DRAINAGE REPORT

TRACT A-6

SUMMARY PLAT OF TRACTS A-6, A-33, A-34 & N¹₂, A-32

NORTHEAST UNIT

TOWN OF ATRISCO GRANT

LADERA SHOPPING CENTER

Prepared for

American Southwest Development Company Incorporated

Albuquerque, New Mexico

MARCH, 1980

Prepared by
Fred Denney & Associates, Inc.
2400 Comanche Road, N.E.
Albuquerque, New Mexico 87107

RECEIPT)

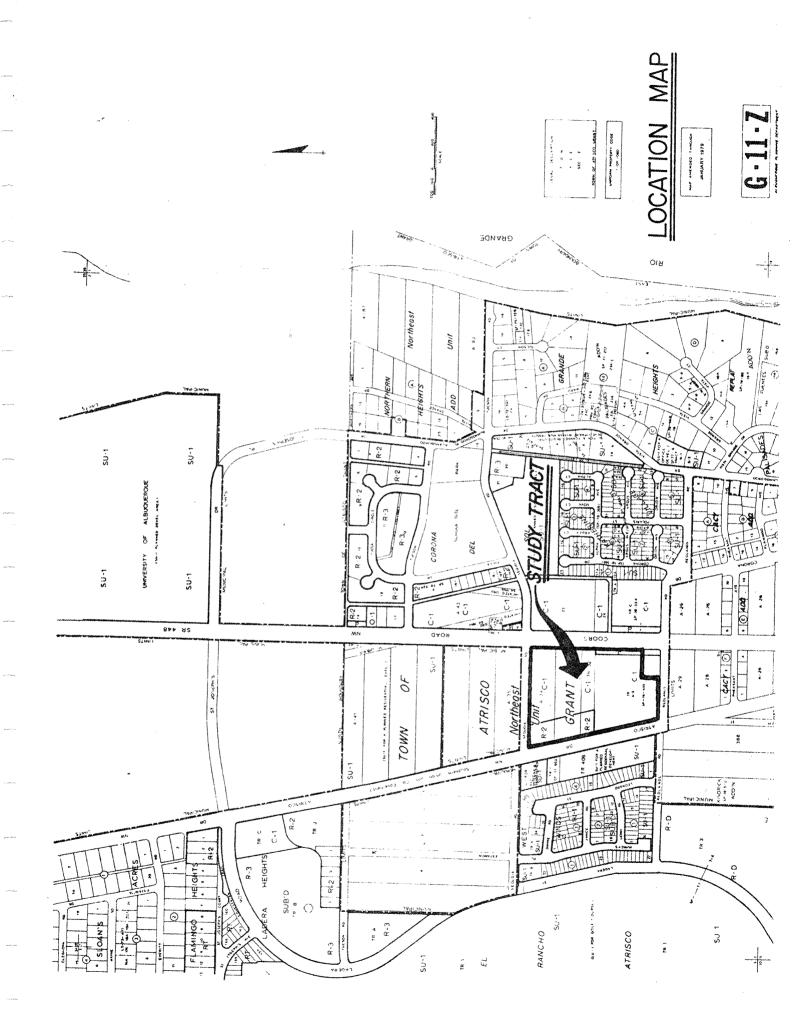
MAR 2 4 1980

CITY ENGINEER

I, Louis W. Gross, hereby certify that the enclosed documents and drawings were prepared under my supervision and are true and correct to the best of my knowledge and belief.

Wew Mexico Registered Professional Engineer and Land Surveyor No. 4100





Drainage Report
Tract A-6
Summary Plat of Tracts A-6, A-33, A-34 & N½, A-32
Northeast Unit
Town of Atrisco Grant
Ladera Shopping Center

I. GENERAL:

The 14.9925 acre tract involved in this study is located as shown on the Location Map (G-11). A copy of the summary plat for Tract A-6 is made a part of this report. A shopping center is planned for the site (see Site and Grading Plan).

No outside drainage enters the site at present. Also, no rain which falls on the site leaves the site. Due to the absence of a storm drainage system within a reasonable distance of the tract, all rainfall must be retained on the tract. In accordance with the policy of the City of Albuquerque, 2.2 inches of rain which is a 6-hour, 100-year rainfall was used to compute the volumes of the storage ponds. There are no outlets to drain the ponds. To conserve usable space for parking, some of the ponds will back up onto the parking lot during the 6-hour, 100-year event.

The soils in the area from the surface down to 2 to $4\frac{1}{2}$ feet are predominantly silty to slightly clayey fine sand (SM-SC). Below this layer is a layer of calcareous cemented silty to clayey sand from $1\frac{1}{2}$ feet to 4 feet thick.

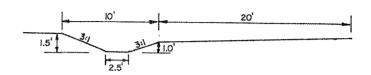
II. CALCULATIONS (Straight Volumetric)

D.A. No. 1

Area = 20,530 sf

Storage Volume Needed: $20,530 \times (2.2/12) = 3,764 \text{ cf}$

Pond No. 1



$$A = (10 + 7/2)(0.5) + (7 + 2.5/2)(1.0) + (.5 * 20/2)$$
$$= (4.25 + 4.75 + 5.0) = 14.0$$

L = 270'

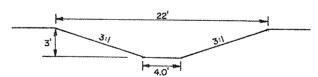
 $V = 14.0 \times 270 = 3,780 \text{ cf}$ O.K.

D.A. No. 2

Area = 205,700 sf

Storage Volume Needed = $205,700 \times (2.2/12) = 37,712 \text{ cf}$

Pond No. 2A



$$A = (22 + 4/2) = 13.0 \times 3 = 39.0$$

L = 970'

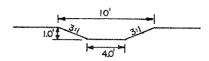
$$V = 39.0 \times 970' = 37,830 \text{ cf}$$
 0.K.

