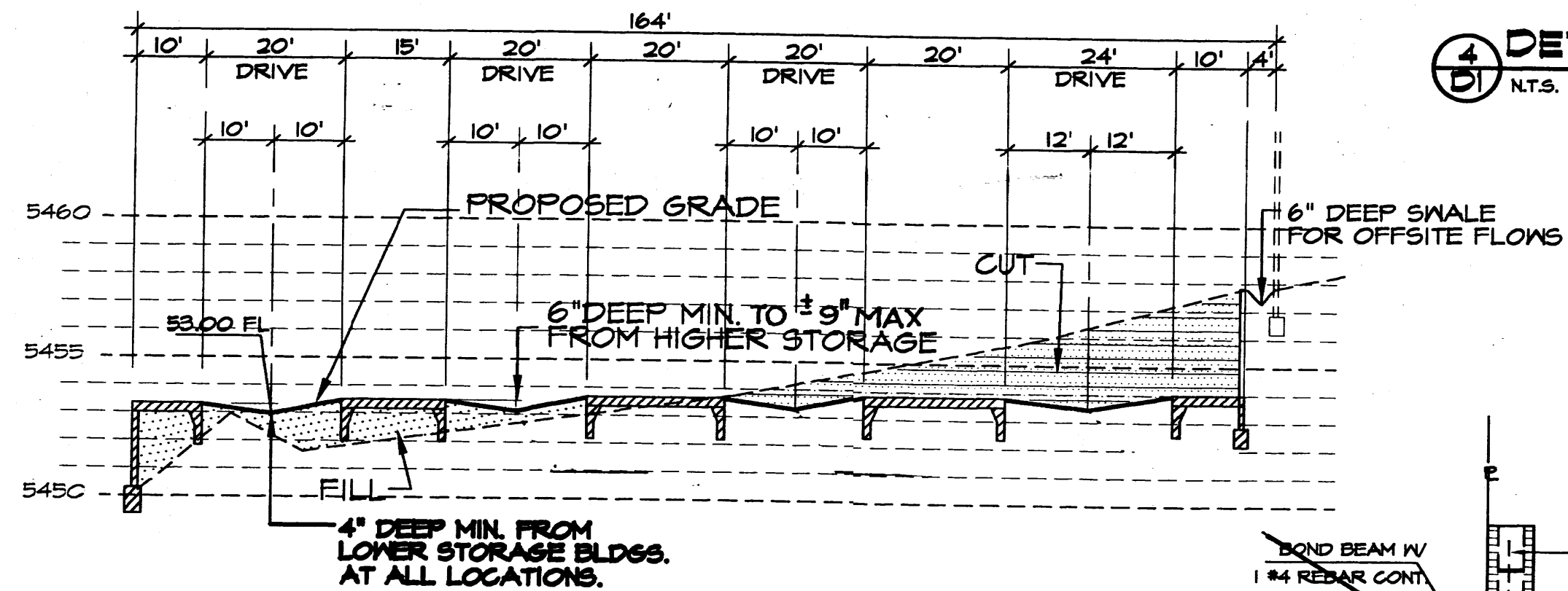
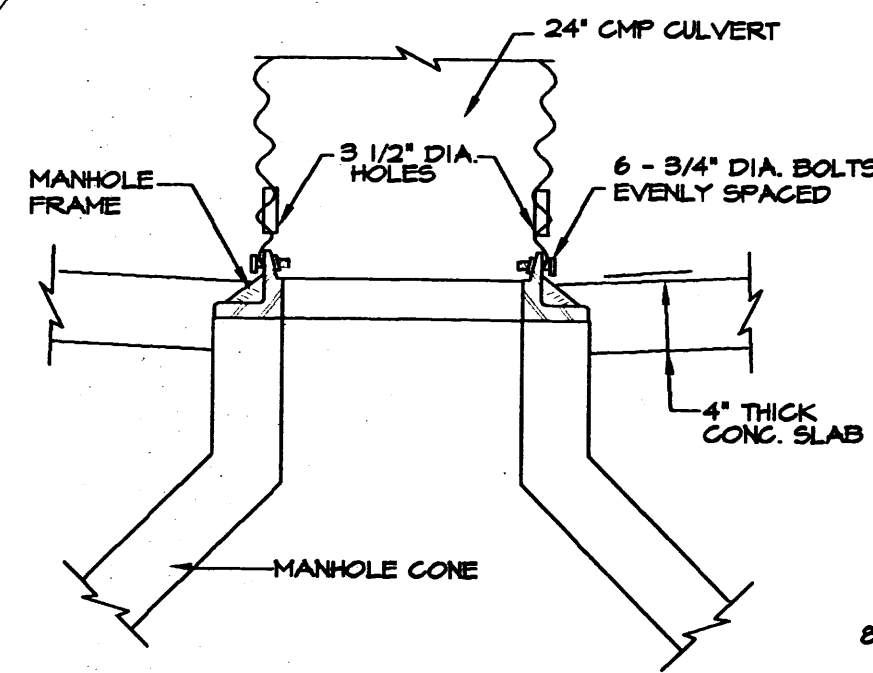
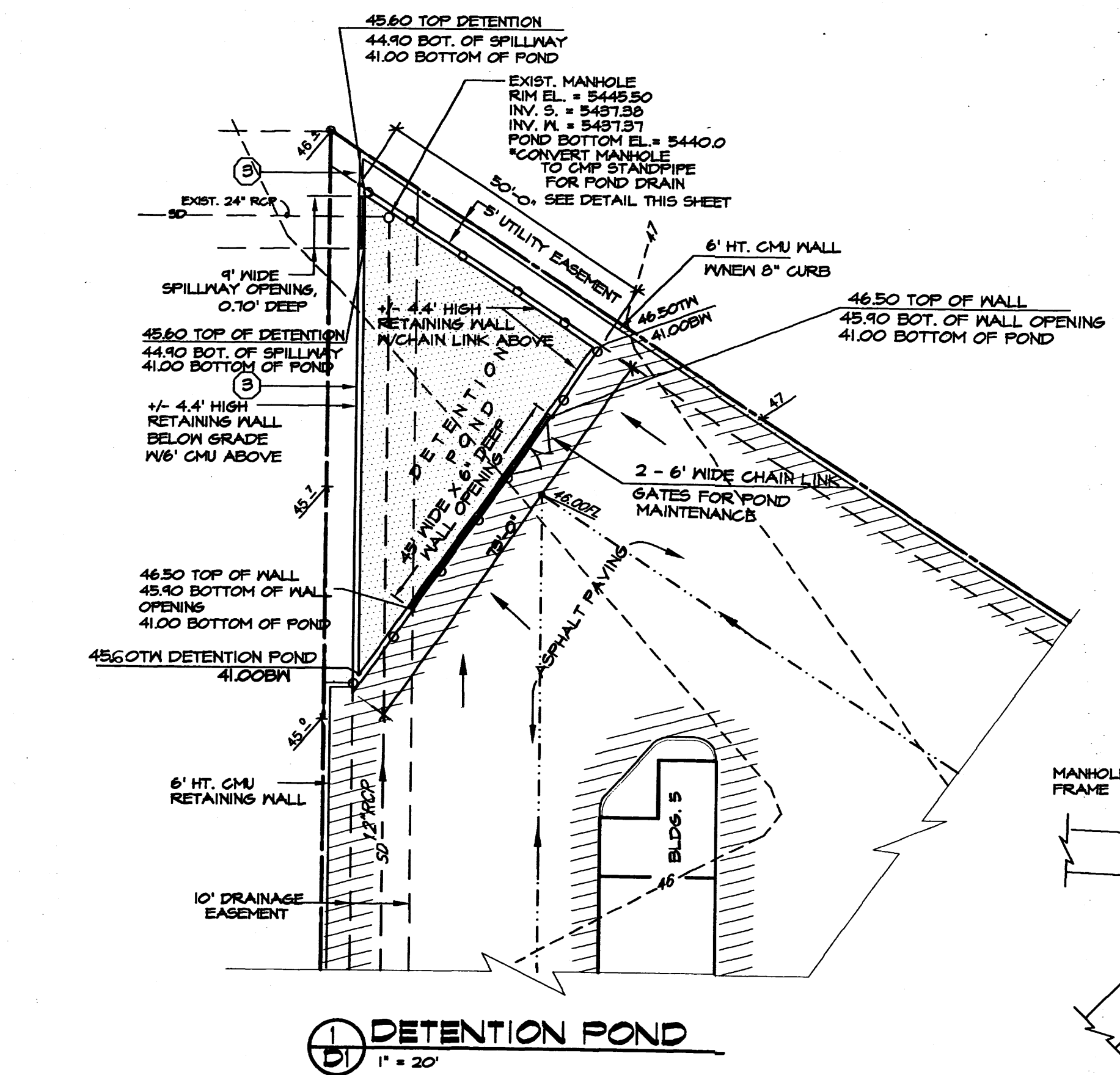
GOLDEN TARGET SELF STORAGE
OWNERS: GEORGE KOURY & RODGER COX
ALBUQUERQUE, NEW MEXICO
PROJECT # 1495

