

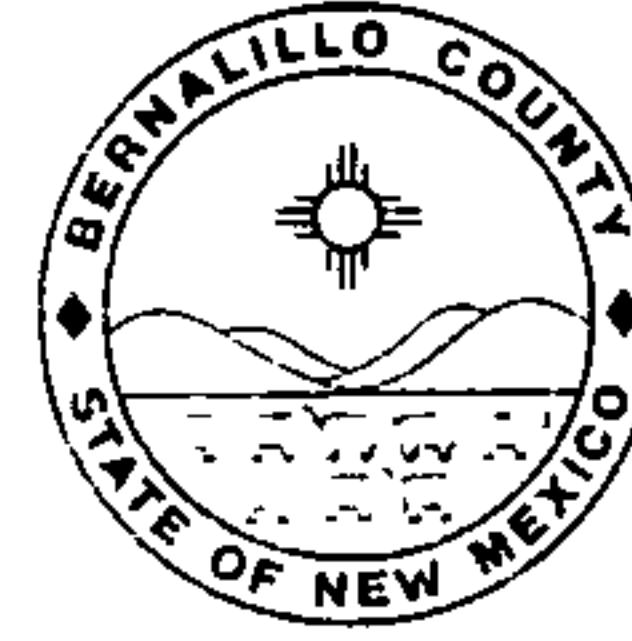
County of Bernalillo

State of New Mexico

BOARD OF COUNTY COMMISSIONERS

BARBARA J. SEWARD, CHAIR
DISTRICT 4
TOM RUTHERFORD, VICE CHAIR
DISTRICT 3
KEN SANCHEZ, MEMBER
DISTRICT 1
STEVE D. GALLEGOS, MEMBER
DISTRICT 2
LES HOUSTON, MEMBER
DISTRICT 5
JUAN R. VIGIL, COUNTY MANAGER

MARK CARRILLO, ASSESSOR
JUDY D. WOODWARD, CLERK
IRA ROBINSON, PROBATE JUDGE
JOE BOWDICH, SHERIFF
ORLANDO VIGIL, TREASURER



2400 BROADWAY, S.E.
ALBUQUERQUE, NEW MEXICO 87102
PUBLIC WORKS (505) 848-1500

March 16, 2001

Ronald R. Bohannan
Tierra West, LLC
8509 Jefferson NE
Albuquerque New Mexico 87113

RE: Grading and Drainage Plan for National Electric Supply (G15-D59) (PWDN 10018)
Dated January 25, 2001

Dear Mr. Bohannan:

The above referenced plan received January 30, 2001 is approved for building permit. Upon completion of the project the engineer will certify the project.

The following items are required by Bernalillo County.

1. Development of this property must conform to this plan. Any proposed changes will require a revision to the plan prior to initiating the proposed change.
2. Inspection required, applicant required to obtain permits from Bernalillo County Public Works Division (Phone 848-1520).
3. The old pond must be filled in with suitable material, and all garbage must be removed.

If you have any questions please contact me at 924-3982.

Sincerely,



Carlos A. Montoya
City/County Floodplain Administrator

C: Lynn Mazur, AMAFCA
Brian Kent, Bernalillo County

~~File~~



Brian Kent
<bkent@mercury.bern
co.gov>

03/15/01 10:43 AM

To: "Carlos Montoya" <cmontoya@cabq.gov>
cc:
Subject: National Electric

Carlos,

These are our comments for the National Electric Grading and Drainage Plan, PWDN #10018.

BK

Based on the information provided on the grading and drainage plan, with engineer's seal dated 01/25/01, this plan appears to meet the requirements established for this area and is therefore acceptable.

Development of this property must conform to this plan. Any proposed changes will require a revision to the plan prior to initiating the proposed change.

Inspection required, applicant required to obtain permits from Bernalillo County Public Works Division. Phone (505)848-1520.

The old pond must be filled in with suitable material, and all garbage must be removed.

BERNALILLO COUNTY

PwD 10123

PWD SUBMITTAL

Use for all PWD applications EXCEPT Street Excavation

- NEW SUBMITTAL
 RESUBMITTAL
 FINAL SIGNOFF

TODAY'S DATE

1/30/01

CASE NO:

PWDN 10018

OWNER

OWNER	Rocky E. Lawrence	PHONE	(505) 345-3577
MAILING ADDRESS	702 Carmony Road, NE	CITY	Alb.
		ZIP	87107

AGENT

AGENT / CONTRACTOR	Tierra West, LLC	PHONE	(505) 858-3100
MAILING ADDRESS	8509 Jefferson, NE	CITY	Alb.
STATE LICENSE NO.	EXP DATE	VOLUME	CLASS

ARCHITECT/ENGINEER	Ronald R. Bohannan	LICENSE NO.	7868	PHONE	(505) 858-3100
--------------------	--------------------	-------------	------	-------	----------------

SITE INFORMATION

SITE ADDRESS / DIRECTIONS	National Electric Supply	ZONE ATLAS NO.:	G-15
Northeast corner of Carmony Road and Alexander Drive			
LEGAL DESCRIPTION	Tract 2-B Land of J.R. Nance		
			LOT SIZE: 4.04 Acres
EXISTING BUILDING(S) AND USE:	National Electric Supply Bldg		
UPC #	PROPOSED BUILDING(S):		
1 0 1 5 0 6 0 4 4 7 3 8 7 1 0 1 2 9			

TYPE OF SUBMITTAL

- | | |
|---|--|
| <input type="checkbox"/> REPLAT | <input type="checkbox"/> TRAFFIC IMPACT ANALYSIS / TRAFFIC STUDY |
| <input type="checkbox"/> MINOR SUBDIVISION | <input type="checkbox"/> INFRASTRUCTURE LIST / DESIGN REVIEW |
| <input type="checkbox"/> MAJOR SUBDIVISION | <input type="checkbox"/> SPECIAL USE PERMIT |
| <input type="checkbox"/> CONSTRUCTION DRAWINGS | <input type="checkbox"/> BARRICADING PERMIT |
| <input checked="" type="checkbox"/> GRADING & DRAINAGE PLAN | <input checked="" type="checkbox"/> BUILDING PERMIT |
| <input type="checkbox"/> AS-CONSTRUCTED GRADING & DRAINAGE PLAN | <input type="checkbox"/> INSPECTION |
| <input type="checkbox"/> VARIANCE REQUEST | <input type="checkbox"/> OTHER (Specify): |
| <input type="checkbox"/> LAND DIVISION | |

The issuance of a permit or a review or approval of plan specifications, computations, and shop drawings, shall not be interpreted to be a permit for, or an approval of any variance or violation of any of the provisions of any COUNTY or STATE codes, ordinances, standards, or policies. Nor shall such issuance of a permit or approval of plans, specifications, computations, and shop drawings prevent any authorized COUNTY representative or COUNTY inspector from thereafter requiring the correction of errors in said plans, specifications, computations, or shop drawings or from stopping construction operations which are being carried on thereunder when in violation of any COUNTY or STATE codes, ordinances, standards, or policies.

Owner Agent Contractor

Signature

Date

1/30/01

COUNTY

BERNALILLO COUNTY USE ONLY		
C/R's:		TOTAL FEE:
		Receipt No.:
		Received By:

DRAINAGE REPORT

for

National Electric Supply

Prepared by

Tierra West, LLC
8509 Jefferson NE
Albuquerque, New Mexico 87113

Prepared for

Rocky E. Lawrence
702 Carmony Road, NE
Albuquerque, NM 87107

January 25, 2001

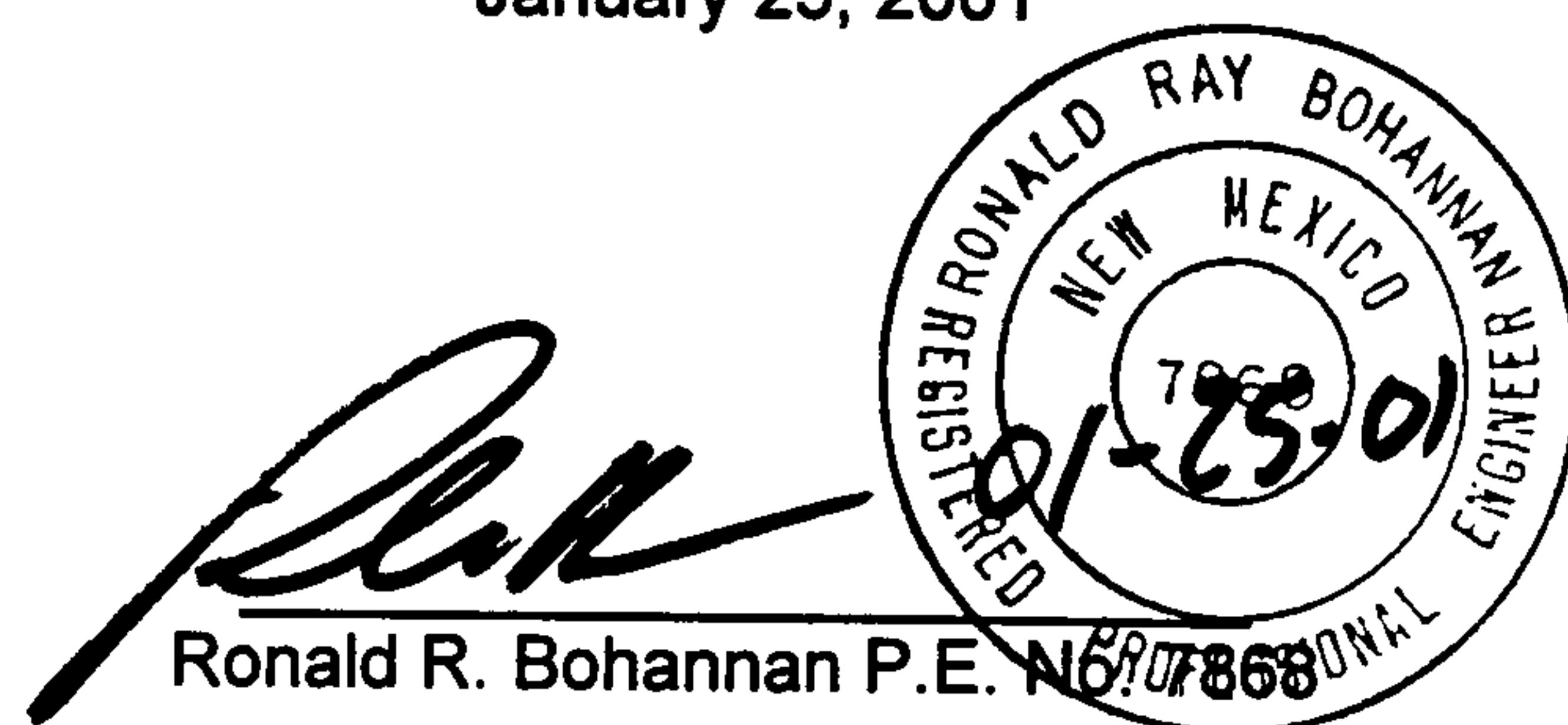


TABLE OF CONTENTS

Exhibit A - Zone Atlas Map G-15-Z	1
Location	2
Existing Drainage Conditions	2
FEMA Map	3
On-Site Drainage Management Plan	3
Criteria	3
Exhibit B - FEMA Map 350001C-0138D	4
Summary	5
Exhibit C - Existing Basin Layout Map	6
Exhibit D - Storm Drain Layout Map	7
Exhibit E - SAD Hymo Basin Parameter Worksheet	8
Exhibit F - SAD Basin Map	9

APPENDICES


Runoff Data	A
Pond Calculations	B
Drop Inlet Calculations	C
Pipe Capacity	D
Emergency Overflow Calculations	E
Runoff Summary Tables for Developed Drainage Basin	F
AHYMO Runoff Input and Summary Output for Developed Drainage Basin	G
AHYMO Input and Summary Output for Pond	H

MAP POCKET

Site Grading and Drainage Plan	A
Paving Sheet	B

EXHIBIT A - ZONE ATLAS MAP: G-15-Z

PAGE 1



Location

National Electric Supply is an existing, established industrial business. The 4.04-acre site is located on the northeast corner of Carmony Road and Alexander Drive. The site is shown on Exhibit A, Zone Atlas Map G-15, on page 1. The legal description is Tract 2-B Land of J.R. Nance. The purpose of this report is to provide the drainage analysis and management plan in connecting the National Electric Supply site's drainage into the new SAD 216 storm drain in Carmony Road and for new parking lot improvements. The new parking lot improvements will consist of the removal of an existing retention pond and the construction of a new detention pond along with repaving the entire parking lot.

Existing Drainage Conditions

The site is currently fully developed with a 56,880 square-foot building and a 100-percent paved parking lot. There is one onsite basin with a 100-year, 24-hour peak discharge of 18.49 cfs. There are no offsite flows that enter the site. Carmony Road, the road fronting the site, is currently under the SAD 216 construction. Before the construction of the SAD, the 18.49 cfs runoff sheet flowed into a retention pond located on the north side of the existing building and had no free discharge, refer to Exhibit C, Existing Basin Layout Map, on page 6. Due to the construction of the SAD in Carmony Road, the retention pond was filled and a new 24" RCP storm drain was stubbed out to the site to allow the developed flow to discharge into a new 84" RCP storm drain in Carmony Road, refer to Exhibit D, Storm Drain Layout Map, on page 7. The drainage basin parameters were set by the SAD 216 under developed conditions shown on Exhibit E, SAD Hymo Basin Parameter Worksheet, on page 8. The National Electric Supply site is located within Basin D-4 of Exhibit F, SAD Basin Map, on page 9, therefore, the developed land treatments for free discharge into the SAD must be less than or equal to 10% B, 10% C, and 80% D. These conditions have limited the free discharge

for the site to 16.95 cfs. Both Exhibit E and Exhibit F were obtained from the Larkin Group's Drainage Report for project SAD-BC-83-1 and SAD 216.

FEMA Map

The site is located on FEMA Map Section 350001C Panel 138D, as shown on Exhibit B page 4. The map shows that the area of the retention pond lies within a 100-year flood plain. However, the SAD 216 has handled upland flows that are in the flood plain and routed those flows into the new 84" RCP. Therefore, the County is responsible for updating the new FEMA Map for this area.

On-Site Drainage Management Plan


As previously mentioned, SAD 216 is currently under construction. The SAD's basin parameters for the developed site restricted its free discharge to 16.95 cfs. Due to, the SAD restrictions, the construction of a small parking lot pond with a storage capacity of .0495 acre-feet is needed to match the discharge rate of the SAD. The pond's maximum water surface elevation is 5029.17 feet. A type double 'D' drop inlet and a 15" orifice plate will control the flow to the 16.95 cfs allowable from the detention pond. The new 24" RCP storm drain pipe will convey the flow from the pond to the new 84" storm drain pipe constructed by SAD 216 located in Carmony Road.

Criteria

The site was analyzed using the procedures outlined in the Development Process Manual Volume 2, Chapter 22. The AHYMO computer program was used to analyze all basins and the

FEMA MAP: 350001C-0138D

PAGE 4



ponds. The existing and proposed conditions for all basins were analyzed for a 100-year, 24-hour rainfall event and an emergency overflow was designed for an additional 100-year, 24-hour storm.

Summary

By making the new parking lot improvements to the site it will allow for free discharge to the SAD 216. The new improvements consist of the removal of the existing retention pond and the construction of a new detention pond along with repaving the entire parking lot. A drop inlet will capture 100-percent of the flows generated within the developed site. A 15" orifice plate will control the flow to the 16.95 cfs allowable from the detention pond and the existing 40-foot wide drivepad will act as an emergency outflow weir located northwest of the drop inlet, refer to Exhibit D, Storm Drain Layout Map, on page 7. The new 24" RCP storm drain pipe will convey the flow from the pond to the new 84" storm drain pipe constructed by SAD 216 located in Carmony Road.