D.A. No. 3

Area = 3,600 sf

Storage Volume Needed = $3,600 \times (2.2/12) = 660 \text{ cf}$

Pond No. 3



$$A = (10 + 4/2)(1) = 7$$

L = 94'

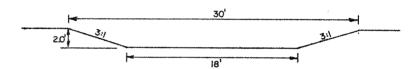
 $V = 7 \times 94' = 658 \text{ cf}$ O.K.

D.A. No. 4

Area = 33,500 sf

Storage Volume Needed = $33.500 \times (2.2/12) = 6.142 \text{ cf}$

Pond No. 4



$$A = (30 + 18/2)(2) = 48$$

L = 128'

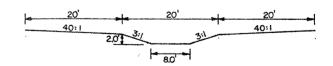
 $V = 48 \times 128' = 6,144 \text{ cf}$ O.K.

D.A. No. 5

Area = $185 \times 220 = 40,700 \text{ sf}$

Storage Volume Needed: $40,700 \times (2.2/12) = 7,462 \text{ cf}$

Pond No. 5



 $A = (20 + 8/2)(2) + (\frac{1}{2} \times .5 \times 20) + (\frac{1}{2} \times .5 \times 20) + (.5 \times 20)$ = 48 sf

$$L = 170'$$

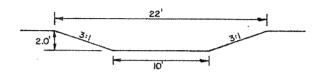
$$V = 48 \times 170' = 8,160 \text{ cf}$$
 O.K.

D.A. No. 6

Area = $150 \times 180 = 27,000 \text{ sf}$

Storage Volume Needed = $27,000 \times (2.2/12) = 4,950 \text{ cf}$

Pond No. 6



$$A = (22 + 10/2)(2) = 32 \text{ sf}$$

$$L = 164'$$

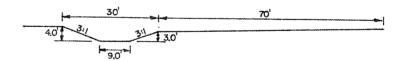
$$V = 32 \times 164' = 5,248 \text{ cf}$$
 O.K.

D.A. No. 7

Area = 96,000 sf

Storage Volume Needed = $96,000 \times (2.2/12) = 17,600 \text{ cf}$

Pond No. 7A



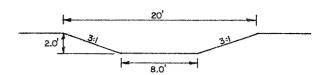
$$A = (30 + 27/2)(1) + (27 + 9/2)(3) + (\frac{1}{2} \times 1 \times 70)$$

$$= 117.5 cf$$

$$L = 126'$$

$$V = 117.5 \times 126 = 14,805 \text{ cf}$$

Pond No. 7B



$$A = (20 + 8/2)(2) = 28 \text{ sf}$$

L = 108'

$$V = 28 \times 108' = 3,024 \text{ cf}$$

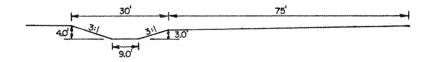
$$V_{7A} + V_{7B} = 14,805 + 3,024 = 17,829 \text{ cf}$$
 O.K.

D.A. No. 8

Area = 145,280 sf

Storage Volume Needed = $145,280 \times (2.2/12) = 26,635 \text{ cf}$

Pond No. 8



$$A = (30 + 27/2)(1) + (\frac{1}{2} \times 1 \times 75) + (27 + 9/2)(3)$$

= 120 sf

L = 222'

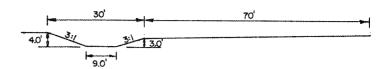
$$V = 120 \times 222' = 26,640 \text{ cf}$$
 O.K.

D.A. No. 9

Area = 81,500 sf

Storage Volume Needed = 81,500 x (2.2/12) = 14,942 cf

Pond No. 9



$$A = (30 + 27/2)(1) + (27 + 9/2)(3) + (\frac{1}{2} \times 1 \times 70) = \frac{1}{2}$$

$$= 117.5$$

$$L = 130'$$

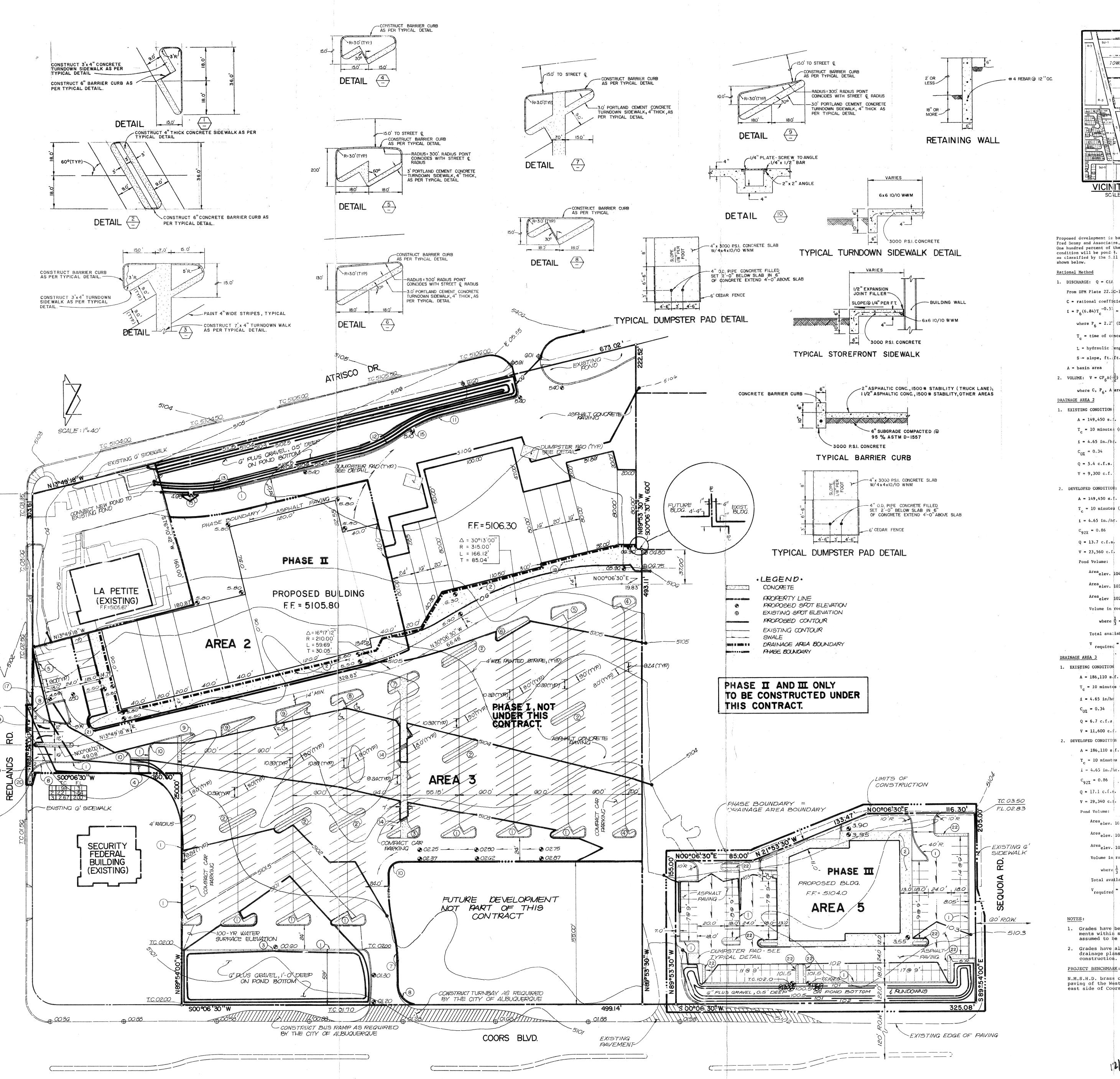
$$V = 117.5 \times 130' = 15,275 \text{ cf}$$
 O.K.