REVISION DATE
Aldaz Engineering and Surveying, Inc. Civil Engineering, Land Planning & Surveying 1000 Blue Sky Dr. Albuquerque, New Mexico 87112 (505) 887-1466
DATE
9-11-95
SHEET NUMBER
D-2

CONSTRUCTION DOCUMENT

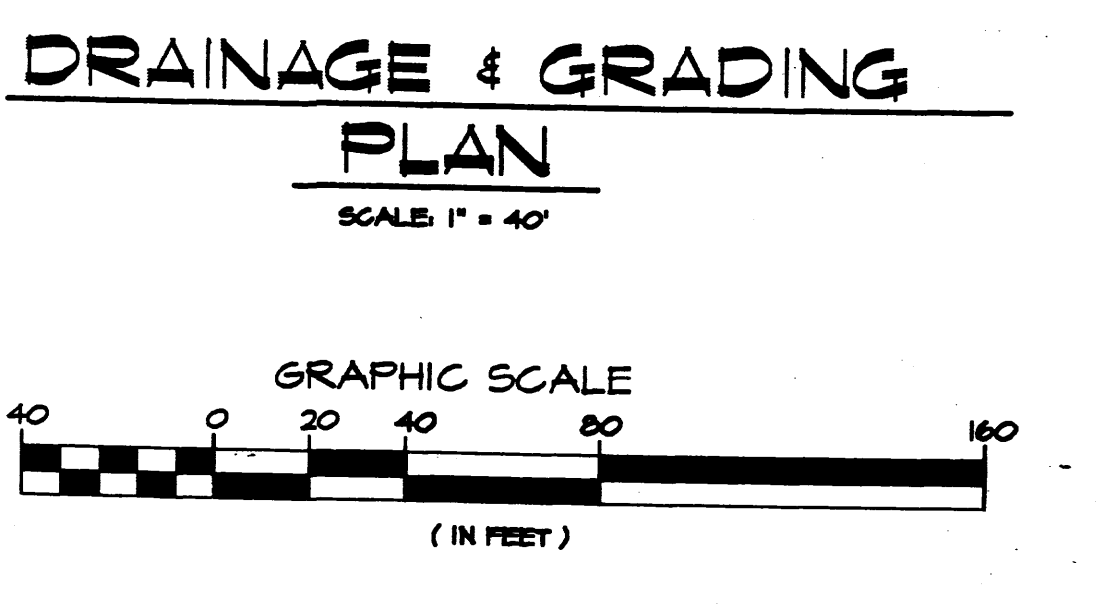
11/30/95

JAN 3 1996



- KEYED NOTES**
1. 6" HIGH FENCE SEE DETAIL: V-5-1
 2. 24" SLIDING GATE
 3. STANDARD CMU
 4. EXISTING 6" SIDEWALK
 5. EXISTING FIRE HYDRANT
 6. NEW 4" SIDEWALK
 7. EXISTING ASPHALT PAVING
 8. NEW ASPHALT PAVING SEE GENERAL NOTES ON PAVING
 9. 6" CONC. CURB SEE DET. 4/5-1
 10. SPLIT-FACED CMU BLOCK WALL
 11. FIRE HYDRANT (RELOCATED)
 12. 1"
 13. 1"
 14. NEW GRASS (500) GROUND COVER

- LEGEND**
- PROPERTY LINE
 - 0-0- EXISTING CONTOUR
 - 0-0- EXISTING SPOT ELEVATION
 - 0-0- PROPOSED SPOT ELEVATION
 - - - DIRECTION OF FLOW
 - - - DRAINAGE BASIN BOUNDARY
 - - - PROPOSED CURB
 - - - EXISTING 100 YR. FLOODPLAIN PER FIRM 550002-001C
- NOTE:** ADD 5400 FEET TO ALL ELEVATIONS ON THIS PLAN IN ORDER TO ESTABLISH FULL MEAN SEA LEVEL DESIGNATION.
