

## City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

December 12, 2000

Kevin Patton, PE Bohannan Huston, Inc 7500 Jefferson NE Albuquerque, NM 87109

Grading and Drainage Certification - Oxbow Village (G-11/D014B), Re:

Engineer's Stamp dated 11-2-1999

**Engineering Certification dated 12-11-2000** 

Dear Mr. Patton:

Based upon the information provided in your submittal dated 12-12-2000, Engineering Certification for the above referenced subdivision is approved for Release of Financial Guaranty.

If I can be of further assistance, please contact me at 924-3986

Sincerely,

Bradly L. Buyham

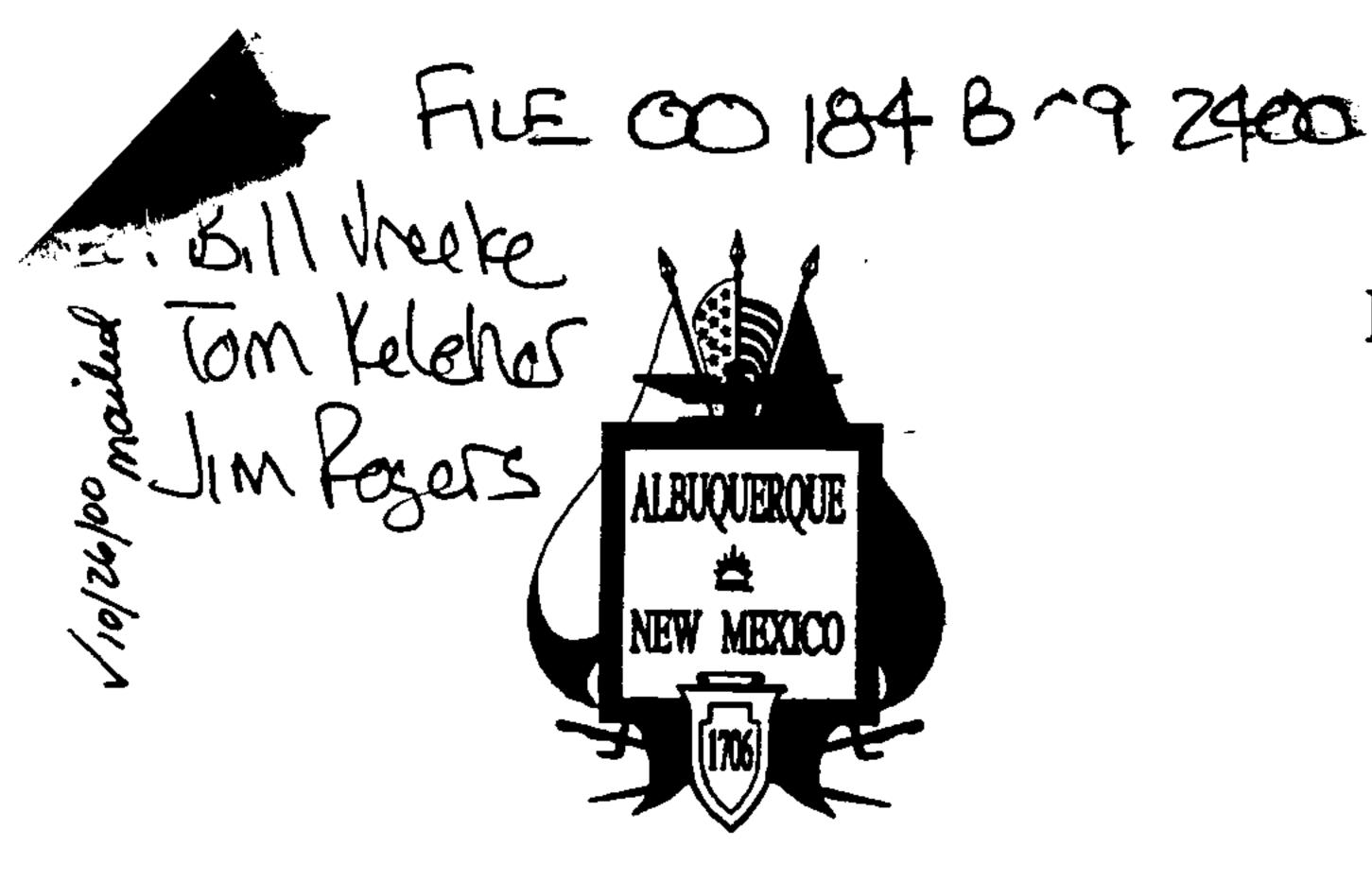
Bradley L. Bingham, PE

Hydrology Review Engineer

Arlene Portillo, PWD – #628981

#### DRAINAGE INFORMATION SHEET

PROJECT TITLE: (3)2000 VIVOQE.  DRB #:	_ ZONE MAP/DRG. FILE #:
LEGAL DESCRIPTION: On VIllage. CITY ADDRESS:	
ENGINEERING FIRM: ROMANNAN HUSTON ADDRESS: CONTROLD I 7500 Jellerson Street CITY, STATE: Albug-NM	CONTACT: KEUIN Patter) PHONE: 823-1000 ZIP CODE: 87109
OWNER: Attha West  ADDRESS: CITY, STATE:	CONTACT: TOM KELEHER  PHONE: 346-4646  ZIP CODE:
ARCHITECT: ADDRESS: CITY, STATE:	CONTACT:PHONE:ZIP CODE:
SURVEYOR: ADDRESS CITY, STATE:	CONTACT:PHONE:ZIP CODE:
CONTRACTOR:  ADDRESS:  CITY, STATE:	CONTACT:PHONE:ZIP CODE:
TYPE OF SUBMITTAL:  DRAINAGE REPORT  DRAINAGE PLAN  CONCEPTUAL GRADING & DRAINAGE PLAN  GRADING PLAN  EROSION CONTROL PLAN  ENGINEER'S CERTIFICATION  CLOMR/LOMR  OTHER  WAS A PRE-DESIGN CONFERENCE ATTENDED:  YES  NO  COPY PROVIDED	K TYPE OF APPROVAL SOUGHT:  X SIA / FINANCIAL GUARANTEE RELEASE PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL WORK ORDER APPROVAL OTHER (SPECIFY)
DATE SUBMITTED:BY:	
Requests for approvals of Site Development Plans and/or Subdivisi The particular nature, location and scope of the proposed development of the following levels of submittal may be required based on a conceptual Grading and Drainage Plan: Required for (5) acres and Sector Plans.  2. Drainage Plans: Required for building permits, grading pacres.  3. Drainage Report: Required for subdivisions containing more.	ment defines the degree of drainage detail. One or the following: approval of Site Development Plans greater than five permits, paving permits and site plans less than five (5)



# City of Albuquerque P.O. Box 1293 Albuquerque, NM 87103 Public Works Department

October 18, 2000

#### Certificate of Work Order Completion

City Engineer 600 Second St. Plaza Del Sol Albuquerque, NM 87102

Re:

Oxbow Village Subdivision (Tract E) Project No. 6289.81

Map No. K-20

Dear Sir:

This is to certify that Project No. 6289.81 has been completed according to approved plans and construction specifications. Please be advised this Certificate of Work-Order Completion does not constitute acceptance of the city infrastructure until all requirements of the subdivision improvements agreement have been satisfied and a Certificate of Completion and Acceptance is issued by the City Engineer.

PRIVATE ROADWAY IMPROVEMENTS – (OXBOW VILLAGE) – 20' F-F/IN/OUT residential paving w/PCC mountable median curb & roll or standard curb on both sides and a 4' wide with PCC sidewalk on the west side\* on Oxbow Village Lane from Oxbow Drive to transition from divided entrance to 28" F-F, and 28' F-F residential paving with PCC standard or mountable roll curb on both sides and a PCC 4' wide sidewalk on the west side\* on Oxbow Village Lane from transition from divided entrance to 28' F-F to south cul-de-sac terminus, and 24' F-F residential paving with PCC standard or mountable roll curb on both sides; no sidewalk on Oxbow Village lane stub street from south cul-de-sac terminus to south end of stub street.

\*Sidewalks to be deferred.

PUBLIC DRAINAGE IMPROVEMENTS – (OXBOW VILLAGE) – 48" DIA RCP with necessary manholes and inlets on public easement between Lots 27 and 28 from east boundary to Oxbow Village Lane, on Oxbow Village Lane) public easement) from public easement between Lots 27 and 28 to public easement between Lots 8 and 9, and on public easement between Lots 8 and 9 from Oxbow Village Lane to Tract E-1.

Construction of a PCC ribbon channel along the eastern boundary within a private drainage easement. Grading and drainage certification of Oxbow Village is required prior to release of Financial Guaranty(s).

PUBLIC SANITARY SEWER IMPROVEMENTS – (OXBOW VILLAGE) – 8" DIA sanitary sewer with necessary manholes and services on Oxbow Village Lane (public easement) from public easement between Lots 27 & 28 to north entrance and on Oxbow Village lane stub street (public easement on south cul-de-sac terminus to south end of stub street, 6" DIA waterline with necessary valves FHs, MJs and RJs on Oxbow Village Lane (public easement) from public easement between Lots 27 and 28 to Lot 1, 8" DIA waterline with necessary valves FHs, MJs and RJs on Oxbow Village Lane (public easement) from public easement between Lots 27 and 28 to public easement between lots 16 and 17, 6" DIA waterline with necessary valves FHs, MJs and RJs on Oxbow Village Lane stub street (public easement) on public easement between Lots 16 and 17 to south end of stub street, 8" DIA waterline with necessary valves FHs, MJs and RJs on public easement between Lots 16 and 17 from south cul-de-sac terminus to Alamogordo Drive and on public easement between Lots 27 and 28 from Oxbow village Lane (public easement) to Mourning Dove Place (Oxbow Bluff Subdivision).

All as shown on sheets 1-13 of City approved project plans.

Oxbow Village Subdivision
Tract E
Project No. 6289.81
Page 2

The contractor's correction period will begin the date of the City Engineers Certificate of Completion and Acceptance letter and is effective for a period of one (1) year.