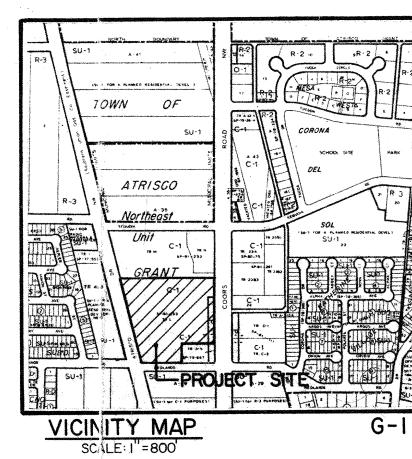
DRAINAGE INFORMATION SHEET

Ladera Shopping Center Ladera 6 Theatre, Goodyear

PROJECT TITLE: Shop "J", Shop "K"

ZONE ATLAS/DRNG.FILE #: G-11-8 LEGAL DESCRIPTION: Replat tr. M, N, and L of corrected summary plat of tracts L-N northeast unit - Town of Atrisco Grant to TR. M-A, N-A, O, P, Q and R CITY ADDRESS: 3301 Coors, N.W. ENGINEERING FIRM: Chavez-Grieves CONTACT: Mr. Vic Chavez ADDRESS: 5420 Montgomery, N.E. PHONE: 881-7376 CONTACT: Mr. Stu Sherman OWNER: Ladera Development Assoc. ADDRESS: 20 First Plaza, Suite 402 PHONE: 243-9511 CONTACT: Mr. George Rainhart ARCHITECT: de la Torre-Rainhart ADDRESS: 6121 Indian School Road, N.E. PHONE: 881-0550 SURVEYOR: DMJM CONTACT: ADDRESS: 4055 Montgomery Blvd, N.E. PHONE: 881-1803 CONTACT: Nick Wallin CONTRACTOR: Wallin Construction Co. PHONE: 292-5500 ADDRESS: P. O. Box 13596 PRE-DESIGN MEETING: DRB NO. 84-833 X YES NO EPC NO. Z-79-78-1 PROJ. NO. COPY OF CONFERENCE SHEET PROVIDED TYPE OF SUBMITTAL: CHECK TYPE OF APPROVAL SOUGHT: DRAINAGE REPORT SKETCH PLAT APPROVAL DRAINAGE PLAN PRELIMINARY PLAT APPROVAL CONCEPTUAL GRADING & DRAINAGE PLAN SITE DEVELOPMENT PLAN APPROVAL GRADING PLAN FINAL PLAT APPROVAL x BUILDING PERMIT APPROVAL EROSION CONTROL PLAN FOUNDATION PERMIT APPROVAL ENGINEER'S CERTIFICATION CERTIFICATE OF OCCUPANCY APPROVAL ROUGH GRADING PERMIT APPROVAL GRADING/PAVING PERMIT APPROVAL (SPECIFY) 'TTED: February 🕏





DRAINAGE PLAN

Proposed development is based upon the approved drainage report prepared by Fred Denny and Associates, Inc., for American Southwest Development Co., Inc. One hundred percent of the 100-year, 6-hour runoff in the fully developed condition will be pond 1. The existing soil is Hydrologic Soil Group B, as classified by the 5.11 Conservation Service. Supporting calculations are shown below.

1. DISCHARGE: Q = CiA

From DPM Plate 22.2C-1,

C = rational coefficient (function of '% impervious') $1 = P_6(6.84)T_c^{-0.5} \approx 4.65 \text{ in/hr; (100-yr., 6-hr.)}$

where P₆ = 2.2' (DPM Plate 22.2D-1)

 T_c = time of concentration = 0.0078L^{0.77}s^{-0.385}

L = hydraulic ength, ft.

S = slope, ft. ft

A = basin area

where C, P6, A are same as above

DRAINAGE AREA 2

1. EXISTING CONDITION

A = 149,450 s.T = 10 minute (minimum)

1 = 4.65 in./h.

 $c_{0%} = 0.34$

Q = 5.4 c.f.s.

V = 9,300 c.f.

A = 149,450 s.

