



#### EXECUTIVE SUMMARY:

THIS GRADING AND DRAINAGE PLAN SUPPORTS THE CONSTRUCTION OF A PROPOSED BUILDING AND ASSOCIATED PAVING AND LANDSCAPING IMPROVEMENTS. THE TRACT IS CURRENTLY UNDEVELOPED. THE TRACT IS ZONED M-1. THE PROPOSED IMPROVEMENTS WILL CAUSE AN INCREASE IN IMPERVIOUS AREA AND WILL IMPACT THE HYDROLOGY OF THE SITE AS DEMONSTRATED IN THE DRAINAGE CALCULATIONS CONTAINED HEREON. THIS PLAN PROPOSES THE FREE DISCHARGE OF DEVELOPED RUNOFF FROM THIS SITE IN ACCORDANCE WITH A PREVIOUSLY APPROVED MASTER DRAINAGE PLAN (G16-D82). THE PURPOSE OF THIS SUBMITTAL IS TO OBTAIN BUILDING PERMIT APPROVAL.

## REFERENCES:

THE FOLLOWING IS A LIST OF PREVIOUSLY APPROVED GRADING AND DRAINAGE PLANS RELATIVE TO THIS SITE. THIS LIST MAY NOT BE INCLUSIVE, HOWEVER, REPRESENTS A SUMMARY OF THOSE PLANS KNOWN TO THIS PREPARER.

1. DRAINAGE ANALYSIS FOR S.A.D. 216 PREPARED BY ANDREWS, ASBURY AND ROBERT FOR THE CITY OF ALBUQUERQUE. THE DRAINAGE REPORT IS DATED 05—30—1989 WITH ADDENDUM DATED 07—18—1989, AND WAS APPROVED BY BOTH THE CITY OF ALBUQUERQUE AND BERNALILLO COUNTY ON 07—24—1989. THIS SITE LIES WITHIN THE AREA AFFECTED BY S.A.D. 216.

2. MASTER DRAINAGE PLAN FOR TRACT B-1, COMANCHE BUSINESS PARK PREPARED BY JEFF MORTENSEN AND ASSOCIATES, INC. DATED 01-17-1997. THIS MASTER DRAINAGE PLAN WAS APPROVED BY THE CITY OF ALBUQUERQUE 01-17-1997 (FILE G16-D82). THE PROPOSED DEVELOPMENT ON TRACT B-1-F-1 IS CONSISTENT WITH THE REQUIREMENTS OF THIS MASTER DRAINAGE PLAN AND DOES NOT EXCEED THE MAXIMUM ALLOWABLE PEAK DISCHARGE OF 3.61 CFS/AC.

### PROJECT DESCRIPTION:

AS SHOWN BY THE VICINITY MAP ON SHEET 1, THE SITE IS LOCATED AT THE EAST END OF VASSAR PLACE N.E., THE SITE ADJOINS AMAFCA RIGHT OF WAY FOR THE NORTH DIVERSION CHANNEL. THE SITE IS CURRENTLY UNDEVELOPED. THE LEGAL DESCRIPTION OF THE SITE IS TRACT B-1-F-1 COMANCHE BUSINESS PARK, FILED 09-15-1997, 97C-277. THE SITE IS ZONED M-1.

AS SHOWN BY PANEL 138 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS FOR BERNALILLO COUNTY, NEW MEXICO, AND INCORPORATED AREAS DATED SEPTEMBER 20, 1996, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE.