Sincerely,

Peter Chang, P.E. RBS-0

Chief Construction Engineer
Public Works Department

c: Sparling Construction

Bohannon-Huston

Master Scheduler

Project Administrator

Martin Barker, Materials Testing Lab

Sam Hall, Water Systems

Dave Harmon, Traffic Engineering, PWD

Josie Jaramillo, New Meter Sales, PWD

Richard Zamora, Maps & Records, PWD

Project No. 6289.81

Warranty:Contract

#### CLIENT/COURIER TRANSMITTAL

À à	To:	To: SUSAN CALOGNE HYDROLOGY SECTION			Requested By: KEVIN PATTON					
		PLAZA DEL SOI LOBBY - WEST		Dat	e:	November	2, 1999			
BOHANNAN HUSTON Courtyard One				Tin	e Due:		X This P. By Tor			
7500 JEFFERSON NE Albuquerque NEW MEXICO 87109	Job N	O.:				XBOW PA	RK AND OX	(BOW		
voice 505.823.1000	<u></u> .	DELIVERY VIA				PICK UP		· ·		
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	COMN	IENTS / INSTRUC	TIONS							
	DR	B-99-293								
	REC'E	) BY:			DATE:	<b>711</b>	/E:			

ENGINEERS PLANNERS PHOTOGRAMMETRISTS SURVEYORS LANDSCAPE ARCHITECTS

SUSAN CALOGNE

To:

### CLIENT/COURIER TRANSMITTAL

Requested By: KEVIN PATTON

	HYDROLOGY SECTION PLAZA DEL SOL LOBBY - WEST	Date:	November 2, 1999
BOHANNAN HUSTON Courtyard One		Time Due:	This A.M. X This P.M.  Rush By Tomorrow
7500 JEFFERSON NE Albuquerque NEW MEXICO 87109	Job No.:		XBOW PARK AND OXBOW UBDIVISIONS
voice 505.823.1000 fax 505.821.0892	DELIVERY VIA	item:_	PICK UP
	Mail UPS  Other		
	2 1 REVISED F	NFRASTRUCTUE	
	COMMENTS / INSTRUCTIONS  DRB-99-293		
	REC'D BY:	<i>DATE:</i>	TIME:
	ENGINEERS PLANNERS PHOTOGRAMMET	RISTS SURVEYORS	LANDSCAPE ARCHITECTS



### City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

November 4, 1999

Kevin Patton, P.E.
Bohannan Huston
7500 Jefferson St. NE
Albuquerque, New Mexico 87111

RE: Drainage Reports and Grading and Drainage Plans for The Oxbow Park and Oxbow Village Subdivision (G11/D14B), Submitted for Preliminary and Final Plat Approval and Grading Permit Release, Reports Dated 10/1/99 and Engineer's Stamp Dated 11/2/99 on the Plans.

Dear Mr. Patton:

Based on the information provided, the above referenced reports, and Grading and Drainage Plans dated November 2, 1999, are approved for Site Development Plan and Preliminary Plat approval by the DRB.

The above referenced plans are also approved for Rough Grading provided that they are approved at the DRB. A separate top-soil disturbance permit must be obtained prior to grading on this site.

Prior to Final Plat sign-off, the Subdivision Improvements Agreement (SIA) must be in place. The Grading and Drainage certification is required prior to release of the SIA for these subdivisions.

If you have any questions, or if I may be of further assistance to you, please call me at 924-3982.

Sincerely,

Susan M. Calongne, P.E.

City/County Floodplain Administrator

c: Whitney Reierson, City Hydrology
Tom Keleher, Altura West Development
File

January 18, 2000



Susan Calongne, P.E.
City/County Floodplain Administrator
City of Albuquerque - Hydrology Division
P. O. Box 1293

BOHANNAN-HUSTON, INC.

Albuquerque, NM 87103

Courtyard One

Re:

Oxbow Village – Minor Storm Drain Revisions COA Project No. 628981, DRB No. 99-293

7500 JEFFERSON NE

Dear Susan:

Albuquerque

NM 87109-4335

The purpose of this letter is to request your concurrence concerning a few isolated revisions to the above referenced project.

voice 505.823.1000

fax 505.821.0892

Sparling Construction is the Oxbow Park and Oxbow Village Grading and Retaining Wall Contractor. Sparling Construction has also been selected to perform the work order construction (COA Project No. 628881 & 628981) for the Oxbow Park and Oxbow Village as well. In order to continue on schedule with the grading and retaining wall construction, the contractor has requested that we make some minor revisions to the storm drain construction on the work order plans so they may construct the walls within the utility easement areas.

I have enclosed sheet 7 of 13 detailing the requested revisions. These revisions reflect keeping the existing 54" RCP storm drain and adding a manhole in place of the 48"x18" Wye, and then constructing another 6'-diameter manhole at Station 3+04.64 as indicated in the plans before.

If you are in agreement with the above requested revisions, please indicate by signing the space provided below.

If you have any questions or require additional information, please call me at 823-1000.

Sincerely,

Bohannan Huston, Inc.

Kevin Patton, P.E. Project Manager

Community Development and Planning Group

KP/am Enclosure

CC:

Pat Wylie, Sparling Construction Bill Vreeke, Bohannan Huston, Inc. Dave Perko, Bohannan Huston, Inc.

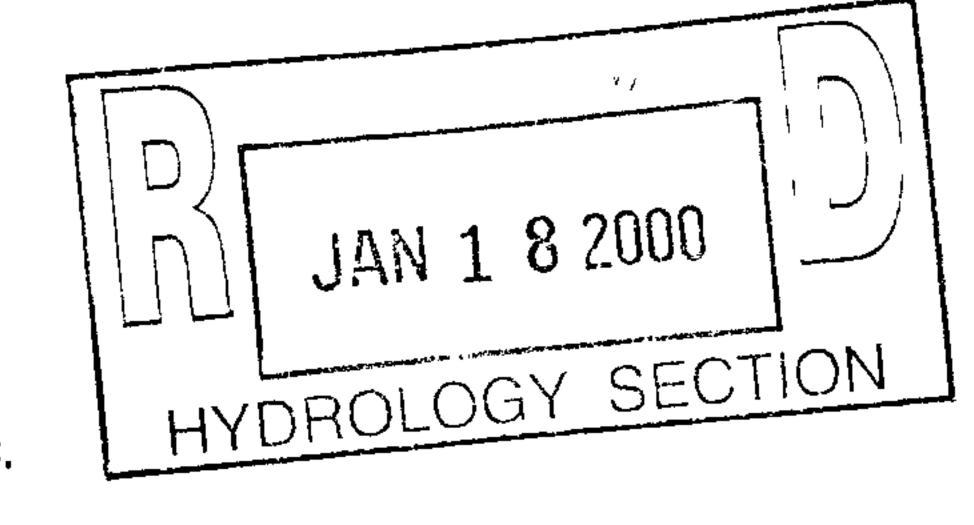
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ACCEPTED:

BY

TITLE

DATE





NGINEERS PLANNERS PHOTOGRAMMETRISTS SURVEYORS SOFTWARE DEVELOPERS



### City of Albuquerque

February 11, 2000

Kevin Patton, P.E.
Bohannan Huston
7500 Jefferson St. NE
Albuquerque, New Mexico 87111

RE: Minor Revisions to the DRC Plans for Oxbow Village Subdivision (G11/D14B), Engineer's Stamp Not Dated on the Plans.

Dear Mr. Patton:

The concept of replacing the proposed 48" x 18" Wye with a 6' diameter manhole in the storm drain system is acceptable to City Hydrology. This revision should appear on the Work Order drawings.

If you have any questions, or if I may be of further assistance to you, please call me at 924-3982.

Sincerely,

Susan M. Calongne, P.E.

City/County Floodplain Administrator

C: File

#### DRAINAGE INFORMATION SHEET

PROJECT TITLE: () YOUN YILLAGE ZON	IE ATLAS/DRNG. FILE #G-11/D0/43/B
PROJECT TITLE: <u>Oxbow Yillage</u> ZON  DRB #: EPC #: <u>2-97-103-1</u>	WORK ORDER #: (THE)
LEGAL DESCRIPTION: TRACT E 0480W SUBS	
CITY ADDRESS: OYBOW DRIVE MO ALAMY	360200 BLUD
ENGINEERING FIRM: BOHANNAN-HUSTON INC.  ADDRESS: 7500 JEFFERSON NE, ALB. NM 87109  OWNER: ALTURA WEST DEVELOPMENT  ADDRESS: DRAWGE AA, ALBUQUEROUE 8714  ARCHITECT: ,  ADDRESS: SURVEYOR:	PHONE: (505) 823-1000  CONTACT: TOM YELEHER  PHONE: 346-4646  CONTACT: PHONE: CONTACT: CONT
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:  SKETCH PLAT APPROVAL  PRELIMINARY PLAT APPROVAL  S. DEV. PLAN FOR SUB®D. APPROVAL  S. DEV. PLAN FOR BLDG. PERMIT APPROVAL  SECTOR PLAN APPROVAL  FINAL PLAT APPROVAL  FOUNDATION PERMIT APPROVAL  BUILDING PERMIT APPROVAL  CERTIFICATE OF OCCUPANCY APPROVAL  GRADING PERMIT APPROVAL  PAVING PERMIT APPROVAL  S.A.D. DRAINAGE REPORT  DRAINAGE REQUIREMENTS  SUBDIVISION CERTIFICATION  OTHER (SPECIFY)
Revised 02/98	OCT 1 1999  HYDROLOGY SECTION

# DRAINAGE REPORT FOR OXBOW VILLAGE SUBDIVISION (TRACT E)

BCHANNAN-HUSTON, INC.

