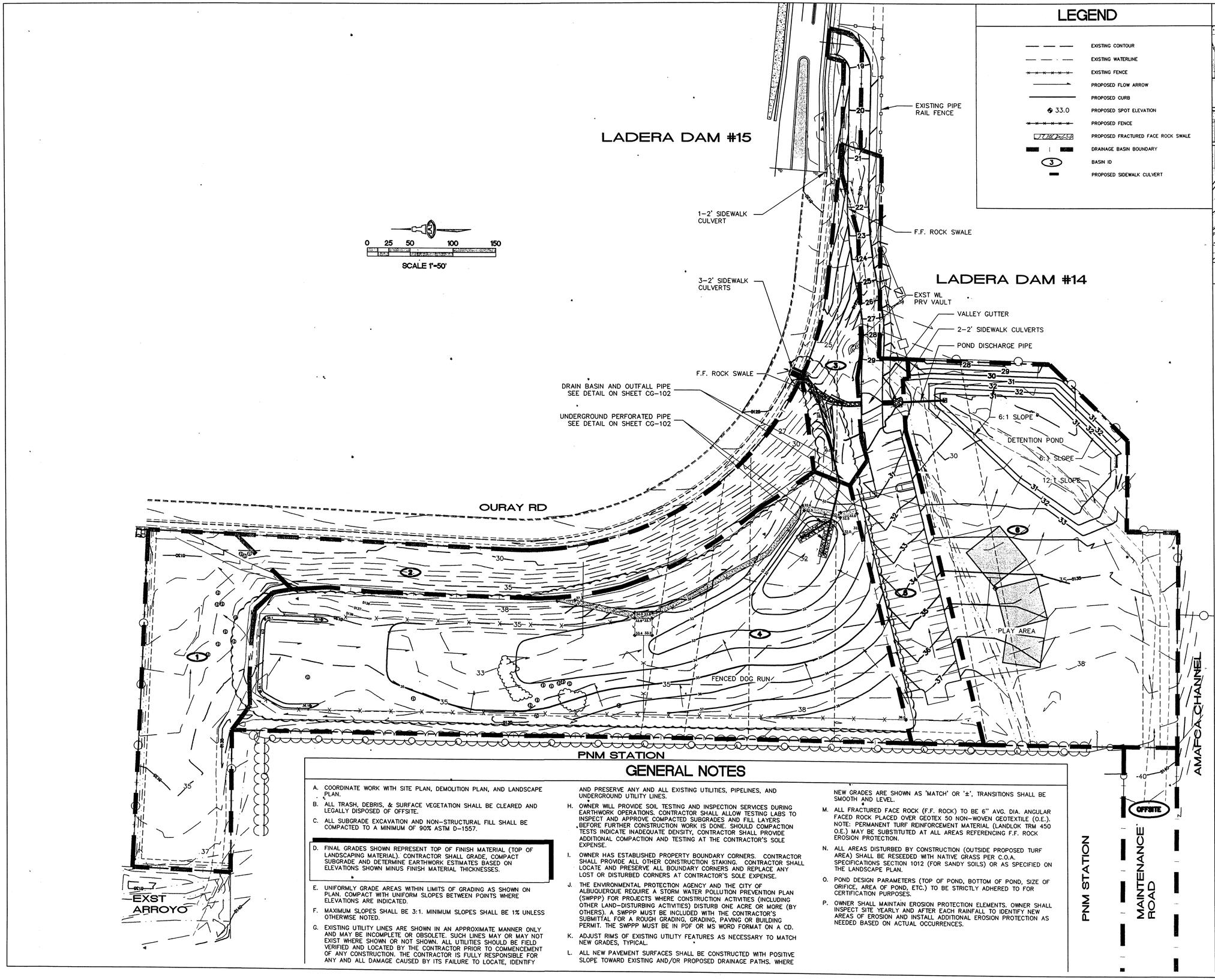
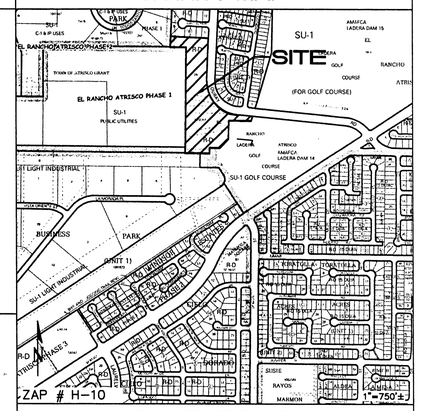


LEGEND

- EXISTING CONTOUR
- EXISTING WATERLINE
- EXISTING FENCE
- PROPOSED FLOW ARROW
- PROPOSED CURB
- 33.0 PROPOSED SPOT ELEVATION
- PROPOSED FENCE
- PROPOSED FRACTURED FACE ROCK SWALE
- DRAINAGE BASIN BOUNDARY
- 3 BASIN ID
- PROPOSED SIDEWALK CULVERT

VICINITY MAP



PROJECT DATA

LEGAL DESCRIPTION: TRACT 2-A, EL RANCHO ATRISCO

SITE AREA: 9.31± AC.

FLOOD ZONE: NO PORTION OF THIS SITE LIES WITHIN THE 100-YEAR FLOOD ZONE PER FEMA FIRM MAP NO. 35001C0326G, DATED 09/26/08.