T = 10 minutes (minimum)

1 = 4.65 in./hr.

 $c_{92\%} = 0.86$

Q = 13.7 c.f.s.V = 23,560 c.f.

Pond Volume:

Area elev. 104.0 = 16,820 s.f.

Volume = 16,090 c.f. Volume = 7,530 c.f.

Volume in rock bed = $(\frac{1}{3})(14,760)0.5 = 2,460 \text{ c.f.}$

where $\frac{1}{3}$ = fraction of voids in rock

Total available ponding volume = 26,080 c.f.

v required V100, developed 23,560 c.f.

DRAINAGE AREA 3 1. EXISTING CONDITION

A = 186,110 s.f

T = 10 minutes (minimum)

1 = 4.65 in/h $c_{0\%} = 0.34$

Q = 6.7 c.f.a V = 11,600 c.f.

2. DEVELOPED CONDITION

A = 186,110 s.f.

T = 10 minutes (minimum)

1 = 4.65 in./hr.

 $C_{92\%} = 0.86$ Q = 17.1 c.f.a.

V = 29,340 c.s.

Pond Volume:

Area elev. 101.5 = 25,970 s.f. Area elev. 101.0 = 21,020 s.f.

Area elev. 100.0 = 9,200 s.f.

Volume in rock bed = $(\frac{1}{3})(9,200)1 = 3,065$

where $\frac{1}{3}$ = fraction of voids in rock

Total available ponding volume = 29,923 c.f. $v_{\text{required}} = v_{100}$, developed = 29,340 c.f.

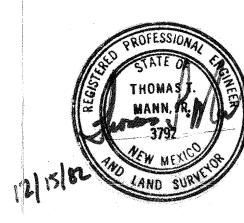
1. Grades have been coordinated with street grades and improvements within street R.O.W. (part of City Project #1584 assumed to be existing).

2. Grades have also been coordinated with approved grading and drainage plans for La Petite and Safeway, currently under construction.

Volume = 11,750 c.f.

Volume = 15,110 c.f.

N.M.S.H.D. brass cap marked "NM448-N3" located on the west edge of paving of the West Mesa Bowling Alley parking lot located on the east side of Coors Road. Elevation = 5099.53 feet (MSLD)



KEYED NOTES

- 1 CONSTRUCT CONCRETE BARRIER CURB AS PER TYPICAL
- (2) 6" STOREFRONT SIDEWALK. SEE TYPICAL STAIL. SEE ARCHITECTURAL NOTES FOR JOINT LABOUT AND PLANTER DETAILS.
- (3) CONSTRUCT 5' 0" CURB OPENING.
- (4) CONSTRUCT 2' WIDE CONCRETE ALLEY GUTTER AS PER CITY OF ALBUQ. STD. DWG. P-8-3.
- (5) CONSTRUCT RETAINING WALL. SEE TYPICAL DETAIL.
- 6 400' RADIUS.
- (7) 428' RADIUS.
- 8 25' RADIUS.
- 9 3' RADIUS. 10 10' RADIUS.
- 11 180' RADIUS.
- (12) 110' RADIUS.
- (13) 160' RADIUS.
- (14) SIDEWALK GUTTER BOX, SEE DETAIL 10
- (15) CONSTRUCT 5' 0" CURB OPENING. (16) RAMP SIDEWALK TO MATCH EXISTING OVER A DISTANCE
- (17) RAMP SIDEWALK.
- (18) BIKE RACK FOR 10 BIKES. LOCATION TO BE COORDINATED
- WITH ARCHITECT.

(19) P.C.C. VALLEY GUTTER AS PER CITY OF ALBUQ. STD. DWG. P-9-2

- 20) PAINTED ARROWS AND LANE STRIPING DIRECTLY ON PAVING.
- (21) 40' x 4' x 4" P.C.C. MEDIAN.
- (22) 3'x4" TURNDOWN SIDEWALK SEE TYPICAL DETAIL
 - GENERAL NOTES
- 1 ALL CONSTRUCTION SHALL COMPLY WITH THE CITY OF ALBUQUERQUE "CONTRACT DOCUMENTS FOR CITY-WIDE UTILITIES AND CASH PAVING NO. 31."
- BY CALLING 765-1234 48 HOURS BEFORE COMMENCING WORK IN AREAS NEAR UNDERGROUND UTILITY LINES. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROTECT AND MAINTAIN IN SERVICE ALL EXISTING UTILITIES THE ENGINEER SHALL BE PROMPTLY NOTIFIED OF ANY

PROBLEMS OR CONFLICTS THAT ARE ENCOUNTERED.

2 THE CONTRACTOR SHALL NOTIFY THE UTILITY COMPANIES

- 3 ALL CONCRETE SHALL BE 3000 PSI CONCRETE WITH 3/4" AGGREGATE
- 4 ALL ASPHALT CONCRETE SHALL BE 1 1/2" EXCEPT WHERE NOTED.

DRAINAGE AREA 5

1. EXISTING CONDITION

A = 59,300 sf = 1.36 Ac

T = 10 minutes (minimum) i = 4.65 in/hr

 $C_{0\%} = 0.34$

Q = 2.2 cfsV = 3695 cf

2. <u>DEVELOPED CONDITION</u>

A - 59,300 sf = 1.36 AcT = 10 minutes (minimum)

i = 4.65 in/hr $c_{7.9\%} = 0.76$

Q = 4.8 cfs

V = 8260 cf

Area elev. 100.5 = 3150 si

Σ Vol. = 8860 cf

Volume = 1690 c

LADERA SHOPPING CENTER

SHOPS A-H job no. |date 6280 12-1-82

Isheet title PAVING AND GRADING PLAH

6121 Indian school road ne suite 1218 albuquerque n.m. 87110/5058810550

REPLAT

TRACTS M, N & L OF CORRECTED SUMMARY PLAT OF

TRACTS L, M AND N; NORTHEAST UNIT TOWN OF ATRISCO GRANT

TRACTS M-A, N-A, O, P, Q & R

APRIL, 1984 DESCRIPTION

The foregoing replac of that certain tract of land situate within the Town of Atrisco Grant, City of Albuquerque, County of Dernolillo, You Mexico, located within Section 2, Township 10 North Range 2 Last, N.M.P.M, being and comprising all of Tract L,M and N; as the same is shown and designated on the CORRECTED SUMMARY PLAT OF TRACTS L,M AND N, NORTHEAST UNIT, TOWN OF ATRISCO GRANT, filed in the Office of the County Clerk of Bernalillo County, New Mexico on August 25, 1983 in Volume: C-22; Folto: 2.