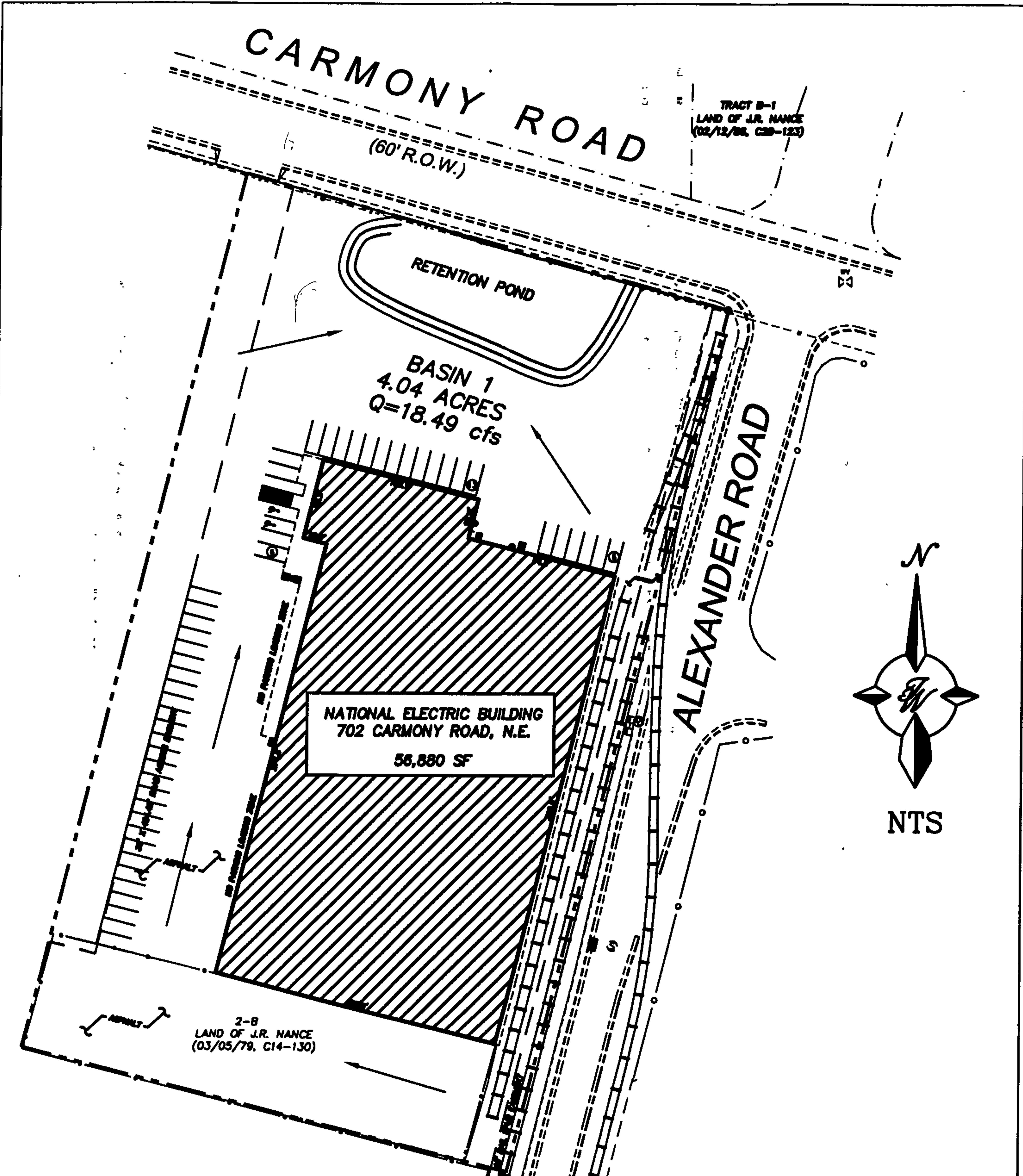


EXHIBIT C - EXISTING BASIN LAYOUT MAP




EXHIBIT D - STORM DRAIN LAYOUT MAP

EXHIBIT E - SAD HYMO BASIN PARAMETER WORKSHEET

AND

EXHIBIT F- SAD BASIN MAP

**NOTE: THESE EXHIBITS ARE FROM THE SAD 216 HYDROLOGY SECTION
AND SHOW THE ALLOWABLE DISCHARGE**

TABLE NO. 2 - CONTD.HYMO BASIN PARAMETER WORKSHEETDEVELOPED CONDITIONS

PROJECT: SAD-BC-83-1 & SAD-216 BY: D.L.A. DATE: 7-3-91

Basin (Sq. Mi.)	Length (ft.)	d Elav (ft.)	Slope (ft/ft)	K	Y (ft/s)	t(c) (hrs.)	t(p) (hrs.)	Surface Treatment Percent		
								A	B	C
D-4	0.0475	100	7.0	0.070	2.0	5.29	0.005	0.003		D
	200	4.0	0.020	2.0	2.83	0.020	0.013			
	300	3.0	0.010	2.0	2.00	0.042	0.028			
	650	2.0	0.003	2.0	1.11	0.163	0.109			
	700	7.0	0.010	3.0	3.00	0.065	0.043			
TOTAL =						0.294	0.196			
D-5	0.0586	550	1.0	0.002	2.0	0.85	0.179	0.119		
	1,100	5.0	0.005	2.0	1.35	0.227	0.151			
	50	6.0	0.120	2.0	6.93	0.002	0.001			
	1,100	20.0	0.018	3.0	4.05	0.076	0.050			
TOTAL =						0.483	0.322			
E-1	0.0506	600	6.0	0.010	2.0	2.00	0.083	0.056		
	1,500	24.0	0.016	3.0	3.79	0.110	0.073			
	700	3.0	0.004	3.0	1.96	0.099	0.066			
TOTAL =						0.292	0.195			
K-1	0.0386	130	26.0	0.200	0.7	3.13	0.012	0.008		
	100	8.0	0.080	2.0	5.66	0.005	0.003			
	150	2.0	0.013	2.0	2.31	0.018	0.012			
	720	5.0	0.007	3.0	2.50	0.080	0.053			
	550	2.5	0.005	3.0	2.02	0.076	0.050			
TOTAL =						0.190	0.127			
								10	10	80

-24-

EXHIBIT E

SAD HYMO BASIN PARAMETER WORKSHEET

12-06-2000 9:00AM

FROM THE LARKIN GROUP 5052750748

P. 3




EXHIBIT F - SAD BASIN MAP

APPENDIX A

Runoff Data

DATA FOR HYDROLOGY CALCULATIONS

RUNOFF CALCULATIONS

THE SITE IS @ ZONE 2

LAND TREATMENT

VARIES (SEE ATTACHMENT)

DEPTH (INCHES) @ 100-YEAR STORM

P60 = 2.01 INCHES

P360 = 2.35 INCHES

P1440 = 2.75 INCHES

DEPTH (INCHES) @ 10-YEAR STORM (FACTOR 0.667)

P60 = 1.34 INCHES

P360 = 1.57 INCHES

P1440 = 1.83 INCHES

* SEE THE SUMMARY OUTPUT FROM AHYMO CALCULATIONS

* ALSO SEE THE FOLLOWING SUMMARY TABLES

LAND TREATMENT

BASIN	CONDITION	A	B	C	D
1	EXISTING	0	0	0	100
1	FUTURE	0	0	0	100
1	SAD DESIGN	0	10	10	80
	CRITERIA				

APPENDIX B

Pond Calculations

VOLUME CALCULATIONS

NEW POND

Ab - Bottom Of The Pond Surface Area

At - Top Of The Pond Surface Area

D - Water Depth

Dt - Total Pond Depth

C - Change In Surface Area / Water Depth

$$\text{Volume} = Ab * D + 0.5 * C * D^2$$

$$C = (At - Ab) / Dt$$

$$Ab = 12.00 \text{ (@ Elevation 5028.50)}$$

$$At = 6,400.00 \text{ (@ Elevation 5029.50)}$$

$$Dt = 1.00$$

$$C = 6388.00$$

$$B \text{ Elev.} = 28.50$$

ACTUAL ELEV.	DEPTH (FT)	VOLUME (AC-FT)	Q (CFS)
23.40	0.00	0.00000	0
24.40	1.00	0.00028	3.618
25.40	2.00	0.00055	6.929
26.40	3.00	0.00083	9.106
27.40	4.00	0.00110	10.855
28.50	5.10	0.00140	12.500
29.50	6.10	0.07350	13.826

Orifice Equation

$$Q = CA \sqrt{2gH}$$

$$C = 0.6$$

$$\text{Diameter (in)} \quad 15$$

$$\text{Area (ft}^2\text{)} = 1.22718463$$

$$g = 32.2$$

$$H (\text{Ft}) = \text{Depth of water above center of orifice}$$

$$Q (\text{CFS}) = \text{Flow}$$

APPENDIX C

Drop Inlet Calculations

DROP INLET CALCULATIONS

POND	TYPE OF INLET	AREA (SF)	Q (CFS)	H (FT)	H ALLOW (FT)
1	Double - D	8.41	18.49	0.2085	0.67

ORIFICE EQUATION

$$Q = CA \sqrt{2gH}$$

$$C = 0.6$$

$$g = 32.2$$

NOTE: No clogging factor has been used in the effective area of the drop inlet because there is an emergency overflow at the drivepad.

STORM DRAIN INLET
EFFECTIVE AREA ASSUMING A 50% CLOGGING FACTOR

SINGLE 'D':

Area at the grate:

$$\begin{aligned} L &= 38.375" - 7(1/2" \text{ middle bars}) \\ &= 34.875" \\ &= 2.906' \end{aligned}$$

$$\begin{aligned} W &= 25.5" - 13(1/2" \text{ middle bars}) \\ &= 19" \\ &= 1.583' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 1.583 \times 2.906 \\ &= 4.601 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Effective Area} &= 4.601 - .5(4.601) \text{ Clogging Factor} \\ &= 2.30 \text{ ft}^2 \text{ at the grate} \end{aligned}$$

DOUBLE 'D':

Area at the grate:

$$\begin{aligned} L &= 76.75" - 14(1/2" \text{ middle bars}) - 6" \text{ center piece} \\ &= 63.75" \\ &= 5.3125' \end{aligned}$$

$$\begin{aligned} W &= 25.5" - 13(1/2" \text{ middle bars}) \\ &= 19" \\ &= 1.583' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 1.583' \times 5.3125' \\ &= 8.410 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Effective Area} &= 8.410 - .5(8.410) \\ &= 4.205 \text{ ft}^2 \end{aligned}$$

APPENDIX D

Pipe Capacity

PIPE CAPACITY

Carmony Street has 24" SD Pipe - Site dev. Flows 16.95 cfs

D (in)	Slope (%)	Area (ft^2)	R	Provide (cfs)	Velocity (ft/s)
18	1	1.77	0.375	10.53	5.96
24	1.65	3.14	0.5	29.14	9.27
30	1	4.91	0.625	41.13	8.38
36	1	7.07	0.75	66.88	9.46
42	1	9.62	0.875	100.88	10.49
48	1	12.57	1	144.03	11.46
54	1	15.90	1.125	197.18	12.40
60	1	19.63	1.25	261.14	13.30
66	1	23.76	1.375	336.71	14.17
72	1	28.27	1.5	424.65	15.02

Manning's Formula:

$$Q = 1.48 / n * A R^{2/3} S^{1/2}$$

$$V = Q * A$$

$$A = d^2 * (3.14) / 4 = \quad 3.14$$

$$R = A / wp = d / 4 = \quad 0.5$$

$$S = 1.65$$

$$d = 24$$

$$Q = 29.14 \text{ cfs}$$

$$V = 9.27 \text{ f/s}$$

Q= discharge in cfs

V= velocity f/s

s= friction slope

A= area of conduit

R= hydraulic radius of conduit

n= 0.013

wp= wetted perimeter

d= diameter of pipe

Q(allowable) = 29.14 cfs > Q(required) = 16.95 cfs

APPENDIX E

Emergency Overflow Calculations

EMERGENCY OVERFLOW

Broad - Crested Weirs

Weir Equation

$$Q = CLH^{(3/2)}$$

Variables:

C = a constant value based on breast of crest of weir and head

L = length of weir

H = Height of water measured from center of orifice

Q = flow

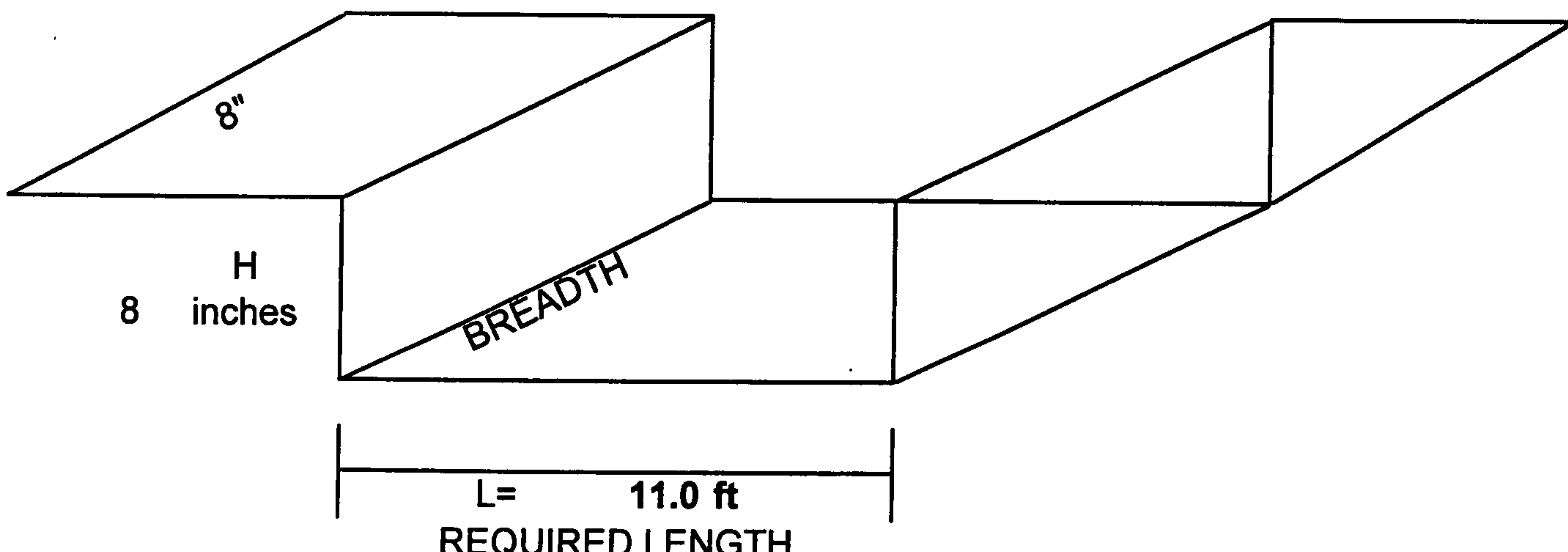
Solve for L:

$$Q = 18.49 \text{ cfs}$$

$$H = 8 \text{ inches}$$

$$C = 3.08 \text{ constant}$$

$$L = 11.0 \text{ ft}$$



NOTE:

40 FT DRIVEPAD WILL BE USED AS EMERGENCY OUTFLOW

APPENDIX F

Runoff Summary Tables for Developed Drainage Basin

DRAINAGE BASIN

Developed

BASIN	CONDITION	AREA (SF)	AREA (AC)	AREA (MI ²)
1	EXISTING	174240.00	4.0000	0.00625
1	FUTURE	174240.00	4.0000	0.00625
1	SAD DESIGN	174240.00	4.0000	0.00625
	CRITERIA			

RUNOFF CALCULATION RESULTS

Developed

BASIN	CONDITION	Q-100 CFS	Q-10 CFS	V-100 AC-FT	V-10 AC-FT
1	EXISTING	18.49	12.22	0.839	0.533
1	FUTURE	18.49	12.22	0.839	0.533
1	SAD DESIGN	16.95	10.80	0.734	0.452
	CRITERIA				

DIFFERENCE	1.54	1.42	0.11	0.08
------------	------	------	------	------

APPENDIX G

AHYMO Runoff Input and Summary Output for Developed Drainage Basin

BASIN2.txt

```
*****  
*  
*          NATIONAL ELECTRIC      *  
*          BASIN 1                *  
*****  
*  100-YEAR, 24-HR STORM (UNDER EXISTING & FUTURE CONDITIONS)  *  
*****  
*  
START        TIME=0.0  
*  
* BASIN 1  
*  
RAINFALL      TYPE=2 RAIN QUARTER=0.0 IN  
RAIN ONE=2.01 IN RAIN SIX=2.35 IN  
RAIN DAY=2.75 IN DT=0.05 HR  
*  
*  
COMPUTE NM HYD    ID=1 HYD NO=100.1 AREA=.00625 SQ MI  
PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00  
TP=-0.1333 HR MASS RAINFALL=-1  
PRINT HYD      ID=1 CODE=1  
  
PRINT HYD      ID=1 CODE=1  
*  
*  
*****  
*          NATIONAL ELECTRIC      *  
*          BASIN 1                *  
*****  
*  10-YEAR, 24-HR STORM (UNDER EXISTING & FUTURE CONDITIONS)  *  
*****  
*  
START        TIME=0.0  
*  
* BASIN 1  
*  
RAINFALL      TYPE=2 RAIN QUARTER=0.0 IN  
RAIN ONE=1.34 IN RAIN SIX=1.57 IN  
RAIN DAY=1.83 IN DT=0.05 HR  
*  
*  
COMPUTE NM HYD    ID=1 HYD NO=100.1 AREA=.00625 SQ MI  
PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00  
TP=-0.1333 HR MASS RAINFALL=-1  
PRINT HYD      ID=1 CODE=1  
*  
*  
*  
FINISH
```

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -
 INPUT FILE = C:\MY FILES\20100\AHYMO\BASIN2.TXT

- VERSION: 1997.02a

RUN DATE (MON/DAY/YR) =01/08/2001
 USER NO.= AHYMO-I-9702a0100011K-SH

COMMAND	HYDROGRAPH IDENTIFICATION	FROM	TO	AREA (SQ MI)	PEAK	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO	CFS	PAGE =
		ID	ID		DISCHARGE (CFS)			PEAK	PER ACRE	NOTATION
START										
RAINFALL	TYPE= 2								TIME= .00	
COMPUTE NM HYD		100.10	-	1	.00625	18.49	.839	2.51715	RAIN24= 2.750	
START								1.500	PER IMP= 100.00	
RAINFALL	TYPE= 2								TIME= .00	
COMPUTE NM HYD		100.10	-	1	.00625	12.22	.533	1.59950	RAIN24= 1.830	
FINISH								1.500	PER IMP= 100.00	

AHYMO PROGRAM (AHYMO_97) - Version: 1997.02a
 RUN DATE (MON/DAY/YR) = 01/08/2001
 START TIME (HR:MIN:SEC) = 15:39:55 USER NO.= AHYMO-I-9702a0100011K-SH
 INPUT FILE = C:\MY FILES\20100\AHYMO\BASIN2.TXT

```
*****
*          NATIONAL ELECTRIC
*
*          BASIN 1
*
* 100-YEAR, 24-HR STORM (UNDER EXISTING & FUTURE CONDITIONS)
*
*          START           TIME=0.0
*
*          BASIN 1
*
*          RAINFALL        TYPE=2 RAIN QUARTER=0.0 IN
*                           RAIN ONE=2.01 IN RAIN SIX=2.35 IN
*                           RAIN DAY=2.75 IN DT=0.05 HR
*
*          COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
*          DT = .050000 HOURS      END TIME = 24.000000 HOURS
*          .0000  .0024  .0049  .0075  .0102  .0130  .0158
*          .0188  .0219  .0252  .0286  .0321  .0358  .0397
*          .0439  .0482  .0529  .0578  .0631  .0689  .0751
*          .0836  .0930  .1201  .1842  .2944  .4649  .7103
*          1.0460 1.3107 1.4303 1.5302 1.6176 1.6959 1.7667
*          1.8313 1.8906 1.9452 1.9955 2.0421 2.0851 2.0946
*          2.1034 2.1115 2.1191 2.1262 2.1330 2.1394 2.1455
*          2.1513 2.1569 2.1622 2.1673 2.1723 2.1771 2.1817
*          2.1862 2.1905 2.1948 2.1989 2.2028 2.2067 2.2105
*          2.2142 2.2178 2.2213 2.2248 2.2282 2.2315 2.2347
*          2.2379 2.2410 2.2440 2.2470 2.2500 2.2529 2.2557
*          2.2585 2.2613 2.2640 2.2666 2.2693 2.2719 2.2744
*          2.2769 2.2794 2.2818 2.2842 2.2866 2.2889 2.2913
*          2.2935 2.2958 2.2980 2.3002 2.3024 2.3046 2.3067
*          2.3088 2.3109 2.3129 2.3150 2.3170 2.3190 2.3209
*          2.3229 2.3248 2.3267 2.3286 2.3305 2.3323 2.3342
*          2.3360 2.3378 2.3396 2.3414 2.3431 2.3449 2.3466
*          2.3483 2.3500 2.3517 2.3534 2.3551 2.3569 2.3586
*          2.3602 2.3619 2.3636 2.3653 2.3669 2.3686 2.3703
*          2.3719 2.3736 2.3752 2.3768 2.3785 2.3801 2.3817
*          2.3833 2.3849 2.3865 2.3881 2.3897 2.3913 2.3929
*          2.3944 2.3960 2.3976 2.3991 2.4007 2.4022 2.4038
*          2.4053 2.4068 2.4084 2.4099 2.4114 2.4129 2.4144
*          2.4159 2.4174 2.4189 2.4204 2.4219 2.4234 2.4248
*          2.4263 2.4278 2.4292 2.4307 2.4322 2.4336 2.4350
*          2.4365 2.4379 2.4394 2.4408 2.4422 2.4436 2.4450
*          2.4464 2.4478 2.4493 2.4506 2.4520 2.4534 2.4548
*          2.4562 2.4576 2.4589 2.4603 2.4617 2.4630 2.4644
*          2.4658 2.4671 2.4685 2.4698 2.4711 2.4725 2.4738
*          2.4751 2.4765 2.4778 2.4791 2.4804 2.4817 2.4830
*          2.4843 2.4856 2.4869 2.4882 2.4895 2.4908 2.4921
*          2.4934 2.4946 2.4959 2.4972 2.4984 2.4997 2.5010
*          2.5022 2.5035 2.5047 2.5060 2.5072 2.5085 2.5097
*          2.5109 2.5122 2.5134 2.5146 2.5158 2.5170 2.5183
*          2.5195 2.5207 2.5219 2.5231 2.5243 2.5255 2.5267
*          2.5279 2.5291 2.5303 2.5314 2.5326 2.5338 2.5350
*          2.5361 2.5373 2.5385 2.5396 2.5408 2.5420 2.5431
*          2.5443 2.5454 2.5466 2.5477 2.5488 2.5500 2.5511
*          2.5523 2.5534 2.5545 2.5556 2.5568 2.5579 2.5590
*          2.5601 2.5612 2.5623 2.5635 2.5646 2.5657 2.5668
*          2.5679 2.5690 2.5701 2.5711 2.5722 2.5733 2.5744
*          2.5755 2.5766 2.5776 2.5787 2.5798 2.5809 2.5819
*          2.5830 2.5841 2.5851 2.5862 2.5872 2.5883 2.5893
```