SURVEYOR: PAIKI, A NATIVE AMERICAN A/E FIRM
2901 JUAN TABO NE, STE 101, ABQ, NM 87112
PHONE: (505) 332-1125
ATTN: PHILLIP TURNER, PLS

BENCHMARK: CITY OF ALBUQUERQUE SURVEY CONTROL 1 3/4" METALLIC DISK, STAMPED "ACS BM, 10-G10", EPOXIED TO TOP OF CURB OF THE SSW CURB RETURN AT THE INTERSECTION OF OURAY ROAD AND PRENDA DE PLATA NW. ELEV=5125.745 (NAVD 1988)

EXISTING CONDITIONS:
THIS SITE IS A 9.31 AC PARCEL THAT IS CURRENTLY UNDEVELOPED. IT IS BOUNDED TO THE EAST BY OURAY RD, TO THE NORTH BY A RESIDENTIAL NEIGHBORHOOD, TO THE WEST BY A PNM POWER STATION, AND TO THE SOUTH BY AN AMAFCA CHANNEL AND DAM.

OFFSITE FLOWS ENTER THE PROPERTY AT THE SOUTHWEST CORNER FROM THE CHANNEL MAINTENANCE ACCESS ROAD. A SMALL BERM PREVENTS STORM WATER FROM THE PNM STATION FROM ENTERING THE SITE.

PROPOSED CONDITIONS:
THIS SITE WILL BE DEVELOPED AS A DOG PARK IN SEVERAL PHASES. THE FIRST PHASE INCLUDES THE FENCED DOG RUN, AND A PARKING LOT. A FUTURE PHASE PAVES THE PARKING LOT AND ENTRY DRIVE, AND BUILDS PLAY AREAS SOUTH OF THE PARKING.

BASINS 1 & 2 AREN'T SIGNIFICANTLY ALTERED BY THIS PLAN, AND CONTINUE TO SHEET FLOW TO OURAY OVER THE CURB. STORM WATER FROM BASIN 1 DRAINS TO OURAY RD AT THE NORTHEAST CORNER OF THE PROPERTY. BASIN 3 RUN OFF EXITS THE SITE AT SIDEWALK CULVERTS WEST OF THE ENTRY DRIVE.

BASIN 4 INCLUDES A SHALLOW RUNOFF DETENTION AREA THAT INCORPORATES PERFORATED PIPE UNDERDRAINS THAT COLLECT RUNOFF AND CONVEY IT TO A MANHOLE. RUNOFF IS THEN RELEASED TO OURAY VIA PROPOSED SIDEWALK CULVERTS.

BASINS 6 AND OFFSITE ARE DETAINED IN A POND IN THE SOUTHEAST CORNER OF THE PROPERTY BEFORE CROSSING THE PARKING LOT ENTRY DRIVE. THERE THEY COMBINE WITH FLOWS FROM BASINS 3 & 5 AND DISCHARGE TO OURAY THROUGH PROPOSED SIDEWALK CULVERTS WEST OF THE ENTRY DRIVE.

GENERAL NOTES

A. COORDINATE WORK WITH SITE PLAN, DEMOLITION PLAN, AND LANDSCAPE PLAN.

B. ALL TRASH, DEBRIS, & SURFACE VEGETATION SHALL BE CLEARED AND LEGALLY DISPOSED OF OFFSITE.

C. ALL SUBGRADE EXCAVATION AND NON-STRUCTURAL FILL SHALL BE COMPACTED TO A MINIMUM OF 90% ASTM D-1557.

D. FINAL GRADES SHOWN REPRESENT TOP OF FINISH MATERIAL (TOP OF LANDSCAPING MATERIAL). CONTRACTOR SHALL GRADE, COMPACT SUBGRADE AND DETERMINE EARTHWORK ESTIMATES BASED ON ELEVATIONS SHOWN MINUS FINISH MATERIAL THICKNESSES.

E. UNIFORMLY GRADE AREAS WITHIN LIMITS OF GRADING AS SHOWN ON PLAN. COMPACT WITH UNIFORM SLOPES BETWEEN POINTS WHERE ELEVATIONS ARE INDICATED.

F. MAXIMUM SLOPES SHALL BE 3:1. MINIMUM SLOPES SHALL BE 1% UNLESS OTHERWISE NOTED.

G. EXISTING UTILITY LINES ARE SHOWN IN AN APPROXIMATE MANNER ONLY AND MAY BE INCOMPLETE OR OBSOLETE. SUCH LINES MAY OR MAY NOT EXIST WHERE SHOWN OR NOT SHOWN. ALL UTILITIES SHOULD BE FIELD VERIFIED AND LOCATED BY THE CONTRACTOR PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES.

H. OWNER WILL PROVIDE SOIL TESTING AND INSPECTION SERVICES DURING EARTHWORK OPERATIONS. CONTRACTOR SHALL ALLOW TESTING LABS TO INSPECT AND APPROVE COMPACTED SUBGRADES AND FILL LAYERS BEFORE FURTHER CONSTRUCTION WORK IS DONE. SHOULD COMPACTION TESTS INDICATE INADEQUATE DENSITY, CONTRACTOR SHALL PROVIDE ADDITIONAL COMPACTION AND TESTING AT THE CONTRACTOR'S SOLE EXPENSE.