- ALL EXISTING BUILDINGS ROOF SLOPES AND LEADERS MUST DRAIN TO THE INTERIOR OF THIS SITE AND CANNOT DRAIN TOWARD ANY PROPERTY LINES.**



LEGAL DESCRIPTION

A CERTAIN TRACT OF LAND SITUATE WITHIN THE SOUTHWEST 1/4 OF THE SOUTHWEST 1/4 OF SECTION 32, T. 11 N., R. 14 E., N.M.P.M., BERNALILLO COUNTY, ALBUQUERQUE, NEW MEXICO, BEING TRACT 11-C BLOCK II, OF OFIMANO J. GUTIERREZ TERRACE SUBDIVISION, THE SAME IS SHOWN AND DESIGNATED ON THE PLAT FILED IN THE OFFICE OF THE COUNTY CLERK OF BERNALILLO COUNTY, NEW MEXICO ON JANUARY 24, 1960 IN VOLUME D2, FOLIO 41.

GOLDEN TARGET SELF STORAGE
OWNERS: GEORGE KOURY & RODGER COX
ALBUQUERQUE, NEW MEXICO
PROJECT # 1445

REVISION DATE
4-25-93 OFFICE/BDWLK
B.L.D.G. LAYOUT
10-10-93 FIN. GR.
REVISED DETAILS
ADDENDUM #2 (11/22/95)

ADAZ Engineering and Surveying, Inc.
Civil Engineering, Land Planning
& Surveying
Gilbert Aldaz, P.E. & P.S.
1602 State Drive NW
Albuquerque, New Mexico 87112 (505) 387-1468