Courtyard One

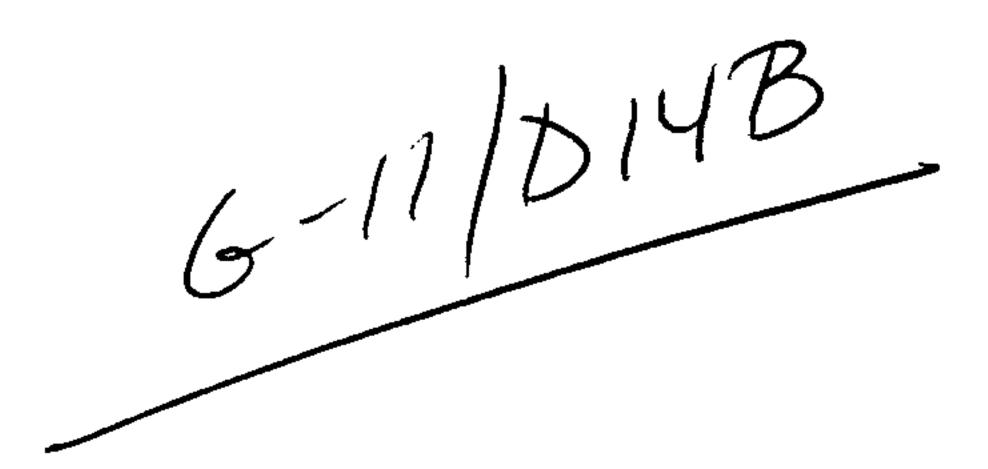
7500 JEFFERSON NE

Albuquerque

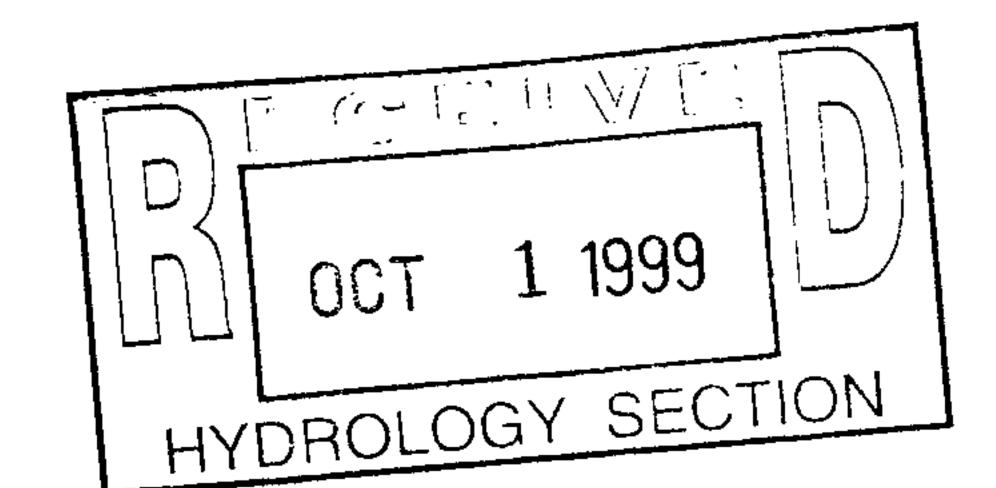
NM 87109-4335

voice 505.823.1000

fax 505.821.0892



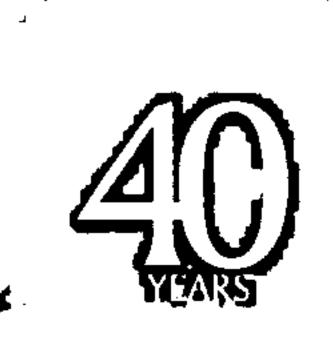
OCTOBER 1, 1999



PREPARED FOR:

ALTURA WEST LTD
P.O. DRAWER AA

ALBUQUERQUE, NEW MEXICO 87107



# DRAINAGE REPORT FOR OXBOW VILLAGE SUBDIVISION (TRACT E)

OCTOBER 1, 1999

PREPARED BY:

BOHANNAN HUSTON COURTYARD I, 7500 JEFFERSON STREET N.E. ALBUQUERQUE, NM 87109

PREPARED FOR:

Altura West Ltd. P.O. DRAWER AA Albuquerque, NM 87107

PREPARED BY:

Yolanda Padilla

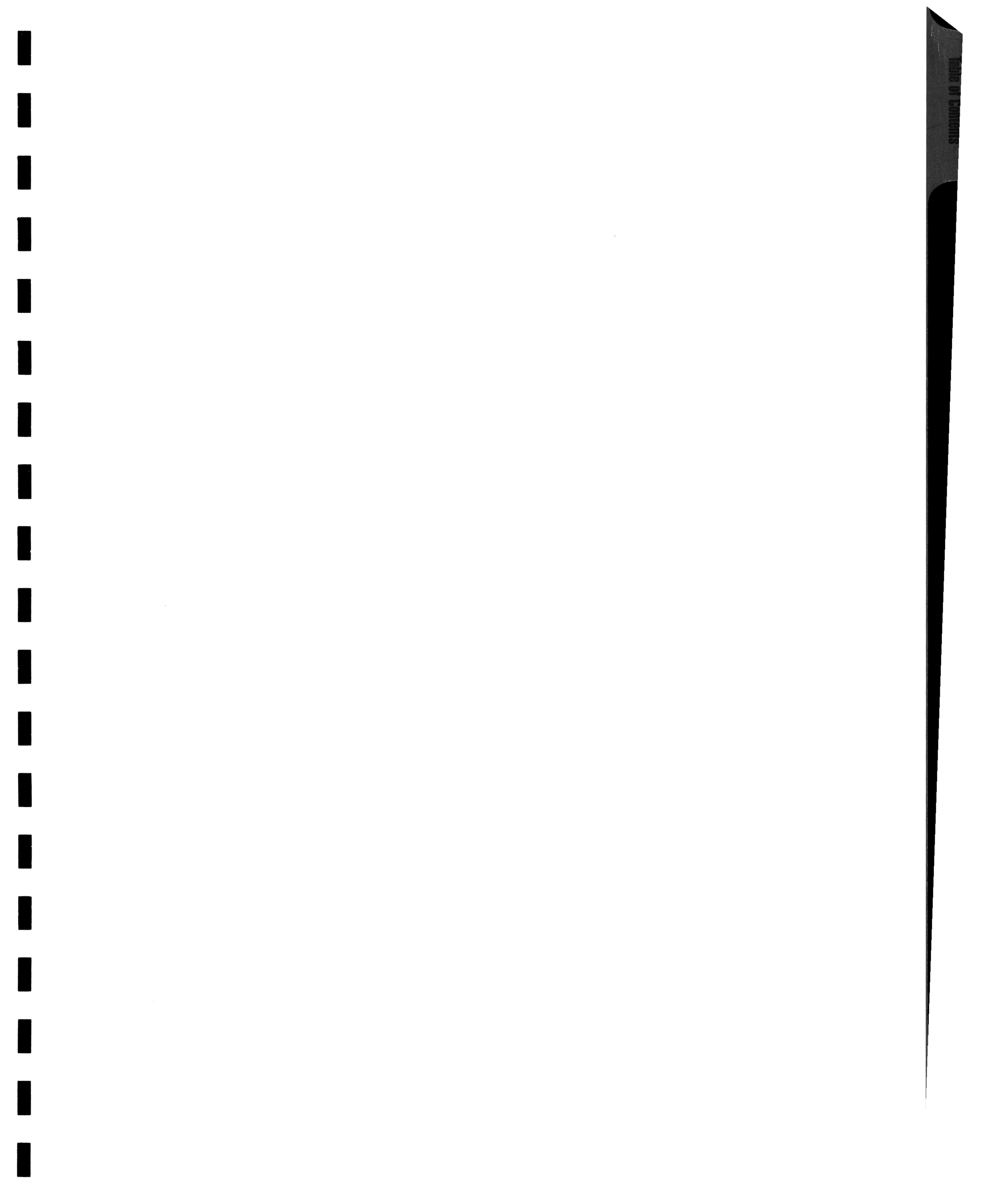
Date

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UNDER THE SUPERVISION OF THE MEXICOL

Kevin Patton, P.E.

(13685)



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APPENDIX D:	RIBBON CHANNEL ANALYSIS
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	PRELIMINARY PLAT
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EXHIBIT 4:	PROPOSED DRAINAGE BASINS MAP
EXHIBIT 5:	PROPOSED ON SITE BASIN MAP
EXHIBIT 6:	GRADING AND DRAINAGE PLAN

#### I. PURPOSE

The purpose of this report is to present historic and proposed drainage conditions for the area pertaining to this site, and to obtain work order, preliminary and final plat approvals.

#### II. METHODOLOGIES

Site conditions will be analyzed for a 10-year and 100-year, 6-hour storm event in accordance with the City of Albuquerque Drainage Ordinance and the Development Process Manual (DPM) Volume 2, Design Criteria, Section 22.2, Hydrology, for the City of Albuquerque, January 1993.

The site, as described in the 'Site Location and Characteristics' section below, is approximately 9.62 acres. Therefore, Part A of the DPM, Section 22.2, which provides a simplified procedure for projects with sub-basins smaller than 40 acres was used.

This report will reference the following City of Albuquerque approved studies prepared for Altura West LTD: 1) *Master Drainage Plan for Altura West and Archdiocese of Santa Fe Properties* dated October, 1997; 2) *Drainage Report for Oxbow Bluff Subdivision (Tract F)*, dated October 17, 1997.

The <u>Master Drainage Plan for Altura West and Archdiocese of Santa Fe Properties</u> dated October, 1997 (hereafter referred to as the Master Plan), which was prepared for Altura West LTD. and Archdiocese of Santa Fe and approved by the City of Albuquerque. That report was prepared to support <u>future</u> drainage plans submitted for the development of individual land parcels within Altura West Development, and to provide design guidance for the installation of major drainage infrastructure to be constructed in advance of, or simultaneously with individual parcel development. In addition, it provides <u>fully developed</u> flow rates for basins within the Altura West and Archdiocese of Santa Fe Properties.

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The <u>Drainage Report for Oxbow Bluff Subdivision (Tract F)</u>, dated October 17, 1997 was prepared for Altura West LTD to support the development of the 63 lot subdivision located directly to the east of this development (Oxbow Village Subdivision – Tract E). The report for the Oxbow Bluff Subdivision constructed a temporary detention pond on Tract E and provided a storm drain extension for future development.

#### III. SITE LOCATION AND CHARACTERISTICS

Please refer to the proposed Bulk Land Plat enclosed with this report and the A.G.I.S. vicinity map page G-11.

The Site, Oxbow Village Subdivision, Tract E Altura West Development, is bounded by St. Pius High School and Oxbow Drive to the north, Alamogordo Boulevard (future Oxbow Park Subdivision) to the west, Oxbow Bluff Subdivision to the east, and Northern Heights Subdivision to the South. The 9.62-acre site proposes to be developed into 40 lots of single family dwelling units. The main road, Oxbow Village Lane (private roadway), is oriented through the center of the development and will provide access to the subdivision from Oxbow Drive.

Vegetation on the site consists primarily of Mesa Dropseed, Indian Ricegrass, Blace Gramma and Sand Sagebrush. The soil at the site has been given the SCS soil classification of BKD (Bluepoint-Kokan association, hilly) and MWA (Madurez-Wink Association, gently sloping). The BKD soils are in the hydrologic soil Group A, which have the highest rate of rainfall absorption. The MWA soils are in the hydrologic soil Group B, which also absorb more water than typical.