Basin2.out

2.5904	2.5914	2.5925	2.5935	2.5946	2.5956	2.5966
2.5977	2.5987	2.5997	2.6008	2.6018	2.6028	2.6038
2.6049	2.6059	2.6069	2.6079	2.6089	2.6099	2.6109
2.6119	2.6129	2.6139	2.6149	2.6159	2.6169	2.6179
2.6189	2.6199	2.6209	2.6219	2.6229	2.6238	2.6248
2.6258	2.6268	2.6278	2.6287	2.6297	2.6307	2.6316
2.6326	2.6336	2.6345	2.6355	2.6364	2.6374	2.6384
2.6393	2.6403	2.6412	2.6421	2.6431	2.6440	2.6450
2.6459	2.6469	2.6478	2.6487	2.6497	2.6506	2.6515
2.6524	2.6534	2.6543	2.6552	2.6561	2.6571	2.6580
2.6589	2.6598	2.6607	2.6616	2.6625	2.6634	2.6644
2.6653	2.6662	2.6671	2.6680	2.6689	2.6698	2.6707
2.6715	2.6724	2.6733	2.6742	2.6751	2.6760	2.6769
2.6778	2.6786	2.6795	2.6804	2.6813	2.6821	2.6830
2.6839	2.6848	2.6856	2.6865	2.6874	2.6882	2.6891
2.6900	2.6908	2.6917	2.6925	2.6934	2.6942	2.6951
2.6959	2.6968	2.6976	2.6985	2.6993	2.7002	2.7010
2.7019	2.7027	2.7035	2.7044	2.7052	2.7061	2.7069
2.7077	2.7085	2.7094	2.7102	2.7110	2.7119	2.7127
2.7135	2.7143	2.7151	2.7160	2.7168	2.7176	2.7184
2.7192	2.7200	2.7209	2.7217	2.7225	2.7233	2.7241
2.7249	2.7257	2.7265	2.7273	2.7281	2.7289	2.7297
2.7305	2.7313	2.7321	2.7329	2.7337	2.7344	2.7352
2.7360	2.7368	2.7376	2.7384	2.7392	2.7399	2.7407
2.7415	2.7423	2.7431	2.7438	2.7446	2.7454	2.7462
2.7469	2.7477	2.7485	2.7492	2.7500		

*

*

COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=.00625 SQ MI
 PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00
 TP=-0.1333 HR MASS RAINFALL=-1

06420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.1
 UNIT PEAK = 24.675 CFS UNIT VOLUME = .9987 B = 526.28 P60 = 2.0100
 AREA = .006250 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 2.51715 INCHES = .8390 ACRE-FEET
 PEAK DISCHARGE RATE = 18.49 CFS AT 1.500 HOURS BASIN AREA = .0063 SQ. MI.

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 2.51715 INCHES = .8390 ACRE-FEET
 PEAK DISCHARGE RATE = 18.49 CFS AT 1.500 HOURS BASIN AREA = .0063 SQ. MI.

*

*

NATIONAL ELECTRIC

*

BASIN 1

*

* 10-YEAR, 24-HR STORM (UNDER EXISTING & FUTURE CONDITIONS) *

*

START TIME=0.0

*

* BASIN 1

*

RAINFALL

TYPE=2 RAIN QUARTER=0.0 IN
RAIN ONE=1.34 IN RAIN SIX=1.57 IN
RAIN DAY=1.83 IN DT=0.05 HR

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
DT = .050000 HOURS END TIME = 24.000000 HOURS

	.0000	.0017	.0034	.0052	.0070	.0089	.0109
	.0129	.0150	.0173	.0196	.0220	.0246	.0272
	.0301	.0331	.0363	.0397	.0433	.0472	.0515
	.0571	.0634	.0815	.1242	.1977	.3113	.4750
	.6988	.8752	.9550	1.0215	1.0798	1.1320	1.1792
1.2223	1.2618	1.2982	1.3318	1.3628	1.3915	1.3979	
1.4037	1.4092	1.4143	1.4191	1.4236	1.4279	1.4320	
1.4359	1.4397	1.4433	1.4467	1.4501	1.4533	1.4564	
1.4594	1.4624	1.4652	1.4680	1.4706	1.4733	1.4758	
1.4783	1.4807	1.4831	1.4854	1.4877	1.4899	1.4921	
1.4943	1.4964	1.4984	1.5004	1.5024	1.5044	1.5063	
1.5082	1.5100	1.5119	1.5137	1.5154	1.5172	1.5189	
1.5206	1.5223	1.5239	1.5255	1.5271	1.5287	1.5303	
1.5318	1.5334	1.5349	1.5364	1.5378	1.5393	1.5407	
1.5421	1.5435	1.5449	1.5463	1.5477	1.5490	1.5503	
1.5517	1.5530	1.5543	1.5555	1.5568	1.5581	1.5593	
1.5605	1.5618	1.5630	1.5642	1.5653	1.5665	1.5677	
1.5689	1.5700	1.5711	1.5722	1.5734	1.5745	1.5756	
1.5767	1.5778	1.5789	1.5799	1.5810	1.5821	1.5832	
1.5843	1.5853	1.5864	1.5875	1.5885	1.5896	1.5906	
1.5917	1.5927	1.5938	1.5948	1.5958	1.5969	1.5979	
1.5989	1.5999	1.6010	1.6020	1.6030	1.6040	1.6050	
1.6060	1.6070	1.6080	1.6090	1.6100	1.6109	1.6119	
1.6129	1.6139	1.6148	1.6158	1.6168	1.6177	1.6187	
1.6197	1.6206	1.6216	1.6225	1.6235	1.6244	1.6253	
1.6263	1.6272	1.6281	1.6291	1.6300	1.6309	1.6318	
1.6328	1.6337	1.6346	1.6355	1.6364	1.6373	1.6382	
1.6391	1.6400	1.6409	1.6418	1.6427	1.6435	1.6444	
1.6453	1.6462	1.6471	1.6479	1.6488	1.6497	1.6505	
1.6514	1.6523	1.6531	1.6540	1.6548	1.6557	1.6565	
1.6574	1.6582	1.6591	1.6599	1.6608	1.6616	1.6624	
1.6633	1.6641	1.6649	1.6657	1.6666	1.6674	1.6682	
1.6690	1.6698	1.6706	1.6715	1.6723	1.6731	1.6739	
1.6747	1.6755	1.6763	1.6771	1.6779	1.6787	1.6795	
1.6802	1.6810	1.6818	1.6826	1.6834	1.6842	1.6849	
1.6857	1.6865	1.6872	1.6880	1.6888	1.6895	1.6903	
1.6911	1.6918	1.6926	1.6933	1.6941	1.6949	1.6956	
1.6964	1.6971	1.6978	1.6986	1.6993	1.7001	1.7008	
1.7016	1.7023	1.7030	1.7038	1.7045	1.7052	1.7059	
1.7067	1.7074	1.7081	1.7088	1.7095	1.7103	1.7110	
1.7117	1.7124	1.7131	1.7138	1.7145	1.7152	1.7159	
1.7166	1.7173	1.7180	1.7187	1.7194	1.7201	1.7208	
1.7215	1.7222	1.7229	1.7236	1.7243	1.7250	1.7257	
1.7263	1.7270	1.7277	1.7284	1.7290	1.7297	1.7304	
1.7311	1.7317	1.7324	1.7331	1.7337	1.7344	1.7351	
1.7357	1.7364	1.7371	1.7377	1.7384	1.7390	1.7397	
1.7403	1.7410	1.7416	1.7423	1.7429	1.7436	1.7442	
1.7449	1.7455	1.7462	1.7468	1.7474	1.7481	1.7487	
1.7493	1.7500	1.7506	1.7512	1.7519	1.7525	1.7531	
1.7538	1.7544	1.7550	1.7556	1.7563	1.7569	1.7575	
1.7581	1.7587	1.7593	1.7600	1.7606	1.7612	1.7618	
1.7624	1.7630	1.7636	1.7642	1.7648	1.7654	1.7661	
1.7667	1.7673	1.7679	1.7685	1.7691	1.7696	1.7702	
1.7708	1.7714	1.7720	1.7726	1.7732	1.7738	1.7744	
1.7750	1.7756	1.7761	1.7767	1.7773	1.7779	1.7785	

	1.7791	1.7796	1.7802	1.7808	1.7814	1.7819	1.7825
	1.7831	1.7837	1.7842	1.7848	1.7854	1.7859	1.7865
	1.7871	1.7876	1.7882	1.7888	1.7893	1.7899	1.7905
	1.7910	1.7916	1.7921	1.7927	1.7932	1.7938	1.7943
	1.7949	1.7955	1.7960	1.7966	1.7971	1.7977	1.7982
	1.7987	1.7993	1.7998	1.8004	1.8009	1.8015	1.8020
	1.8025	1.8031	1.8036	1.8042	1.8047	1.8052	1.8058
	1.8063	1.8068	1.8074	1.8079	1.8084	1.8090	1.8095
	1.8100	1.8105	1.8111	1.8116	1.8121	1.8127	1.8132
	1.8137	1.8142	1.8147	1.8153	1.8158	1.8163	1.8168
	1.8173	1.8178	1.8184	1.8189	1.8194	1.8199	1.8204
	1.8209	1.8214	1.8219	1.8225	1.8230	1.8235	1.8240
	1.8245	1.8250	1.8255	1.8260	1.8265	1.8270	1.8275
	1.8280	1.8285	1.8290	1.8295	1.8300		

*

*

COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=.00625 SQ MI
 PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00
 TP=-0.1333 HR MASS RAINFALL=-1

06420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.1

UNIT PEAK = 24.675 CFS UNIT VOLUME = .9987 B = 526.28 P60 = 1.3400

AREA = .006250 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 1.59950 INCHES = .5332 ACRE-FEET
 PEAK DISCHARGE RATE = 12.22 CFS AT 1.500 HOURS BASIN AREA = .0063 SQ. MI.