I. OWNER HAS ESTABLISHED PROPERTY BOUNDARY CORNERS. CONTRACTOR SHALL PROVIDE ALL OTHER CONSTRUCTION STAKING. CONTRACTOR SHALL LOCATE AND PRESERVE ALL BOUNDARY CORNERS AND REPLACE ANY LOST OR DISTURBED CORNERS AT CONTRACTOR'S SOLE EXPENSE.

J. THE ENVIRONMENTAL PROTECTION AGENCY AND THE CITY OF ALBUQUERQUE REQUIRE A STORM WATER POLLUTION PREVENTION PLAN (SWPPP) FOR PROJECTS WHERE CONSTRUCTION ACTIVITIES (INCLUDING OTHER LAND-DISTURBING ACTIVITIES) DISTURB ONE ACRE OR MORE (BY OTHERS). A SWPPP MUST BE INCLUDED WITH THE CONTRACTOR'S SUBMITTAL FOR A ROUGH GRADING, GRADING, PAVING OR BUILDING PERMIT. THE SWPPP MUST BE IN PDF OR MS WORD FORMAT ON A CD.

K. ADJUST RIMS OF EXISTING UTILITY FEATURES AS NECESSARY TO MATCH NEW GRADES, TYPICAL.

L. ALL NEW PAVEMENT SURFACES SHALL BE CONSTRUCTED WITH POSITIVE SLOPE TOWARD EXISTING AND/OR PROPOSED DRAINAGE PATHS. WHERE NEW GRADES ARE SHOWN AS 'MATCH' OR '±', TRANSITIONS SHALL BE SMOOTH AND LEVEL.

M. ALL FRACTURED FACE ROCK (F.F. ROCK) TO BE 6" AVG. DIA. ANGULAR FACED ROCK PLACED OVER GEOTEX 50 NON-WOVEN GEOTEXTILE (O.E.). NOTE: PERMANENT TURF REINFORCEMENT MATERIAL (LANDLOK TRM 450 O.E.) MAY BE SUBSTITUTED AT ALL AREAS REFERENCING F.F. ROCK EROSION PROTECTION.

N. ALL AREAS DISTURBED BY CONSTRUCTION (OUTSIDE PROPOSED TURF AREA) SHALL BE RESEEDED WITH NATIVE GRASS PER C.O.A. SPECIFICATIONS SECTION 1012 (FOR SANDY SOILS) OR AS SPECIFIED ON THE LANDSCAPE PLAN.

O. POND DESIGN PARAMETERS (TOP OF POND, BOTTOM OF POND, SIZE OF ORIFICE, AREA OF POND, ETC.) TO BE STRICTLY ADHERED TO FOR CERTIFICATION PURPOSES.

P. OWNER SHALL MAINTAIN EROSION PROTECTION ELEMENTS. OWNER SHALL INSPECT SITE YEARLY AND AFTER EACH RAINFALL TO IDENTIFY NEW AREAS OF EROSION AND INSTALL ADDITIONAL EROSION PROTECTION AS NEEDED BASED ON ACTUAL OCCURRENCES.

ASA M. NILSSON-WEBER NEW MEXICO 17631 LICENSED PROFESSIONAL ENGINEER 12-15-09

ISAACSON & ARFMAN, P.A.
Consulting Engineering Associates
128 Monroe Street N.E.
Albuquerque, New Mexico 87108
Ph: 505-268-8828 Fax. 505-268-2632

1731 CG-101.dwg Dec 15,2009

OURAY DOG PARK MASTER PLAN

CITY OF ALBUQUERQUE

MASTER GRADING PLAN

No.	Revision	Date

8/6/09
Drawn By: GLD
Ckd By: GLD

Job No. 1731
CG-101
SH. OF

RECEIVED
DEC 16 2009
HYDROLOGY SECTION

M:\CAD FILES\1700-1799\1731\dwg\1731 CG-101.dwg, 12/15/2009 2:40:29 PM, thor

BASIN NO. 1	DESCRIPTION	Existing area north of fenced dog run
Area of basin flows =	48760 SF =	1.1 Ac.
The following calculations are based on Treatment areas as shown in table to the right		LAND TREATMENT
Sub-basin Weighted Excess Precipitation (see formula above)		A = 0%
Weighted E =	0.73 in.	B = 80%
Sub-basin Volume of Runoff (see formula above)		C = 20%
V_{10} =	2982 CF	D = 0%
Sub-basin Peak Discharge Rate: (see formula above)		
Q_P =	2.5 cfs	

BASIN NO. 2	DESCRIPTION	Existing slope adjacent to Ouray
Area of basin flows =	40182 SF =	0.9 Ac.
The following calculations are based on Treatment areas as shown in table to the right		LAND TREATMENT
Sub-basin Weighted Excess Precipitation (see formula above)		A = 0%
Weighted E =	0.73 in.	B = 80%
Sub-basin Volume of Runoff (see formula above)		C = 20%
V_{100} =	2458 CF	D = 0%
Sub-basin Peak Discharge Rate: (see formula above)		
Q_P =	2.0 cfs	