#### IV. EXISTING HYDRAULIC AND HYDROLOGIC CONDITIONS

The existing drainage basins and patterns are shown graphically on the Existing Drainage Conditions Map (Exhibit 3) located in the plates section of this report. Please note that the Existing Drainage Basin Map was taken from the approved Oxbow Bluff Subdivision Report and is labeled as

the "Oxbow-Bluff Subdivision Proposed Conditions Basin Map". This map adequately depicts the existing condition of this site.

The existing site consists of Basin M, that flows to an existing temporary detention pond, which was built with Oxbow Bluff Subdivision, see Exhibit 3. The runoff from the on-site basins is approximately 27.82cfs in the 100-year, 6-hour storm event.

For additional information regarding the existing drainage conditions, please refer to the approved Master Plan (October 1997).

#### V. PROPOSED HYDRAULIC AND HYDROLOGIC CONDITIONS

For Additional assistance throughout this section, please refer to the Grading and Drainage Plan (Exhibit 6) and/or the Proposed On-Site Conditions Map (Exhibit 5) enclosed within the plate's section of this report.

The proposed site will be divided into six on site basins (Basins A through F) and one off-site basin (a portion of Tract E-1). On-site Basins A and B will drain to the roadway below referred to as Oxbow Village Lane. This private roadway slopes to the center of the development where the site has provided off-street parking. This parking area contains a double grate Type "A" inlet in a sump condition to capture this runoff. The inlet has been designed to capture 2 x 100-year runoff. The runoff from On-site Basin "C" drains to Oxbow Village Lane as well, where it is then redirected to Oxbow Drive. Oxbow Drive is an existing roadway that was constructed with the Oxbow Bluff Subdivision. Runoff within Oxbow Drive is conveyed to the existing inlets located at the entrance to the Oxbow Bluff Subdivision (Mourning Dove Place). The runoff from On-site Basins D, E and F drain to a concrete ribbon channel along the retaining walls adjacent to the eastern subdivision boundary. The ribbon channel then directs runoff to center of development along the eastern boundary. A double grate Type "C" inlet has been designed to capture 2 x 100 year storm event. The Off-site Basin within a portion of Tract E-1 will convey a small amount of the existing and/or the proposed fully developed runoff into a concrete ribbon channel along the retaining wall proposed along the western

boundary to Oxbow Village Subdivision. The runoff is directed down the ribbon channel where is captured by a double grate Type "D" inlet that has been designed to capture 2 x 100 year storm event.

A cross lot drainage easement outside of the building areas allows runoff to be conveyed to the roadway and ribbon channel. Due to the erosiveness of the soil in this area and the steep grades, each lot owner and/or homebuilder should take additional care in order to convey all developed runoff in a non-erosive manner to the roadway or ribbon channel below. This can be accomplished with cobble swales, piping and other non-erosive solutions. A breakdown of the On-site Basins is provided below.

#### A. On-Site Basins

The proposed site is broken into six (6) basins. Basin A (2.00ac, Q<sub>100</sub>=6.62cfs) is located on the west side of Oxbow Village Lane and is in far southwest corner of the Oxbow Village Subdivision. Basin A consists of eight (8) lots, 12 through 19. Run-off flows from this basin drain to the front of the lot and into Oxbow Village Lane and discharge into inlets at the lowpoint.

Basin B (1.78ac, Q<sub>100</sub>=5.89cfs) is located on the west side of Oxbow Village Lane and is adjacent to and northwest of Basin A. Basin B consists of eight (8) lots, 4 through 11. Run-off flows from this basin drain to the front of the lot and into Oxbow Village Lane and discharge into inlets at the lowpoint.

Basin C (0.83ac, Q<sub>100</sub>=2.75cfs) is located on the west side of Oxbow Village Lane and is adjacent to and northwest of Basin B. Basin C consists of three (3) lots, 1 through 3. Run-off from this basin will drain to the front of the lot and into Oxbow Village Lane where it is directed into Oxbow Drive and collected by inlets at the intersection of Oxbow Drive and Mourning Dove Place. The runoff is then conveyed to the Rio Grande Storm Drain, which was built with the Oxbow Bluff Subdivision.

Basin D (1.10ac, Q<sub>100</sub>=3.64cfs) is on the east side of Oxbow Village Lane and is the northeast corner of Oxbow Village Subdivision. Basin D consists of five (5) lots, 36 through 40. Run-off flows from this basin drain to the back of the lot and into a concrete ribbon channel, which will then be collected by a double grate Type "C" inlet. However the front portion of Lot 40 will flow north through a turned block and into Oxbow Drive.

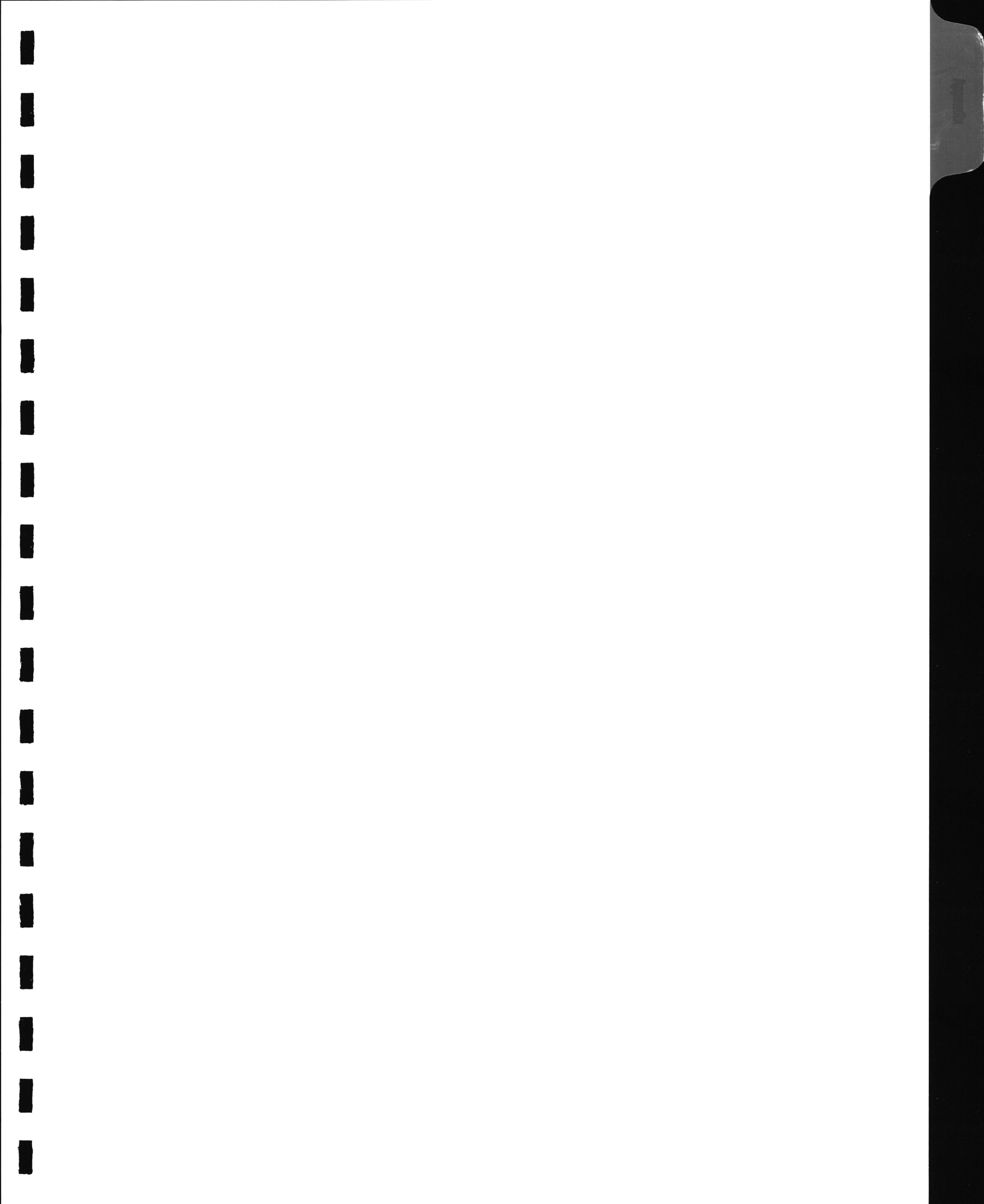
Basin E (1.54ac, Q<sub>100</sub>=5.11cfs) is on the east side of Oxbow Village Lane and is adjacent to and southeast of Basin D. Basin E consists of eight (8) lots, 28 through 35. Runoff flows from this basin drain to the back of the lot and into a concrete ribbon channel, which will then be collected by a double grate Type "C" inlet.

Basin F (1.74ac, Q<sub>100</sub>=5.76cfs) is on the east side of Oxbow Village Lane and is adjacent to and southeast of Basin E. Basin E consists of eight (8) lots, 20 through 27. Runoff flows from this basin drain to the back of the lot and into a concrete ribbon channel, which will then be collected by a double grate Type "C" inlet.

A 48" RCP storm drain will connect the existing 54" storm drain that was constructed with the Oxbow Bluff Subdivision. This storm drain is located in the center of the development along the eastern boundary. The proposed 48" storm drain will continue westward through the Oxbow Village Subdivision in order to serve future development in accordance with the Master Drainage Report. The analysis of this storm drain can be found in the appendix of this report, see Appendix E.

#### VI. CONCLUSION

This report has provided hydrologic and hydraulic considerations of the proposed development of Altura West Tract E, the Oxbow Village Subdivision. This information provides adequate supporting documentation and guidance for approval of this report and to guide future development and phasing of the properties previously mentioned. It is recommended that this plan be approved as requested.