DATE
11-30-95

SHEET NUMBER
D-1

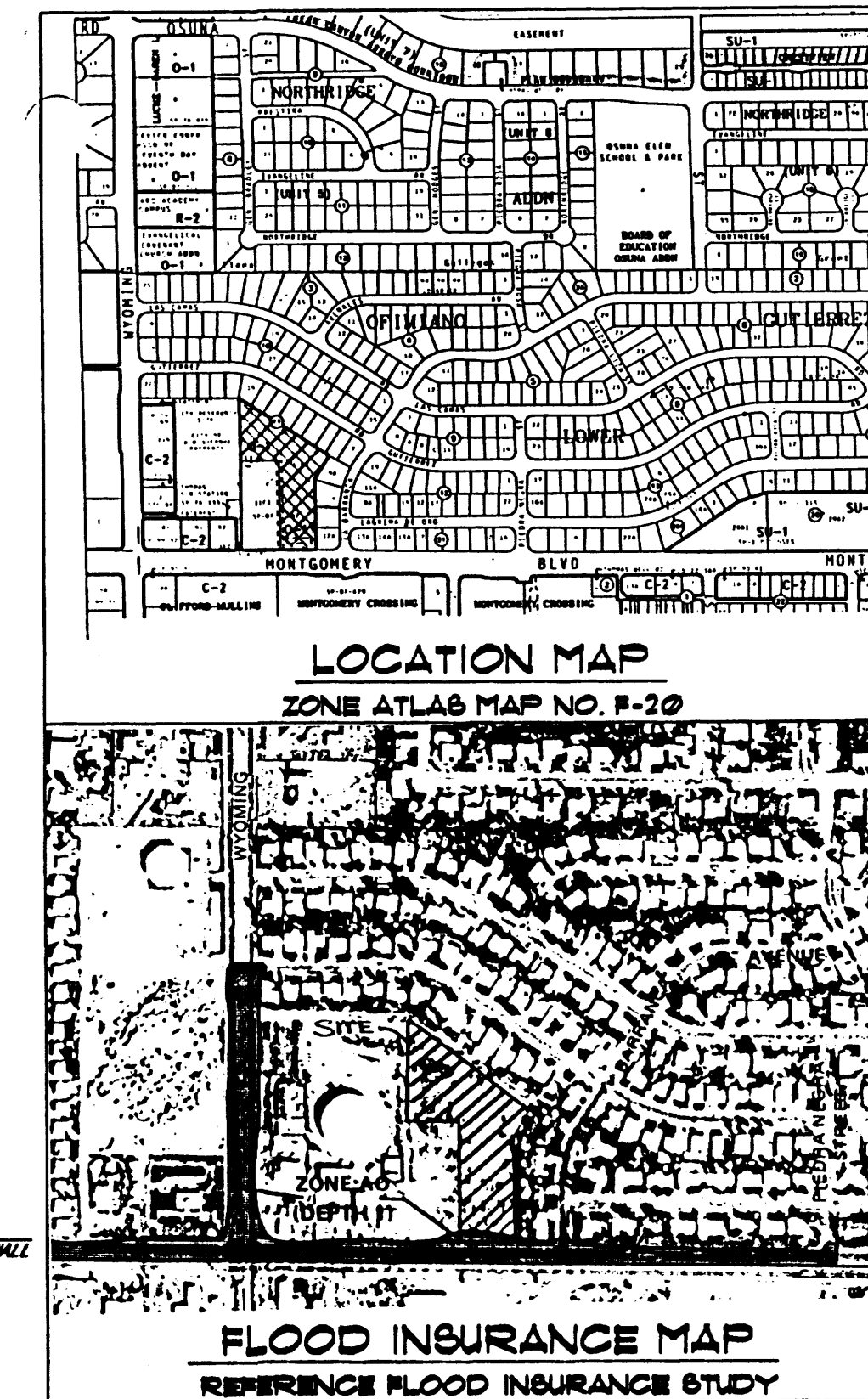
CONSTRUCTION DOCUMENT
11/30/95

TEMPORARY CERTIFICATION

As Indicated by the As-Built information shown hereon, the Golden Target Self Storage Grading and Drainage Plan is in near substantial conformance with the approved Grading and Drainage Plan. The only items remaining is the fencing around the pond and grouting of the manholes. This certification is presented in fulfillment of drainage requirements requested by the City of Albuquerque. The information shown hereon has been obtained by me or under my direct supervision and is true and correct to the best of my knowledge and belief.