BASIN NO. 3	DESCRIPTION	Area between Ouray and new entry drive
Area of basin flows =	16019 SF =	0.4 Ac.
The following calculations are based on Treatment areas as shown in table to the right		LAND TREATMENT
Sub-basin Weighted Excess Precipitation (see formula above)		A = 0%
Weighted E =	0.99 in.	B = 0%
Sub-basin Volume of Runoff (see formula above)		C = 100%
V_{100} =	1322 CF	D = 0%
Sub-basin Peak Discharge Rate: (see formula above)		
Q_P =	1.1 cfs	

BASIN NO. 4	DESCRIPTION	Fenced dog run
Area of basin flows =	149344 SF =	3.4 Ac.
The following calculations are based on Treatment areas as shown in table to the right		LAND TREATMENT
Sub-basin Weighted Excess Precipitation (see formula above)		A = 0%
Weighted E =	0.99 in.	B = 0%
Sub-basin Volume of Runoff (see formula above)		C = 100%
V_{100} =	12321 CF	D = 0%
Sub-basin Peak Discharge Rate: (see formula above)		
Q_P =	9.8 cfs	

BASIN NO. 5	DESCRIPTION	Parking lot/entry road
Area of basin flows =	44466 SF =	1.0 Ac.
The following calculations are based on Treatment areas as shown in table to the right		LAND TREATMENT
Sub-basin Weighted Excess Precipitation (see formula above)		A = 0%
Weighted E =	1.73 in.	B = 15%
Sub-basin Volume of Runoff (see formula above)		C = 5%
V_{100} =	6396 CF	D = 80%
Sub-basin Peak Discharge Rate: (see formula above)		
Q_P =	4.0 cfs	

BASIN NO. 6	DESCRIPTION	South detention pond/play area
Area of basin flows =	109831 SF =	2.5 Ac.
The following calculations are based on Treatment areas as shown in table to the right		LAND TREATMENT
Sub-basin Weighted Excess Precipitation (see formula above)		A = 0%
Weighted E =	0.76 in.	B = 78%
Sub-basin Volume of Runoff (see formula above)		C = 20%
V_{100} =	6956 CF	D = 2%
Sub-basin Peak Discharge Rate: (see formula above)		
Q_P =	5.7 cfs	

BASIN NO. OFFSITE	DESCRIPTION	Offsite maintenance road
Area of basin flows =	185950 SF =	4.3 Ac.
The following calculations are based on Treatment areas as shown in table to the right		LAND TREATMENT
Sub-basin Weighted Excess Precipitation (see formula above)		A = 0%
Weighted E =	0.99 in.	B = 0%
Sub-basin Volume of Runoff (see formula above)		C = 100%
V_{100} =	15341 CF	D = 0%
Sub-basin Peak Discharge Rate: (see formula above)		
Q_P =	12.3 cfs	

HYDROGRAPH FOR SMALL WATERSHED
DPM SECTION 22-2 * PAGE A-13/14

Base time, t_B, for a small watershed hydrograph is,

$tB = \sqrt{2.107 * E * A / (Q_P) - (0.25 * A_D / A)}$

Where

E	=	0.95 inches
A	=	9.38 acres
A_D	=	0.87 acres
Q_P	=	25.1 cfs

t_B = 0.73 hours

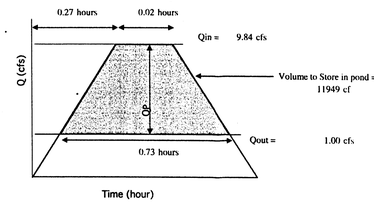
E is the excess precipitation in inches (from DPM TABLE A-8), Q_P is the peak flow, A_D is the area (acres) of treatment D, and A_T is the total area in acres. Using the time of concentration, t_C (hours), the time to peak in hours is:

$t_P = (0.7 * tC) + ((1.6 - (A_D / A)) / 12)$

Where t_C = 0.20 hours

t_P = 0.27 hours

Continue the peak for 0.25 * A_D / A_T hours. When A_D is zero, the hydrograph will be triangular. When A_D is not zero, the hyrograph will be trapezoidal. see the graph below:



DOG RUN POND
INFLOW / OUTFLOW HYDROGRAPH

HYDROGRAPH FOR SMALL WATERSHED
DPM SECTION 22-2 * PAGE A-13/14

Base time, t_B, for a small watershed hydrograph is,

$tB = (2.107 * E * A / Q_P) - (0.25 * A_D / A)$

Where

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A	=	9.38 acres
A_D	=	0.87 acres
Q_P	=	25.1 cfs

t_B = 0.73 hours

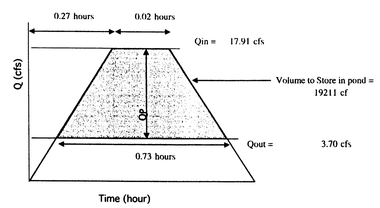
E is the excess precipitation in inches (from DPM TABLE A-8), Q_P is the peak flow, A_D is the area (acres) of treatment D, and A_T is the total area in acres. Using the time of concentration, t_C (hours), the time to peak in hours is:

$t_P = (0.7 * tC) + ((1.6 - (A_D / A)) / 12)$

Where t_C = 0.20 hours

t_P = 0.27 hours

Continue the peak for 0.25 * A_D / A_T hours. When A_D is zero, the hydrograph will be triangular. When A_D is not zero, the hyrograph will be trapezoidal. see the graph below:



OFFSITES POND
INFLOW / OUTFLOW HYDROGRAPH

DOG RUN POND		
Contour	Area	Volume
5130.50	925	
5132.60	12298	13884 CF
TOTAL VOL.		13884 CF