#### APPENDICES

APPENDIX A: BASINS

APPENDIX B: STREET HYDRAULICS

APPENDIX C: INLET ANALYSIS

APPENDIX D: RIBBON CHANNEL ANALYSIS

APPENDIX E: STORM DRAIN ANALYSIS

**APPENDIX A** 

**BASINS** 

					ОХ	BOW VILL	AGE (TRA	CT E)					
BASIN I.D.	AREA	UNITS		Α	% LAND T	REATMEN	•			F (AC-FT)		DISCHARGE (CFS)	
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	HYRDOLOGICAL VOLUMETRIC & DISCHARGE DATA (EXISTING) <sup>1</sup>												
		<del></del>		· <del>-</del> .									
Tract E	9.62	· · · · · · · · · · · · · · · · · · ·	97.	.0%	0.0%	0.0%	3.0%						20.4
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			III	JLUG	ICAL VOL	JIVIE I RIC A	DISCHAR	GEDA	TA (DEVE	LOPED)	<del></del>		
A	2.00	8	0.0	)%	27.5%	27.5%	45.0%		0.12	0.22		3.8	6.62
В	1.78	8	0.0	)%	27.5%	27.5%	45.0%		0.11	0.20		3.4	5.89
С	0.83	3	0.0	)%	27.5%	27.5%	45.0%		0.05	0.09	<u> </u>	1.6	2.75
D	1.10	5	0.0	)%	27.5%	27.5%	45.0%		0.07	0.12	<u> </u>	2.1	3.64
Ε	1.54	88	0.0	)%	27.5%	27.5%	45.0%		0.10	0.17		3.0	5.11
F	1.74	8	0.0	)%	27.5%	27.5%	45.0%		0.11	0.19		3.3	5.76
TOTAL	8.99	40				<del></del> -	·	<del></del>					29.8

NOTES: 1) In the "Master Drainage Plan for Altura West and Archdiocese of Santa Fe Properties Near St. Pius High School" is shows that Tract E lies primarily within existing Basins M. The existing 100yr discharge was also taken from this report w/ approximately 73% of the Tract in Basin M. See Exhibit 3.

2) Impervious percentages for developed flows were calculated from the DPM equation a-4. The remaining percentages were distributed evenly between land treatment types B and C, except for Basin G which were divided accordingly.

45 %

N=UNITS/ACRES = 
$$4.4$$
  
%D=  $7*SQRT((N*N)+(5*N))$  =

# APPENDIX B STREET HYDRAULICS

### SUMMARY OF THE ROADWAY CAPACITY ANALYSIS FOR OXBOW VILLAGE SUBDIVISION - TRACT E

Oxbow Village Lane - Drainage Basins A, B, & C Max Q (cfs)=11.66

	TOTAL CA (OTO)			1,2,00	ECI	ROW	COMMENTS
Roadway Grade	Q(100 YR)	Depth of	Velocity of	V <sup>2</sup> /2*g	EGL		
	in roadway	water in	storm water			Elevation	
(%)	(cfs)	roadway (ft)	in roadway (ft/s)	(ft)	(ft)	(ft)	
			2.56	0.10	0.44	1.00	OK
0.70	15.26	0.34			0.45	1.00	OK
0.72	15.26	0.34	2.69	0.11			ОК
1.20	15.26	0.31	3.01	0.14	0.45	1.00	<b>1</b>
2.64	15.26	0.27	3.80	0.22	0.49	1.00	OK

NOTE:

There is a inverted 2% crown in the roadway, which transistions to a 2% cross-slope to the lowpoint.

1.98

1.28

33.33

9.06

PC PROGRAM STREAM

@ S=2.64%

\*\*\*\*

STREET

SLOPE= .0264 .017 MANNING'S N= ELEV DIST POINT DIST ELEV ELEV POINT DIST POINT 0.44 33.17 0.24 7.00 0.70 0.00 0.44 33.83 0.00 10 19.00 0.78 4.17 0.52 38.00 0.24 11 31.00 0.78 -4.630.00 12 0.00 0.11 33,00 0.11 8 5.00 ENERGY VEL TOPWID FLOW WETTED FLOW FLOW DEPTH WSEL HEAD HEAD VEL PER RATE AREA INC (FT) (FT) (FT) (FPS) (FT) (CFS) SQ.FT. (FT) (FT) 0.07 0.02 5.00 1.21 5.00 0.13 0.2 0.05 0.05 0.16 0.06 10.00 1.93 10.00 1.0 0.50 0.10 0.10 0.24 16.27 0.09 2.42 16.33 2.8 1.15 0.15 0.15 0.33 0.13 22.87 2.91 22.98 6.2 2.13 0.20 0.20 0.44 0.19 28.15 3.48 28.33 11.9 3.43 0.25 0.25 0.60 0.30 28.20 4.36 28.44 21.1 4.84 0.30 0.30 0.76 0.41 28.26 5.16 28.56 32.2 6.25 0.35 0.35 0.94 0.54 28.31 5.89 28.67 7.66 45.2 0.40 0.40 1.11 0.66 29.54 6.51 29.29 59.1 9.08 0.45 0.45 1.20 0.70 32.17 6.73 32.62 71.5 10.63 0.50 0.50 1.37 0.82 33.24 7.25 33.72 89.0 12.28 0.55 0.55 1.56 0.96 33.27 7.88 33.77 109.9 13.95 0.60 0.60 1.77 1.12 33.30 8.48 33.83 132.4 15.61 0.65 0.65

33.89

156.6

17.28

0.70

0.70

STREET @ S=.70%

MANNING'S N= .017 SLOPE= .007

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DI	_	ELEV
1	0.00	0.70	5	7.00	0.24	9	33.1	7 0	. 44
2	4.17	0.78	6	19.00	0.00	10	33.83	3 0	. 44
3	4.63	0.78	7	31.00	0.24	11	38.0	0 0	.52
4	5.00	0.11	8	33.00	0.11	12	0.0	0 0	.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWI	D '	VEL	ENERGY
•••	INC	AREA	RATE	PER	VEL		•	HEAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT	)	(FT)	(FT)
0.05	0.05	0.13	0.1	5.00	0.63	5.	00	0.01	0.06
0.10	0.10	0.50	0.5	10.00	0.99	10.	00	0.02	0.12
0.15	0.15	1.15	1.4	16.33	1.25	16.	27	0.02	0.17
0.20	0.20	2.13	3.2	22.98	1.50	22.	87	0.03	0.23
0.25	0.25	3.43	6.1	28.33	1.79	28.	15	0.05	0.30
0.30	0.30	4.84	10.9	28.44	2.25	28.	20	0.08	0.38
0.35	0.35	6.25	16.6	28.56	2.66	28.	26	0.11	0.46
0.40	0.40	7.66	23.3	28.67	3.04	28.	31	0.14	0.54
0.45	0.45	9.08	30.4	29.29	3.35	29.	54	0.17	0.62
0.50	0.50	10.63	36.8	32.62	3.46	32.	17	0.19	0.69
0.55	0.55	12.28	45.8	33.72	3.73	33.	24	0.22	0.77
0.60	0.60	13.95	56.6	33.77	4.06	33.	27	0.26	0.86
0.65	0.65	15.61	68.2	33.83	4.37	33.	30	0.30	0.95
0.70	0.70	17.28	80.6	33.89	4.67	33.	33	0.34	1.04

B-3

\*\*\*\*
PC PROGRAM STREAM

STREET @ S=.72%

MANNING'S N= .017 SLOPE= .0072

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIS	r F	ELEV
1	0.00	0.70	5	7.00	0.24	9	33.17	0.	. 44
2	4.17	0.78	6	19.00	0.00	10	33.83		. 44
3	4.63	0.78	7 :	31.00-	0.24	11	38.00		. 52
4	5.00	0.11	8	33.00	0.11	12	0.00		.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWI		EL	ENERGY
	INC	AREA	RATE	PER	VEL			EAD	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT	,	FT)	(FT)
0.05	0.05	0.13	0.1	5.00	0.63	5.0		0.01	0.06
0.10	0.10	0.50	0.5	10.00	1.01	10.		0.02	0.12
0.15	0.15	1.15	1.5	16.33	1.27	16.		0.02	0.17
0.20	0.20	2.13	3.2	22.98	1.52	22.		0.04	0.24
0.25	0.25	3.43	6.2	28.33	1.82	28.	<del></del>	0.05	0.30
0.30	0.30	4.84	11.0	28.44	2.28	28.		0.08	0.38
0.35	0.35	6.25	16.8	28.56	2.69	28.		0.11	0.46
0.40	0.40	7.66	23.6	28.67	3.08	28.	31	0.15	0.55
0.45	0.45	9.08	30.9	29.29	3.40	29.	54	0.18	0.63
0.50	0.50	10.63	37.4	32.62	3.51	32.	17	0.19	0.69
0.55	0.55	12.28	46.5	33.72	3.78	33.	24	0.22	0.77
0.60	0.60	13.95	57.4	33.77	4.11	33.	27	0.26	0.86
0.65	0.65	15.61	69.1	33.83	4.43	33.	30	0.30	0.95
0.70	0.70	17.28	81.8	33.89	4.73	33.	33	0.35	1.05