STATEMENT and AFFIDAVIT of OWNER

To the extent that strom drainage waters are not contained within the boundaries of the Six Tracts depicted on this Plat, the Owner(s) hereby grant reciprocal easements in and among the Six Tracts depicted for purposes of storm drainage in a manner substantially in accordance with the Final Drainage Plan as shall be submitted to the City of Albuquerque by Owner(s). Owner(s), its Successors and Assigns, further agree to maintain the existing drainage ponds previously approved by the City of said Tracts, including the drainage ponds on Tract R.

LADERA DEVELOPMENT ASSOCIATES

by: AMERICAN SOUTHWEST DEVELOPMENT COMPANY

SHERMAN, VICE PRESIDENT

STATE OF NEW MEXICO COUNTY OF BERNALILLO

The foregoing instrument was acknowledged before me this 19 th day of 1984, by Stuart C. Sherman.

Notary Public Share Catan

APPROVALS

PLANNING DIRECTOR, CITY OF ALBUQUERQUE, NEW MEXICO TIVE ENGINEER, AMAFCA CITY OF ALBUQUERQUE, NEW MEXICO AND RECREATION, CITY OF ALBUQUERQUE, NM 6/36/84 De Com PROPERTY MANAGEMENT, CITY OF ALBUQUERQUE, NEW MEXICO DATE 06198d HIEF CITY SURVEYOR, CITY OF ALBUQUERQUE, NEW MEXICO

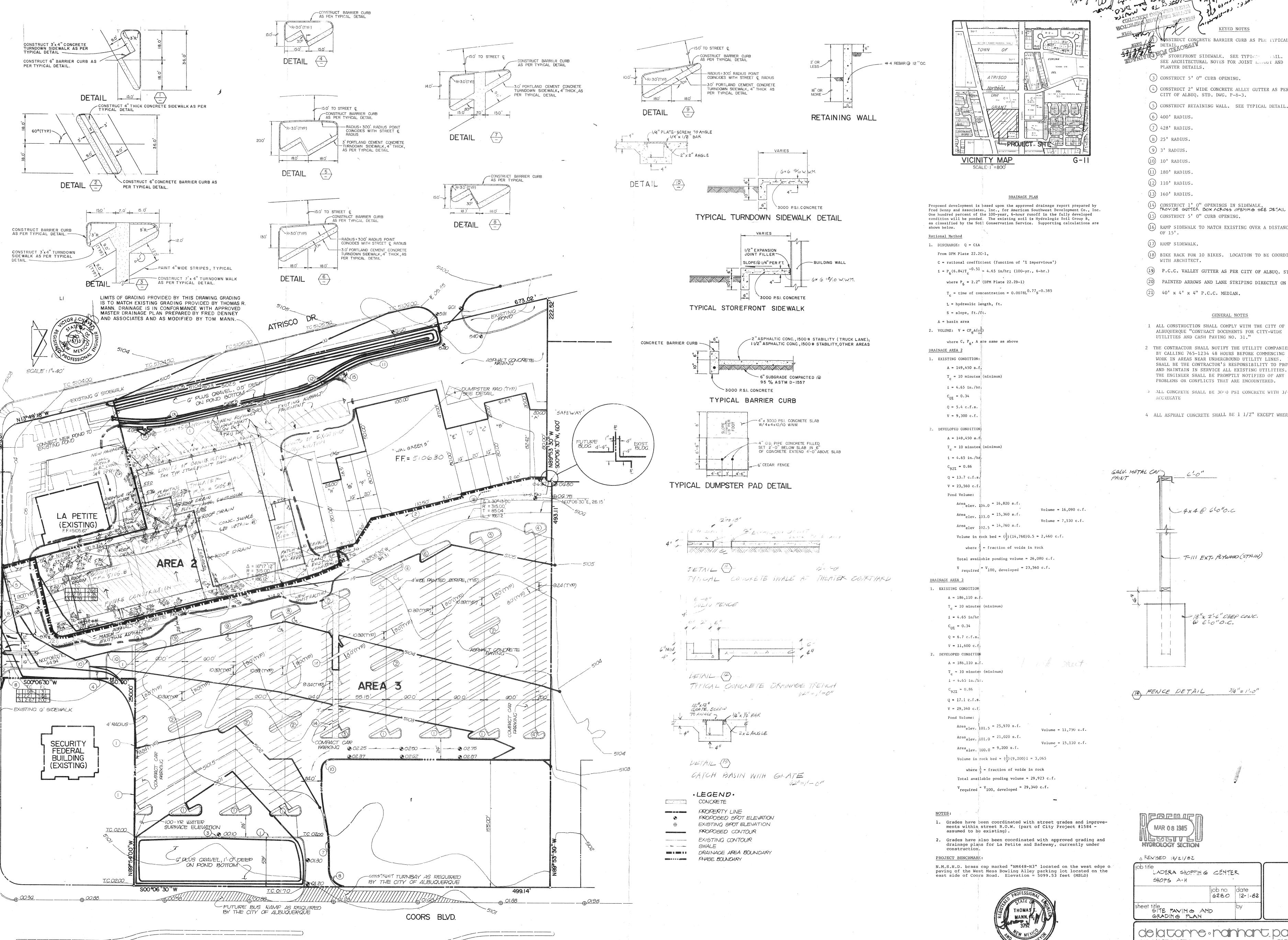
> I, UNDER THE LAWS OF THE STATE OF NEW MEXICO CERTIFY THAT I AM A REGISTERED PROFESSIONAL ENGINEER AND LAND SURVEYOR AND THAT THIS PLAT WAS PREPARED BY ME OR UNDER MY SUPERVISION AND MEETS THE MINIMUM REQUIREMENTS FOR MONUMENTS AND SURVEYS OF THE ALBUQUERQUE SUBDIVISION ORDINANCE AND ALL EASEMENTS OF RECORD ARE SHOWN AND THAT THE REPESSION PUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF THE ROPESSION OF THE BEST OF MY KNOWLEDGE AND