Gilbert Aldaz
Gilbert Aldaz, NMPE No. 10848

05-19-96
Date



GOLDEN TARGET SELF STORAGE
OWNERS: GEORGE KOURY & RODGER COX
ALBUQUERQUE, NEW MEXICO
PROJECT # 1495

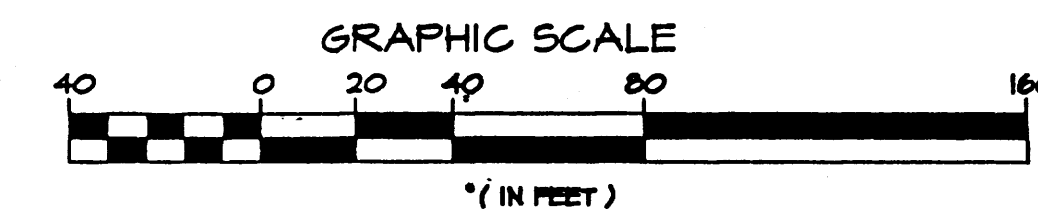
- KEYED NOTES**
1. 6" HIGH FENCE SEE DETAIL 1/5-1
 2. 24" SLIDING GATE
 3. STANDARD CHU
 4. EXISTING 6" SIDEWALK
 5. EXISTING FIRE HYDRANT
 6. NEW 4" SIDEWALK
 7. EXISTING ASPHALT PAVING
 8. NEW ASPHALT PAVING SEE GENERAL NOTES ON PAVING
 9. 4" CONC. CURB SEE DETAIL 4/5-1
 10. SPLIT-FACED CHU BLOCK WALL
 11. FIRE HYDRANT (RELOCATED)
 12. " "
 13. " "
 14. NEW GRASS (500) GROUND COVER

- LEGEND**
- PROPERTY LINE
 - 54- EXISTING CONTOUR
 - 54.12E EXISTING SPOT ELEVATION
 - 54.12E PROPOSED SPOT ELEVATION
 - DIRECTION OF FLOW
 - DRAINAGE BASIN BOUNDARY
 - PROPOSED CURB
 - EXISTING 100 YR. FLOODPLAIN PER FIRM 550002-0011C

NOTE: ADD 8400 FEET TO ALL ELEVATIONS ON THIS PLAN IN ORDER TO ESTABLISH FULL MEAN SEA LEVEL DESIGNATION.
ALL EXISTING STORAGE BUILDINGS ROOF SLOPES AND LEADERS MUST DRAIN TO THE INTERIOR OF THIS SITE AND CANNOT DRAIN TOWARD ANY PROPERTY LINES.

DRAINAGE & GRADING PLAN

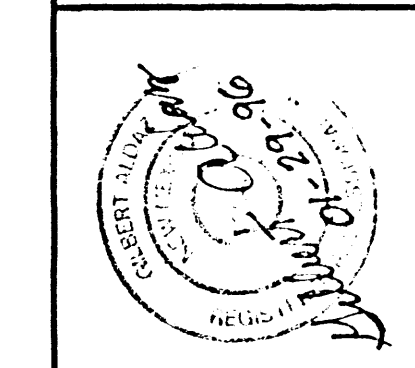
SCALE 1" = 40'



LEGAL DESCRIPTION

A CERTAIN TRACT OF LAND SITUATE WITHIN THE SOUTHWEST 1/4 OF THE SOUTHWEST 1/4 OF SECTION 32, T. 11 N. R. 4 E. N.M.P.M., BERNALILLO COUNTY, ALBUQUERQUE, NEW MEXICO AND BEING TRACT II-C BLOCK II, OF OPTIMANO J. GUTIERREZ TERRACE SUBDIVISION AS THE SAME IS SHOWN AND DESIGNATED ON THE PLAT FILED IN THE OFFICE OF THE COUNTY CLERK OF BERNALILLO COUNTY, NEW MEXICO ON JANUARY 29, 1960 IN VOLUME D2, FOLIO 41.