*

*

*

FINISH

NORMAL PROGRAM FINISH END TIME (HR:MIN:SEC) = 15:39:55

BASIN1.txt

```
*****  
*  
*      NATIONAL ELECTRIC          *  
*      BASIN 1                   *  
*****  
*      100-YEAR, 24-HR STORM (ALLOWABLE CONDITIONS)      *  
*****  
*  
START        TIME=0.0  
*  
* BASIN 1  
*  
RAINFALL      TYPE=2 RAIN QUARTER=0.0 IN  
               RAIN ONE=2.01 IN RAIN SIX=2.35 IN  
               RAIN DAY=2.75 IN DT=0.05 HR  
*  
*  
COMPUTE NM HYD    ID=1 HYD NO=100.1 AREA=.00625 SQ MI  
                  PER A=0.00 PER B=10.00 PER C=10.00 PER D=80.00  
                  TP=-0.1333 HR MASS RAINFALL=-1  
PRINT HYD       ID=1 CODE=1  
  
PRINT HYD       ID=1 CODE=1  
*  
*  
*****  
*      NATIONAL ELECTRIC          *  
*      BASIN 1                   *  
*****  
*      10-YEAR, 24-HR STORM (ALLOWABLE CONDITIONS)      *  
*****  
*  
START        TIME=0.0  
*  
* BASIN 1  
*  
RAINFALL      TYPE=2 RAIN QUARTER=0.0 IN  
               RAIN ONE=1.34 IN RAIN SIX=1.57 IN  
               RAIN DAY=1.83 IN DT=0.05 HR  
*  
*  
COMPUTE NM HYD    ID=1 HYD NO=100.1 AREA=.00625 SQ MI  
                  PER A=0.00 PER B=10.00 PER C=10.00 PER D=80.00  
                  TP=-0.1333 HR MASS RAINFALL=-1  
PRINT HYD       ID=1 CODE=1  
*  
*  
*  
FINISH
```

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -
 INPUT FILE = c:\my files\20100\ahymo\basin1.txt

- VERSION: 1997.02a

 RUN DATE (MON/DAY/YR) =01/08/2001
 USER NO.= AHYMO-I-9702a0100011K-SH

COMMAND	HYDROGRAPH IDENTIFICATION	FROM	TO	AREA (SQ MI)	PEAK	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO	CFS	PAGE =
		ID NO.	ID NO.		DISCHARGE (CFS)			PEAK (HOURS)	PER ACRE	NOTATION
START										
RAINFALL TYPE= 2								TIME= .00		
COMPUTE NM HYD	100.10	-	1	.00625	16.95	.734	2.20093	RAIN24= 2.750	PER IMP= 80.00	
START								TIME= .00		
RAINFALL TYPE= 2								RAIN24= 1.830	PER IMP= 80.00	
COMPUTE NM HYD	100.10	-	1	.00625	10.80	.452	1.35543	1.500	2.700 PER IMP= 80.00	
FINISH										

AHYMO PROGRAM (AHYMO_97) - Version: 1997.02a
 RUN DATE (MON/DAY/YR) = 01/08/2001
 START TIME (HR:MIN:SEC) = 22:05:10 USER NO.= AHYMO-I-9702a0100011K-SH
 INPUT FILE = c:\my files\20100\ahymo\basin1.txt

```
*****
*          NATIONAL ELECTRIC
*
*          BASIN 1
*
*          100-YEAR, 24-HR STORM (ALLOWABLE CONDITIONS)
*
*
*          START           TIME=0.0
*
*          * BASIN 1
*
*          RAINFALL        TYPE=2 RAIN QUARTER=0.0 IN
*                           RAIN ONE=2.01 IN RAIN SIX=2.35 IN
*                           RAIN DAY=2.75 IN DT=0.05 HR
*
*          COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
*          DT = .050000 HOURS      END TIME = 24.000000 HOURS
*          .0000   .0024   .0049   .0075   .0102   .0130   .0158
*          .0188   .0219   .0252   .0286   .0321   .0358   .0397
*          .0439   .0482   .0529   .0578   .0631   .0689   .0751
*          .0836   .0930   .1201   .1842   .2944   .4649   .7103
*          1.0460  1.3107  1.4303  1.5302  1.6176  1.6959  1.7667
*          1.8313  1.8906  1.9452  1.9955  2.0421  2.0851  2.0946
*          2.1034  2.1115  2.1191  2.1262  2.1330  2.1394  2.1455
*          2.1513  2.1569  2.1622  2.1673  2.1723  2.1771  2.1817
*          2.1862  2.1905  2.1948  2.1989  2.2028  2.2067  2.2105
*          2.2142  2.2178  2.2213  2.2248  2.2282  2.2315  2.2347
*          2.2379  2.2410  2.2440  2.2470  2.2500  2.2529  2.2557
*          2.2585  2.2613  2.2640  2.2666  2.2693  2.2719  2.2744
*          2.2769  2.2794  2.2818  2.2842  2.2866  2.2889  2.2913
*          2.2935  2.2958  2.2980  2.3002  2.3024  2.3046  2.3067
*          2.3088  2.3109  2.3129  2.3150  2.3170  2.3190  2.3209
*          2.3229  2.3248  2.3267  2.3286  2.3305  2.3323  2.3342
*          2.3360  2.3378  2.3396  2.3414  2.3431  2.3449  2.3466
*          2.3483  2.3500  2.3517  2.3534  2.3551  2.3569  2.3586
*          2.3602  2.3619  2.3636  2.3653  2.3669  2.3686  2.3703
*          2.3719  2.3736  2.3752  2.3768  2.3785  2.3801  2.3817
*          2.3833  2.3849  2.3865  2.3881  2.3897  2.3913  2.3929
*          2.3944  2.3960  2.3976  2.3991  2.4007  2.4022  2.4038
*          2.4053  2.4068  2.4084  2.4099  2.4114  2.4129  2.4144
*          2.4159  2.4174  2.4189  2.4204  2.4219  2.4234  2.4248
*          2.4263  2.4278  2.4292  2.4307  2.4322  2.4336  2.4350
*          2.4365  2.4379  2.4394  2.4408  2.4422  2.4436  2.4450
*          2.4464  2.4478  2.4493  2.4506  2.4520  2.4534  2.4548
*          2.4562  2.4576  2.4589  2.4603  2.4617  2.4630  2.4644
*          2.4658  2.4671  2.4685  2.4698  2.4711  2.4725  2.4738
*          2.4751  2.4765  2.4778  2.4791  2.4804  2.4817  2.4830
*          2.4843  2.4856  2.4869  2.4882  2.4895  2.4908  2.4921
*          2.4934  2.4946  2.4959  2.4972  2.4984  2.4997  2.5010
*          2.5022  2.5035  2.5047  2.5060  2.5072  2.5085  2.5097
*          2.5109  2.5122  2.5134  2.5146  2.5158  2.5170  2.5183
*          2.5195  2.5207  2.5219  2.5231  2.5243  2.5255  2.5267
*          2.5279  2.5291  2.5303  2.5314  2.5326  2.5338  2.5350
*          2.5361  2.5373  2.5385  2.5396  2.5408  2.5420  2.5431
*          2.5443  2.5454  2.5466  2.5477  2.5488  2.5500  2.5511
*          2.5523  2.5534  2.5545  2.5556  2.5568  2.5579  2.5590
*          2.5601  2.5612  2.5623  2.5635  2.5646  2.5657  2.5668
*          2.5679  2.5690  2.5701  2.5711  2.5722  2.5733  2.5744
*          2.5755  2.5766  2.5776  2.5787  2.5798  2.5809  2.5819
*          2.5830  2.5841  2.5851  2.5862  2.5872  2.5883  2.5893
```

2.5904	2.5914	2.5925	2.5935	2.5946	2.5956	2.5966
2.5977	2.5987	2.5997	2.6008	2.6018	2.6028	2.6038
2.6049	2.6059	2.6069	2.6079	2.6089	2.6099	2.6109
2.6119	2.6129	2.6139	2.6149	2.6159	2.6169	2.6179
2.6189	2.6199	2.6209	2.6219	2.6229	2.6238	2.6248
2.6258	2.6268	2.6278	2.6287	2.6297	2.6307	2.6316
2.6326	2.6336	2.6345	2.6355	2.6364	2.6374	2.6384
2.6393	2.6403	2.6412	2.6421	2.6431	2.6440	2.6450
2.6459	2.6469	2.6478	2.6487	2.6497	2.6506	2.6515
2.6524	2.6534	2.6543	2.6552	2.6561	2.6571	2.6580
2.6589	2.6598	2.6607	2.6616	2.6625	2.6634	2.6644
2.6653	2.6662	2.6671	2.6680	2.6689	2.6698	2.6707
2.6715	2.6724	2.6733	2.6742	2.6751	2.6760	2.6769
2.6778	2.6786	2.6795	2.6804	2.6813	2.6821	2.6830
2.6839	2.6848	2.6856	2.6865	2.6874	2.6882	2.6891
2.6900	2.6908	2.6917	2.6925	2.6934	2.6942	2.6951
2.6959	2.6968	2.6976	2.6985	2.6993	2.7002	2.7010
2.7019	2.7027	2.7035	2.7044	2.7052	2.7061	2.7069
2.7077	2.7085	2.7094	2.7102	2.7110	2.7119	2.7127
2.7135	2.7143	2.7151	2.7160	2.7168	2.7176	2.7184
2.7192	2.7200	2.7209	2.7217	2.7225	2.7233	2.7241
2.7249	2.7257	2.7265	2.7273	2.7281	2.7289	2.7297
2.7305	2.7313	2.7321	2.7329	2.7337	2.7344	2.7352
2.7360	2.7368	2.7376	2.7384	2.7392	2.7399	2.7407
2.7415	2.7423	2.7431	2.7438	2.7446	2.7454	2.7462
2.7469	2.7477	2.7485	2.7492	2.7500		