### EXISTING CONDITIONS:

THE SITE IS UNDEVELOPED. THE SITE DRAINS FROM EAST TO WEST TO VASSAR PLACE N.E. THE SITE IS PART OF BASIN B-3A AS IDENTIFIED IN THE AFOREMENTIONED MASTER DRAINAGE PLAN FOR THE SITE. THE MASTER DRAINAGE PLAN ALSO IDENTIFIES OFFSITE FLOWS ENTERING THE SITE FROM THE NORTH AND FROM THE EAST. THE LOCATIONS AND PEAK FLOW RATES OF THESE OFFSITE FLOWS ARE SHOWN ON THE GRADING PLAN ON SHEET 1. OFFSITE FLOWS DO NOT ENTER THE SITE FROM THE TRACT TO THE SOUTH WHICH EXHIBITS PARALLEL TOPOGRAPHY, OR FROM THE TRACT TO THE WEST WHICH LIES TOPOGRAPHICALLY LOWER. CITY WORK ORDER 554481 CONSTRUCTED THE EXISTING PUBLIC PAVING AND STORM DRAINAGE IMPROVEMENTS WITHIN VASSAR PLACE N.E. THIS SITE DRAINS TO THE EXISTING PUBLIC STORM DRAIN WITHIN VASSAR PLACE N.E. WHICH CURRENTLY OUTFALLS TO A TEMPORARY RETENTION POND CONSTRUCTED BY CITY WORK ORDER ON LOT B-1-J-1. AS IDENTIFIED IN THE MASTER DRAINAGE PLAN, THIS STORM DRAIN WILL OUTLET TO THE TEMPORARY RETENTION POND UNTIL SUCH TIME AS S.A.D. 216 IMPROVEMENTS ARE CONSTRUCTED AND SUBSEQUENT DRAINAGE IMPROVEMENTS ARE MADE. THESE FUTURE IMPROVEMENTS WILL ELIMINATE THE NEED FOR THE TEMPORARY RETENTION POND. IN THE INTERIM CONDITION, THE POND IS SIZED FOR FREE DISCHARGE OF DEVELOPED RUNOFF FROM THE CONTRIBUTING BASINS. THE EXISTING GRADES, AS INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1. 0" INTERVALS SUPPORT THESE OBSERVATIONS.

# DEVELOPED CONDITION:

AS PREVIOUSLY INDICATED, THE PROPOSED IMPROVEMENTS CONSIST OF THE CONSTRUCTION OF A NEW BUILDING WITH ASSOCIATED PAVING AND LANDSCAPING. THE SITE WILL DISCHARGE FREELY TO VASSAR PLACE N.E. VIA A PROPOSED DRIVEPAD. SECTIONS A-A AND B-B CONTAINED HEREON SHOW PROPOSED ON-SITE DRAINAGE FEATURES USED TO CONVEY ON-SITE AND OFF-SITE FLOWS TO THE PROPOSED DRIVEPAD. AS SHOWN BY THE CALCULATIONS, THE PROPOSED DISCHARGE RATE OF 3.4 CFS/ACRE IS LESS THAN THE ALLOWABLE RATE OF 3.61 CFS/ACRE SET FORTH FOR THE SITE BY THE PREVIOUSLY REFERENCED MASTER DRAINAGE PLAN. FREE DISCHARGE FROM THIS SITE IS THEREFORE JUSTIFIED. THE EXISTING OFFSITE FLOWS, WHICH ENTER THE SITE, WILL BE PASSED THROUGH THE SITE. VASSAR PLACE N.E. WILL CONTINUE TO DRAIN TO THE AFOREMENTIONED PUBLIC STORM DRAIN AND TEMPORARY INTERIM RETENTION POND. FUTURE DOWNSTREAM STORM DRAINAGE AND/OR SAD 216 IMPROVEMENTS WILL NOT RESTRICT OR OTHERWISE AFFECT DRAINAGE FROM THIS SITE. THE PROPOSED SPOT ELEVATIONS AND CONTOURS AT 1'0" INTERVALS SHOWN ON THE PLAN DEMONSTRATE CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. NO OFFSITE IMPROVEMENTS ARE REQUIRED OR PROPOSED IN CONJUNCTION WITH THIS PROJECT.