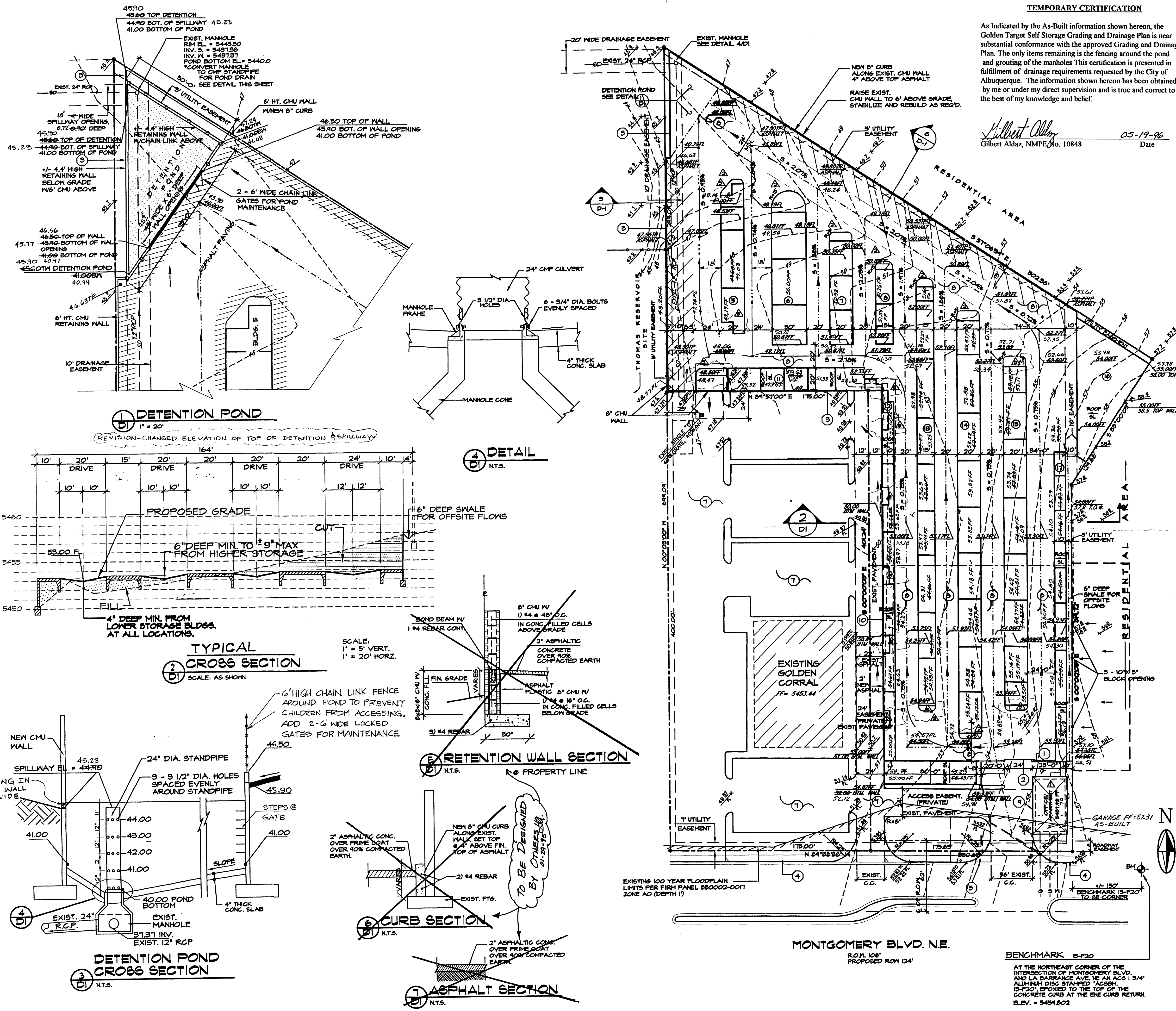
REVISION DATE
Δ 4-25-95 OFFICE/BDMLK
Δ BLDG. LAYOUT
Δ 10-10-95 FIN. GR.
Δ REVISED DETAILS
Δ ADDENDUM #2 (11/22/95)



Aldaz Engineering and Surveying, Inc.
Civil Engineering, Land Planning & Surveying
Gilbert Aldaz, P.E. & P.S.
1800 Blinn Drive NE
Albuquerque, New Mexico 87112 (505) 257-1408

DATE
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11/30/95



MAY 20 1996