ACTUAL WSEL:
Required Volume = 11949 CF
WSEL = 5132.45

OFFSITES POND

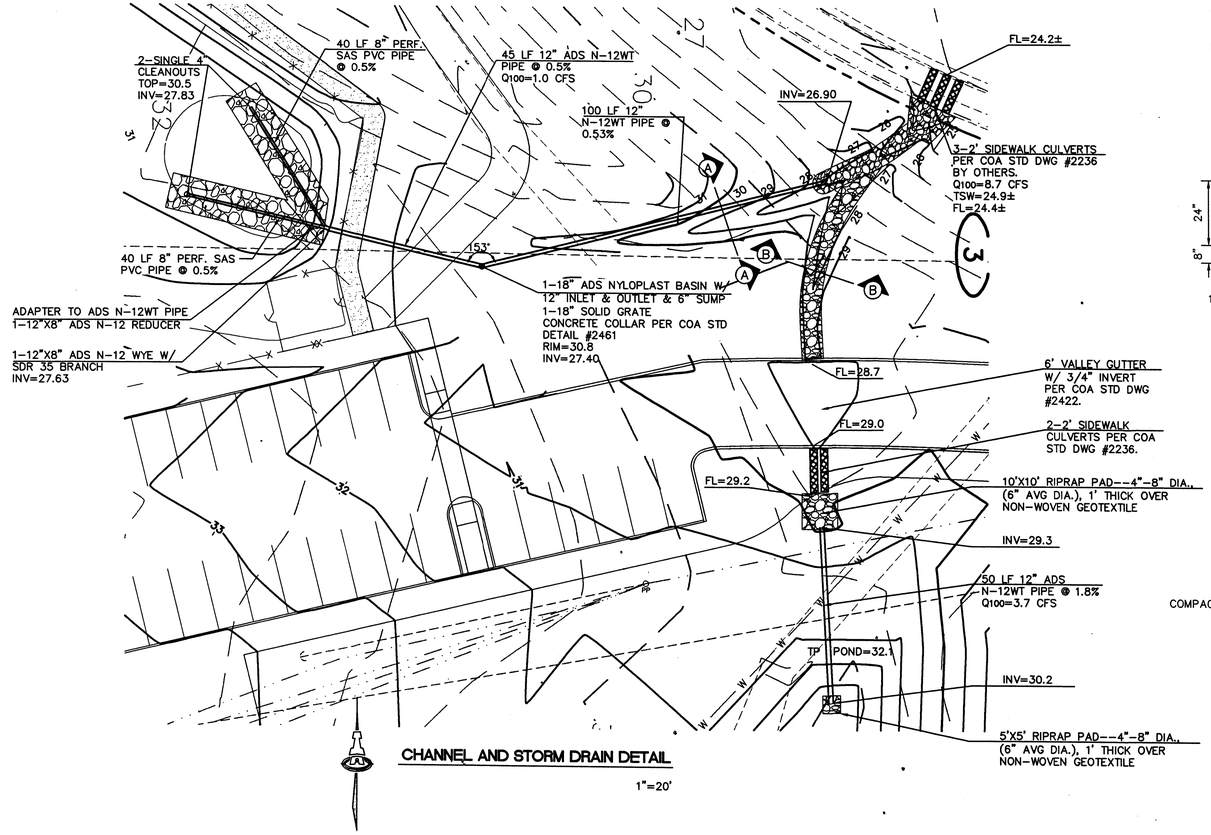
GENERAL STORM DRAIN NOTES

A. ALL STORM DRAIN PRODUCT TO BE FURNISHED BY CONTRACTOR.

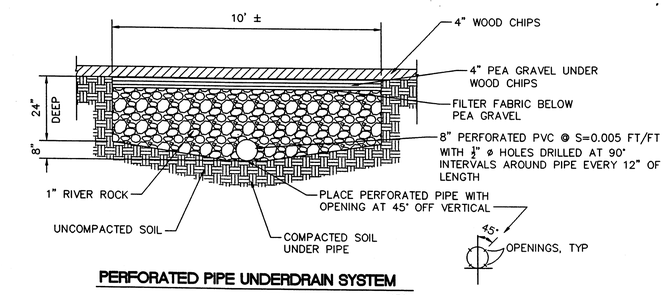
B. INSTALL ALL STORM DRAIN PIPE, FITTINGS AND DRAIN BASIN PER MANUFACTURER'S REQUIREMENTS.

C. ALL NEW STORM DRAIN LINE IS TO BE INSTALLED AT RIM AND INVERT ELEVATIONS SHOWN.

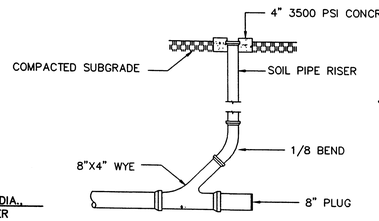
D. ALL STORM DRAIN TO BE A.D.S. N-12WT (WATERTIGHT) PIPE OR APPROVED EQUAL, EXCEPT PERFORATED PIPE THAT SHALL BE 8" PVC SAS PIPE.