# CALCULATIONS:

THE CALCULATIONS, WHICH APPEAR HEREON, ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100—YEAR, 6—HOUR RAINFALL EVENT. THE PROCEDURE FOR 40—ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS SHOWN BY THESE CALCULATIONS, AN INCREASE IN THE PEAK RATE AND VOLUME OF DISCHARGE IS ANTICIPATED. THIS INCREASE IS JUSTIFIED BECAUSE BOTH THE EXISTING AND FUTURE DOWNSTREAM IMPROVEMENTS ARE SIZED FOR THE PROPOSED FREE DISCHARGE FROM THIS TRACT AS IDENTIFIED HEREIN.

# CONCLUSION:

THIS PROPOSED GRADING AND DRAINAGE PLAN PROPOSES A RESPONSIBLE APPROACH TO MANAGING THE STORM WATER RUNOFF ASSOCIATED WITH THE PROPOSED CONSTRUCTION. THE INTRODUCTION OF IMPERVIOUS AREA TO THE TRACT WILL CAUSE AN INCREASE IN THE PEAK RATE AND VOLUME OF RUNOFF GENERATED BY THIS SITE. THE PROPOSED DEVELOPED DISCHARGES FROM THIS SITE ARE IN ACCORDANCE WITH THE APPROVED MASTER DRAINAGE PLAN AND S.A.D. REPORT FOR BOTH THE INTERIM. AND ULTIMATE CONDITIONS. NO NEW DRAINAGE COVENANTS, EASEMENTS OR VARIANCES ARE REQUESTED AS PART OF THIS PLAN.

## CALCULATIONS

I. SITE CHARACTERISTICS

II. EXISTING CONDITION

A. VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C E_D A_D)/A_T$ 

 $E_W = [(1.13)(2.61)]/2.61 = 1.13 \text{ IN}.$ 

 $V_{100} = (E_W/12)A_T$  $V_{100} = (1.13/12)2.61 = 0.2458 \text{ AC.FT.} = 10,700 \text{ CF}$ 

B. PEAK DISCHARGE

 $Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B} + Q_{PC}A_{C} + Q_{PD}A_{D}$ 

 $Q_p = Q_{100} = (3.14)(2.61) = 8.2 \text{ CFS}$ III. DEVELOPED CONDITION

A. VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C E_D A_D) / A_T$   $E_W = [(0.78(0.19) + 1.13(1.92) + (2.12)(0.50)] / 2.61 = 1.39$  IN.

 $V_{100} = (E_W/12)A_T$  1.39  $V_{100} = (1.29/12)2.61 = 0.30$  0.30 0.30AC.FT. = 12,220 CF

B. PEAK DISCHARGE

 $Q_{p} = Q_{pA}A_{A} + Q_{pB}A_{B} + Q_{pC}A_{C} + Q_{pD}A_{D}$  $Q_{p} = Q_{100} = (2.28)(0.19) + (3.14)(1.92) + (4.70)(0.50) = 8.8 \text{ CFS}$ 

C. ALLOWABLE DISCHARGE

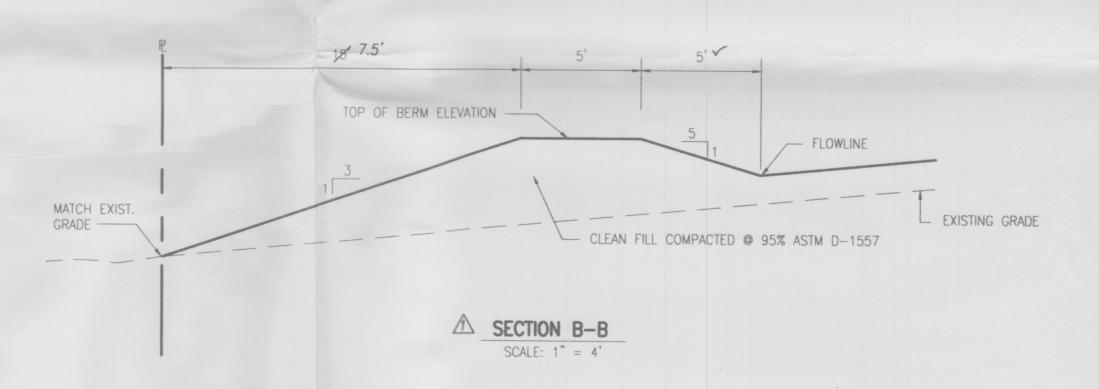
q<sub>PLAN</sub> = 8.8/2.61 = 3.4 CFS/ACRE < q ALLOW