*

*

COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=.00625 SQ MI
 PER A=0.00 PER B=10.00 PER C=10.00 PER D=80.00
 TP=-0.1333 HR MASS RAINFALL=-1

06420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.1
 UNIT PEAK = 19.740 CFS UNIT VOLUME = .9986 B = 526.28 P60 = 2.0100
 AREA = .005000 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

44947 K = .119767HR TP = .133300HR K/TP RATIO = .898476 SHAPE CONSTANT, N = 3.9
 UNIT PEAK = 3.2960 CFS UNIT VOLUME = .9971 B = 351.48 P60 = 2.0100
 AREA = .001250 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 2.20093 INCHES = .7336 ACRE-FEET
 PEAK DISCHARGE RATE = 16.95 CFS AT 1.500 HOURS BASIN AREA = .0063 SQ. MI.

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 2.20093 INCHES = .7336 ACRE-FEET
 PEAK DISCHARGE RATE = 16.95 CFS AT 1.500 HOURS BASIN AREA = .0063 SQ. MI.

```

*
*
*****
*          NATIONAL ELECTRIC
*
*          BASIN 1
*
*          10-YEAR, 24-HR STORM (ALLOWABLE CONDITIONS)
*
*
START      TIME=0.0
*
* BASIN 1
*
RAINFALL   TYPE=2 RAIN QUARTER=0.0 IN
            RAIN ONE=1.34 IN RAIN SIX=1.57 IN
            RAIN DAY=1.83 IN DT=0.05 HR

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
DT = .050000 HOURS    END TIME = 24.000000 HOURS
  .0000  .0017  .0034  .0052  .0070  .0089  .0109
  .0129  .0150  .0173  .0196  .0220  .0246  .0272
  .0301  .0331  .0363  .0397  .0433  .0472  .0515
  .0571  .0634  .0815  .1242  .1977  .3113  .4750
  .6988  .8752  .9550  1.0215  1.0798  1.1320  1.1792
  1.2223  1.2618  1.2982  1.3318  1.3628  1.3915  1.3979
  1.4037  1.4092  1.4143  1.4191  1.4236  1.4279  1.4320
  1.4359  1.4397  1.4433  1.4467  1.4501  1.4533  1.4564
  1.4594  1.4624  1.4652  1.4680  1.4706  1.4733  1.4758
  1.4783  1.4807  1.4831  1.4854  1.4877  1.4899  1.4921
  1.4943  1.4964  1.4984  1.5004  1.5024  1.5044  1.5063
  1.5082  1.5100  1.5119  1.5137  1.5154  1.5172  1.5189
  1.5206  1.5223  1.5239  1.5255  1.5271  1.5287  1.5303
  1.5318  1.5334  1.5349  1.5364  1.5378  1.5393  1.5407
  1.5421  1.5435  1.5449  1.5463  1.5477  1.5490  1.5503
  1.5517  1.5530  1.5543  1.5555  1.5568  1.5581  1.5593
  1.5605  1.5618  1.5630  1.5642  1.5653  1.5665  1.5677
  1.5689  1.5700  1.5711  1.5722  1.5734  1.5745  1.5756
  1.5767  1.5778  1.5789  1.5799  1.5810  1.5821  1.5832
  1.5843  1.5853  1.5864  1.5875  1.5885  1.5896  1.5906
  1.5917  1.5927  1.5938  1.5948  1.5958  1.5969  1.5979
  1.5989  1.5999  1.6010  1.6020  1.6030  1.6040  1.6050
  1.6060  1.6070  1.6080  1.6090  1.6100  1.6109  1.6119
  1.6129  1.6139  1.6148  1.6158  1.6168  1.6177  1.6187
  1.6197  1.6206  1.6216  1.6225  1.6235  1.6244  1.6253
  1.6263  1.6272  1.6281  1.6291  1.6300  1.6309  1.6318
  1.6328  1.6337  1.6346  1.6355  1.6364  1.6373  1.6382
  1.6391  1.6400  1.6409  1.6418  1.6427  1.6435  1.6444
  1.6453  1.6462  1.6471  1.6479  1.6488  1.6497  1.6505
  1.6514  1.6523  1.6531  1.6540  1.6548  1.6557  1.6565
  1.6574  1.6582  1.6591  1.6599  1.6608  1.6616  1.6624
  1.6633  1.6641  1.6649  1.6657  1.6666  1.6674  1.6682
  1.6690  1.6698  1.6706  1.6715  1.6723  1.6731  1.6739
  1.6747  1.6755  1.6763  1.6771  1.6779  1.6787  1.6795
  1.6802  1.6810  1.6818  1.6826  1.6834  1.6842  1.6849
  1.6857  1.6865  1.6872  1.6880  1.6888  1.6895  1.6903
  1.6911  1.6918  1.6926  1.6933  1.6941  1.6949  1.6956
  1.6964  1.6971  1.6978  1.6986  1.6993  1.7001  1.7008
  1.7016  1.7023  1.7030  1.7038  1.7045  1.7052  1.7059
  1.7067  1.7074  1.7081  1.7088  1.7095  1.7103  1.7110
  1.7117  1.7124  1.7131  1.7138  1.7145  1.7152  1.7159
  1.7166  1.7173  1.7180  1.7187  1.7194  1.7201  1.7208
  1.7215  1.7222  1.7229  1.7236  1.7243  1.7250  1.7257
  1.7263  1.7270  1.7277  1.7284  1.7290  1.7297  1.7304
  1.7311  1.7317  1.7324  1.7331  1.7337  1.7344  1.7351
  1.7357  1.7364  1.7371  1.7377  1.7384  1.7390  1.7397
  1.7403  1.7410  1.7416  1.7423  1.7429  1.7436  1.7442

```

Basin1.out

	1.7449	1.7455	1.7462	1.7468	1.7474	1.7481	1.7487
	1.7493	1.7500	1.7506	1.7512	1.7519	1.7525	1.7531
	1.7538	1.7544	1.7550	1.7556	1.7563	1.7569	1.7575
	1.7581	1.7587	1.7593	1.7600	1.7606	1.7612	1.7618
	1.7624	1.7630	1.7636	1.7642	1.7648	1.7654	1.7661
	1.7667	1.7673	1.7679	1.7685	1.7691	1.7696	1.7702
	1.7708	1.7714	1.7720	1.7726	1.7732	1.7738	1.7744
	1.7750	1.7756	1.7761	1.7767	1.7773	1.7779	1.7785
	1.7791	1.7796	1.7802	1.7808	1.7814	1.7819	1.7825
	1.7831	1.7837	1.7842	1.7848	1.7854	1.7859	1.7865
	1.7871	1.7876	1.7882	1.7888	1.7893	1.7899	1.7905
	1.7910	1.7916	1.7921	1.7927	1.7932	1.7938	1.7943
	1.7949	1.7955	1.7960	1.7966	1.7971	1.7977	1.7982
	1.7987	1.7993	1.7998	1.8004	1.8009	1.8015	1.8020
	1.8025	1.8031	1.8036	1.8042	1.8047	1.8052	1.8058
	1.8063	1.8068	1.8074	1.8079	1.8084	1.8090	1.8095
	1.8100	1.8105	1.8111	1.8116	1.8121	1.8127	1.8132
	1.8137	1.8142	1.8147	1.8153	1.8158	1.8163	1.8168
	1.8173	1.8178	1.8184	1.8189	1.8194	1.8199	1.8204
	1.8209	1.8214	1.8219	1.8225	1.8230	1.8235	1.8240
	1.8245	1.8250	1.8255	1.8260	1.8265	1.8270	1.8275
	1.8280	1.8285	1.8290	1.8295	1.8300		

*

*

COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=.00625 SQ MI
 PER A=0.00 PER B=10.00 PER C=10.00 PER D=80.00
 TP=-0.1333 HR MASS RAINFALL=-1

06420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.1

UNIT PEAK = 19.740 CFS UNIT VOLUME = .9986 B = 526.28 P60 = 1.3400

AREA = .005000 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

12062 K = .123717HR TP = .133300HR K/TP RATIO = .928113 SHAPE CONSTANT, N = 3.8

UNIT PEAK = 3.2112 CFS UNIT VOLUME = .9973 B = 342.44 P60 = 1.3400

AREA = .001250 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 1.35543 INCHES = .4518 ACRE-FEET
 PEAK DISCHARGE RATE = 10.80 CFS AT 1.500 HOURS BASIN AREA = .0063 SQ. MI.