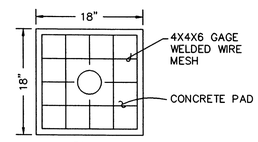
CHANNEL AND STORM DRAIN DETAIL
1"=20'



PERFORATED PIPE UNDERDRAIN SYSTEM
NTS

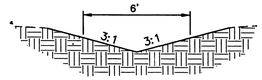


4" SINGLE CLEANOUT DETAIL
NTS

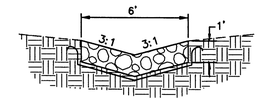


PLAN

CONCRETE PAD FOR SAS CLEANOUT
SCALE: 1"=1'



SECTION A-A
1' DEEP EARTHEN SWALE
NTS



- VARY COBBLE SIZE BETWEEN 4" AND 8" DIA. (AVG.=6")
- PLACE GEOTEX 501 NON-WOVEN GEOTEXTILE (O.E.)BENEATH COBBLE SWALE PER MANUFACTURER'S RECOMMENDATIONS.

SECTION B-B
1' DEEP COBBLE LINED SWALE
NTS

ASA M. NILSSON-WEBER NEW MEXICO 17631 LICENSED PROFESSIONAL ENGINEER 12-15-09

ISAACSON & ARFMAN, P.A.
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128 Monroe Street N.E.
Albuquerque, New Mexico 87108
Ph. 505-268-8828 Fax. 505-268-2632
1731 CG-501.dwg Dec 15,2009

OURAY DOG PARK MASTER PLAN

CITY OF ALBUQUERQUE

GRADING DETAILS

No.	Revision	Date

8/6/09
Drawn By: GLD
Ckd By: GLD

Job No. 1731
CG-102
SH. OF

RECEIVED
DEC 16 2009
HYDROLOGY SECTION

M:\CAD FILES\1700-1799\1731\dwg\1731 CG-501.dwg, 12/15/2009 2:42:01 PM, thor

PROJECT DATA

LEGAL DESCRIPTION: TRACT 2-A, EL RANCHO ATRISCO

SITE AREA: 9.31± AC.

FLOOD ZONE: NO PORTION OF THIS SITE LIES WITHIN THE 100-YEAR FLOOD ZONE PER FEMA FIRM MAP NO. 35001C0326G, DATED 09/26/08.

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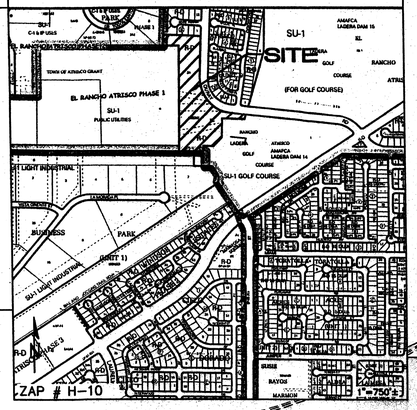

DRAINAGE CERTIFICATION

I, Asa M. Nilsson-Weber, NMPE #17631, of the firm Isaacson & Arfman, P.A., hereby certify that Phase 1 of this project has been graded and will drain in substantial compliance with and in accordance with the design intent of the approved master plan dated 12/15/09.

The record information edited onto the original design document has been obtained by Rex Vogler, NMPS #10466, of the firm Rio Grande Surveying Co., P.C. I further certify that I have personally visited the project site on 11/05/10 and have determined by visual inspection that the survey data provided is representative of actual site conditions and is true and correct to the best of my knowledge and belief.

The record information presented hereon is not necessarily complete and intended only to verify substantial compliance of the grading and drainage aspects of this project. Those relying on this record document are advised to obtain independent verification of its accuracy before using it for any other purpose.

Asa M. Nilsson-Weber NMPE #17631

11-08-10
Date

ASA M. NILSSON-WEBER NEW MEXICO 17631 LICENSED PROFESSIONAL ENGINEER

LEGEND

- 6135 — EXISTING CONTOUR
- EXISTING WATERLINE
- EXISTING FENCE
- 35 — PROPOSED CONTOUR
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- PROPOSED CURB
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L. ALL NEW PAVEMENT SURFACES SHALL BE CONSTRUCTED WITH POSITIVE SLOPE TOWARD EXISTING AND/OR PROPOSED DRAINAGE PATHS. WHERE NEW GRADES ARE SHOWN AS 'MATCH' OR '±', TRANSITIONS SHALL BE SMOOTH AND LEVEL.

M. ALL FRACTURED FACE ROCK (F.F. ROCK) TO BE 6" AVG. DIA. ANGULAR FACED ROCK PLACED OVER GEOTEX 50 NON-WOVEN GEOTEXTILE (O.E.). NOTE: PERMANENT TURF REINFORCEMENT MATERIAL (LANDLOK TRM 450 O.E.) MAY BE SUBSTITUTED AT ALL AREAS REFERENCING F.F. ROCK EROSION PROTECTION.

N. ALL AREAS DISTURBED BY CONSTRUCTION (OUTSIDE PROPOSED TURF AREA) SHALL BE RESEEDED WITH NATIVE GRASS PER C.O.A. SPECIFICATIONS SECTION 1012 (FOR SANDY SOILS) OR AS SPECIFIED ON THE LANDSCAPE PLAN.

O. POND DESIGN PARAMETERS (TOP OF POND, BOTTOM OF POND, SIZE OF ORIFICE, AREA OF POND, ETC.) TO BE STRICTLY ADHERED TO FOR CERTIFICATION PURPOSES.