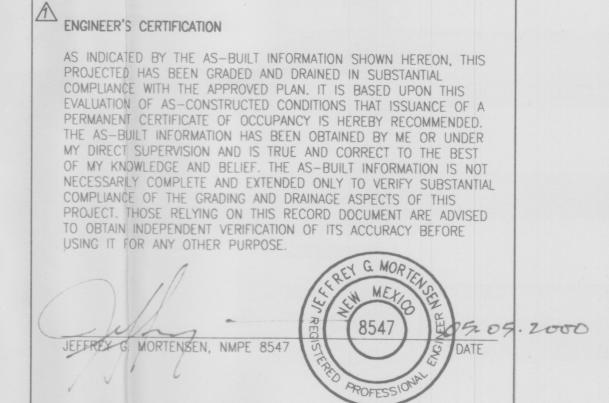
 $q_{ALLOW} = 3.61 CFS/ACRE (PER MDP)$ 

IV. COMPARISON

 $\Delta V_{100} = \frac{13,180}{12,220} - 10,700 = \frac{2480}{1520} \text{ CF (INCREASE)}$   $9.2 \qquad 1.0$   $\Delta Q_{100} = 8.8 - 8.2 = 0.6 \text{ CFS (INCREASE)}$ 

2'-7 1/2" 2'-0" 2" 5 1/2' LANDSCA (GRAVEL M INVERT **ELEVATION** VARIES EXIST. GRADE 3000 PSI ( COMPACTED -STM D-1557 -2 1/2" CLR., TYP. REBAR @ 12" O.C. E.W. △ SECTION A-A DELETED AND REPLACED SCALE: 1" = 1' WITH STANDARD CURB AND GUTTER





BEAT STORMS OF THE PROPESSION AND APPOPESSION APPO

JEFF MORTENSEN & ASSOCIATES, INC.

G010-B MIDWAY PARK BLVD. N.E.