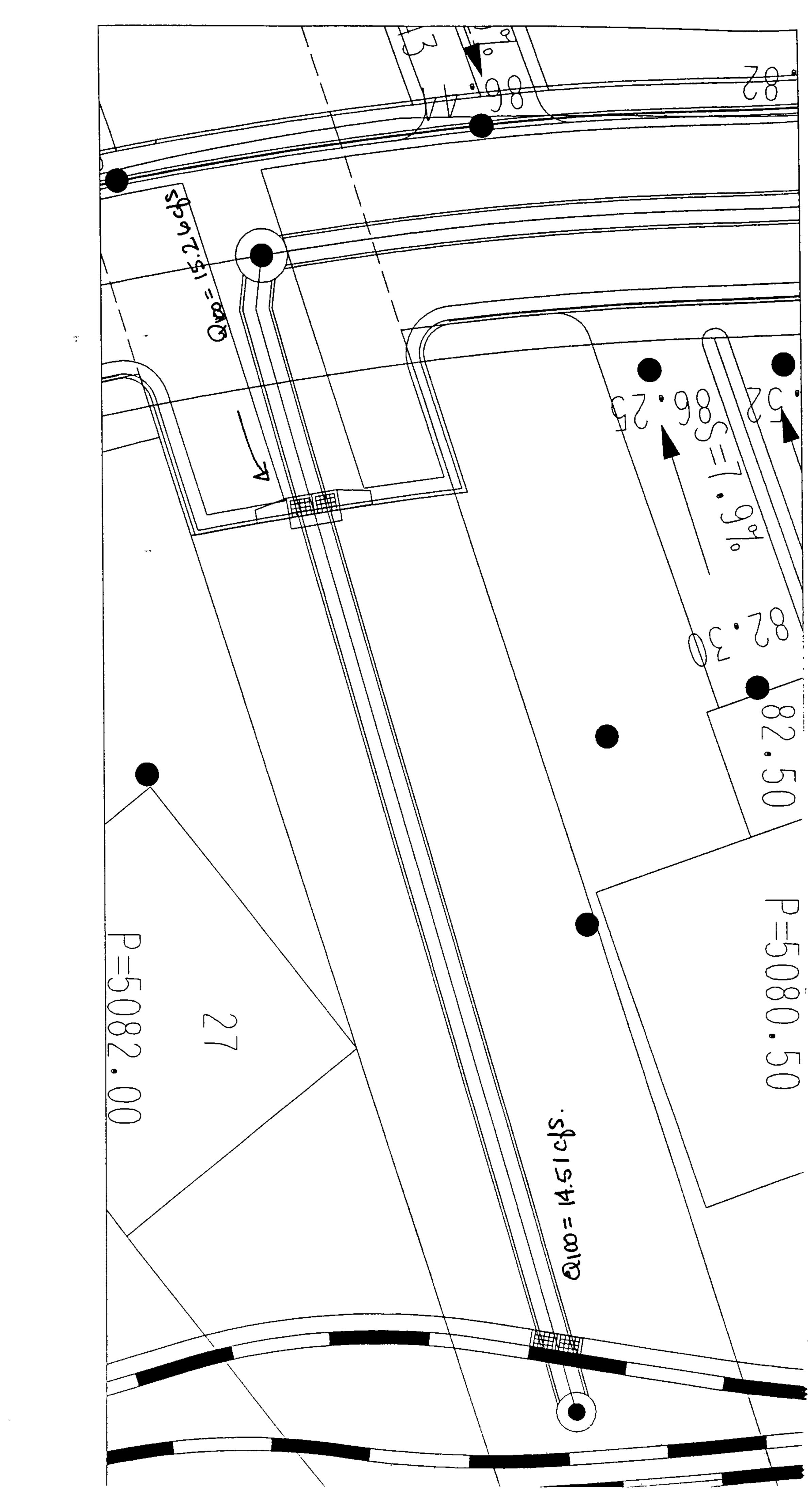
STREET @ S=1.20%

MANNING'S N= .017 SLOPE= .012

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	]	ELEV
1	0.00	0.70	5	7.00	0.24	9	33.17	0	. 44
2	4.17	0.78	6	19.00	0.00	10	33.83	0	. 44
3	4.63	0.78	7	31.00	0.24	11	38.00	0	.52
4	5.00	0.11	8	33.00	0.11	12	0.00	0	.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWII	) VEI	ı	ENERGY
	INC	AREA	RATE	PER	VEL		HEA	7D	HEAD
(FT)	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(F)	?)	(FT)
0.05	0.05	0.13	0.1	5.00	0.82	5.0		.01	0.06
0.10	0.10	0.50	0.6	10.00	1.30	10.0	0.0	. 03	0.13
0.15	0.15	1.15	1.9	16.33	1.63	16.3	27 0.	.04	0.19
0.20	0.20	2.13	4.2	22.98	1.96	22.	37 0	.06	0.26
0.25	0.25	3.43	8.0	28.33	2.34	28.	15 0	.09	0.34
0.30	0.30	4.84	14.2	28.44	2.94	28.	_	. 13	0.43
0.35	0.35	6.25	21.7	28.56	3.48	28.	26 0	.19	0.54
0.40	0.40	7.66	30.5	28.67	3.97	28.	31 0	. 25	0.65
0.45	0.45	9.08	39.9	29.29	4.39	29.	54 0	.30	0.75
0.50	0.50	10.63	48.2	32.62	4.54	32.	17 0	.32	0.82
0.55	0.55	12.28	60.0	33.72	4.88	33.	24 0	.37	0.92
0.60	0.60	13.95	74.1	33.77	5.31	33.		. 44	1.04
0.65	0.65	15.61	89.3	33.83	5.72	33.	30 0	.51	1.16
0.70	0.70	17.28	105.6	33.89	6.11	33.	33 0	.58	1.28

APPENDIX C

INLET ANALYSIS



#### Double Type "A" Sump

#### Oxbow Village

ANALYSIS OF AN INLET IN A SUMP CONDITION -

Oxbow Village Lane

INLET TYPE: Double Grate Type "A" with curb opening wings on both sides on inlet.

WEIR:

 $Q=C*L*H^1.5$ 

ORIFICE: Q=C\*A\*(2\*G\*H)\*\*0.5

Wing opening

Grate opeining

Grate opening

Wing opening

C = 3.0

C = 3.0

 $Q=3.0(4.0')H**1.5=12.0H**1.5 Q=3.0(8.94)H^1.5=26.82*H^1.5$ 

C = 0.6

C = 0.6

L=4.0 ft

L(double grate)=[2(2.67')+2(1.8')]=8 A(double grate)=8.19 sf A=2.0 sf

Q=4.194\*(64.4\*H)^0.5 Q=1.2\*(64.4\*H)^0.5

			Q (CFS)	Q (CFS)	Q (CFS)	TOTAL	
			WEIR	WEIR	ORIFICE	Q	
	WS	HEIGHT	"A"	DOUBLE	DOUBLE	(CFS)	
	ELEVATION	ABOVE INLET	<b>OPENING</b>	GRATE	GRATE		COMMENTS:
-FL @ INLET	0.00	0.00	0.00	0.00	0.00	0.00	Flow at double "A" inlet w/ two wing openings
	0.10	0.10	0.38	0.85	12.47	1.61	Weir controls on grate analysis
	0.20	0.20	1.07	2.40	17.64	4.55	
	0.30	0.30	1.97	4.41	21.60	8.35	
<u></u>	0.40	0.40	3.04	6.78	24.94	12.86	
	0.50	0.50	4.24	9.48	27.88	17.97	Q(100  yr) = 15.26  cfs is provided at this depth
<u></u>	0.60	0.60	5.58	12.46	30.55	23.62	
TOP OF CURB	0.70	0.70	7.03	15.71	32.99	29.76	
	0.80	0.80	8.59	19.19	35.27	36.36	Q(2x100  yr) = 30.50  cfs is provided at this depth
	0.90	0.90	10.25	22.90	37.41	43.39	· · · · · · · · · · · · · · · · · · ·
ROW LIMIT	1.00	1.00	12.00	26.82	39.43	50.82	

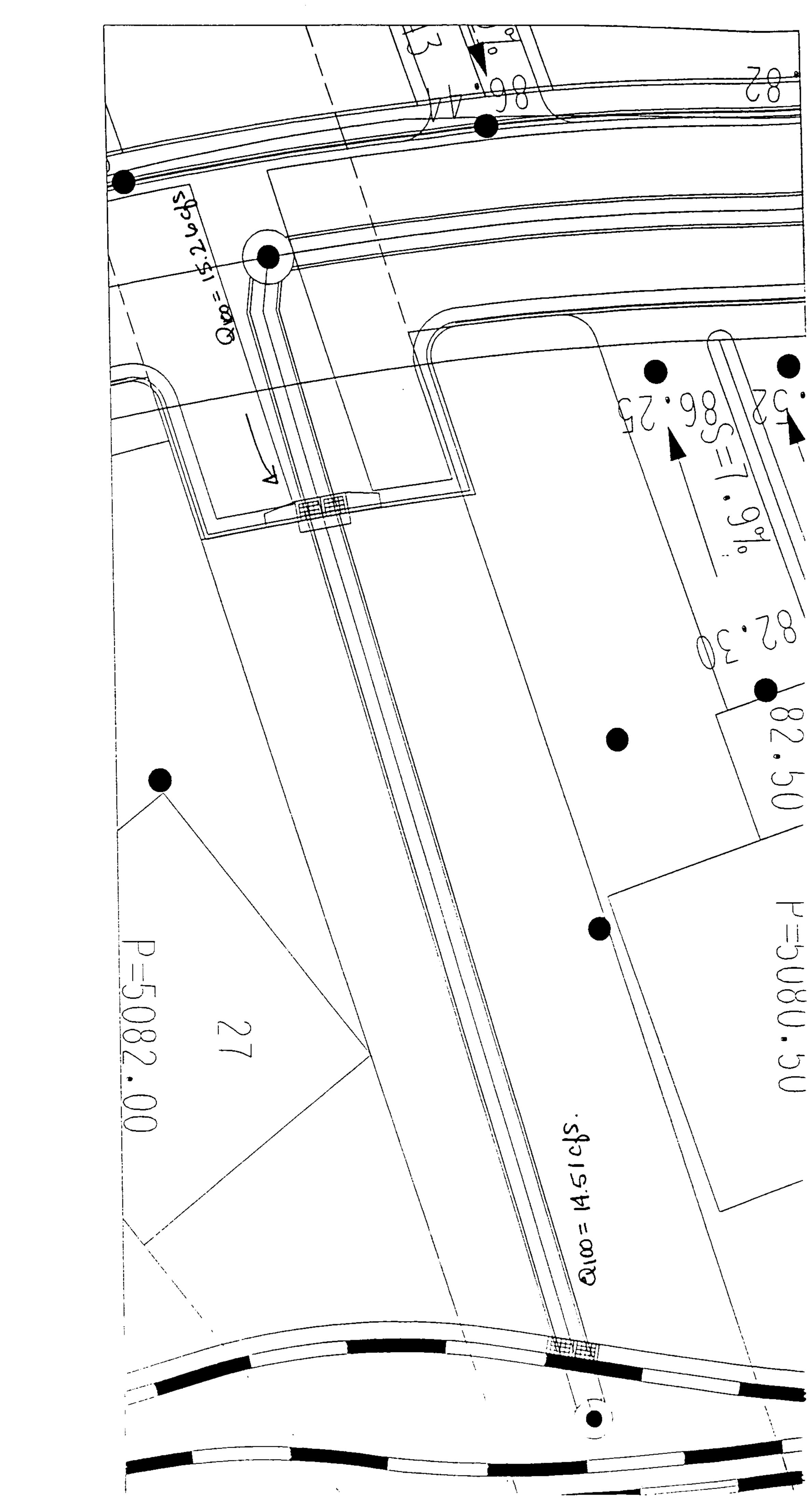
NOTE:

The total runoff intercepted by the inlet at the low point in the road is:

Qr(100) = 2\*[(runoff of the wing opening) + (the lesser of the weir or orifice amount taken by the double grate)].