P. OWNER SHALL MAINTAIN EROSION PROTECTION ELEMENTS. OWNER SHALL INSPECT SITE YEARLY AND AFTER EACH RAINFALL TO IDENTIFY NEW AREAS OF EROSION AND INSTALL ADDITIONAL EROSION PROTECTION AS NEEDED BASED ON ACTUAL OCCURRENCES.

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1731 CG-101-PHASE 1.dwg Jan 25,2010

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NOV 08 2010

CITY OF ALBUQUERQUE
PARK DESIGN AND CONSTRUCTION DIVISION
DEPARTMENT OF MUNICIPAL DEVELOPMENT

OURAY OFF-LEASH DOG PARK

PHASE 1 GRADING PLAN

Design Review Committee	City Engineer Approval	Lost Design Update	Mo./Day/Yr.	Mo./Day/Yr.

City Project No. 585691 | Zone Map No. H-10 | Sheet 4 OF 9

REVISIONS

NO.	DATE	REMARKS	BY
DESIGNED BY	ÅNW	DATE	10/27/09
DRAWN BY	JTS	DATE	10/27/09
CHECKED BY	ÅNW	DATE	10/27/09

AS-BUILT INFORMATION
CONTRACTOR
WORK REVIEWED BY DATE
INSPECTORS DATE
ACCEPTANCE BY
FIELD VERIFICATION BY DATE
DOWNREGS
RECORDED BY DATE
MICRO-FILM INFORMATION
RECORDED BY DATE
NO.

BENCH MARKS

SURVEY INFORMATION
FIELD NOTES
NO. BY DATE

ENGINEER'S SEAL
ASA M. NILSSON-WEBER NEW MEXICO 17631 LICENSED PROFESSIONAL ENGINEER
1-25-10

FINAL FOR CONSTRUCTION
DATE: JANUARY 25, 2010

Ouray Off-Leash Dog Park - Phase One

PROJECT# 585691

RECORD DRAWINGS
DATE:

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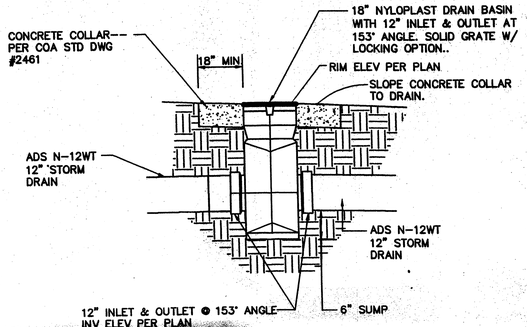