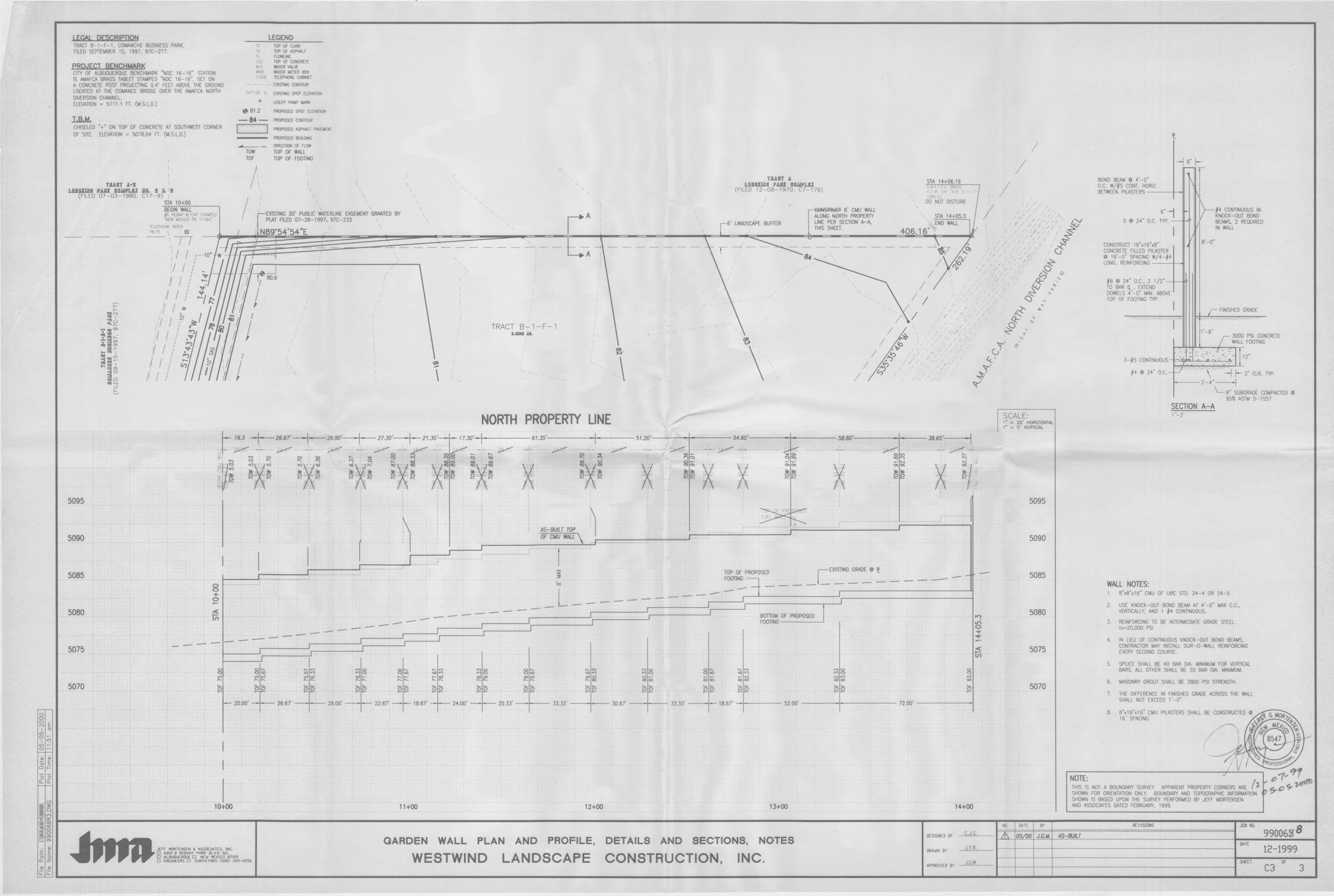
ALBUQUERQUE | NEW MEXICO 87109

ENGINEERS | SURVEYORS (505) 345-4250

DRAINAGE PLAN, CALCULATIONS, SECTIONS AND DETAILS WESTWIND LANDSCAPE CONSTRUCTION INC.

4/990068\ Plot Date: 05-05-2000

e Name: 990068R2.DWG





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### CALCULATIONS

I. SITE CHARACTERISTICS

A. PRECIPITATION ZONE = 1B.  $P_{6,100} = P_{360} = 2.35$  IN. C. TOTAL AREA  $(A_T) = 113,800 \text{ SF}/2.61 \text{ AC}$ D. EXISTING LAND TREATMENT

TREATMENT AREA SF/AC) % 113,800/2.61 100 E. DEVELOPED LAND TREATMENT AREA (SF/AC) % 8,280/0.19 83,910/1.92 21,610/0.50

### II. EXISTING CONDITION

A. VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C E_D A_D)/A_T$  $E_W = [(1.13)(2.61)]/2.61 = 1.13 \text{ IN}.$ 

 $V_{100} = (E_W/12)A_T$ 

 $V_{100} = (1.13/12)2.61 = 0.2458 \text{ AC.FT.} = 10,700 \text{ CF}$ 

B. PEAK DISCHARGE

 $Q_{P} = Q_{PA}^{A}_{A} + Q_{PB}^{A}_{B} + Q_{PC}^{A}_{C} + Q_{PD}^{A}_{D}$  $Q_P = Q_{100} = (3.14)(2.61) = 8.2 \text{ CFS}$ 