PRINTED JUN 28 1984

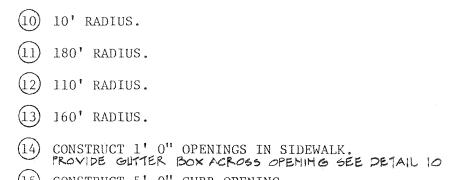
TRACTS are subject to Easements with Covenants and Restrictions affecting Land (ECR) as recorded in the Office of the County Clerk of Bernalillo County, New Mexico in Misc. Book 875, Pages 831 thru 853 inclusive and amendments as shown is Misc. Book 50-A, Pages 825 thru 828 inclusive filed September 27,1983; and Misc. Book 56-A, Pages 22 thru 32 inclusive, filed October 14, 1983.

Notes:

- 1. Bearings are based on CORRECTED SUMMARY PLAT OF TRACTS L,M, AND N, NORTHEAST UNIT, TOWN OF ATRISCO GRANT, filed August 25, 1983 in Volume: C-2?; Folio: 2.
- 2. Distances are ground distances.
- 3. No field Survey was made.







SONSTRUCT CONCRETE BARRIER CURB AS PER TYPICAL

SEE ARCHITECTURAL NOTES FOR JOINT LINOUT AND

(4) CONSTRUCT 2' WIDE CONCRETE ALLEY GUTTER AS PER

(5) CONSTRUCT RETAINING WALL. SEE TYPICAL DETAIL.

- (15) CONSTRUCT 5' O" CURB OPENING. (16) RAMP SIDEWALK TO MATCH EXISTING OVER A DISTANCE
- (17) RAMP SIDEWALK.

PLANTER DETAILS.

(6) 400' RADIUS.

7 428' RADIUS.

(8) 25' RADIUS.

9 3' RADIUS.

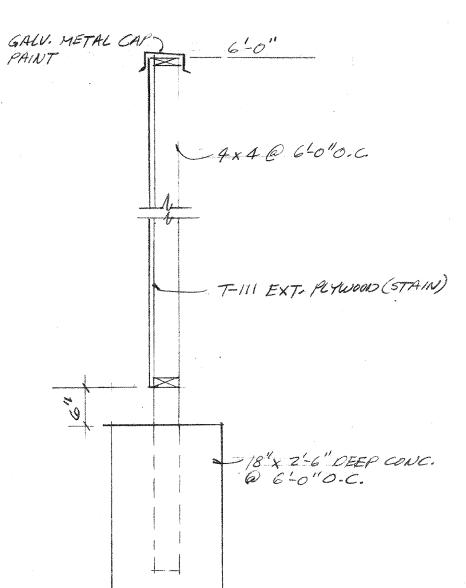
3 CONSTRUCT 5' 0" CURB OPENING.

CITY OF ALBUQ. STD. DWG. P-8-3.

- (18) BIKE RACK FOR 10 BIKES. LOCATION TO BE COORDINATED WITH ARCHITECT.
- (19) P.C.C. VALLEY GUTTER AS PER CITY OF ALBUQ. STD. DWG. P-9-2.
- PAINTED ARROWS AND LANE STRIPING DIRECTLY ON PAVING.
- (21) 40' x 4' x 4" P.C.C. MEDIAN.

GENERAL NOTES 1 ALL CONSTRUCTION SHALL COMPLY WITH THE CITY OF

- ALBUQUERQUE "CONTRACT DOCUMENTS FOR CITY-WIDE UTILITIES AND CASH PAVING NO. 31."
- 2 THE CONTRACTOR SHALL NOTIFY THE UTILITY COMPANIES BY CALLING 765-1234 48 HOURS BEFORE COMMENCING WORK IN AREAS NEAR UNDERGROUND UTILITY LINES. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROTECT AND MAINTAIN IN SERVICE ALL EXISTING UTILITIES. THE ENGINEER SHALL BE PROMPTLY NOTIFIED OF ANY PROBLEMS OR CONFLICTS THAT ARE ENCOUNTERED.
- 3 ALL CONCRETE SHALL BE 3000 PSI CONCRETE WITH 3/4" ACCREGATE
- 4 ALL ASPHALT CONCRETE SHALL BE 1 1/2" EXCEPT WHERE NOTED.