THE 100 YR STORM EVENT = 15.26 CFS at the sump condition THE 2 x 100 YR STORM EVENT = 30.50 at the sump condition

# APPENDIX D RIBBON CHANNEL ANALYSIS



#### Oxbow Village

ANALYSIS OF AN INLET IN A SUMP CONDITION -

Ribbon Channel

INLET TYPE: Double Grate Type "C" inlet.

WEIR:

 $Q=C*L*H^1.5$ 

ORIFICE: Q=C\*A\*(2\*G\*H)\*\*0.5

Wing opening

Grate opeining

Grate opening

Wing opening

C = 3.0

C = 3.0

 $Q=3.0(4.0')H**1.5=12.0H**1.5 Q=3.0(8.94)H^1.5=26.82*H^1.5$ 

C = 0.6

C = 0.6

L=4.0 ft

L(double grate)=[2(2.67')+2(1.8')]=8 A(double grate)=8.19 sf A=2.0 sf

Q=4.194\*(64.4\*H)^0.5 Q=1.2\*(64.4\*H)^0.5

			Q (CFS)	Q (CFS)	Q (CFS)	TOTAL	
			WEIR	WEIR	ORIFICE	Q	
	WS	HEIGHT	"A"	DOUBLE	DOUBLE	(CFS)	
	ELEVATION	ABOVE INLET	OPENING	GRATE	GRATE		COMMENTS:
~FL @ INLET	0.00	0.00	0.00	0.00	0.00	0.00	Flow at double "C" inlet
	0.10	0.10	0.38	0.85	12.47	1.61	Weir controls on grate analysis
	0.20	0.20	1.07	2.40	17.64	4.55	
	0.30	0.30	1.97	4.41	21.60	8.35	
	0.40	0.40	3.04	6.78	24.94	12.86	
	0.50	0.50	4.24	9.48	27.88	17.97	Q(100  yr) = 14.51  cfs is provided at this depth
	0.60	0.60	5.58	12.46	30.55	23.62	
TOP OF CURB	0.70	0.70	7.03	15.71	32.99	29.76	
	0.80	0.80	8.59	19.19	35.27	36.36	Q(2x100  yr) = 29.02  cfs is provided at this depth
	0.90	0.90	10.25	22.90	37.41	43.39	· · · · · · · · · · · · · · · · · · ·
ROW LIMIT	1.00	1.00	12.00	26.82	39.43	50.82	

NOTE:

The total runoff intercepted by the inlet at the low point in the road is:

Qr(100) = (the lesser of the weir or orifice amount taken by the double grate)].

THE 100 YR STORM EVENT = 14.51 CFS at the sump condition THE 2 x 100 YR STORM EVENT = 29.02 at the sump condition

****	PC PROGRAM STREAM	SEPTEMBER 1994
------	-------------------	----------------

Ribbon Ch		@ S=0.50%	_	· ^ -				
MANNING	G'S N=	.017 SLO		05	ተግተ ተግኝ ፖ	$D \cap TMM$	DIST	ELEV
POINT	DIST	ELEV	POINT		ELEV	POINT		0.00
1	0.00	1.00	4	3.00	0.00	_	0.00	0.00
2	0.50	1.00	5	3.00	1.00	_	0.00	
3	0.50	0.00	6	3.50	1.00	9	0.00	0.00
WSEL	DEPTH	FLOW	FLOW	WETTED	FLOW	TOPWID	VEL	ENERGY
	INC	AREA	RATE	PER	VEL.	( <del></del>	HEAD	HEAD
2 (FT)*	(FT)	SQ.FT.	(CFS)	(FT)	(FPS)	(FT)	(FT)	(FT)
0.05	0.05	0.13	0.1	2.60	0.82	2.50		
0.10	0.10	0.25	0.3	2.70	1.27	2.50		
0.15	0.15	0.38	0.6	2.80	1.62	2.50		
0.20	0.20	0.50	1.0	2.90	1.91	2.50		
0.25	0.25	0.63	1.4	3.00	2.17	2.50		
0.30	0.30	0.75	1.8	3.10	2.40	2.50		
0.35	0.35	0.88	2.3	3.20	2.60	2.50		
0.40	0.40	1.00	2.8	3.30	2.79	2.50		
0.45	0.45	1.13	3.3	3.40	2.96	2.5	0.1	
0.50	0.50	1.25	3.9	3.50	3.11	2.50	0 0.1	
0.55	0.55		4.5	3.60	3.25	2.5	0 0.1	•
0.60	0.60	1.50	5.1	3.70	3.39	2.5	0 0.1	8 0.78
0.65	0.65	1.63	5.7	3.80	3.51	2.5	0 0.1	9 0.84
0.03	0.70	1.75	6.3		3.62	2.5	0 0.2	0.90
0.75	0.75		7.0		3.73	2.5	0 0.2	2 0.97
0.80	0.80		7.7	_	3.83	2.5	0 0.2	3 1.03
0.85	0.85	_	_ <del>8.3</del>		3.92	2.5	0 0.2	4 1.09
0.90	0.90		9.0		4.01	2.5	0 0.2	5 1.15
0.95	0.95		9.7		4.10	2.5	0 0.2	6 1.21
1.00	1.00		10.4		4.18	3.5	0 0.2	7 1.27
1.00		<del>-</del> -						

\*\*\*\*

# APPENDIX E STORM DRAIN ANALYSIS

## CULVERT RATING TABLE

## 48. INCH DIAMETER PIPE

N = 0.01300 INCREMENT = 0.50 SLOPE = 0.04300

FLOW DEPTH (IN)	FLOW AREA (SQ FT)	DISCHARGE (CFS)	VELOCITY (FPS)
		0.04898 0.21854 0.52272 0.96862 1.56055 2.30125 3.19242 4.23499 5.42930 6.77523 8.27225 9.91953 11.71597 13.66019 15.75064 17.98552 20.36294 22.88074 25.53674 28.32852 31.25362 34.30937 37.49311 40.80195 44.23300 47.78323 51.44952 55.22869 59.11745 63.11246 67.21028 71.40740 75.70026 80.08516 84.55841 89.11620 93.75468 98.46993 103.25796 108.11620 93.75468 98.46993 103.25796 108.11620 93.75468 98.46993 103.25796 108.11620 93.75468 98.46993 103.25796 108.31466 113.03595	(FPS)  2.16667 3.42851 4.47789 5.40664 6.25293 7.03731 7.77253 8.46714 9.12726 9.75743 10.36116 10.94125 11.49993 12.03907 12.56021 13.06467 13.55357 14.02786 14.48842 14.93597 15.37118 15.79461 16.20680 16.60820 16.99925 17.38031 17.75173 18.11383 18.46688 18.81116 19.14688 19.47429 19.79357 20.10491 20.40847 20.70442 20.99289 21.27402 21.54792 21.54792 21.54792 21.54792 21.54792 21.54792 21.54792 21.54792 21.54792 21.54792
21.00000 21.50000 22.00000 22.50000 23.00000	5.28579 5.45135 5.61728 5.78350 5.94994	118.01762 123.05542 128.14499 133.28195 138.46187	22.32734 22.57337 22.81263 23.04519 23.27114
FLOW DEPTH (IN)	FLOW ARE		VELOCITY (FPS)

Page 1

Cul-2.out

23.50000	6.11652	143.68015 148.93219	23.49050 23.70333	Reach
24.00000	6.28318 6.44983	154.21336	23.90967	
25.00000	6.61641	159.51894	24.10958	Q100 = 149.80
25.50000	6.78285	164.84396	24.30305	d = 24.08 in
26.00000	6.94907	170.18359	24.49012	
26.50000	7.11500	175.53288	24.67081	
27.00000	7.28057	180.88675	24.84514	
27.50000	7.44570	186.23996	25.01310	
28.00000	7.61031	191.58730	25.17469	
28.50000	7.77434	196.92343	25.32991	
29.00000	7.93771	202.24292	25.47875	
29.50000	8.10034	207.54016	25.62117 25.75717	
30.00000	8.26215	212.80952	25.88670	
30.50000	8.42306	218.04524 223.24139	26.00973	
31.00000	8.58300 8.74187	223.24133	26.12620	
31.50000	8.89961	233.49066	26.23606	
32.00000	9.05612	238.53130	26.33925	
32.50000	9.03012	243.50723	26.43569	
33.00000 33.50000	9.36509	248.41190	26.52530	
34.00000	9.51738	253.23836	26.60798	
34.50000	9.66808	257.97943	26.68362	
35.00000	9.81709	262.62793	26.75212	
35.50000	9.96431	267.17621	26.81332	
36.00000	10.10963	271.61633	26.86709	
36.50000	10.25295	275.94009	26.91325	
37.00000	10.39414	280.13907	26.95162	
37.50000	10.53310	284.20404	26.98198	
38.00000	10.66970	288.12570	27.00410	
38.50000	10.80380	291.89398	27.01771	
39.00000	10.93527	295.49835	27.02249	
39.50000	11.06396	298.92737	27.01811	
40.00000	11.18971	302.16895	27.00418	
40.50000	11.31236	305.20984	26.98022	
41.00000	11.43171	308.03555	26.94571	
41.50000	11.54758	310.63022	26.90003	
42.00000	11.65975	312.97598	26.84243	
42.50000	11.76798	315.05286	26.77205 26.68779	•
43.00000	11.87200	316.83746	26.58833	
43.50000	11.97152	318.30273 319.41611	26.30033	
44.00000	12.06620 12.15563	320.13705	26.33652	
44.50000 45.00000	12.23936	320.13703	26.17898	
45.50000	12.23930	320.17709	25.99516	
45.50000	12.31079	319.32672	25.77874	
46.50000	12.36721	317.70551	25.51927	
47.00000	12.50262	315.02682	25.19686	
47.50000	12.54376	310.59998	24.76131	
48.00000	12.56637	297.86487	23.70334	
<del>_</del>				