III. DEVELOPED CONDITION

A. VOLUME

 $E_W = (E_A A_A + E_B A_B + E_C A_C E_D A_D)/A_T$ 

 $E_W = [(0.78(0.19) + 1.13(1.92) + (2.12)(0.50)]/2.61 = 1.29 \text{ IN}.$ 

 $V_{100} = (E_W/12)A_T$ 

 $V_{100} = (1.29/12)2.61 = 0.2806 \text{ AC.FT.} = 12,220 \text{ CF}$ 

B. PEAK DISCHARGE

 $Q_{P} = Q_{PA}^{A}_{A} + Q_{PB}^{A}_{B} + Q_{PC}^{A}_{C} + Q_{PD}^{A}_{D}$ 

 $Q_{p} = Q_{100} = (2.28)(0.19) + (3.14)(1.92) + (4.70)(0.50) = 8.8 CFS$ 

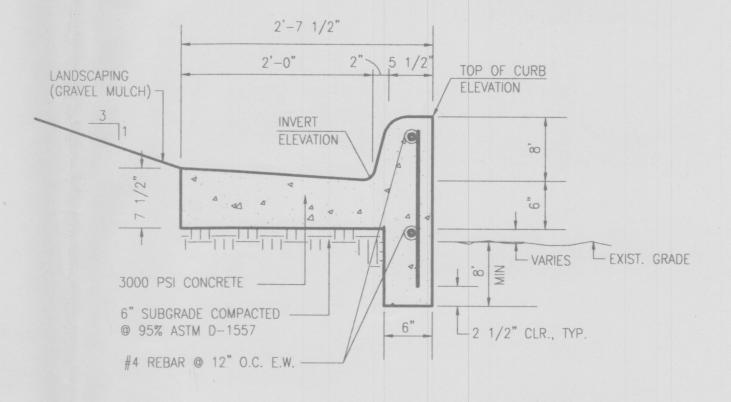
C. ALLOWABLE DISCHARGE

 $q_{PLAN} = 8.8/2.61 = 3.4 CFS/ACRE$ 

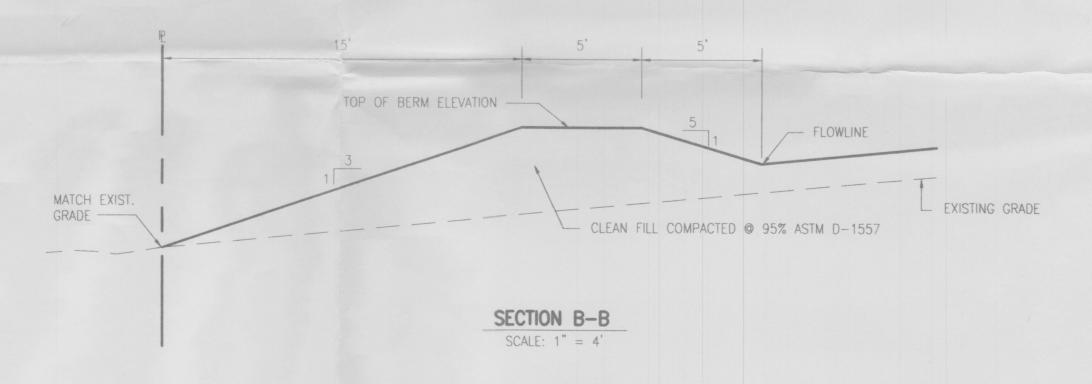
IV. COMPARISON

 $\Delta V_{100} = 12,220 - 10,700 = 1520 \text{ CF (INCREASE)}$ 

 $\Delta Q_{100} = 8.8 - 8.2 = 0.6 \text{ CFS (INCREASE)}$ 



SECTION A-A SCALE: 1" = 1'





DRAINAGE PLAN, CALCULATIONS, SECTIONS AND DETAILS WESTWIND LANDSCAPE CONSTRUCTION INC.

NO. DATE BY REVISIONS JOB NO. 990062 DESIGNED BY J.G.M./G.M. 03-1999 DRAWN BY D.L.M./J.Y.I APPROVED BY J.G.M.