TA FENCE DETAIL

MAR 08 1985 MULTISTITUTE LINE
HYDROLOGY SECTION

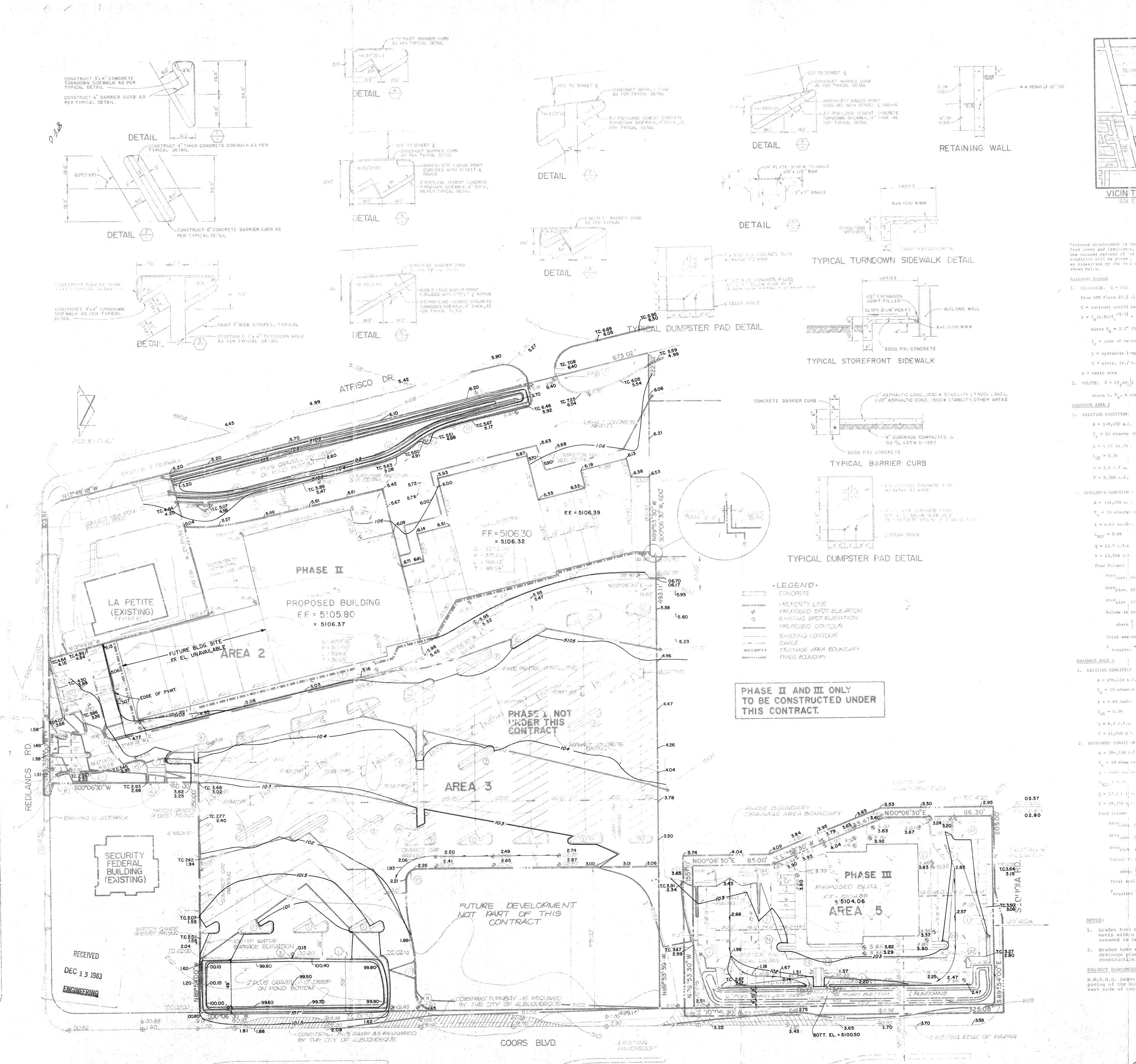
GRADING PLAH

0 REVISED 12/21/82

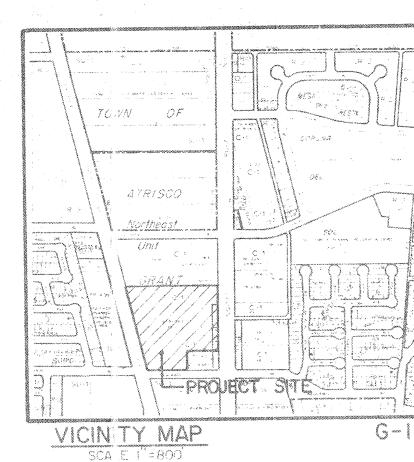
LADERA SHOPPING CENTER SHOPS A-H liob no. Idate 6280 12-1-82 sheet title SITE PAVIMO AND

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C1 albuquenque. n.m. 87110/505-881-0550



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DRAINAGE PLAN

Proposed development is based upon the approved drainage report prepared by Fred Denny and Associatis, Inc., for American Southwest Development Co., Inc. One hundred percent of the 100-year, 6-hour runoff in the fully developed condition will be ponder. The existing soil is Mydrologic Soil Group B, as classified by the Soul Conservation Service. Supporting calculations are shown below.

Rational Method 1. DISCHARGE: Q = C1A From DPM Plate 22.2 -1.

C * rational coefficient (function of '% impervious') $1 = P_6(6.84)T_c^{-0.51} = 4.65 \text{ in/hr; (100-yr., 6-hr.)}$

where P₆ = 2.2" (DPM Plate 22.2D-1) $T_c = \text{time of concentration} = 0.0078 L^{0.77} s^{-0.383}$

L = hydraulic length, ft. S = slope, ft.//t.

2. VOLUME: $V = CP_6A(\frac{1}{12})$

where C. Pg. Alare same as above

1. EXISTING CONDITION: A = 149,450 s.j.

T = 10 minute: (minimum) $i = 4.65 \text{ in } / \text{b}^{-1}$

C_{O%} = 0.34 0 = 5,4 c.f.s.

DEVELOPED CONDITIO:

A = 149,450 s.f 1 = 10 minutec (minimum)

> -1 = 4.65 in./hC_{92%} ≈ 0.86 Q = 13.7 c.f.(.

V = 23,560 c.f Pond Volume:

Area elev. 104.0 = 16,820 s.f. Volume = 16.090 c.f. Area_{elev.} 103.0 = 15,360 s.f. Volume = 7,530 c.f.

Area elev 102.5 = 14,760 s.f. Volume in rock bed = $(\frac{1}{5})(14.760)0.5 = 2.460$ c.f.

where $\frac{1}{2}$ = fraction of voids it rock Total available ponding volume = 26,080 c.f.