### CULVERT RATING TABLE

## 48. INCH DIAMETER PIPE

N = 0.01300	INCREMENT	· = 0.50	SLOPE = 0.01400
FLOW DEPTH (IN)	FLOW AREA	DISCHARGE	VELOCITY
	(SQ FT)	(CFS)	(FPS)
(IN)  0.50000 1.00000 1.50000 2.00000 2.50000 3.00000 3.50000 4.00000 4.50000 5.50000 6.00000 7.50000 8.00000 9.00000 10.50000 10.50000 11.50000 11.50000 12.50000 12.50000 13.00000 13.50000 14.50000 15.50000 15.50000 15.50000 15.50000 16.50000 17.50000 17.50000 17.50000 18.50000 19.50000 19.50000 19.50000	0.02260 0.06374 0.11673 0.17915 0.24957 0.32701 0.41073 0.50017 0.59484 0.69437 0.79839 0.90662 1.01879 1.13465 1.25401 1.37665 1.50240 1.63109 1.76256 1.89666 2.03326 2.17222 2.31342 2.45673 2.60206 2.74927 2.89828 3.04898 3.20127 3.35506 3.51025 3.66675 3.82449 3.98336 4.14330 4.30421 4.46602 4.62865 4.79201 4.95604	0.02795 0.12470 0.29827 0.55269 0.89044 1.31309 1.82159 2.41647 3.09795 3.86593 4.72012 5.66006 6.68510 7.79447 8.98727 10.26249 11.61904 13.05569 14.57120 16.16418 17.83324 19.57684 21.39347 23.28149 25.23923 27.26499 29.35696 31.51335 33.73227 36.01181 38.35001 40.74488 43.19437 45.69638 48.24881 50.84948 53.49618 56.18669 58.91873 61.68995	1.23630 1.95630 2.55507 3.08501 3.56790 4.01547 4.43499 4.83133 5.20799 5.56756 5.91205 6.24305 6.56183 6.86946 7.16683 7.45467 7.73363 8.00426 8.52243 8.77076 9.01236 9.24756 9.47660 9.69973 9.91716 10.12909 10.33570 10.53715 10.73360 10.92516 11.11198 11.29416 11.47181 11.64502 11.81389 11.97849 12.13890 12.29519 12.44742
20.50000	5.12066	64.49802	12.59565
21.00000	5.28579	67.34055	12.73993
21.50000	5.45135	70.21510	12.88031
22.00000	5.61728	73.11920	13.01683
22.50000	5.78350	76.05034	13.14953
23.00000	5.94994	79.00598	13.27845
FLOW DEPTH	FLOW ARE	A DISCHARGE	E VELOCITY
(IN)	(SQ FT)	(CFS)	(FPS)

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Cul-3.out

23.50000 24.00000 25.00000 25.00000 26.00000 26.50000 27.00000 28.00000 29.50000 30.00000 31.00000 31.50000 31.50000 32.00000 32.50000 33.00000 34.00000 34.50000 35.00000 35.50000 36.00000 36.50000 37.50000	6.11652 6.28318 6.44983 6.61641 6.78285 6.94907 7.11500 7.28057 7.44570 7.61031 7.77434 7.93771 8.10034 8.26215 8.42306 8.58300 8.74187 8.89961 9.05612 9.21131 9.36509 9.51738 9.66808 9.81709 9.96431 10.10963 10.25295 10.39414 10.53310 10.66970 10.80380 10.93527 11.06396 11.18971 11.31236 11.43171 11.54758 11.65975 11.65975 11.65975 11.65975 11.76798 11.87200 11.97152 12.06620 12.15563 12.23936 12.31679 12.38721	81.98351 84.98033 87.99375 91.02109 94.05954 97.10632 100.15862 103.21351 106.26804 109.31921 112.36401 115.39928 118.42188 121.42856 124.41605 127.38097 130.31987 133.22917 136.10535 138.94461 141.74318 144.49715 147.20241 149.85483 152.45004 154.98357 157.45071 159.84662 162.16609 164.40376 166.55396 168.61058 170.56720 172.41681 174.15193 175.76428 177.24481 174.15193 175.76428 177.24481 178.58328 179.76834 180.78664 181.62273 182.62937 182.69233 182.20702	13.40361 13.52506 13.64280 13.75686 13.86726 13.97400 14.07710 14.17657 14.27241 14.36461 14.45318 14.61938 14.61938 14.69697 14.77088 14.90754 14.90754 14.90754 15.02911 15.08414 15.13527 15.18244 15.22561 15.26469 15.33029 15.35663 15.37853 15.39585 15.40847 15.41623 15.41623 15.41647 15.40851 15.39484 15.37515 15.37606 14.83277 14.70928	Reach 2 Q100 = 165.06 d = 38.15 in
44.50000 45.00000 45.50000	12.15563 12.23936 12.31679	182.82735 182.69223	14.93766 14.83277	
46.00000 46.50000 47.00000 47.50000 48.00000	12.38721 12.44963 12.50262 12.54376 12.56637	182.20702 181.28195 179.75349 177.22755 169.96094	14.70928 14.56123 14.37726 14.12874 13.52507	

## CULVERT RATING TABLE

## 48. INCH DIAMETER PIPE

	FLOW AREA	DISCHARGE		
(IN)	(52 - 1)	(CFS)	VELOCITY (FPS)	
0.50000	0.02260 0.06374 0.11673 0.17915 0.24957 0.32701 0.41073 0.50017 0.59484 0.69437 0.79839 0.90662 1.01879 1.13465 1.25401 1.37665 1.50240 1.63109 1.63109 1.76256 1.89666 2.03326 2.17222 2.31342 2.45673 2.60206 2.74927 2.89828 3.04898 3.20127 3.35506 3.51025 3.66675 3.82449	0.08044 0.35895 0.85855 1.59091 2.56314 3.77971 5.24343 6.95580 8.91741 11.12804 13.58684 16.29244 19.24301 22.43632 25.86979 29.54049 33.44530 37.58070 41.94308 46.52846 51.33282 56.35176 61.58093 67.01557 72.65093 78.48204 84.50377 90.71091 97.09805 103.65969 110.39021 117.28380 124.33463	(FPS)  3.55867 5.63119 7.35476 8.88019 10.27018 11.55850 12.76608 13.90694 14.99116 16.02618 17.01780 17.97056 18.88818 19.77369 20.62965 21.45819 22.26119 23.04021 23.79666 24.53174 25.24655 25.94201 26.61902 27.27831 27.92059 28.54647 29.15651 29.75124 30.33111 30.89657 31.44799 31.98574 32.51014	
16.50000 17.00000 17.50000 18.00000	3.82449 3.98336 4.14330 4.30421	124.33463 131.53667 138.88380 146.36980	32.51014 33.02150 33.52010 34.00618	
18.50000 19.00000 19.50000 , 20.00000	4.46602 4.62865 4.79201 4.95604	153.98833 161.73291 169.59706 177.57401	34.47998 34.94172	Reach 3
20.0000 20.50000 21.00000 21.50000	5.12066 5.28579 5.45135	185.65701 193.83922 202.11359	36.25646 36.67177 37.07585	$Q_{100}=779.77$ $d=20.14$ in
22.00000 22.50000 23.00000 FLOW DEPTH (IN)	5.61728 5.78350 5.94994 FLOW ARE (SQ FT)	210.47301 218.91026 227.41805 A DISCHARGE (CFS)	37.46883 37.85081 38.22190 VELOCITY (FPS)	

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23.50000	6.11652	235.98888	38.58220
24.00000	6.28318	244.61516	38.93176
24.50000	6.44983	253.28928	39.27068
25.00000	6.61641	262.00345	39.59901
25.50000	6.78285	270.74960	39.91678
26.00000	6.94907	279.51974	40.22404
26.50000	7.11500	288.30576	40.52083
27.00000	7.28057	297.09921	40.80715
27.50000	, , , , , , ,	305.89166	41.08301
28.00000	7.61031	314.67444	41.34842
28.50000	7.77434	323.43884	41.60336
29.00000	7.93771	332.17587	41.84782
29.50000	8.10034	340.87643	42.08175
30.0000	8.26215	349.53113	42.30512
30.50000	8.42306	358.13058	42.51787
31.00000	8.58300	366.66507	42.71994
31.50000	8.74187	375.12466	42.91124
32.00000	8.89961	383.49905	43.09168
32.50000	9.05612	391.77811	43.26117
33.00000	9.21131	399.95090	43.41956
33.50000	9.36509	408.00659	43.56674
34.00000	9.51738	415.93387	43.70255
34.50000	9.66808	423.72089	43.82678
35.00000	9.81709	431.35587	43.93928
35.50000	9.96431	438.82617	44.03981
36.00000	10.10963	446.11893	44.12812
36.50000	10.25295	453.22058	44.20394
37.00000	10.39414	460.11719	44.26696
37.50000	10.53310	466.79376	44.31683
38.00000	10.66970	473.23489	44.35316
38.50000	10.80380	479.42416	44.37550
39.00000	10.93527	485.34421	44.38337
39.50000	11.06396	490.97626	44.37617
40.00000	11.18971	496.30038	44.35328
40.50000	11.31236	501.29489	44.31393
41.00000	11.43171	505.93604	44.25725
41.50000	11.54758	510.19769	44.18222
42.00000	11.65975	514.05054	44.08762
42.50000	11.76798	517.46167	43.97202
43.00000	11.87200	520.39282	43.83363
43.50000	11.97152	522.79950	43.67027
44.00000	12.06620	524.62817	43.47916
44.50000	12.15563	525.81226	43.25667
45.00000	12.23936	526.26697	42.99792
45.50000	12.31679	525.87805	42.69601
46.00000	12.38721	524.48138	42.34055
46.50000	12.44963	521.81860	41.91439
47.00000	12.50262	517.41895	41.38483
47.50000	12.54376	510.14804	40.66947
48.00000	12.56637	489.23114	38.93179

## **EXHIBITS**

EXHIBIT 1: BULK LAND PLAT

EXHIBIT 2: PRELIMINARY PLAT

EXHIBIT 3 EXISTING DRAINAGE BASIN MAP

EXHIBIT 4 PROPOSED DRAINAGE BASIN MAP

EXHIBIT 5 PROPOSED ON-SITE BASIN MAP

EXHIBIT 6 GRADING AND DRAINAGE PLAN

**EXHIBIT 1** 

**BULK LAND PLAT** 

# EXHIBIT 2 PRELIMINARY PLAT

## EXHIBIT 3 EXISTING DRAINAGE BASIN MAP

## EXHIBIT 4 PROPOSED DRAINAGE BASIN MAP

## EXHIBIT 5 PROPOSED ON-SITE BASIN MAP

## EXHIBIT 6 GRADING AND DRAINAGE PLAN



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