DRAIN BASIN DETAIL

NTS

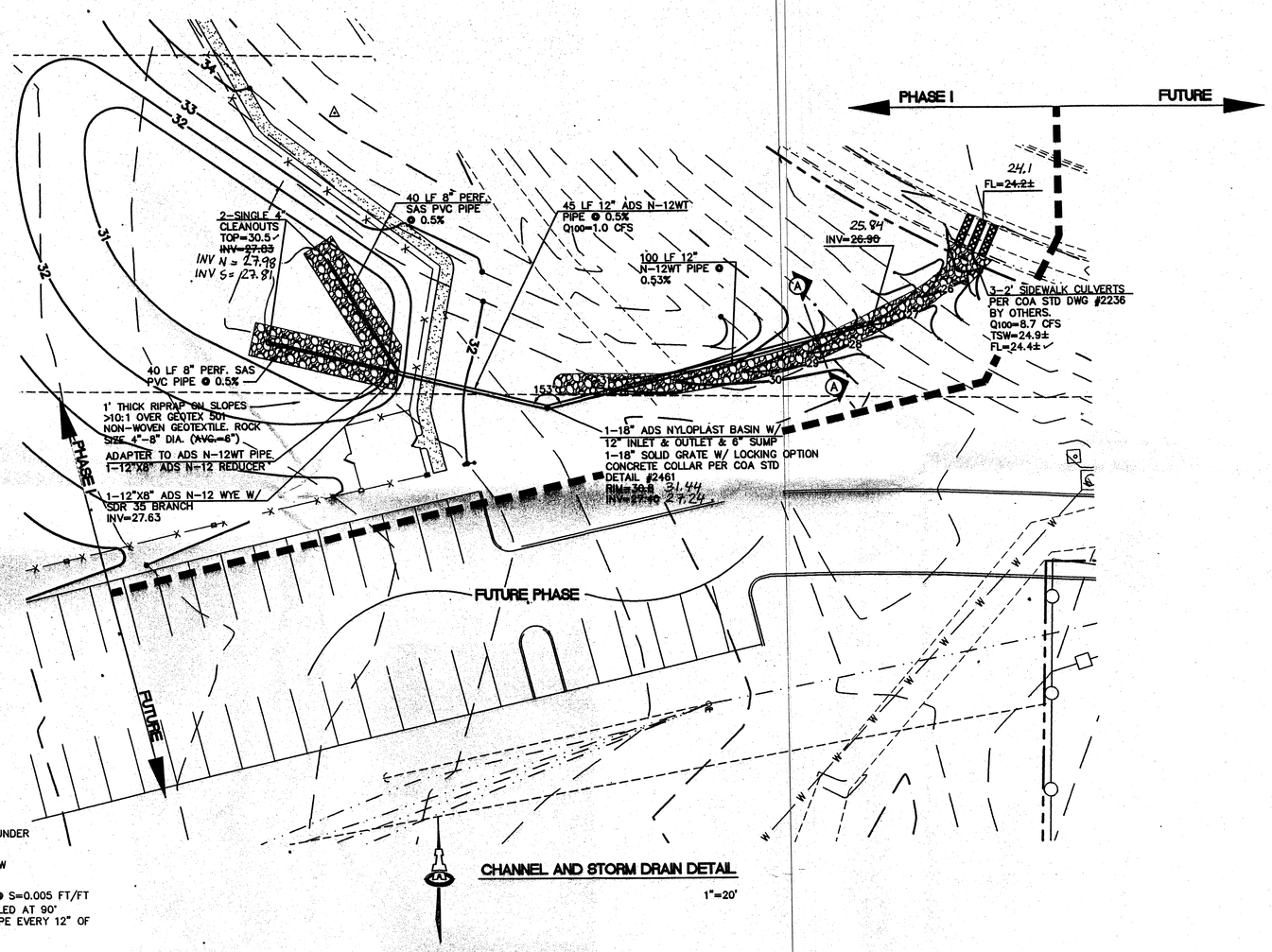
GENERAL STORM DRAIN NOTES

A. ALL STORM DRAIN PRODUCT TO BE FURNISHED BY CONTRACTOR.

B. INSTALL ALL STORM DRAIN PIPE, FITTINGS AND DRAIN BASIN PER MANUFACTURER'S REQUIREMENTS.

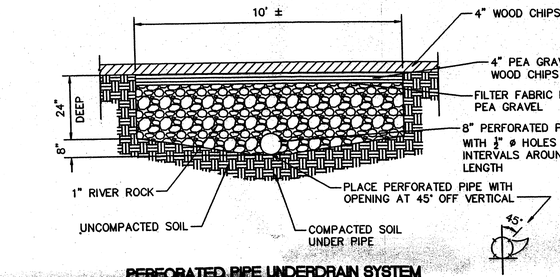
C. ALL NEW STORM DRAIN LINE IS TO BE INSTALLED AT RIM AND INVERT ELEVATIONS SHOWN.

D. ALL STORM DRAIN TO BE A.D.S. N-12WT (WATERTIGHT) PIPE OR APPROVED EQUAL, EXCEPT PERFORATED PIPE THAT SHALL BE 8" PVC SAS PIPE.



CHANNEL AND STORM DRAIN DETAIL

1"=20'



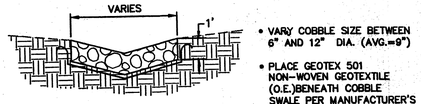
PERFORATED PIPE UNDERDRAIN SYSTEM

NTS

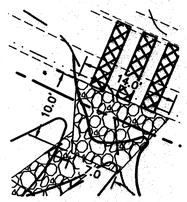
- VARY COBBLE SIZE BETWEEN 4" AND 8" DIA. (AVG.=6")
- PLACE GEOTEX 501 NON-WOVEN GEOTEXTILE (O.E.) BENEATH COBBLE SWALE PER MANUFACTURER'S RECOMMENDATIONS.

SECTION A-A
1' DEEP COBBLE LINED SWALE

NTS

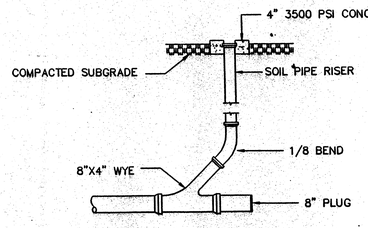


- VARY COBBLE SIZE BETWEEN 6" AND 12" DIA. (AVG.=9")
- PLACE GEOTEX 501 NON-WOVEN GEOTEXTILE (O.E.)BENEATH COBBLE SWALE PER MANUFACTURER'S RECOMMENDATIONS.



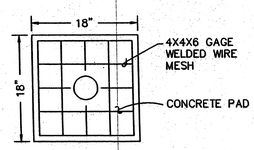
1' DEEP COBBLE EROSION PROTECTION AT BACK OF SIDEWALK CULVERTS

NTS



4' SINGLE CLEANOUT DETAIL

NTS



PLAN

CONCRETE PAD FOR SAS CLEANOUT

SCALE:1"=1'

AS-BUILT INFORMATION
CONTRACTOR
WORK INSPECTOR'S ACCEPTANCE BY DATE
VERIFICATION BY DATE
DRAWINGS POSTED BY DATE
MICRO-FILM INFORMATION
RECORDED BY DATE
NO.

BENCH MARKS

SURVEY INFORMATION
FIELD NOTES BY DATE
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ENGINEER'S SEAL

NEW MEXICO 17631 REGISTERED PROFESSIONAL ENGINEER ASA W. NILSSON-WEBER
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Ouray Off-Leash Dog Park - Phase One

PROJECT# 585691

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10/27/09
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NOV 08 2010

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PARK DESIGN AND CONSTRUCTION DIVISION
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OURAY OFF-LEASH DOG PARK

PHASE 1 GRADING AND DRAINAGE DETAILS

Design Review Committee	City Engineer Approval	Last Design Update	Mo./Day/Yr.	Mo./Day/Yr.

City Project No.	Zone Map No.	Sheet
585691	H-10	5 OF 9

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