V required * V100, developed * 23,560 c.f.

I. EXISTING CONDITION

A = 186,110 s.f. Tc = 10 minutes (minimus)

1 = 4.65 in/ha C_{OZ} = 0.34-

Q = 6.7 c.f.m.

V ≈ 11,600 c.f

2. DEVELOPED CONDITION

-A = 186,110.8.6.

T = 10 minu es (minimum)

V = 29,340 c/1.

Fond Volume:

Area elev. 101.5 = 25,970 e.f. Arca elev. 101.0 = 21,020 s.f.

Area elev. 100.0 = 9,200 s.f. Volume in rock bed = $\binom{1}{3}(9,200)! = 3.065$

where $\frac{k}{2}$ = fraction of voids in rock

Total available ponding volume = 29,923 c.f. Vrequired " Vi00, developed " 29,340 c.f.

1. Grades have been coordinated with street grades and improvements within street R.O.W. (part of City Project #1584 assumed to be existing). 2. Grades have also been coordinated with approved grading and

drainage plans for La Petite and Safeway, currently under construction. PROJECT BENCHMARK:

N.M.S.H.D. brass cap marked "NM448 N3" located on the west edge of paving of the West Mesa Bowling Alley parking lot located on the east side of Coors Road. Elevation = 5039.53 feet (MSLD)



KEYFD MOTES

(1) CONSTRUCT CONCRETE BARKTER CURE AS PLANTED CAL DETAIL

SEE AMCHITECTURAL NOTES FOR JOINT COLOR AND PLANTER DETAILS.

(3) CONSTRUCT 5' O" CURB OPENING.

(4) CONSTRUCT 2' WIDE CONCRETE ALLEA GUITTER AS PER CITY OF ALBUQ. STD. DWG. P-5-3.

(5) CONSTRUCT RETAINING WALL. SEE TYPICAL DELATE. (a) 400' RADIUS.

(7) 428' RADIUS.

(a) 25' RADIUS.

(9) 3 RATHUS.

(10) 10' RADIES.

(1) 180' KADIUS.

(12) 110' KADIUS.

(13) 160' RAPHUS. (14) SIDEWALK GUTTER BOX. SEE DETAIL 10

16) CONSTRUCT S' O" CURB OPENING.

(16) RAMP SIDEMALK TO MATCH EXISTING OVER A DISTANCE

(F) RAMP SIDEWALK.

of 15%

(18) BIKE RACK FOR TO BIKES. LOCATION TO BE COORDINATED WITH ARCHITECT.

(19) P.C.C. VALLEY GUTTER AS PER CITY OF ALBUQ. STD. DWG. P-

20 PAINTED ARROWS AND LANE STRIPING DIRECTLY ON PAVING. (21) 40' x 4' x 4" P.C.C. MEDIAN.

(22) 3'x4" TURNDOWN SIDEWALK - SEE TYPICAL DETAIL

CEMERAL NOTES 1 ALL CONSTRUCTION SHALL COMPLY WITH THE CITY OF "ALBUQUERQUE "CONTRACT DOCUMENTS FOR CITY-WIDE"

UTILITIES AND CASH PAVING NO. 31." 2 THE CONTRACTOR SHALL NOTIFY THE DILLETY COMPANIES. BY CALLING 765-1234 48 HOURS BEFORE COMPLECING WORK IN AREAS NEAR UNDERGROUND UTILITY LINES. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROTECT

AND MAINTAIN IN SERVICE ALL EXISTING UTILITIES. THE ENGINEER SHALL HE PROMPTLY NOTIFIED OF ANY PROBLEMS OR CONFLICTS THAT ARE ENCOUNTERED.

4 ALL ASPHAUT CONCRETE SHALL BE 1 1/2" EXCLPT WHERE WOTEN

DRAINAGE CERTIFICATION CALCULATIONS

Brainage Area 2 - Fond Volume Area - Elev. 105 = 14780 s.f.

Volume = 12317 c.f. Area - Elev. 104 = 9853 s.f. Volume = 8641 c.f. Area - Elev. 103 = 7428 s.f.

Area - Elev. 102.6= 0 Volume in Rock Bed = 7428 (1/3) (.5) = 1238 c.f.Total Available Ponding Volume = 23682 c.f.

Volume Required = 23,560 c.f. Drainage Area 3 - Pond Volume

Area - Elev. 101.5 = 28903 c.f.

Area - Elev. 101 = 17783 c.f. Volume = 13584 c.f. Area - Elev. 100 = 9384 c.f.

Area - Elev. 99.5 = 0Volume in Rock Bed = 9384(1/3)(1) = 3128 c.f.Total Available Ponding Volume = 30,730 c.f. Volume Required = 29,340 c.f.

Drainage Area 5 - Pond Volume Area - Elev. 102 = 8915 s.f. Volume = 6757 c.f

Area - Elev. 101 = 4598 s.f.Area - Elev. 100.5 = 2580 c.f.Total Available Ponding Volume = 8552 c.f. Volume Required = 8260 c.f.

> DRAINAGE AREA 5 1. EXISTING CONDITION

A = 39,360 sf = 1.36 Ac T_ * 10 minutes (minimum) 1 = 4.65 in/hr

 $C_{0Z} = 0.34$ Q = 2.2 cfs

V = 3695 of Promotes commission

& - 59,300 sf = 1.36 Ac T_ = 10 minutes (minimum)

i = 4.65 in/hr

C741 = 0.76 5 = 4.9 dfs

Fond Volume:

Area elev. 102 - 10,725 sf

Volume = 1690 cf Area eley. 100.5 - 3130 st I vol. = 3860 cf

ONEVIDED 12/21/62

LADERA SHOPPING CENTER job no date

6280 12-20-82 sheet title PAVING AND GRADING PLAH

anchitects. o 21 Indian school road ne 1 suite 141 abuquerque nm 87110/505881:0550