*

*

*

FINISH

NORMAL PROGRAM FINISH END TIME (HR:MIN:SEC) = 22:05:10

APPENDIX H

AHYMO Input and Summary Output for Ponding

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -
 INPUT FILE = C:\MY FILES\20100\AHYMO\POND2.TXT

- VERSION: 1997.02a

RUN DATE (MON/DAY/YR) =01/08/2001
 USER NO.= AHYMO-I-9702a0100011K-SH

COMMAND	HYDROGRAPH IDENTIFICATION	FROM	TO	AREA (SQ MI)	PEAK	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO	CFS	PAGE =
		ID	ID		DISCHARGE (CFS)			PEAK (HOURS)	PER ACRE	NOTATION
START										
RAINFALL TYPE= 2								TIME= .00		
COMPUTE NM HYD	100.10	-	1	.00625	18.49	.839	2.51715	1.500	4.624 PER IMP=	100.00
ROUTE RESERVOIR	501.10	1	2	.00625	13.38	.841	2.52217	1.600	3.346 AC-FT=	.049
FINISH										

AHYMO PROGRAM (AHYMO_97) - Version: 1997.02a
 RUN DATE (MON/DAY/YR) = 01/08/2001
 START TIME (HR:MIN:SEC) = 23:08:25 USER NO.= AHYMO-I-9702a0100011K-SH
 INPUT FILE = C:\MY FILES\20100\AHYMO\POND2.TXT

```
*****
*          NATIONAL ELECTRIC
*
*          BASIN & POND
*****
*          INPUT FILE FOR 100 YR, 24 HR
*****
*          100-YEAR, 24-HR STORM (UNDER PROPOSED CONDITIONS)
*****
*          START           TIME=0.0
*
*          BASIN 1
*
RAINFALL      TYPE=2 RAIN QUARTER=0.0 IN
               RAIN ONE=2.01 IN RAIN SIX=2.35 IN
               RAIN DAY=2.75 IN DT=0.05 HR

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
DT = .050000 HOURS    END TIME = 24.000000 HOURS
  .0000   .0024   .0049   .0075   .0102   .0130   .0158
  .0188   .0219   .0252   .0286   .0321   .0358   .0397
  .0439   .0482   .0529   .0578   .0631   .0689   .0751
  .0836   .0930   .1201   .1842   .2944   .4649   .7103
  1.0460  1.3107  1.4303  1.5302  1.6176  1.6959  1.7667
  1.8313  1.8906  1.9452  1.9955  2.0421  2.0851  2.0946
  2.1034  2.1115  2.1191  2.1262  2.1330  2.1394  2.1455
  2.1513  2.1569  2.1622  2.1673  2.1723  2.1771  2.1817
  2.1862  2.1905  2.1948  2.1989  2.2028  2.2067  2.2105
  2.2142  2.2178  2.2213  2.2248  2.2282  2.2315  2.2347
  2.2379  2.2410  2.2440  2.2470  2.2500  2.2529  2.2557
  2.2585  2.2613  2.2640  2.2666  2.2693  2.2719  2.2744
  2.2769  2.2794  2.2818  2.2842  2.2866  2.2889  2.2913
  2.2935  2.2958  2.2980  2.3002  2.3024  2.3046  2.3067
  2.3088  2.3109  2.3129  2.3150  2.3170  2.3190  2.3209
  2.3229  2.3248  2.3267  2.3286  2.3305  2.3323  2.3342
  2.3360  2.3378  2.3396  2.3414  2.3431  2.3449  2.3466
  2.3483  2.3500  2.3517  2.3534  2.3551  2.3569  2.3586
  2.3602  2.3619  2.3636  2.3653  2.3669  2.3686  2.3703
  2.3719  2.3736  2.3752  2.3768  2.3785  2.3801  2.3817
  2.3833  2.3849  2.3865  2.3881  2.3897  2.3913  2.3929
  2.3944  2.3960  2.3976  2.3991  2.4007  2.4022  2.4038
  2.4053  2.4068  2.4084  2.4099  2.4114  2.4129  2.4144
  2.4159  2.4174  2.4189  2.4204  2.4219  2.4234  2.4248
  2.4263  2.4278  2.4292  2.4307  2.4322  2.4336  2.4350
  2.4365  2.4379  2.4394  2.4408  2.4422  2.4436  2.4450
  2.4464  2.4478  2.4493  2.4506  2.4520  2.4534  2.4548
  2.4562  2.4576  2.4589  2.4603  2.4617  2.4630  2.4644
  2.4658  2.4671  2.4685  2.4698  2.4711  2.4725  2.4738
  2.4751  2.4765  2.4778  2.4791  2.4804  2.4817  2.4830
  2.4843  2.4856  2.4869  2.4882  2.4895  2.4908  2.4921
  2.4934  2.4946  2.4959  2.4972  2.4984  2.4997  2.5010
  2.5022  2.5035  2.5047  2.5060  2.5072  2.5085  2.5097
  2.5109  2.5122  2.5134  2.5146  2.5158  2.5170  2.5183
  2.5195  2.5207  2.5219  2.5231  2.5243  2.5255  2.5267
  2.5279  2.5291  2.5303  2.5314  2.5326  2.5338  2.5350
  2.5361  2.5373  2.5385  2.5396  2.5408  2.5420  2.5431
  2.5443  2.5454  2.5466  2.5477  2.5488  2.5500  2.5511
  2.5523  2.5534  2.5545  2.5556  2.5568  2.5579  2.5590
  2.5601  2.5612  2.5623  2.5635  2.5646  2.5657  2.5668
  2.5679  2.5690  2.5701  2.5711  2.5722  2.5733  2.5744
```

2.5755	2.5766	2.5776	2.5787	2.5798	2.5809	2.5819
2.5830	2.5841	2.5851	2.5862	2.5872	2.5883	2.5893
2.5904	2.5914	2.5925	2.5935	2.5946	2.5956	2.5966
2.5977	2.5987	2.5997	2.6008	2.6018	2.6028	2.6038
2.6049	2.6059	2.6069	2.6079	2.6089	2.6099	2.6109
2.6119	2.6129	2.6139	2.6149	2.6159	2.6169	2.6179
2.6189	2.6199	2.6209	2.6219	2.6229	2.6238	2.6248
2.6258	2.6268	2.6278	2.6287	2.6297	2.6307	2.6316
2.6326	2.6336	2.6345	2.6355	2.6364	2.6374	2.6384
2.6393	2.6403	2.6412	2.6421	2.6431	2.6440	2.6450
2.6459	2.6469	2.6478	2.6487	2.6497	2.6506	2.6515
2.6524	2.6534	2.6543	2.6552	2.6561	2.6571	2.6580
2.6589	2.6598	2.6607	2.6616	2.6625	2.6634	2.6644
2.6653	2.6662	2.6671	2.6680	2.6689	2.6698	2.6707
2.6715	2.6724	2.6733	2.6742	2.6751	2.6760	2.6769
2.6778	2.6786	2.6795	2.6804	2.6813	2.6821	2.6830
2.6839	2.6848	2.6856	2.6865	2.6874	2.6882	2.6891
2.6900	2.6908	2.6917	2.6925	2.6934	2.6942	2.6951
2.6959	2.6968	2.6976	2.6985	2.6993	2.7002	2.7010
2.7019	2.7027	2.7035	2.7044	2.7052	2.7061	2.7069
2.7077	2.7085	2.7094	2.7102	2.7110	2.7119	2.7127
2.7135	2.7143	2.7151	2.7160	2.7168	2.7176	2.7184
2.7192	2.7200	2.7209	2.7217	2.7225	2.7233	2.7241
2.7249	2.7257	2.7265	2.7273	2.7281	2.7289	2.7297
2.7305	2.7313	2.7321	2.7329	2.7337	2.7344	2.7352
2.7360	2.7368	2.7376	2.7384	2.7392	2.7399	2.7407
2.7415	2.7423	2.7431	2.7438	2.7446	2.7454	2.7462
2.7469	2.7477	2.7485	2.7492	2.7500		

*

*

COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=0.00625 SQ MI
 PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00
 TP=-0.1333 HR MASS RAINFALL=-1

06420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.1
 UNIT PEAK = 24.675 CFS UNIT VOLUME = .9987 B = 526.28 P60 = 2.0100
 AREA = .006250 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 2.51715 INCHES = .8390 ACRE-FEET
 PEAK DISCHARGE RATE = 18.49 CFS AT 1.500 HOURS BASIN AREA = .0063 SQ. MI.

*
 *
 *
 *POND
 *

ROUTE RESERVOIR ID=2 HYD NO=501.1 INFLOW ID=1 CODE=24
 OUTFLOW (CFS) STORAGE(AC-FT) ELEVATION(FT)

0.0000	0.00000	23.40
3.6180	0.00028	24.40
6.9290	0.00055	25.40
9.1060	0.00083	26.40
10.855	0.00110	27.40
12.500	0.00140	28.50
13.826	0.07350	29.50

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	23.40	.000	.00
1.20	.54	23.54	.000	.52
2.40	.78	23.40	.000	.00
3.60	.13	23.41	.000	.04
4.80	.11	23.43	.000	.10
6.00	.13	23.44	.000	.13
7.20	.13	23.44	.000	.13
8.40	.12	23.43	.000	.12
9.60	.11	23.43	.000	.11
10.80	.10	23.43	.000	.10
12.00	.10	23.43	.000	.10
13.20	.09	23.43	.000	.09
14.40	.09	23.42	.000	.09
15.60	.08	23.42	.000	.08
16.80	.08	23.42	.000	.08
18.00	.08	23.42	.000	.08
19.20	.07	23.42	.000	.07
20.40	.07	23.42	.000	.07
21.60	.07	23.42	.000	.07
22.80	.06	23.42	.000	.06
24.00	.06	23.42	.000	.06

PEAK DISCHARGE = 13.384 CFS - PEAK OCCURS AT HOUR 1.60

MAXIMUM WATER SURFACE ELEVATION = 29.166

MAXIMUM STORAGE = .0495 AC-FT INCREMENTAL TIME= .050000HRS

*

*

*

PRINT HYD

ID=2 CODE=1

OUTFLOW HYDROGRAPH RESERVOIR 501.10

RUNOFF VOLUME = 2.52217 INCHES = .8407 ACRE-FEET

PEAK DISCHARGE RATE = 13.38 CFS AT 1.600 HOURS BASIN AREA = .0063 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 23:08:25